Appendix A: Comments and Selection Process

- 1. Review Comments
- 2. Selection of Study Locations
- 3. Public Participation

Part 1: Review Comments

From: Lee Toma @ Bike Milton

Sent: Wednesday, September 26, 2018 4:30 PM

To: sasante@ctps.org
Cc: William Clark

Subject: Re: Route 138 Priority Corridor Study in Milton - feedback

Dear Mr. Asante,

I'd like to thank you and your team for putting so much effort into the Route 138 Corridor Study. Most of the proposed changes look very promising for local residents like me.

First, I'd like to offer some suggested corrections and notes:

- Appendix Figure 1 Route 138 is not mapped across Milton in the correct location
- Appendix P 46 & 47

Milton - Route 138 and Mystic Valley Parkway Should be Neponset Valley Parkway

- Image quality of traffic report summaries is poor, which makes them difficult to read. I don't know if this is from the source, or from the PDF export.
- Do the Future Conditions charts (Figures 42 and 43) include the effects of adding a northbound lane of traffic?

General comments:

I am pleased to see the proposed intersection improvements, and consideration of a shared-use path. In the Tucker section especially, I would prefer to see an elevated or at least a curb-separated shared-use path over painted bike lanes to prevent vehicle obstructions. I expect that many neighbors would complain about the loss of the area that they currently use for parking, but on the other hand, this could also improve visibility for people pulling out of driveways and side streets. The shared-use path option would encourage more people to bike on this very intimidating corridor, more so than bike lanes would.

I'd like to suggest adding curb bump-outs in the Tucker area to improve sight lines at intersections. I would also like to encourage your team to include additional improvements at the Oak Street crosswalk, such as a signal, improved sight lines, and possibly a raised crosswalk. This school has the highest rate of pedestrian access in town, and anything we can do to improve pedestrian safety at this site should be a high priority.

Finally, I am concerned by the option that would add a second northbound travel lane from the Canton town line to Canton Avenue. I believe that this would increase traffic volumes at some times and increase speeds at other times, both of which are extremely unpopular among many Milton residents. This option would also shift merge conflicts from the current location in Canton up into Milton. Adding the third lane would also make road crossings more challenging for the many people who hike in the Blue Hills. I expect that most Milton residents would prefer to see the second northbound lane end at the Park and Ride parking lot.

Once again, I'd like to thank you for collecting and analyzing so much data and public input to help improve the corridor for residents and travelers.

Sincerely,

Lee (Leonardo) Toma
Member, Milton Traffic Commission
Chair, Milton Bicycle Advisory Committee Member, Milton Public Schools Transportation and Traffic Safety
Subcommittee

On Sep 24, 2018, at 9:16 PM, William Clark < wclark@townofmilton.org > wrote: Sent from my iPhone

Begin forwarded message:

From: Seth Asante <sasante@ctps.org<mailto:sasante@ctps.org>>

Date: September 20, 2018 at 2:08:57 PM EDT Subject: Route 138 Priority Corridor Study in Milton

Good Afternoon,

The draft report for the Route 138 Priority Corridor Study in Milton is available for final review. I have already sent you a Dropbox email and link to download the report. If you did not receive that email, then please use the link provided below to download a copy of the report.

https://www.dropbox.com/sh/v9pdd0455p6j7qu/AADPX2tN3pKDkG06X-2HCApVa?dl=0

Please send me your final comments by September 27. If you do not have any comment, please let me know.

Thank you, Seth

Seth A. Asante, P.E. | Chief Transportation Planner CENTRAL TRANSPORTATION PLANNING STAFF 857.702.3644 | sasante@ctps.org<mailto:sasante@ctps.org> www.ctps.org/bostonmpo<http://www.ctps.org/bostonmpo>

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This email has been scanned for spam and viruses by Proofpoint Essentials. Click here to report this email as spam.

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From: John Thompson

Sent: Thursday, September 27, 2018 9:21 AM

To: Seth Asante

Cc: Michael D. Dennehy; Chase Berkeley; William Clark; geraldine.vatan@state.ma.us

Subject: Route 138 Priority Corridor Study - Comments

Good Morning Seth,

I would like to offer the following comment on the Route 138 Priority Corridor Study:

If not already examined, the Town would be extremely interested in the looking at the feasibility of adding a left turn lane on Route 138 at Brush Hill Road for the northbound approach. The current configuration requires northbound vehicles use the Canton Avenue "jug-handle" for all turns. This creates a conflict at the Canton Avenue/Brush Hill Road intersection, particularly in peak hours, as southbound vehicles on Canton Avenue (bound for Route 138 South to expressway) have to criss-cross with the northbound vehicles on Canton Avenue (bound for Brush Hill Road to Paul's Bridge). We believe this condition exacerbates the already heavy backups on Canton Avenue southbound and creates an unsafe condition. Also of note, several of the DCR parking areas on Route 138 (ski area, Trailside Museum, etc.) do not allow exiting vehicles to make left turns to Route 138 south. Vehicles ultimately have to proceed northbound on Route 138 and use the Canton Avenue jug-handle to reverse direction, inviting additional vehicles into the congested Canton Avenue/Brush Hill road and Route 138/Brush Hill Road intersections.

Thank-you for all the work you do and please let me know if you have any questions.

Regards,

John P. Thompson, P.E. Town Engineer

Town of Milton – Engineering Dept. 525 Canton Avenue Milton, MA 02186

(617) 898-4869

From: Pounds, Bryan (DOT)

Sent: Tuesday, September 25, 2018 3:58 PM

To: Seth Asante
Cc: Mark Abbott

Subject: RE: Route 138 Priority Corridor Study in Milton

Seth -

Planning has no comments, thanks.

BP

From: Seth Asante [mailto:sasante@ctps.org] **Sent:** Thursday, September 20, 2018 2:09 PM

To: Vatan, Geraldine T. (DOT); Dwyer, Courtney (DOT); Kulen, Raj (DOT); Pervez, Hameed (DOT); Gascon, Cassandra (DOT); Pounds, Bryan (DOT); John Thompson; William Clark; Polin, Bonnie S. (DOT); Chase Berkeley; Michael D. Dennehy; Diaz, John; Reardon, Muazzez G. (DOT); Pastore, Karl (DCR); Jahnige, Paul (DCR); Keating, Connor (DOT);

John King; John Grant

Cc: Mark Abbott

Subject: Route 138 Priority Corridor Study in Milton

Good Afternoon,

The draft report for the Route 138 Priority Corridor Study in Milton is available for final review. I have already sent you a Dropbox email and link to download the report. If you did not receive that email, then please use the link provided below to download a copy of the report.

https://www.dropbox.com/sh/v9pdd0455p6j7qu/AADPX2tN3pKDkG06X-2HCApVa?dl=0

Please send me your final comments by **September 27**. If you do not have any comment, please let me know.

Thank you, Seth

Seth A. Asante, P.E. | Chief Transportation Planner CENTRAL TRANSPORTATION PLANNING STAFF 857.702.3644 | sasante@ctps.org www.ctps.org/bostonmpo







Part 2: Selection of Study Locations



BOSTON REGION METROPOLITAN PLANNING ORGANIZATION

Stephanie Pollack, MassDOT Secretary and CEO and MPO Chair Karl H. Quackenbush, Executive Director, MPO Staff

TECHNICAL MEMORANDUM

DATE: January 18, 2018

TO: Boston Region Metropolitan Planning Organization

FROM: Seth Asante, MPO Staff

RE: Selection of Study Locations for the FFY 2018 Addressing Priority

Corridors from the Long-Range Transportation Plan Needs

Assessment

1 BACKGROUND

During the development of the Boston Region Metropolitan Planning Organization (MPO) Long-Range Transportation Plan (LRTP), *Charting Progress to 2040*, the MPO staff identified the existing needs for all transportation modes in the region. The results were compiled in the LRTP Needs Assessment, which is used to guide the MPO's decision-making process for selecting transportation projects to fund in future Transportation Improvement Programs (TIP). The MPO goals that guided the development of the LRTP Needs Assessment include the following:

- Safety—make all modes safe
- Preservation—maintain and modernize the system
- Capacity Management and Mobility—use existing facility capacity more efficiently and increase healthy transportation capacity
- Clean Air/Clean Communities—create an environmentally friendly transportation system
- Transportation Equity—provide comparable transportation access and service quality among communities, regardless of income level or minority population
- Economic Vitality—ensure our transportation network serves as a strong foundation for economic vitality

Based on previous and ongoing transportation-planning work—including the MPO's Congestion Management Process (CMP) and planning studies—MPO staff identified several priority arterial roadway segments that require maintenance, modernization, and safety and mobility improvements. These locations are documented in the LRTP Needs Assessment.

¹ Boston Region Metropolitan Planning Organization, *Charting Progress to 2040: The New Long-Range Transportation Plan of the Boston Region Metropolitan Planning Organization*, endorsed by the Boston Region MPO on July 30, 2015.

To address problems on some of these arterial segments, the *Addressing Priority Corridors from the Long-Range Transportation Plan Needs Assessment* study was included in the federal fiscal year (FFY) 2018 Unified Planning Work Program (UPWP).² This memorandum presents the results of Task 2 of the work program for that study.³ Task 2 involves presenting a recommendation for locations to study to the MPO board for discussion.

By focusing on arterial segments rather than intersections, planners can evaluate multimodal transportation needs comprehensively (with the goal of creating Complete Streets). A holistic approach to analyzing problems and forming recommendations ensures that the needs of all public transportation users—including pedestrians, bicyclists, and motorists—are considered. Ultimately, this approach will result in roadways where it is safe to cross the street and walk or cycle to shops, schools, train stations, and recreational facilities, and where buses can run on time. Typically, the recommended improvements are within a roadway's right-of-way. They take into account the needs of abutters and users, and the interests and support of stakeholders.

2 PROCEDURE FOR SELECTING STUDY LOCATIONS

The process for selecting study locations consisted of three steps. First, MPO staff assembled data about the arterial segments identified in the LRTP Needs Assessment and used the data to prioritize the roadway segments. Next, MPO staff examined the arterial segments more closely by applying specific criteria. Finally, staff scored each arterial segment and assigned a priority of *low*, *medium*, or *high* to each segment. Details about each step in the process are provided below.

2.1 Gathering Data

MPO staff identified 44 arterial segments in 33 municipalities in the Boston region based on the following data sources:

 The Massachusetts Department of Transportation (MassDOT) 2016 Road Inventory File and 2010–14 crash database were used to assemble the following information for each arterial segment: roadway jurisdiction,

² Boston Region Metropolitan Planning Organization, Unified Planning Work Program, Federal Fiscal Year 2018, endorsed by the Boston Region Metropolitan Planning Organization on June 15, 2017 and was approved by our federal partners and took effect on October 1, 2017.

³ Karl H. Quackenbush, CTPS Executive Director, memorandum of a work program to the Boston Region Metropolitan Planning Organization, "Addressing Priority Corridors from the Long-Range Transportation Plan Needs Assessment: Federal Fiscal Year (FFY) 2017," October 19, 2017.

- National Highway System status, average daily traffic (ADT), high-crash locations, and crash rates.
- The MPO's CMP data on arterial congestion were used to determine average travel speeds, travel-time index (travel time in the peak period divided by travel time at free-flow conditions), and speed index (average travel speed divided by the speed limit) on each arterial segment.
- The MPO's data on gaps in the bike network and data on the location of MassDOT bike facilities were used to identify needs for the bicycle mode, including locations where connectivity between bicycle facilities could be improved and where bicyclists' accommodations could be improved.
- Data on MBTA bus service performance and passenger loads were used to determine the percentage of bus trips that do not adhere to the schedule (in other words, that provide late service) or do not adhere to passenger load standards (resulting in crowding).
- Data on MBTA bus routes, subway lines, and commuter rail lines were used to identify which arterial segments serve MBTA buses or stations.
- Data on the MPO's Environmental Justice (EJ) transportation analysis zones were used to identify areas of concern as relates to environmental justice.
- Data selected from MassDOT's project-information database, the MPO's FFY 2018–22 TIP projects, MPO planning studies and other studies, and municipal websites were used to obtain data on projects, studies, and TIP projects that are planned or programmed for each arterial segment.

Table 1 (attached) presents the data and information gathered on each of the following arterial segments:

- Municipality
- Metropolitan Area Planning Council (MAPC) subregion
- Jurisdiction
- MassDOT district office
- Crash rate per million vehicle-miles traveled
- Number of top-200 high-crash locations
- Number of crash clusters that are eligible for Highway Safety Improvement Program (HSIP) funding
- Travel-time index
- Transit service performance
- Proximity to an EJ transportation analysis zone (within one-half mile distance)
- Relevant studies or projects within or near the segment

Table 1 also includes the score and priority rating that were determined by applying the selection criteria. The processes for scoring and assigning priority ratings to segments are described below.

2.2 Applying Criteria

MPO staff examined the arterial segments more closely by applying the following six criteria and assigning points based on the number of criteria that apply to each location:

- 1. Safety Conditions, 0–4 points (each of the four criteria is worth one point)
 - Location has a higher-than-average crash rate for its functional class
 - Location contains an HSIP-eligible crash cluster
 - Location is identified in the Massachusetts Top High Crash Locations Report
 - Location has a significant number of pedestrian and bicycle crashes per year (two or more per mile) or contains one or more HSIP-eligible bike-pedestrian crash cluster
- 2. Congested Conditions, 0–2 points (each of the two criteria is worth one point)
 - o Travel-time index is at least 1.3
 - o Travel-time index is at least 2.0
- 3. Multimodal Significance, 0–3 points (each of the three criteria is worth one point)
 - o Location currently supports transit, bicycle, or pedestrian activities
 - Location needs to have improved transit, bicycle, or pedestrian facilities
 - Location has a high volume of truck traffic serving regional commerce
- 4. Regional Significance, 0–4 points (each of the four criteria is worth one point)
 - Location is in the National Highway System
 - Location carries a significant portion of regional traffic (ADT is greater than 20,000)
 - o Location lies within 0.5 miles of an EJ transportation analysis zone
 - Location is essential for the region's economic, cultural, or recreational development
- 5. Regional Equity, 0–2 points (each of the two criteria is worth one point)
 - Location is in an MAPC subregion for which there has not been a Priority Corridors study

- Location is in an MAPC subregion for which there has not been a Priority Corridors study in the previous three years.
- 6. Implementation Potential, 0–3 points (each of the three criteria is worth one point)
 - Location is proposed or endorsed for study by the agency that administers the roadway
 - Location is proposed or endorsed by its MAPC subregional group and is a priority for that subregional group
 - o Other stakeholders strongly support improvements for the location

2.3 Scoring and Rating

MPO staff rated arterial segments with a total score of 10 or fewer points as *low* priority; those with a score of 11 to 12 points as *medium* priority; and those with a total score of 13 or more points as *high* priority. MPO staff gave 15 arterial segments a high-priority rating based on safety and operational needs, multimodal and regional significance, regional equity, and support for improvements from agencies and municipalities. Staff then examined high-priority segments more closely, and excluded arterials that had projects meeting any of the following criteria from further consideration for this cycle of the Priority Corridors study: recently completed, in construction, in design, under study, or programmed in the TIP with the 25 percent design completed.

The four arterial segments with the highest scores were:

- Route 138 in Milton
- Route 114 in Peabody
- Route 3A in Quincy
- Route 16 in Wellesley

Staff also evaluated the pedestrian accommodation and safety improvement needs for these segments by applying the MPO's recently developed Pedestrian Report Card Assessment.⁴ All four locations highly qualify based on pedestrian accommodation or safety improvement requirements. Appendix A contains detailed results of the assessments. Based on this evaluation, MPO staff recommends studying the segment on Route 138 in Milton.

⁴ Ryan Hicks and Casey-Marie Claude, Boston Region Metropolitan Organization, *Pedestrian Level-of-Service Memorandum*, January 19, 2017.

3 ARTERIAL SEGMENT SELECTED FOR STUDY: ROUTE 138 IN MILTON

The arterial segment that was selected for study was Route 138 in Milton, based on a total score of 14, using the five selection criteria (safety, congestion, multimodal and regional significance, regional equity, and implementation potential). Route 138 runs north-south through Milton, from the border of Boston to the north to the border of Canton to the south. In Milton, the roadway primarily passes through residential areas and the Blue Hills Reservation area. Current evaluation indicates that there are safety, capacity, and mobility problems in the segment. Two locations along the segment contain HSIP-eligible crash clusters and the segment has a higher-than-average crash rate for its functional class. The corridor also sees a high injury rate with 39 percent of collisions causing injuries. Additionally, several intersections in the segment are congested, which create long traffic queues during peak travel periods. Finally, accommodations for pedestrians and bicyclists are poor and need improvement—several sections in the Blue Hills Reservation and Curry College lack sidewalks.

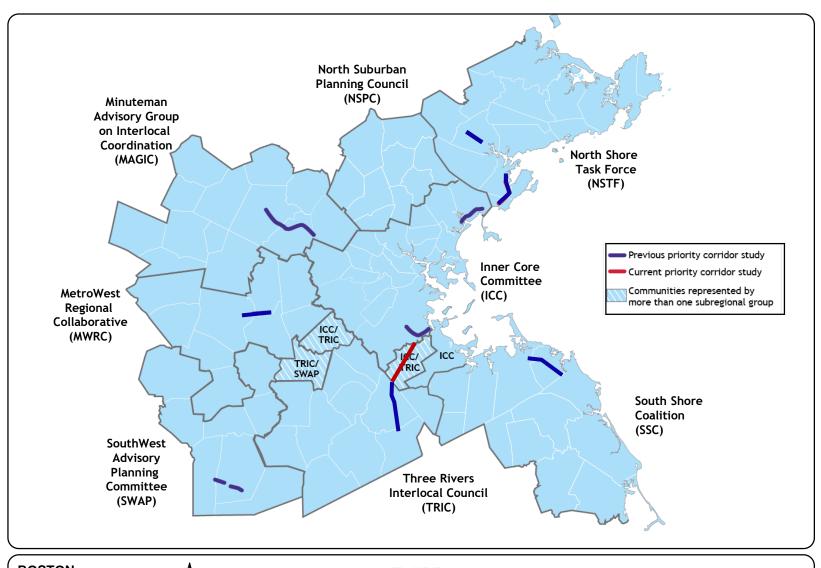
The Town of Milton is considering capacity and mobility improvements in the corridor and has expressed support for and willingness to participate in a study of this arterial segment (See Appendix B). MassDOT Highway Division District 6 supports this study and asked the MPO staff to identify the problems and develop Complete Street solutions that could be implemented by MassDOT in tandem with a future roadway improvement project.

The recommended arterial segment on Route 138 in Milton meets the selection criteria of this study, especially by supporting the transportation improvement priorities of the MPO's LRTP. While the work program for this study assumed that "as many as two" arterial segments would be selected, the MPO staff does not propose studying a second arterial segment because Route 138 in Milton is approximately 3.5 miles long and this study would require considerable resources for evaluating alternative improvement plans. Figure 1 shows the general locations of previous Priority Corridor studies, and the location identified for this year's study.

4 NEXT STEPS

After the MPO board discusses this recommendation, staff will meet with officials from the Town of Milton, MassDOT, MAPC, and other stakeholders to discuss the study specifics, conduct field visits, collect data, identify needs, and develop solutions.

SA/sa



BOSTON REGION MPO



FIGURE 1
Previous and Current LRTP Priority Corridor Studies
By MAPC Subregion

Route 138 Milton LRTP Priority Corridors Study

TABLE 1 Arterial Segments Considered for Study: Priority Corridors for Long-Range Transportation Plan Needs Assessment Study (Arterial Segment Selected for Study Is Highlighted in Green)

									T			(Ar	terial Segment Selec	ted for Study Is Highlighted in Green)									
Arterial			MassDOT		National Highway	Functional	Crash Rate	Number of Top-200 High Crash Locations	Number of HSIP-Eligible				In or Near Environmental		Safety	Congested	Multimodal	Regional	Regional			iority	
Segment	Community	MAPC Subregion	District	Jurisdiction	System	Class*	(MVMT)	2012–14	Crash Clusters 2012–14**	Index	Transit Service	Late Bus	Justice Zone	Study, Project, or TIP Project	Conditions	Conditions	Significance	Significance	Equity	Implementation Potential	Score Ra	iting	Summary of Comments
Route 138	Milton	ICC and TRIC	6	MassDOT	Yes	2	1.5	0	1		MBTA bus Routes 245 and 716 MBTA Commuter Rail at Route 128 Station MBTA Red Line Rapid Transit at Mattapan Station			MassDOT Poject #608484, Roadway Improvement on Route 138, project is planned to be funded through the 2020 Transportation Improvement Program for the Boston Metropolitan Planning Organization (MPO); project will also incorporate work planned originally for 607763 (described below); in the preliminary design phase. MassDOT Project #607763, Intersection and Signal Improvements at Two Locations: Route 138 (Blue Hill Avenue) at Atherton Street and Bradlee Road and Route 138 (Blue Hill Avenue) at Mitton Street and Dollar Lane, s programmed in federal fiscal year (FFY) 2019 Transportation Improvement Program (TIP); in the preliminary design phase.		2	2	4	1	3	14 Hig		Safety and congestion issues have been identified on this route and many locations in the segment need pedestrian and bicycle improvements. In addition, several intersections in the segment have congestion and safety issues. The Town of Milton is looking at pedestrian improvements in the corridor and has expressed unanimous support for the study. MassDOT Highway District 6 is in support of this study to identify problems and solutions that can be implemented in tandem with a future roadway improvement project in the segment. The location was suggested in the 2017 MPO outreach program.
Route 114	Peabody	NSTF	4	MassDOT and Town	Yes	2	3.7	2	8	3.60	Three MBTA bus stops MBTA bus Routes 435 and 465	s Yes	Yes Half the segment abuts an EJ zone.	MassDOT Project # 608567, Improvements at Route 114 at Sylvan Street, Cross Street, Northshore Mall, Loris Road, Route 128 Interchange, and Esquire Drive, in design	4	2	2	3	1	2	14 Hig		Route 114 in Peabody was listed as a potential corridor in need of signal progression and improvements to accommodate pedestrians and bicyclists. However, the arterial segment was not selected because according to MassDOT Highway District 4, a road safety audit was completed for the segment in August 2016 and a consultant is starting design work as part of project #608567. The location was suggested in the 2017 MPO outreach program.
											MBTA bus Routes 201, 202, 210, 211, 212, 217, 275, 276, and 21, MBTA Red Line			MassDOT Project #608569, Intersection Improvements at Route 3A (Southern Artery) and Broad Street. The project is planned to be funded through the FFY 2021 TIP; in the preliminary design phase. MassDOT Project #605729, Intersection and Signal Improvements at Hancock	1								
											Rapid Transit at Quincy Center, Wollaston, and North Quincy		Yes	Street and East/West Squantum streets. The project consists of widering and improvements to the intersection of Hancock Street with East and West Squantum streets and improvements along Hancock Street to the MBTA access drive; completed in fall 2015.									Route 3A (Hancock Street and Southern Artery) has
				MassDOT, DCR, and							MBTA Commuter Rail at Quincy		lies within or near EJ	MassDOT Project #602237, Traffic Signal Installation and Intersection improvements, Hancock Street and Southern Artery. The project reconstructed	d								received several improvement projects and a CTPS study. The location was suggested in the 2017 MPO
Route 3A	Quincy	ICC	6	City	Yes	3	2.9	1	4	2.76	Center	Yes	zones.	Hancock Street from Saint Ann's Road to Fenno Street, completed in fall 2007.	7. 3	2	2	4	1	2	14 Hig	gh	outreach program.
Route 16	Wellesley	MWRC	6	MassDOT and Town	Yes	4	6.4	0	5	2.57	MBTA Commuter Rail at Wellesley Square, Wellesley Hills, and Wellesley Farms MWRTA Route 8	у		MassDOT Project #606518. As part of the Quincy redevelopment project, the city plans to construct a new bridge over the existing MBTA tracks that will connect the downtown area at Market Square and Hancock Street. The main goal of the new bridge will be improved pedestrian conditions along Hancock Street; 25% package received (as of 12/16/2016) An FFY 2012 CTPS safety and operations study addressed problems at Route 3A and Coddington Street intersection. MassDOT Project #94762, Bridge Rehabilitation, Route 16 (Washington Street) over Route 9, including relocation of retaining wall; completed summer 2010. MassDOT Project #600712, Reconstruction of Route 16 from Grantland Road to the Newton City Line. The work consisted of paving, drainage improvements, sidewalk reconstruction, traffic signals, and ornamental lighting on Route 16. A signal was installed at the Washington Street/Walnut Street intersection, and the pedestrian crossing 150 feet south of Hillside Road was upgraded, completed in 2004.	1	2	2	4	1	2	14 Hig		The location was suggested in 2014 LRTP outreach through verbal comments at a 495/MetroWest Partnership meeting.
											Eight MBTA bus stops			CTPS and MAPC Community Transportation Technical Assistance Program evaluated the high-crash location at the intersection at Massachusetts Avenue, March 2010.	,								
											MBTA bus Routes 67, 62, 76, 77, 78		V	MassDOT Project #606885, the contractor is planning to finish the rest of the									
Route 60	Arlington	icc	4	Town	Yes	3	5.7	0	1	2.92	79, 80, 84, and 350	Yes	Yes	bike route symbols and electric work, weather permitting (as of 01/06/2017); in construction.	3	2	3	3	1	1	13 Hig	gh	N/A
Alewife Brool Parkway		ICC	6	DCR	Yes	2	9.3	0	3	4.77	MBTA bus Router 79, 350, 62, 67, 74, 76, 78, 84, and 351 MBTA Rapid Transit on the Re- Line MBTA Commuter Rail at Porter Square	d		Alewife Studies, Phase II, CTPS study (2009). DCR announced a comprehensive study of the parkway system for bike lanes. MassDOT Project #605637, Improvements at Route 2 and Route 16. The purpose of this project is to perform minor widening, eliminate a merge condition, and improve throughput capacity and vehicle queue storage at the intersection of Route 2 and Route 16 (Alewife Brook Parkway); under construction.	3	2	2	4	1	1	13 Hig		The Fresh Pond Residents Alliance identified Fresh Pond Parkway and Alewife Brook Parkway as locations in need of transportation improvements. Concerns include pedestrian safety of young students who walk to Shady Hill School because of high traffic volumes, environmental issues, and lack of livability.
Route 16 (Revere Beach Parkway)	Everett	ICC	4	DCR	Yes	2	2.2	1	7	1.97	MBTA bus Routes 97, 99, 106, 110, 112, 104, 105, and 109 MBTA Orange Line Rapid Transi at Wellington and MBTA Commuter Rail at Chelsea	it		DCR announced a \$500,000 comprehensive study of the parkway system for bike lanes in FFY 2015. The goals of the study include updated traffic information, assessment of parkway conditions, and assessment and understanding of deficiencies along the heavily cycled parkways.	3	1	3	4	1	1	13 Hig		This arterial segment was not selected because it is part of the Mystic River Working Group Study. In addition, the Wynn Everett DEIR (2015) includes intersection improvements and mitigated traffic operations for Revere Beach Parkway and Mystic Valley Parkway.

TABLE 1

Arterial Segments Considered for Study: Priority Corridors for Long-Range Transportation Plan Needs Assessment Study
(Arterial Segment Selected for Study Is Highlighted in Green)

												(Ar	terial Segment Selec	ted for Study Is Highlighted in Green)								
Arterial Segment	Community	MAPC Subregion	MassDOT	Jurisdiction	National Highway System	Functional Class*	Crash Rate (MVMT)	Number of Top-200 H Crash Locations 2012–14	igh- Number of HSIP-Eligible Crash Clusters 2012–14**		Transit Service		In or Near Environmental Justice Zone	Study, Project, or TIP Project	Safety Conditions	Congested Conditions	Multimodal Significance	Regional Significance	Regional Equity	Implementation Potential	Priority Score Rating	Summary of Comments
Route 9	Framingham	MWRC	3	MassDOT	Yes	2	2.8	0	7	3.47	MWRTA bus Routes 1, 2, 3, 7 and 9	None	Yes Over half the route lies within or adjacen to an EJ zone.	MAPC Land Use/Route 9 Corridor Study (fall 2013) MassDOT Project #603865 is located in Framingham at the intersection of Route 9 and Temple Street; in preliminary design MassDOT Project #608006, Pedestrian Hybrid Beacon Installation at Route 9 and Maynard Road; 25% design stage MassDOT Project #604991, Resurfacing and Related Work on Route 9, includes wheelchair ramp upgrades, additional sidewalks/repairs, and signal improvements; completed in autumn 2011 #602522: Framingham- Bridge Replacement, Br# F-07-006, Route 9 over the Sudbury River — This project proposes to replace the Route 9 Bridge over the Sudbury River and includes minor incidental roadway work. (2009) #602930: Brookline- Framingham- Natick- Newton- Southborough- Wellesley-Westborough- Traffic Signs on Route 9 — This project will replace and update all overhead and ground-mounted guide sign panels, exit gore, warning, regulatory, and route marker panels on Route 9 and secondary roadways for the Boston-Brookline town line to 1-495 in Westborough, with the exception of signing, which was updated under the 1-495 Milford to Bolton contract. (2009) #604991: Framingham- Natick- Resurfacing and Related Work on Route 9 — Route 9 will be resurfaced from approximately the Southborough/Framingham Line easterly to the Natick/Wellesley Line. (2011) #605228: Framingham- bridge replacement, F-07-001, Route 9 (Worcester Road) over reservoir outlet — The purpose of this project is to replace the superstructure of the Route 9 bridge over the Reservoir Outlet connecting the Foss Reservoir No. 3 to the Stearnes Reservoir No. 1. (2017)	2 2	2	3	4	1	1	13 High	This arterial segment was not selected because according to MassDOT District 3, most of the intersections on this corridor have already been studied. In addition, MPO staff studied Route 30 in Framingham and Natick under the FFY 2013 Priority Corridors for LRTP Needs Assessment.
											MBTA bus Route 424,426, 436,	rs.		#608006: Framingham- Pedestrian Hybrid Beacon installation at Route 9 And Maynard Road The proposed project will construct an at-grade pedestrian crossing across Route 9 in the vicinity of Maynard Road and the Framingham Fire Station. (Design public hearing 2017)								
Route 107	Lynn	ICC	4	MassDOT and Town	Yes	3	20.6	3	21		441, 442, 450, 455, 456, 459, 429, and 435 MBTA Commute Rail at River Works, Lynn/Central Square, and Swampscott Ferry service	Yes	Yes The entire segment lies within EJ zones.	MassDOT Project #604952, Bridge Replacement, Route 107 over the Saugus River; Design exception submitted (as of 01/26/2017); The construction will begin in autumn 2018. MassDOT Project #26710, Bridge Replacement, Route 107 over the Saugus River (Fox Hill Bridge); completed spring 2013 MassDOT Project #603938, Western Avenue Bridge over Saugus River (Fox Hill Bridge) TIP Project #374, Lynn Garage (transit)		1	2	4	1	1	13 High	This arterial segment was not selected for study because there is an ongoing Route 107 Corridor Study in Lynn and Salem, which is being conducted by MassDOT in conjunction with Lynn and Salem.
Route 16 (Revere Beach Parkway and Mystic Valley Parkway)	Medford	ICC	4	DCR	Yes	2, 3	2.2	2	4		MBTA bus Route 90, 97, 99, 100, 106, 108, 110, 112, and 134 MBTA Rapid Transit on the Orange Line at Wellington and o the Red Line at Porter Square MBTA Commute Rail at West Medford and Porter Square	n	segment in Somerville and Everett and 0.2	DCR announced a \$500,000 comprehensive study of the parkway system for bike lanes in FFY 2015. The goals of the study include updating traffic information, assessing parkway conditions, and assessing and understanding deficiencies along the heavily cycled parkways. #604660: Everett- Medford- Bridge Replacements, Revere Beach Parkway (Route 16), E-12-004=M-12-018 Over The Malden River (Woods Memorial Bridge) & M-12-017 Over Mbta And Rivers Edge Drive The purpose of this project is to replace the existing non-operating draw bridge with a new fixed bridge. (2020)	3	2	2	4	1	1	13 High	This arterial segment was not selected because it is part of the Mystic River Working Group Study. In addition, the Wynn Everett DEIR (2015) includes intersection improvements and mitigated traffic operations for Revere Beach Parkway and Mystic Valley Parkway.
Route 9	Natick	MWRC	3	MassDOT	Yes	2	4.4	1	10		MWRTA bus Routes 1, 4, 9, and 10	None	Yes One EJ zone is 0.5 miles away.	MAPC Land Use/Route 9 Corridor Study (fall 2013) MassDOT Project #608821, Installation of adaptive traffic control signal equipment, vehicle detection, communication equipment, and managing software at 5 traffic signals (3 in Framingham and 2 in Natick) on Route 9; in construction. MassDOT Project #605091, Work consists of bridge repairs on 4 bridges ove Route 9 and Speen Street, in preliminary design MassDOT Project #601586 was completed in autumn 2015. MassDOT Project #6005313 will reconstruct the Route 9/Route 27 interchange 25% project design stage. MassDOT Project #604991, Resurfacing and Related Work on Route 9, includes wheelchair ramp upgrades, additional sidewalks/repairs, and signal improvements; completed in 2011	er e;	2	1	4	1	1	13 High	This segment was not selected because according to MassDOT District 3, the installation of an adaptive traffic control system for five signals and the reconstruction of the Route 9 and Oak Street intersection are currently under construction. The Route 9 and Route 27 interchange is currently in design.

TABLE 1

Arterial Segments Considered for Study: Priority Corridors for Long-Range Transportation Plan Needs Assessment Study
(Arterial Segment Selected for Study is Highlighted in Green)

														ea for Study is Highlighted in Green)									
					National			Number of Top-200	High-	Travel-			In or Near										
Arterial	Community	MAPC Subregion	MassDOT	Jurisdiction	Highway System	Functional Class*	Crash Rate	Crash Locations 2012–14	Number of HSIP-Eligible Crash Clusters 2012–14**				Environmental	Study, Project, or TIP Project	Safety Conditions	Congested Conditions	Multimodal Significance	Regional Significance	Regional Equity	Implementation Potential		Priority	Summary of Comments
Segment	Community	MAPC Subregion	District	Jurisdiction	System	Class*	(MVM1)	2012–14	Crash Clusters 2012–14**	index	Transit Service	Late Bus	Justice Zone	#602930: Brookline- Framingham- Natick- Newton- Southborough- Wellesley- Westborough- Traffic Signs On Route 9 This project will replace and update all overhead and ground-mounted guide sign panels, exit gore, warning, regulatory, and route marker panels on Route 9 and secondary roadways for the Boston-Brookline town line to I-495 in Westborough (completed 2009)	9	Conditions	Significance	Significance	Equity	implementation Potential	Score	Rating	Summary of Comments
														#603004: Natick- Bridge Replacement, Br# N-03-021, Route 9 Over Lake Cochituate (2007)									
														#607732: Framingham- Natick- Cochituate Rail Trail Construction Including Pedestrian Bridge, N-03-014, Over Route 9 & F-07-033=N-03-029 Over Rout 30 (begins 2018/2019)	e								
														#607993: Ayer- Natick- Lancaster- Leominster- Worcester- Stormwater Improvements Along Route 2, Route 9, Route 12, Route 2a, Route 110, Rout 111 And I-290 (2018)	е								
														#608281: Framingham- Natick- Adaptive Signal Control On Route 9 (Worcester Road) – Installation of adaptive traffic control signal equipment, wehicle detection, communication equipment, and managing software at 5 traffic signals (3 in Framingham + 2 in Natick) on Route 9. (completed summe 2017)	r								
											18 MBTA bus stops MBTA bus Routes 450, 451, 455, 456, 459, and 465 MBTA Commuter Rail at Salem and		Yes	Transportation Improvement Study for Routes 1A, 114, and 107 and Other Roadways in Downtown Salem, 2005 CTPS study									This arterial segment was not selected because of regional equitythe NSTF subregion was the recipient of the FFY 2016 LRTP Priority Corridor study. This location was suggested for study in 2012 UPVP outreach via an NSTF letter. NSTF suggested that a study on Routes 114/14 and Route 127 from Swampscott to Gloucester would include suggestions about how to improve bike facilities and bike-to-rail connections in this healty traveled tourist region. This
Route 114	Salem	NSTF	4	MassDOT and City	Yes	2, 3	10.4	1	5	2.06	Beverly Ferry service		Half the segment abuts EJ zones.	MassDOT Project #605332, Bridge Replacement (Route 114) North Street over North River; in preliminary design	4	2	2	4	0	1	13	High	builds on the NSTF's primary recommendation for that year and the anticipated popularity of the Essex Coastal Scenic Byway in the region.
											30 MBTA bus stops MBTA bus Routes 220, 221, and 222 MBTA Commuter Rail at Quincy Center, Weymouth Landing/ East Braintree, and West Hingham		Yes An EJ zone in Quincy	MassDOT Project #608231. The intent of this project is to reconstruct Route 3A and address poor traffic operations along the corridor. The project will als upgrade accomodations for bicyclists and pedestrians; in preliminary design MassDOT Project #604382, Route 3A (Washington Street) Bridge; construction ends winter 2016/2017 MassDOT Project #608483, Work consists of resurfacing on Route 3A; in preliminary design									A road safety audit was completed for Route 3A in Weymouth in September 2016. The audit identified the problems and needs on the roadway, and suggested short-, medium-, and long-term improvements. MassDOT District 6 indicated that a study would probably be redundant as the audit
Bouto 2A	Weymouth	SSC	6	MassDOT	Vaa	2	3.5		2	1.74	Ferry service			MassDOT Project #602703, Bridge Rehabilitation, Route 3A (Lincoln Street) over the Weymouth Back River; completed in autumn 2006	2	4	2	4	4	2	12	High	provided the information needed to advance Project #608321 in design.
Route 18	Weymouth	ssc	6	MassDOT	Yes	3	7.1	0	10	2.55	Nine MBTA bus stops MBTA bus Route 225 MBTA Commuter Rail at South Weymouth		Yes EJ zones lie adiacent	Programmed TIP (2017) and MassDOT Project #601630, Reconstruction and Widening on Route 18 (Main Street), from Highland Place to Route 139; construction begins summer 2017 MassDOT Project #603161, Signalization and Improvements on Route 18 (Three Locations) at West Street, Park Avenue, and Columbian Street; completed in spring 2009 MassDOT Project #603738, Traffic Signal Improvements on Route 18 at Pon Street and Pleasant Street; completed in summer 2006		2	2	4	1	1	13	High	This arterial segment was not selected because according to MassDOT District 6, a MassDOT project is underway, and no project is needed at this time.
Route 129	Wilmington	NSPC	4	MassDOT and Town	n Yes	3	6.1	0	7	3.31	MBTA Commuter Rail at Wilmington, North Wilmington, Anderson/Woburn, and Reading		None	MassDOT Project #601732, Rehabilitation, Route 129 (Lowell Street) from Route 38 (Main Street) to Woburn Street. The project includes full-depth reconstruction and widening, accessible (ADA-compliant) sidewalks, new tree plantings, and bicycle accommodation within the newly paved shoulders. The intersection of Route 129 and 38 was realigned with new traffic signals and the bridge over Maple Meadow Brook was replaced; completed in 2009. MassDOT Project #608051 will reconstruct Route 38 from Route 62 to the Woburn city line and will add bike lanes, sidewalks, turn lanes, and signal upgrades; in preliminary design.		2	2	3	2	1	13	High	NA
	I Bedford and										Three MBTA bus stops MBTA bus Route			Great Road Project: Master Plan and Conceptual Design, prepared by VHB for the Town of Bedford in 2011, in preliminary design The MassDOT-administered section, from I-95 to Hartwell Avenue, was the subject of a Town study (Hartwell Avenue Traffic Mitigation Plan Bedford Street Concept Plan), and a road safety audit was performed for this segmer in November 2011 #29500: Bedford- Roadway Reconstruction And Traffic Signal Installation On A Section Of Great Road (Routes 4 & 225) (complete 2000) #607409: Lexington- Reconstruction On Massachusetts Avenue, From Marret Road To Pleasant Street The proposed project will address safety and capacity deficiencies at three intersections along Massachusetts Avenue; Marrett Road (Route 2A), Maple Street (Route 2A) and Pleasant Street									This arterial segment was not selected because it did not have the support of MassDOT District 4 and also sections of it had already been studied. The Town of Bedford requested in FFY 2017 that the MPO study this arterial segment from I-95 in Lexington to Loomis Street in Bedford. The MAGIC subregion requested that the FFY 2012 UPWP and FFY 2013 UPWP include a study of Routes 4 and 225. The MassDOT section from I-95 to Hartwell Avenue
Route 16 (Revere Beach Parkway)	Lexington Chelsea	MAGIC	6	MassDOT and Town	Yes (part)	3, 5	2.8	2	3	2.33	MBTA bus Routes 112 and 111 MBTA Commuter Rail at Chelsea		Yes The entire segment lies within EJ zone.	(Routes 4/225). (construction 2016-2018) The Lower North Shore Transportation Improvement Study, CTPS study (2000) DCR announced a comprehensive study of the parkway system for bike lanes	3 5. 3	2	2	4	1	0	12	Medium Medium	was the subject of a Town study. This arrenta segment was not selected because it is part of the Mystic River Working Group Study. In addition, the Wynn Everett DEIR (2015) includes intersection improvements and mitigated traffic operations for Revere Beach Parkway and Mystic Valley Parkway.

TABLE 1 Arterial Segments Considered for Study: Priority Corridors for Long-Range Transportation Plan Needs Assessment Study (Arterial Segment Selected for Study Is Highlighted in Green)

	· in											(/	Arterial Segment Selec	ted for Study Is Highlighted in Green)									
					N-4i			Number of Ten 200	18-b	T1			In No										
Arterial	Community	MAPC Subregio	MassDOT	Jurisdiction	Nationa Highwa System	y Functional	Crash Rate (MVMT)	Number of Top-200 Crash Locations 2012–14	Number of HSIP-Eligible Crash Clusters 2012–14*		Transit Service		In or Near Environmental Justice Zone	Study Project or TIP Project	Safety Conditions	Congested Conditions	Multimodal Significance	Regional Significance	Regional Equity	Implementation Potential	Caara	Priority Rating	Summary of Comments
Segment	Community	MAPC Subregio	n District	Jurisdiction	System	Class	(INIVINII)	2012-14	Crash Clusters 2012–14	index	51 MBTA bus	Late Bus	Justice Zone	Study, Project, or TIP Project	Conditions	Conditions	Significance	Significance	Equity	implementation Potential	Score	Rating	Summary or Comments
											stops												
											MBTA bus Route 240, 245, 24, 28	3,											
											26, 30, 31, and 3			MassDOT Project #607342, Intersection and Signal Improvements at Route 28 (Randolph Avenue) and Chickatawbut Road; in preliminary design									
											MBTA Red Line Rapid Transit at			MassDOT Project #106901, Roadway Reconstruction on Route 28 (Randolph									
											Mattapan/Ashmo Station	ont	Yes	Avenue) from Reedsdale Road to Milton/Quincy town line; completed 2008									This arterial segment was not selected because there
Route 28	Milton	ICC and TRIC	6	MassDOT and Town	Yes	3	4.2	0	1	2.48	BAT Route 12	Yes	at the northern end.		2	2	2	3	1	2	12	Medium	have been several improvements in this segment in recent years.
														MassDOT Project #604327, Resurfacing and Related Work on Route 9 (Boylston Street) from the Wellesley/Newton city line to Newton/Brookline city	ty								
														line; completed in summer 2012									
														MassDOT Project #601704, Reconstruction and Signal Improvements on Walnut Street, from Homer Street to Route 9; in design; 25% package received (as of 12/23/2013)									
											Six MBTA bus			MassDOT Project #606635, Reconstruction of Highland Avenue, Needham									
											stops		Yes	Street, and Charles River Bridge, from Webster Street to Route 9; 75% package received (as of 09/23/2016).									According to MassDOT District 6, improvements
											MBTA bus Route 60, 52, and 59	es	An EJ zone in	MassDOT Project #604327, resurfaced this segment, including updates to									were recently made to accommodate new developments. An analysis of the new existing
Route 9	Newton	ICC	6	MassDOT	Yes	2	2.0	0	8	4.99	MBTA Green Lir	ne Yes	Brookline is 0.3 mi	guardrails and improvements to the existing drainage structures; construction was completed in 2012.	n 2	2	2	4	1	1	12	Medium	conditions would be helpful to compare with the future projected conditions.
											MBTA Routes 59	9.											
											170, 505, 553, 554, and 556	,											
											MBTA Green Lir	ne		MassDOT Project #606780, Bridge Rehabilitation, Route 16 (Washington									In FFY 2014, a subregional study was conducted on
											Rapid Transit		Yes	Street) over I-90, MBTA/CSX Corporation and Access Road; 25% package comments to DE (as of 02/19/2016).									Washington Street in Newton.
											MBTA Commute Rail at West	r	An EJ zone lies adjacent to the	Conceptual TIP #1067, Washington Street (Phase 2), from Commonwealth									The location was suggested in 2014 LRTP outreach through verbal comments at a 495/MetroWest
Route 16	Newton	ICC	6	MassDOT and City	Yes	3	2.9	0	4	1.86	Newton	Yes	segment.	Avenue to Perkins Street MassDOT Project #603716, Resurfacing and Related Work on a Section of	2	1	2	4	1	2	12	Medium	partnership meeting.
											50 MBTA bus			Route 28; completed 2007/2008									
											stops			Conceptual TIP #1002, Route 28 (N. Main Street) Bridge									
											MBTA bus Route 240 and 238	es		Conceptual TIP #1010, Route 28 (N. Main Street) and Liberty Street intersection									
											MBTA Commute	r		Conceptual TIP #1011, Route 28 (N. Main Street) and West Street intersection									
											Holbrook/Randol	lp	Yes	FFY 2008 Safety and Operations Analyses at Intersections study									
Route 28	Randolph	TRIC	6	MassDOT and Town	Yes	3	5.5	0	6	2.00	BAT Route 12	Yes	The entire segment	Arterial Coordination Study, CTPS study (2010)	3	2	2	4	0	1	12	Medium	The location has several MassDOT projects and CTPS studies and it is not recommended for study.
Titodio 20	ransopri	1140		masser and rown	. 100		0.0			2.00	MBTA bus Route	es	iloo Waliii Ee Eerioo.	The first of the f		-	-					Wodam	on a statute and his not recommended for stady.
											110, 116, 117, 119, 424, 426,												
											428, 448, 449, 450, 455, and 45	59		DCR announced a \$500,000 comprehensive study of the parkway system for bike lanes in FFY 2015. The goals of the study include updating traffic	r								
											MBTA Rapid Transit on Blue			information, assessing parkway conditions, and assessing and understanding deficiencies along the heavily cycled parkways.	9								This arterial segment was not selected because it is part of the Mystic River Working Group Study. In
Route 16 (Revere											Line		Yes	The Wynn Everett DEIR (2015) includes intersection improvements and									addition, the Wynn Everett DEIR (2015) includes intersection improvements and mitigated traffic
Beach Parkway)	Revere	ICC	4	DCR	Yes	2	1.5	0	4	1.86	MBTA Commute Rail at Chelsea		The entire segment lies within EJ Zones.	mitigated traffic operations for Revere Beach Parkway and Mystic Valley	2	1	3	4	1	1	12	Medium	operations for Revere Beach Parkway and Mystic Valley Parkway.
																							and LRTP outreach in 2012, 2013, and 2014 by
											MBTA bus Route	е	Voo										MAGIC; a formal letter was submitted and verbal comments were made at an MWRC subregion meeting.
											MBTA Commute	er.	Yes An EJ Zone is locate	1									The location was resubmitted in a comment on Draft
Route 20	Weston	MWRC	6	MassDOT	Yes	3	2.6	0	2	3.06	Rail at Waltham and Kendal Gree		0.1 mi from the end of the segment.		1	2	2	4	1	2	12	Medium	FFY 2014 UPWP and was suggested in the 2017 MPO outreach program.
						-					MBTA bus Route 75, 71, 72, 73, 7	es		The project		_							,
											and 78	.,		DCR announced that the agency will conduct a traffic study of several intersections along Mount Auburn Street and Fresh Pond Parkway, in									
											MBTA Red Line Rapid Transit		Yes	partnership with the City of Cambridge and the MBTA. The study will focus o safety measures, bus prioritization, and accessibility.	on								The Fresh Pond Residents Alliance identified Fresh Pond Parkway and Alewife Brook Parkway as
Route 2											MBTA Commute	er	Two EJ zones are	Conceptual TIP project #987 would acquire Minuteman Path right-of-way in									locations in need of transportation improvements. Concerns include pedestrian safety of young students
(Fresh Pond Parkway)	Cambridge	ICC	6	DCR	Yes	2	1.8	1	3	1.31	Rail at Porter Square	Yes	located within 0.5 miles of the segment	Watertown to connect Minuteman Bikeway from Arlington, Cambridge, and . Watertown to Dr. Paul Dudley White Bike Path in Boston.	3	1	2	4	0	1	11	Medium	who walk to Shady Hill School because of high traffic volumes, environmental issues, and lack of livability.
											MBTA bus Route												
											70, 70A, 71, 73, 86, and 701												
											MBTA Rapid												
											Transit available on the Red and												
											Green Lines												
											MBTA Commute Rail at North		Yes										
Memorial											Station, Back Ba Yawkey, Porter	ay,		DCR announced a \$500,000 comprehensive study of the parkway system for bike lanes in FFY 2015. The goals of the study include updating traffic									
Drive (Route 2 and 3)	Cambridge	ICC	6	DCR	Yes	2	3.6	0	4	3.99	Square, and Belmont	Yes	to EJ Zones.	t information, assessing parkway conditions, and assessing and understanding deficiencies along the heavily cycled parkways.	3	2	1	4	1	0	11	Medium	None

TABLE 1

Arterial Segments Considered for Study: Priority Corridors for Long-Range Transportation Plan Needs Assessment Study
(Arterial Segment Selected for Study Is Highlighted in Green)

							1						(Arteria	al Segment Selecte	d for Study Is Highlighted in Green)									
						National			Number of Top-200 Hig		Travel-			or Near										
Arte Seg		ommunity	MAPC Subreg	MassDOT jion District	Jurisdiction	Highway System	Functional Class*	Crash Rate (MVMT)	Crash Locations 2012–14	Number of HSIP-Eligible Crash Clusters 2012–14**		ransit Service	Crowded or En Late Bus Ju		Study, Project, or TIP Project	Safety Conditions	Congested Conditions	Multimodal Significance	Regional Significance	Regional Equity	Implementation Pote	ntial Score	Priority Rating	Summary of Comments
Rou	99 E	verett	ıcc	4	City	Yes	3	2.6	0	3	st M 9: 1:	00 MBTA bus stops MBTA bus Routes 97, 104, 105, 109, 110, 112, 99, and 06	Ye Th	es ne entire segment	MassDOT Project #602383 reconstructed Route 99 with a traffic signal upgrade, from Second Street to the Malden city line in 2008; completed autumn 2007; All work is complete except punch list work (as of 02/15/2008) MassDOT Project #601580 reconstructed Route 99 from Sweetser Circle to Second Street in 2004; completed in summer 2004. MassDOT Project #602382 reconstructed Route 99 from Sweetser Circle to the Alford Street Bridge in 2013; completed spring 2013.	2	2	2	4	o	1	11	Medium	Not recommended for study because the MassDOT projects listed completely reconstructed Route 99 with signal improvements from Alford Street Bridge to the Malden city line.
Bout	:1 N	anuard.	TRIC		MassDOT	Yes	2	0.8			R D C N N	MBTA Commuter Rail at Islington, bedham Corp Center, Endicott, Vorwood Depot, Vorwood Central, Vindsor Gardens, and Plimptonville	On adj soi	es ne EJ zones lies djacent to the outhern end of the	MassDOT's I-95 South Corridor Study, provided a comprehensive evaluation of the I-95 and Route 1 corridors south of Route 128 that included a recommended plan of short-term and long-term improvements (June 2010) MassDOT Project #608052, Route 1 at Morse Street (approved by PRC Nov 2014); in preliminary design MassDOT Project #605857, Route 1 at University Avenue and Everett Street, Town design is at pre-25% MassDOT Project #605321, Bridge Preservation, Route 1 over the Neponset							44	Medium	The location has MassDOT projects and studies and it is not recommended for study.
	· 1A R		ICC	4	MassDOT	Yes		0.0			st M 1: 4: 4: 4: M Ti Li	MBTA bus Routes 110, 116, 117, 111, 424, 426, 139, 441, 442, 148, 449, 450, IMBTA Rapid Transit on Blue ine	Ye Th	es ne entire segment	River; in design stage CTPS Lower North Shore Transportation Improvement Study proposed improvements for Route 1A in Revere in October 2000; an update may be necessary. Conceptual TIP Project #982, Mahoney Circle (Bell Circle) Grade Separation		2	2		0			Medium	This arterial segment was not selected because it is part of the Mystic River Working Group Study. In addition, the Wynn Everett DEIR (2015) includes intersection improvements and mitigated traffic operations for Revere Beach Parkway and Mystic Valley Parkway.
Rou	TIA K	evere		4	MassuO1	Yes		2.1	U		м	MBTA Commuter MBTA Commuter Rail at Sharon and	Ye On adj	es ne EJ zones lies tjacent to the	MassDOT's I-95 South Corridor Study presented a comprehensive evaluation of the I-95 and Route 1 corridors south of Route 128 and included a recommended plan of short-term and long-term improvements (June 2010) MassDOT Project #608480, Resurfacing and related work on Route 1; in preliminary design MassDOT Project #608599, Stormwater Improvements to treat discharges from Route 1, I-95 and Route 1A to the Neponset River and an Unnamed		2	2	4	U			Medium	The location has MassDOT projects and studies and was not recommended for study by MassDOT
Rou	1 W	/alpole	TRIC	6	MassDOT and Town	Yes Yes	3	6.7	0	2	1.53 W	Valpole MBTA Commuter Rail at Natick, Vellesley Square, and Wellesley Hills MWRTA bus Route 8	Ye Mc lies	egment. es ost of the segment is adjacent to EJ	Tributary; in preliminary design No projects	3	1	2	3	1	1	11	Medium Medium	Highway District 5.
Roul	:2 A	cton	MAGIC	3	MassDOT	Yes	2	1.3	0	1	R A	MBTA Commuter Rail at South Acton and West Concord	N/A Ye		MassDOT Project #604472, Resurfacing and Related Work on Route 2 (includes all of Acton); completed in spring 2014 MassDOT Project #607748, Intersection and Signal Improvements on Route and Route 111 at Piper Road and Taylor Road; in preliminary design MassDOT Project #604609, Traffic Sign Replacement and Safety Improvements on Route 2; completed in summer 2009 TIP Project #606223, Bruce Freeman Rail Trail Construction (Phase II-B) in Acton and Concord to connect the trail across Route 2, programmed in FFY 2018 TIP	2	1	2	4	1	1	10	Low	Location has MassDOT projects. A MassDOT road safety audit is scheduled for the Piper Road/Taylor Road intersection; the project is in the preliminary design phase. The MAGIC subregion expressed interest in a Route 2 study.
	: 62 B		MAGIC	4	MassDOT and Town	n No	5	7.0	0	0	TI	Three MBTA bus stops MBTA bus Route	Yes No		Great Road Project: Master Plan and Conceptual Design, prepared by Vanasse Hagen Brustlin Inc. (VHB) for the Town of Bedford in 2011. The plar was to improve pedestrian and bicycle access, recommend streetscape improvements that would highlight the "center" of Bedford while taking into consideration traffic flow through the area, crosswalk locations, intersection and traffic control improvements, property access, and parking.	2	2	2	2	1	1	10	Low	Forms part of Routes 4 and 225 arterial segment.
												MWRTA bus			MassDOT Project #605745, Reconstruction of Route 16 from Quail Run to th Sherborn town line; in preliminary design MassDOT Project #602462 will enhance safety and improve efficiency by installing a new traffic signal at the intersection of Route 16 at Route 126 and at Oak Street in Holliston; 25% design stage (as of 12/08/1999) 2011 CTPS study, Route 126 Corridor: Transportation Improvement Study									Location has MassDOT projects and CTPS studies, which have not been implemented. The 495/MetroWest Partnership expressed interest in a Route 16 study. The section that experiences the most crashes is the town center portion (under town jurisdiction). A road safety audit was performed for the town center
	: 16 H		MWRC MWRC	3	MassDOT and Town	Yes Yes	3	6.7	1	3	M R M R	MWRTA bus Routes 10 and 11 MBTA Commuter Rail at Natick and			2008 CTPS study, Washington Street (Route 16/126) at Hollis Street MassDOT Project #600573 reconstructed Route 135 in Natick in 2008. More extensive improvements were proposed in the downtown area, on East Centr Street between North Main Street and Union Street, including signal upgrades new sidewalks, pavement rehabilitation, and shoulders; Contract #32302 was completed; all construction operations have been suspended (as of 06/30/2007) 2010 CTPS study, West Central Street (Route 135) at Speen Street.	al s,	1	2	1	1	1	10	Low	portion in December 2012. Congestion in the downtown area; likely focus area would be on the intersection of Route 135 at Route 27 and the intersection of Route 135 at Speen Street because of the crash history of those locations.

TABLE 1 Arterial Segments Considered for Study: Priority Corridors for Long-Range Transportation Plan Needs Assessment Study (Arterial Segment Selected for Study is Highlighted in Green)

												(Arterial Segment Selec	cted for Study Is Highlighted in Green)								
Arterial Segment Community	y MAPC Subregi	MassDOT on District	Jurisdiction	National Highway System	Functional Class*	Crash Rate (MVMT)	Number of Top-200 H Crash Locations 2012–14	ligh- Number of HSIP-Eligible Crash Clusters 2012–14**		Transit Service	Crowde	In or Near d or Environmental s Justice Zone	Study, Project, or TIP Project	Safety Conditions	Congested Conditions	Multimodal Significance	Regional Significance	Regional Equity	Implementation Potential	Priority Score Rating	
										11 MBTA bus stops MBTA bus Route											
										MBTA Commuter Rail at Wakefield,											
Route 129 Reading	NSPC	4	MassDOT and Town	Yes	3	3.9	0	1		Reading, and Woburn	Yes	None	No projects	2	2	2	1	2	1	10 Low	None
													MassDOT Project #601586, Intersection Improvements at Route 9 (Worchester Street) and Oak Street, from 1500 feet West of Oak Street to 300 feet East of Overbrook Drive; construction ended in spring 2015								
										MBTA Commuter			MassDOT Project #607340, Resurfacing on Route 9, from Dearborn Street to the Natick town line; in preliminary design	о							
										Rail at Wellesley Hills and Wellesley Farms	у		MassDOT Project #606530, Drainage Improvements along Route 9 Boulder Creek Culvert (Design Only); 25% design stage (as of 06/10/2015)								
	NIN DO			.,		0.5				MWRTA bus			CTPS study: Route 9 Corridor in Wellesley, 2003								MassDOT has a preliminary assessment of this corridor that will develop into 25% design plans for
Route 9 Wellesley	MWRC	6	MassDOT	Yes	2	3.5	0	11	1.76	Route 1	None	None	MAPC Land Use/Corridor Study (fall 2013)	2	1	2	3	1	1	10 Low	roadway improvements. Location was suggested in 2014 LRTP outreach at a 495/MetroWest Partnership meeting.
													2002 CTPS study, Traffic Congestion in SWAP Subregion: Sherborn Town Center Traffic-Flow Improvement Study								The section that experiences the most crashes and congestion is the town center portion, where Route
Route 16 Sherborn	SWAP	3	Town	Yes	3	1.3	0	1	1.96	None	N/A	None	Conceptual TIP #915, Washington Street (Route 16)	1	1	1	3	1	2	9 Low	16 and Route 27 combine and split.
Route 62 Concord	MAGIC	4	Town	Yes	3	43	0	0		MBTA Commuter Rail at Concord and West Concord		None	No projects	2	2	1	1	1	1	8 Low	None
Notice 02 Concord	MAGIC	4	Town	165	3	4.5		0		GATRA bus	u IVA	None		2	2	1		'		o Low	None
Route 3A Marshfield	SSC	5	MassDOT	Yes	3	2.0	0	2		MBTA Commuter Rail at Greenbush	n None	None	The corridor is within the limits of MassDOT Project #605664, Resurfacing ar Related Work on Route 3A (Duxbury town line northerly to Scituate town line) work includes patching and microsurfacing, shoulder reconstruction, and drainage structures; 100% design stage; no construction funding identified	nd),	1	2	2	1	1	8 Low	None
																					The 495/MetroWest Partnership expressed interest in a Route 16 study. Specific issues in this segment include improvements to accommodate pedestrians
Route 16 Natick	MWRC	3	Town	Yes	3	1.5	0	0	1.21	None	N/A	Yes	No projects	0	0	2	3	1	2	8 Low	and bicyclists.
										MBTA Commuter			MassDOT's I-95 South Corridor Study, provided a comprehensive evaluation of the I-95 and Route 1 corridors south of Route 128 that included a recommended plan of short-term and long-term improvements (June 2010)								
Route 1 Sharon	TRIC	5	MassDOT	Yes	3	1.3	0	1		Rail at Sharon and Walpole	d N/A	None	MassDOT Project #603622, Bridge Rehabilitations, Route 1/Route I-95; completed in 2010	1	1	3	2	0	1	8 Low	Segment has MassDOT projects and studies.
													MassDOT's I-95 South Corridor Study provided a comprehensive evaluation of the I-95 and Route 1 corridors south of Route 128 and included a recommended plan of short-term and long-term improvements (June 2010)	of							
Route 1 Westwood	TRIC	6	MassDOT	Yes	3	1.1	0	0	2.49	None	N/A	None	MassDOT Project #603162, Route 128 Add-a-Lane Bridges (Bridge III), Route 1 and 1A over I-95/128; completed in 2012 MAPC Land Use/Route 9 Corridor Study (fall 2013).	0	2	2	3	0	1	8 Low	Segment has MassDOT projects and studies.
													The CTPS Safety and Operations at Intersections study evaluated congestior and safety issues at the Route 9/Oak Hill Road/Central Street intersection in FFY 2012.	n							
													MassDOT's I-495/Route 9 study, November 2013. The western section of Route 9 in Southborough between the I-95 interchange and Crystal Pond Roa was evaluated for short-term and long-term improvements as part of this study.	ad							
Route 9 Southborou	igh MWRC	3	MassDOT	Yes	2	1.4	0	0		MWRTA bus Route 7	None	None	MassDOT Project #607172, Resurfacing and Related Work on Route 9, from Westborough to just west of White Bagley Road; construction ends in summe 2016	n er 0	2	2	2	1	0	7 Low	Most of the intersections on this corridor have already been studied, as MassDOT District 3 has noted.
													FFY 2013 Subregional Priority Corridor Study								
Route 3A Scituate	SSC	5	MassDOT	Yes	3	1.2	0	0		MBTA Commuter Rail at Greenbush North Scituate, and Cohasset		None	The corridor is within the limits of MassDOT Project #605664, Resurfacing ar Related Work on Route 3A (Dubury town line northerly to Scituate town line) no construction funding identified. Work includes patching and microsurfacing, shoulder reconstruction, and drainage structures; 100% design stage.);	0	2	1	1	1	5 Low	The FFY 2013 Subregional Priority Corridors Study was conducted within the segment. MassDOT District 5 comments refer to MassDOT Project #605664 (in the 100% design stage).

Abbreviations

AADT = Annual average daily traffic. ADA = Americans with Disabilities Act. ADT = Average daily traffic. BAT = Brockton Areas Transit Authority. CTPS = Central Transportation Planning Staff. DCR = Department of Conservation and Recreation. DEIR = Draft Environmental Justice. ENHC = Essex National Heritage Commission.

EPDO = Equivalent property damage only. FFY = Federal fiscal year. GATRA = Greater Attlebor o Taunton Regional Transit Authority. MSIP = Highway Safety Improvement Program. ICC = Inner Core Committee. LRTP = Long-Range Transportation Planning Advisory Group on Interlocat Coordination. MAPC = Metropolitan Area Planning Council.

MassaChusetts Department of Transportation. MBTA = Massachusetts Bay Transportation Authority. MPC = Boston Region Metropolitan Planning Organization. MVMT = MetroWest Regional Collaborative. MWRTA = MetroWest Regional Transit Authority. MSPC = South Shore Coalition. SWAP = South Shore Coalition. SWAP = South Shore Coalition. SWAP = South Shore Coalition. DEIR = Transportation Improvement Program. TRIC = Three Rivers Interlocal Council. UPWP = Unified Planning Work Program.

Selection Criteria
Safety Conditions: Segment has a high crash rate for its functional class, contains an HSIP-eligible crash location, a top-200 high-crash location, and/or a significant number or HSIP-eligible clusters of pedestrian or bicycle crashes.

Congested Conditions: Segment has a Travel-Time Index of at least 1.3 and/or of at least 2.0,that is, which signify that it experiences delays during peak periods.

Multimodal Significance: Segment supports transit or bicycle or pedestrian activities, has a need to improve these activities, and/or has a high volume of truck traffic serving regional commerce.

Regional Significance: Segment is in the National Highway System, carries a significant proportion of use within 0.5 miles of Emvironmental Justice transportation analysis zones, and/or is essential for regional economic, cultural, or recreational development in the area.

Regional Equity: Location is in a subregion that has not had a priority corridor study before, or location is in a subregion that has not had a priority corridor study before, or location is in a subregion that has not had a priority corridor study before, or location is in a subregion that has not had a priority corridor study before, or location is in a subregion that has not had a priority corridor study in the last three years.

Implementation Potential: Improvements to the segment are proposed or endorsed by the roadway administrative agency (agencies), proposed or endorsed by the subregion and are a priority for the subregion, and/or have strong support from other stakeholders.

*Functional Class

2 = principal arterial; 3 = principal arterial other (rural minor arterial or urban principal arterial); 5 = minor arterial (urban minor arterial or rural major collector)

Number of HSIP-eligible crash clusters

**HSIP-eligible crash clusters are defined by MassDOT as crash clusters that rank within the top five percent of crash clusters for each regional planning agency, based on the Equivalent Property Damage Only (EPDO) index. In the EPDO index, property damage only crashes are awarded one point each, crashes involving injuries are given five points each, and fatal crashes are given 10 points each. In the Boston region, the 896 intersections in the top five percent have crash clusters with a minimum EPDO value of 42.

Source: Central Transportation Planning Staff.





Central Transportation Planning Staff (CTPS) to the Boston Region MPO: www.ctps.org | 857.702.3700 | ctps@ctps.org

Ryan Hicks, Congestion Management Process Manager: www.ctps.org/cmp | 857.702.3661 | rhicks@ctps.org

Casey Claude, Bicycle and Pedestrian Program Manager: www.ctps.org/livability | 857.702.3707 | cclaude@ctps.org

Pedestrian Report Card Assessment (PRCA):

Roadway Segment

Roadway Segment Location

Route 138 - Milton, MA

Grading Categories	Score	Rating
Safety	2.8	Good
System Preservation	N/A	Fair
Capacity Management and Mobility	1.5	Poor
Economic Vitality	1.5	Poor

Transportation E	quity
High Priority Area	✓
Moderate Priority Area	
Not a Priority Area	

Category Ratings

Good: Score of 2.3 or more (maximum 3.0)

Fair: Score is between 1.7 and 2.3 Poor: Score is 1.7 or less (minimum 0)

Grading Categories: Scoring Breakdown Roadway Segment

Capacity Manageme	nt an	d Mol	oility
Performance Measure	Weight	Rating	Weighted Score
Sidewalk Presence	3	Fair	6
Crossing Opportunities	2	Poor	2
Walkway Width	1	Poor	1
Total	6		9

Economic V	⁄itali	ty	
Performance Measure	Weight	Rating	Weighted Score
Pedestrian Volumes	1	Fair	2
Adjacent Bicycle Accommodations	1	Poor	1
Total	2		3

Category rating = total rating/total weight Rating Score: Good = 3 Fair = 2 Poor = 1

Safety			
Performance Measure	Weight	Rating	Weighted Score
Pedestrian Crashes	3	Good	9
Pedestrian-Vehicle Buffer	1	Good	3
Vehicle Travel Speed	1	Fair	2
Total	5		14

System Prese	ervation
Performance Measure	Rating
Sidewalk Condition	Fair

Transportation Equity Priority			
Area Condition	Yes/No		
Environmental Justice zone?	✓		
School or college within one-quarter mile?	✓		
More than 8.9% of population older than 75 years?	✓		
More than 27.5% of households do not own a vehicle?			

Category Ratings

Good: Score of 2.3 or more (maximum 3.0)

Fair: Score is between 1.7 and 2.3 Poor: Score is 1.7 or less (minimum 0)

Detailed Performance Measure Information: Roadway Segment

Goal	Performance Measure	Features of Analyzed Locations
Mobility	Sidewalk Presence	Sidewalks are present on one side of the street
	Crossing Opportunities	9 crosswalks/ 3.6 miles = 2.5 crosswalks per mile
	Walkway Width	4 foot sidewalks
Economic Vitality	Pedestrian Volumes	Estimated 5 to 60 pedestrians
Safety	Adjacent Bicycle Accommodations	Some bike lanes are present at the southern portion of the corridor but the bike lanes are inconsistent
	Pedestrian Crashes	Not in HSIP cluster
	Pedestrian-Vehicle Buffer	13 feet
	Vehicle Travel Speed	32 MPH
System Preservation	Sidewalk Condition	Fair

From: John Thompson

Sent: Friday, October 20, 2017 9:21 AM

To: sasante@ctps.org

Cc: geraldine.vatan@state.ma.us; Chase Berkeley; Michael D. Dennehy; William Clark

Subject: Rt. 138 corridor study extension and Rt. 28 corridor study - Milton

Good Morning Seth,

Over the past few weeks I have had a few conversations with Geri Vatan at MassDOT District 6 about an ongoing corridor study for Route 138 in Canton and the possibility of extending the study to include Milton. My understanding is that MassDOT will be undertaking a resurfacing project in FY19 for Route 138 through Canton and Milton, and would like to incorporate additional complete streets principles and improvements that may be identified by extending the corridor study. I am writing to relay Milton's enthusiastic support for this effort. The town would benefit greatly from any improvements to this corridor that would increase efficiency and increase accessibility for all users, including bicyclists and pedestrians, especially given the number of area amenities and destinations directly adjacent to Rt. 138 (Blue Hills Ski Area, DCR's Blue Hills Reservation and Trailside Museum, and Curry College to name a few).

In addition to supporting the Route 138 study, I would also like to request that State Route 28, primarily the section between I-93 and Reedsdale Road in Milton, be considered for a corridor study as well. Officials from the Town recently met with MassDOT to discuss ongoing safety issues along this section of state highway. There was recently another fatality on the roadway (there have been numerous fatalities over the past several years) in addition to many other accidents that seemingly occur on a regular basis. I know that an intersection project at the intersection of Randolph Avenue and Chickatawbut Road is already moving forward, which is fantastic, but we believe the entire corridor should be looked at for possible improvements. The corridor sees a tremendous amount of cut-through traffic which floods the town with vehicles looking to avoid and bypass the Braintree split in both the AM (northbound) and PM (southbound). The cut through traffic is travelling at high rates of speed on a four lane highway through a residential neighborhood with many driveways, no shoulders, no accommodations for bikes, and very uncomfortable conditions for pedestrians. Currently, the layout of Route 28 does very little to promote safe driving habits. Compounding the issue is the fact that a lot of traffic, particularly during peak hours, is finding its way onto smaller neighborhood streets to avoid queues and delays due to high volume. We feel that a corridor study would be a very logical and beneficial first step to begin addressing these issues.

Thank you for your attention to this matter and please feel free to reach out to me directly if you have any questions or would like any further information.

Respectfully,

John P. Thompson, P.E. Town Engineer

Town of Milton – Engineering Dept. 525 Canton Avenue Milton, MA 02186

(617) 898-4869

From: Vatan, Geraldine (DOT)

Sent: Thursday, October 19, 2017 10:33 AM **To:** Seth Asante (sasante@ctps.org)

Cc: Rose, Marie (DOT); Paul, Andrew (DOT); Polin, Bonnie (DOT); Sutton, Peter (DOT);

Dwyer, Courtney (DOT)

Subject: FW: Route 138 Corridor Study Canton-Milton

Attachments: Emailing: Ma Ped Plan_DRAFT Corridor Analysis (2).jpg (96.4 KB)

Hi Seth,

I am writing to update you regarding Route 138 in Canton and Milton. As you know, there is a corridor study underway in Canton and Milton is a potential study for next year. I would like to re-iterate MassDOT's support of the Milton corridor study. OTP is developing a Statewide Pedestrian Plan, ranking corridors for improvement. Route 138 Canton-Milton has been ranked as a high priority corridor and as such may be eligible for additional funding (see attached). In the email below statements relating to support for the corridor study have been highlighted. It is my understanding that the Town of Milton has expressed their support for this study to you as well.

Thank you and I look forward to learning what CTPS decides on this issue.

Geri

Geraldine Vatan | District 6 Project Development Engineer

185 Kneeland Street Boston, MA 02111 | Office (857) 368-6115 | Cell (508) 330-1078 MassDOT Highway Division geraldine.vatan@dot.state.ma.us

From: Polin, Bonnie (DOT)

Sent: Tuesday, October 3, 2017 8:36 AM

To: 'sasante@ctps.org'
Cc: Vatan, Geraldine (DOT)

Subject: Route 138 Corridor Study Canton-Milton

Seth – Good morning. I understand CTPS will be conducting a corridor study of Route 138 in Canton. It is perfect because there is a resurfacing job of Canton/Milton Route 138 on the STIP for 2020 (608484). It would be great if we could incorporate the recommendations and actually make the corridor study applicable. Therefore, is your intention to actually pull the crashes? Is it possible to conduct the RSAs for the HSIP eligible locations along the corridor? If not, let me know as soon as possible so we can do it (but it would make sense to have one as part of the corridor rather than piecemeal). Furthermore, because the resurfacing job is for both Canton and Milton, is there a chance you could extend the corridor study to cover the area of the project and then add the HSIP clusters. If not, let me know. We want to work with CTPS to make this effective for Milton, Canton, MassDOT and CTPS.

Also, just so you know, 2015 just closed so we will be updating the high crash cluster map.

Thanks, Bonnie



Bonnie Polin, Manager Highway Safety Programs *MassDOT* | Highway Division | Traffic Safety Section 10 Park Plaza Suite 7210 | Boston, MA 02116 Phone: 857-368-9636 | Fax: 857-368-0628

Email: Bonnie.Polin@state.ma.us

Part 3: Public Participation

Route 138 Priority Corridor Study in Milton

Blute Conference Room

Milton Town Hall,

July 12, 2018

Name	Affiliation	<u>Email</u>
John Thompson	Town of Milton	jthompson@townofmilton.org
William Clark	Town of Milton	wclark@townofmilton.org
Chase Berkeley	Town of Milton	cberkeley@townofmilton.org
Michael D. Dennehy	Town of Milton	mdennehy@townofmilton.org
John King, Police Chief	Town of Milton	jking@mpdmilton.org
Mark Alba	Town of Milton	malba@mpdmilton.org
√John Grant, Fire Chief	Town of Milton	jgrant@townofmilton.org
Tindsey Barbee	GPI	lbarbee@gpinet.com
Diaz, John	GPI	jdiaz@gpinet.com
Karl Pastore	DCR	Kpastore@MassMail.state.ma.us
Paul Jahnige	DCR	paul.jahnige@state.ma.us
Michael Clark	MassDOT—Planning	michael.clark@state.ma.us
Cassandra Gascon	MassDOT—Planning	Cassandra.Gascon@dot.state.ma.us
Bryan Pounds	MassDOT—Planning	bryan.pounds@state.ma.us
Raj Kulen	MassDOT—District 6	raj.kulen@state.ma.us
Courtney Dwyer	MassDOT—District 6	courtney.dwyer@state.ma.us
Geraldine Vatan	MassDOT—District 6	geraldine.vatan@state.ma.us
Amitai Lipton	MassDOT—District 6	amitai.lipton@state.ma.us
Hameed Pervez	MassDOT—District 6	hameed.pervez@state.ma.us
Muazzez Reardon	MassDOT—Project Development	muazzez.reardon@state.ma.us
Bonnie Polin	MassDOT—Safety	bonnie.polin@state.ma.us
Connor Keating	MassDOT—Safety	connor.keating@state.ma.us
Mark Abbott	Boston Region MPO	mabbott@ctps.org
Seth Asante	Boston Region MPO	sasante@ctps.org
Ben Erban	Boston Region MPO	berban@ctps.org

Route 138 Priority Corridor Study in Milton
Blute Conference Room
Milton Town Hall
July 12, 2018

Meeting Summary

- MassDOT/GPI is considering a "shared use trail" or "shared use sidewalk" concept on Route 138 south of Canton Avenue. This would take advantage of the relatively low bicycle and pedestrian volumes on this stretch and replace the proposed dual bike lanes and dual sidewalks with a single path. That would leave room for a second northbound travel lane which would complement the second northbound lane previously added up to the park-and-ride lot.
- There was also some thought about continuing the shared use trail concept past Neponset Valley Parkway. That would preserve the right-of-way which may be tight at this intersection. In this area it would make sense to keep it on the east side of the road since there's it would provide access to a pond there, and also most of the utility poles are already located on the west side of the road.
- Courtney Dwyer would provide CTPS (Seth Asante) with more information about the shared use trail or shared-use sidewalk concept.
- The island put in at Atherton Street in front of the fire station was installed to prevent Stop and Shop trucks who missed the turn onto Neponset Valley Parkway from using the space to make a U-turn. (Stop and Shop is gone now.)
- Raj Kulen asked if it might be worth moving the existing pedestrian signal at Aberdeen Road away from the intersection to make it more obvious that it's a mid-block crossing.
- Raj was also concerned how the residents would react to losing the ability to park in the shoulder, particularly on the north portion of the corridor.
- There was a discussion about the regional model projections, no future growth expected in the corridor.
- John Diaz expressed uncertainty about constructability due to factors like retaining walls or existing pavement widths.
- This project is programmed for the 2020 TIP.
- Next steps
 - o Comments by Friday July 20
 - o Include shared use trail/sidewalk concept and second northbound lane
 - Draft report sent out mid/late September

Route 138 Priority Corridor Study in Milton Blute Conference Room Milton Town Hall, February 20, 2018

In Attendance:

Name Af	filiation	<u>Email</u>
John Thompson	Town of Milton	jthompson@townofmilton.org
William Clark	Town of Milton	wclark@townofmilton.org
Chase Berkeley	Town of Milton	cberkeley@townofmilton.org
Michael D. Dennehy	Town of Milton	mdennehy@townofmilton.org
Diaz, John	GPI	jdiaz@gpinet.com
Lindsey Barbee	GPI	lbarbee@gpinet.com
Michael Clark	MassDOT—Planning	michael.clark@state.ma.us
Cassandra Gascon	MassDOT—Planning	cassandra.gascon@dot.state.ma.us
Raj Kulen	MassDOT—District 6	raj.kulen@state.ma.us
Courtney Dwyer	MassDOT—District 6	courtney.dwyer@state.ma.us
Hameed Pervez	MassDOT—District 6	hameed.pervez@state.ma.us
Muazzez Reardon	MassDOT—Projects	muazzez.reardon@state.ma.us
Bonnie Polin	MassDOT—Safety	bonnie.polin@state.ma.us
Connor Keating	MassDOT—Safety	connor.keating@state.ma.us
Mark Abbott	Boston Region MPO	mabbott@ctps.org
Seth Asante	Boston Region MPO	sasante@ctps.org
Ben Erban	Boston Region MPO	berban@ctps.org

Meeting Summary

Summary of study tasks and expected completion date

- Collect stakeholder input throughout length of project.
- Collect data: intersection geometry, signal timings, turning movement counts (TMCs) for winter and spring, automatic traffic recorder (ATR) counts, spot speed studies, crash data, community survey data – by April
- Analyze existing conditions by May
- Develop conceptual improvements by June
- Prepare document by July
- Reviews and editing by September

Issues and concerns raised

- Safety concerns the corridor experiences high number of crashes at many of the intersections. CTPS staff will perform further analysis on the crash data using the new methodology from the Highway Safety Manual. The following locations were expressed as areas of particular interest:
 - <u>Left turn onto Milton Street</u> many accidents are reported at this intersection
 - Parking along Route 138 it is not allowed, but the shoulders are used for parking anyway between Blue Hill Terrace and Route 28. The parked cars crowd the travelled way and are a safety risk. The corridor lacks "No Parking" signs. Parking enforcement is state highway jurisdiction, but is enforced by town police.
 - Curry College entrance (Blue Jay Way) this location lacks sidewalks and sees a fair amount of pedestrian traffic, especially from people using the bus stops on the Route 716. Installing sidewalks in the vicinity was previously planned but not completed.
 - <u>Pedestrian crossing at Oak Street</u> Tucker Elementary School generates significant pedestrian traffic at this location. There is no pedestrian signal for crossing Route 138; usually a crossing guard is employed during school hours.
 - Milton is considering asking this to either be designated a school zone or installing some kind of pedestrian signal. It was proposed to move the signalized crossing at Aberdeen Road to this location to better reflect usage patterns.
 - Pedestrian/bicycle accommodations at Blue Hill Recreational Area the lack of these facilities (sidewalks, crosswalks, and bicycle lanes) leading up to the recreation area are a safety and comfort issue for visitors.

- Milton Fire Station at Bradlee Road/Atherton Street employing signal preemption at the existing traffic signal could improve safety and responsiveness for emergency vehicles leaving the garage.
- No additional RSAs should be needed. Bradlee Road/Atherton Street intersection and Milton Street/Dollar Street intersection were part of previous Road Safety Audits. Both RSAs are available online and Seth Asante has the link.
- Include the fire and police chiefs on the project team in order to have their input.
- Congestion the entire corridor has congestion issues. Conditions are often just as bad on weekends due to recreational activity. Particular areas of concern are:
 - Signal optimization the signal timing plans are several decades old and will need to be updated. In particular, the signal at Royall Street intersection was brought up as a bottleneck location due to the north/south movements not receiving adequate green time.
 - <u>Cut-through traffic</u> during the PM peak period, drivers will often take Blue Hills Parkway instead of Route 138 southbound to avoid congestion, and then proceed back to Route 138 via Canton Avenue or the side streets.
 - Route 138 and Canton Avenue has congestion issues
 - Paul's Bridge this bridge, which is west of the corridor where Neponset Valley Parkway crosses the Neponset River, receives high volume traffic and is used by commuters and truckers to bypass major roads into Boston and to the trucking depot down the road in Boston. However, they avoid the Route 138 and Neponset Valley Parkway intersection because it is not signalized.
 - Trucks can use Neponset Valley Parkway unlike all the other parkways in the state.
 - The sports field near Paul's Bridge on Neponset Valley Parkway also sees high traffic volume on weekends.
- Additional issues and concerns from Milton residents were gathered using an online mapping tool and can be viewed here:
 http://wikimapping.com/wikimap/Milton.html. This outreach effort was well received by citizens.

Prior and future projects along the corridor

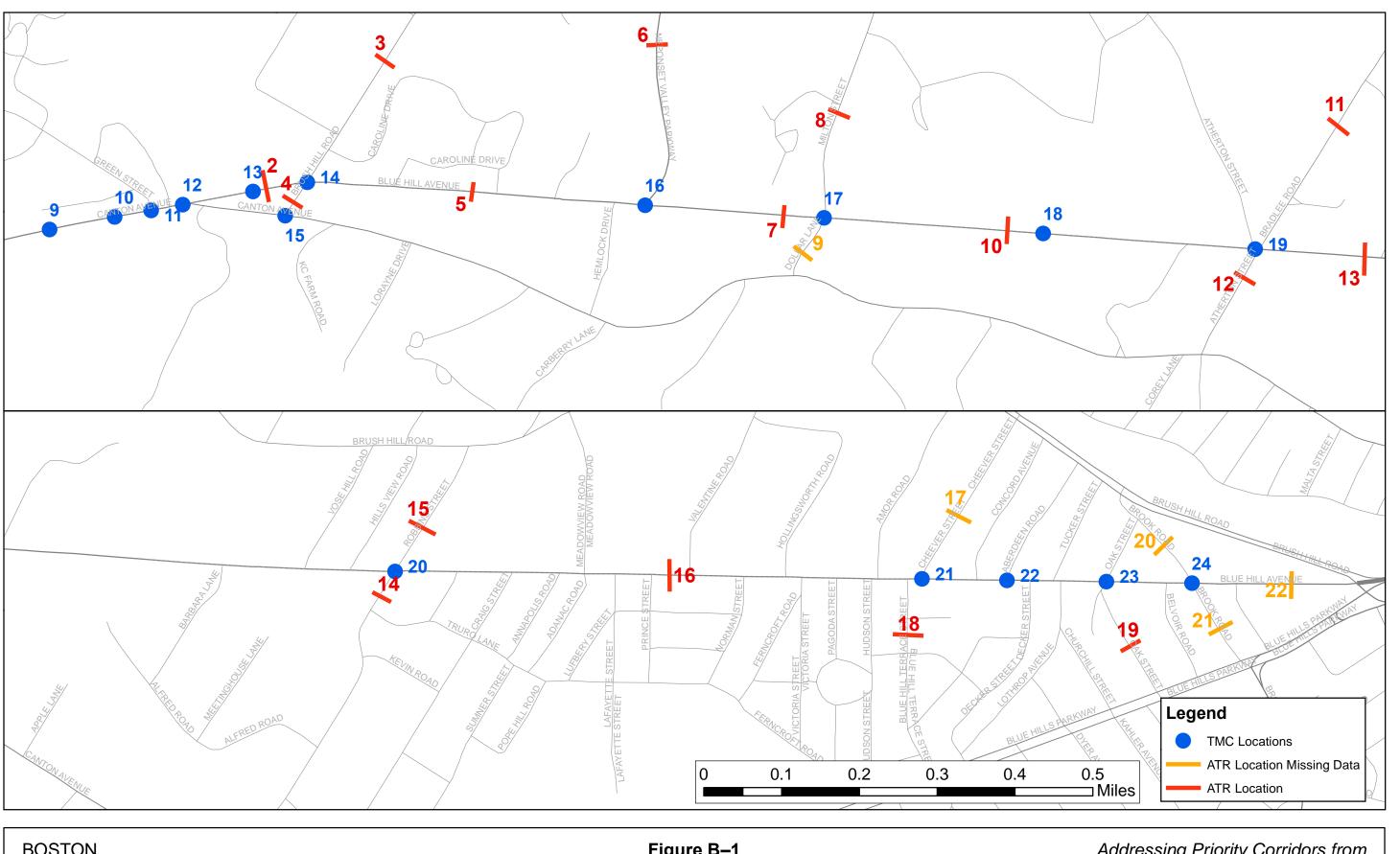
- Prior studies along this corridor include 25% design plans for a signal at Neponset Valley Parkway put together by GPI, and Road Safety Audits at Route 138 intersections at Milton Street/Dollar Street and Bradlee Road/Atherton Street.
- Expanded scope and funding: The Route 138 corridor has come up on lists because of its high pedestrian potential. MassDOT is expanding the scope of the existing resurfacing project for Route 138 to incorporate complete streets objectives. The results of the CTPS investigation will be incorporated into the new project scope. This expansion will require new funding sources which are yet to be determined.
- Water infrastructure upgrades may also be required. In particular, drainage in the 700 block of Blue Hill Avenue (Route 138) was brought up as an issue. Milton will investigate aging water infrastructure on Blue Hill Avenue and plan the timeline for those repairs to align with any resurfacing and other roadway improvements.
- All parties are interested in keeping communication channels open throughout the course of these projects.

Follow-up tasks

- Work with MassDOT Safety to obtain any missing police crash reports along the corridor.
- Have MassDOT Safety assist with using the improved "expected crashes" metric from the Highway Safety Manual for safety analysis. Talk to Connor Keating.
- Should look at the crashes between clusters #5 and #6.
- The winter counts are in March, which may be after the peak skiing season. It may be worth it to reach out to the Blue Hill Ski Area to get historical data so we can figure out what traffic would look like on crowded ski days. DCR has jurisdiction of this property. Contact Karl Pastore for traffic data for the Blue Hill Ski Area on heavy skiing days.

Appendix B: **Traffic Data Collection**

- 1. V`¦}āj*ÁT[ç^{ ^}ơÁÔ[`}ơÁÇVTÔĐÁÖæææÁ 2. Œq[{ææã&ÁV¦ææã&ÁÜ^&[¦å^¦ÁÇCEVÜDÁÖæææÁ



BOSTON REGION MPO Figure B–1
Map of Turning Movement Count and Automatic Traffic Recorder Locations
Route 138 in Milton

Addressing Priority Corridors from the LRTP Needs Assessment: Route 138 in Milton

Part 1: Turning Movement Count (TMC) Data

Study Name Milton - Route 138 and Blue Hills South Parking Lot Entrance TM8 TMC
Start Date Thursday, March 29, 2018 6:00 AM
Saturday, March 31, 2018 5:00 PM
Site Code

			So	uthbou	ınd			V	/estbou	nd			No	rthbou	ind				Crossw	alk
Time Period	Class.	T	L	U		0	R		U		0	R	T	U		0	Total		Pedestrians	T
Peak 1	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	0	
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
5:00 AM - 9:00 AM	Cars	917	1	0	918	1253	0	0	0	0	2	1	1253	0	1254	917	2172	E	0	
One Hour Peak	%	85%	100%	0%	85%	84%	0%	0%	0%	0%	100%	100%	84%	0%	84%	85%	85%		0%	
7:30 AM - 8:30 AM	Light Goods Vehicles	89	0	0	89	165	0	0	0	0	0	0	165	0	165	89	254	S	0	
	%	8%	0%	0%	8%	11%	0%	0%	0%	0%	0%	0%	11%	0%	11%	8%	10%		0%	
	Buses	26	0	0	26	5	0	0	0	0	0	0	5	0	5	26	31		0	
	%	2%	0%	0%	2%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	2%	1%			
	Single-Unit Trucks	23	0	0	23	50	0	0	0	0	0	0	50	0	50	23	73			
	%	2%	0%	0%	2%	3%	0%	0%	0%	0%	0%	0%	3%	0%	3%	2%	3%			
	Articulated Trucks	19	0	0	19	15	0	0	0	0	0	0	15	0	15	19	34			
	%	2%	0%	0%	2%	1%	0%	0%	0%	0%	0%	0%	1%	0%	1%	2%	1%			
	Bicycles on Road	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	1074	1	0	1075	1488	0	0	0	0	2	1	1488	0	1489	1074	2564			
	PHF	0.95	0.25	0	0.95	0.97	0	0	0	0	0.5	0.25	0.97	0	0.97	0.95	0.96			
	Approach %				42%	58%				0%	0%				58%	42%				
Peak 2	Motorcycles	0	0	0	0	1	0	0	0	0	0	0	1	0	1	0	1	N	0	
Specified Period	%	0%	-0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
3:00 PM - 6:00 PM	Cars	1028	2	0	1030	1025	0	0	0	0	12	10	1025	0	1035	1028	2065	E	0	
One Hour Peak	%	81%	100%	0%	81%	88%	0%	0%	0%	0%	92%	91%	88%	0%	88%	81%	84%		0%	
3:30 PM - 4:30 PM	Light Goods Vehicles	199	0	0	199	99	0	0	0	0	0	0	99	0	99	199	298	S	0	
	%	16%	0%	0%	16%	9%	0%	0%	0%	0%	0%	0%	9%	0%	8%	16%	12%		0%	
	Buses	10	0	0	10	10	0	0	0	0	1	1	10	0	11	10	21		0	
	%	1%	0%	0%	1%	1%	0%	0%	0%	0%	8%	9%	1%	0%	1%	1%	1%			
	Single-Unit Trucks	25	0	0	25	15	0	0	0	0	0	0	15	0	15	25	40			
	%	2%	0%	0%	2%	1%	0%	0%	0%	0%	0%	0%	1%	0%	1%	2%	2%			
	Articulated Trucks	6	0	0	6	9	0	0	0	0	0	0	9	0	9	6	15			
	%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	1%	0%	1%	0%	1%			
	Bicycles on Road	0	0	0	0	5	0	0	0	0	0	0	5	0	5	0	5			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	1268	2	0	1270	1164	0	0	0	0	13	11	1164	0	1175	1268	2445			
	.PHF	0.98	0.5	0	0.98	0.88	0	0	0	0	0.54	0.55	0.88	0	0.88	0.98	0.95			
	Approach %				52%	48%				0%	1%				48%	52%				

Study Name Milton - Route 138 and Blue Hills South Parking Lot Entrance TM8 TMC
Start Date Thursday, March 29, 2018 6:00 AM
Saturday, March 31, 2018 5:00 PM
Site Code

			So	uthbou	ınd			V	Vestbou	nd			No	orthbou	und				Crossw	alk
Time Period	Class.	Ī	L	U	ı	0	R		U		0	R	ī	U	1	0	Total		Pedestrians	To
Peak 1	Motorcycles	1	0	0	1	1	0	0	0	0	0	0	1	0	1	1	2	N	0	(
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
8:00 AM - 11:00 AM	Cars	897	16	0	913	816	0	0	0	0	83	67	816	0	883	897	1796	E	0	4
One Hour Peak	%	86%	100%	0%	87%	85%	0%	0%	0%	0%	98%	97%	85%	0%	86%	86%	86%		0%	
0:00 AM - 11:00 AM	Light Goods Vehicles	109	0	0	109	109	0	0	0	0	2	2	109	0	111	109	220	S	0	
	%	10%	0%	0%	10%	11%	0%	0%	0%	0%	2%	3%	11%	0%	11%	10%	11%		0%	
	Buses	6	0	0	6	4	0	0	0	0	0	0	4	0	4	6	10		0	
	%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%			
	Single-Unit Trucks	21	0	0	21	19	0	0	0	0	0	0	19	0	19	21	40			
	%	2%	0%	0%	2%	2%	0%	0%	0%	0%	0%	0%	2%	0%	2%	2%	2%			
	Articulated Trucks	4	0	0	4	5	0	0	0	0	0	0	5	0	5	4	9			
	%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%			
	Bicycles on Road	1	0	0	1	9	0	0	0	0	0	0	9	0	9	1	10			
	%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	1%	0%	1%	0%	0%			
	Total	1039	16	0	1055	963	0	0	0	0	85	69	963	0	1032	1039	2087			
	PHF	0.93	0.8	0	0.93	0.92	0	0	0	0	0.79	0.78	0.92	0	0.93	0.93	0.96			
	Approach %				51%	46%				0%	4%				49%	50%				
Peak 2	Motorcycles	3	0	0	3	6	0	0	0	0	0	0	6	0	6	3	9	N	3	
Specified Period	%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%		100%	
2:00 PM - 5:00 PM	Cars	1142	14	0	1156	1067	0	0	0	0	76	62	1067	0	1129	1142	2285	E	0	
One Hour Peak	%	89%	100%	0%	89%	90%	0%	0%	0%	0%	95%	94%	90%	0%	90%	89%	90%		0%	
2:00 PM - 3:00 PM	Light Goods Vehicles	113	0	0	113	78	0	0	0	0	3	3	78	0	81	113	194	S	6	
	%	9%	0%	0%	9%	7%	0%	0%	0%	0%	4%	5%	7%	0%	6%	9%	8%		100%	
	Buses	5	0	0	5	5	0	0	0	0	0	0	5	0	5	5	10		9	
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Single-Unit Trucks	15	0	0	15	17	0	0	0	0	0	0	17	0	17	15	32			
	%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	1%	0%	1%	1%	1%			
	Articulated Trucks	3	0	0	3	2	0	0	0	0	0	0	2	0	2	3	5			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Bicycles on Road	2	0	0	2	8	0	0	0	0	1	1	8	0	9	2	11			
	%	0%	0%	0%	0%	1%	0%	0%	0%	0%	1%	2%	1%	0%	1%	0%	0%			
	Total	1283	14	0	1297	1183	0	0	0	0	80	66	1183	0	1249	1283	2546			
	PHF	0.94	0.88	0	0.94	0.92	0	0	0	0	0.8	0.72	0.92	0	0.93	0.94	0.96			
	Approach %				51%	46%	10 13 1			0%	3%	100			49%	50%				

Study Name Milton - Route 138 and Blue Hills South Parking Lot Exit TM9 TMC
Start Date Thursday, March 29, 2018 6:00 AM
Saturday, March 31, 2018 5:00 PM

Site Code

	电流 电影	NAME OF THE PARTY OF		South	bound			S.N.A.		West	bound					North	bound					East	bound			W498156		Crossw	valk
Time Period	Class.	R	T	L	U	1	0	R	T	L	U	ı	0	R	T	L	U	1	0	R	Ţ	L	U		0	Total		Pedestrians	s Tot
Peak 1	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	. 0	0	N	0	0
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
6:00 AM - 9:00 AM	Cars	0	917	0	0	917	1253	1	0	0	0	1	0	0	1252	0	0	1252	917	0	0	0	0	0	0	2170	E	0	0
One Hour Peak	%	0%	85%	0%	0%	85%	84%	100%	0%	0%	0%	100%	0%	0%	84%	0%	0%	84%	85%	0%	0%	0%	0%	0%	0%	84%		0%	
7:30 AM - 8:30 AM	Light Goods Vehicles	0	97	0	0	97	164	0	0	0	0	0	0	0	164	0	0	164	97	0	0	0	0	0	0	261	S	0	0
	%	0%	9%	0%	0%	9%	11%	0%	0%	0%	0%	0%	0%	0%	11%	0%	0%	11%	9%	0%	0%	0%	0%	0%	0%	10%		0%	
	Buses	0	26	0	0	26	6	0	. 0	0	0	0	0	0	6	0	0	6	26	0	0	0	0	0	0	32	W	0	0
	%	0%	2%	0%	0%	2%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	2%	0%	0%	0%	0%	0%	0%	1%		0%	
	Single-Unit Trucks	0	23	0	0	23	51	0	0	0	0	0	0	0	51	0	0	51	23	0	0	0	0	0	0	74		0	0
	%	0%	2%	0%	0%	2%	3%	0%	0%	0%	0%	0%	0%	0%	3%	0%	0%	3%	2%	0%	0%	0%	0%	0%	0%	3%			
	Articulated Trucks	0	19	0	0	19	15	0	0	0	0	0	0	0	15	0	0	15	19	0	0	0	0	0	0	34			
	%	0%	2%	0%	0%	2%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	2%	0%	0%	0%	0%	0%	0%	1%			
	Bicycles on Road	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	0	1082	0	0	1082	1489	1	0	0	0	1	0	0	1488	0	0	1488	1082	0	0	0	0	0	0	2571			
	PHF	0	0.94	0	0	0.94	0.96	0.25	0	0	0	0.25	0	0	0.96	0	0	0.96	0.94	0	0	0	0	0	0	0.95			
	Approach %					42%	58%					0%	0%					58%	42%					0%	0%				
Peak 2	Motorcycles	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	N	0	0
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
3:00 PM - 6:00 PM	Cars	0	1123	0	0	1123	1048	8	0	0	0	8	0	0	1040	0	0	1040	1123	0	0	0	0	0	0	2171	E	0	0
One Hour Peak	%	0%	88%	0%	0%	88%	90%	80%	0%	0%	0%	80%	0%	0%	90%	0%	0%	90%	88%	0%	0%	0%	0%	0%	0%	89%		0%	
4:45 PM - 5:45 PM	Light Goods Vehicles	0	128	0	0	128	86	2	0	0	0	2	0	0	84	0	0	84	128	0	0	0	0	0	0	214	S	0	0
	%	0%	10%	0%	0%	10%	7%	20%	0%	0%	0%	20%	0%	0%	7%	0%	0%	7%	10%	0%	0%	0%	0%	0%	0%	9%		0%	
	Buses	0	7	0	0	7	11	0	0	0	0	0	0	0	11	0	0	11	7	0	0	0	0	0	0	18	W	0	0
	%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	1%		0%	
	Single-Unit Trucks	0	15	0	0	15	16	0	0	0	0	0	0	0	16	0	0	16	15	0	0	0	0	0	0	31		0	0
	%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	1%			
	Articulated Trucks	0	2	0	0	2	3	0	0	0	0	0	0	0	3	0	0	3	2	0	0	0	0	0	0	5			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Bicycles on Road	0	1	0	0	1	2	0	0	0	0	0	0	0	2	0	0	2	1	0	0	0	0	0	0	3		R.T. E. S.	
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	0	1277	0	0	1277	1166	10	0	0	0	10	0	0	1156	0	0	1156	1277	0	0	0	0	0	0	2443			
	PHF	0	0.95	0	0	0.95	0.91	0.5	0	0	0	0.5	0	0	0.9	0	0	0.9	0.95	0	0	0	0	0	0	0.93			
	Approach %					52%	48%					0%	0%					47%	52%					0%	0%	1			
						LIE.																							

Site Code

Study Name Milton - Route 138 and Blue Hills South Parking Lot Exit TM9 TMC

Start Date Thursday, March 29, 2018 6:00 AM End Date Saturday, March 31, 2018 5:00 PM

Company of the second				South	bound					West	bound	SELET	274 17		THE PARTY	North	bound					East	bound					Crossy	walk
Time Period	Class.	R	T	L	U		0	R	MIT	L	U		0	R	Т	L	U		0	R	Т	L	U	1	0	Total		Pedestrian	is To
Peak 1	Motorcycles	0	1	0	0	1	1	0	0	0	0	0	0	0	1	0	0	1	1	0	0	0	0	0	0	2	N	0	
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
8:00 AM - 11:00 AM	Cars	0	907	0	0	907	854	22	0	10	0	32	1	1	832	0	0	833	917	0	0	0	0	0	0	1772	E	1	
One Hour Peak	%	0%	87%	0%	0%	87%	85%	100%	0%	100%	0%	100%	100%	100%	84%	0%	0%	84%	88%	0%	0%	0%	0%	0%	0%	86%		100%	
9:45 AM - 10:45 AM	Light Goods Vehicles	0	98	0	0	98	116	0	0	0	0	0	- 0	.0	116	0	0	116	98	0	0	0	0	0	0	214	S	0	
	%	0%	9%	0%	0%	9%	12%	0%	0%	0%	0%	0%	0%	0%	12%	0%	0%	12%	9%	0%	0%	0%	0%	0%	0%	10%		0%	
	Buses	0	6	0	0	6	3	0	0	0	0	0	0	0	3	0	0	3	6	0	0	0	0	0	0	9	W	0	
	%	0%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%		0%	
	Single-Unit Trucks	0	20	0	0	20	19	0	0	0	0	0	0	0	19	0	0	19	20	0	0 .	0	0	0	0	39		1	
	%	0%	2%	0%	0%	2%	2%	0%	0%	0%	0%	0%	0%	0%	2%	0%	0%	2%	2%	0%	0%	0%	0%	0%	0%	2%			
	Articulated Trucks	0	5	0	0	5	6	0	0	0	0	0	0	0	6	0	0	6	5	0	0	0	0	0	0	11			
	%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	1%			
	Bicycles on Road	0	1	0	0	1	8	0	0	0	0	0	0	0	8	0	0	8	1	0	0	0	0	0	0 .	9			
	%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	0	1038	0	0	1038	1007	22	0	10	0	32	1	1	985	0	0	986	1048	0	0	0	0	0	0	2056			
	PHF	0	0.92	0	0	0.92	0.95	0.61	0	0.62	0	0.62	0.25	0.25	0.94	0	0	0.94	0.93	0	0	0	0	0	0	0.97			
	Approach %					50%	49%					2%	0%					48%	51%					0%	0%				
Peak 2	Motorcycles	0	3	0	0	3	6	0	0	0	0	0	0	0	6	0	0	6	3	0	0	0	0	0	0	9	N	2	
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%		100%	
2:00 PM - 5:00 PM	Cars	0	1143	0	0	1143	1138	67	0	10	0	77	0	0	1071	0	0	1071	1153	0	0	0	0	0	0	2291	E	0	
One Hour Peak	%	0%	89%	0%	0%	89%	90%	91%	0%	100%	0%	92%	0%	0%	90%	0%	0%	90%	89%	0%	0%	0%	0%	0%	0%	89%		0%	
2:00 PM - 3:00 PM	Light Goods Vehicles	0	119	0	0	119	85	6	0	0	0	6	1	1	79	0	0	80	119	0	0	0	0	0	0	205	S	0	
	%	0%	9%	0%	0%	9%	7%	8%	0%	0%	0%	7%	100%	100%	7%	0%	0%	7%	9%	0%	0%	0%	0%	0%	0%	8%		0%	
	Buses	0	4	0	0	4	5	0	0	0	0	0	0	0	5	0	0	5	4	0	0	0	0	0	0	9	W	1	
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		100%	
	Single-Unit Trucks	0	14	0	0	14	19	0	0	0	0	0	0	0	19	0	0	19	14	0	0	0	0	0	0	33		3	
	%	0%	1%	0%	0%	1%	2%	0%	0%	0%	0%	0%	0%	0%	2%	0%	0%	2%	1%	0%	0%	0%	0%	70%	0%	1%			
	Articulated Trucks	0	3	0	0	3	1	0	0	0	0	0	0	0	1	0	0	1	3	0	0	0	0	0	0	4			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Bicycles on Road	0	1	0	0	1	9	1	0	0	0	1	0	0	8	0	0	8	1	0	0	0	0	0	0	10			
	%	0%	0%	0%	0%	0%	1%	1%	0%	0%	0%	1%	0%	0%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	0	1287	0	0	1287	1263	74	0	10	0	84	1	1	1189	0	0	1190	1297	0	0	0	0	0	0	2561			
	PHF	0	0.97	0	0	0.97	0.94	0.84	0	0.42	0	0.75	0.25	0.25	0.93	0	0	0.93	0.97	0	0	0	0	0	0	0.96			
	Approach %					50%	49%					3%	0%					46%	51%				CVA	0%	0%				

Study Name Milton - Route 138 and Blue Hills North Parking Lot Entrance TM10 TMC
Start Date Thursday, March 29, 2018 6:00 AM
Saturday, March 31, 2018 5:00 PM
Site Code

		al same	So	uthbou	ınd			L W	Vestbou	ınd			No	orthbou	ınd				Crossw	alk
Time Period	Class.	T	L	U	1	0	R	L	U	1	0	R	T	U		0	Total		Pedestrians	1
Peak 1	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	0	
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
6:00 AM - 9:00 AM	Cars	932	6	0	938	1259	0	0	0	0	11	5	1259	0	1264	932	2202	Ε	0	
One Hour Peak	%	86%	100%	0%	86%	84%	0%	0%	0%	0%	85%	71%	84%	0%	84%	86%	85%		0%	
7:30 AM - 8:30 AM	Light Goods Vehicles	91	0	0	91	162	0	0	0	0	1	1	162	0	163	91	254	S	0	
	%	8%	0%	0%	8%	11%	0%	0%	0%	0%	8%	14%	11%	0%	11%	8%	10%		0%	
	Buses	26	0	0	26	6	0	0	0	0	0	0	6	0	6	26	32		0	
	%	2%	0%	0%	2%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	2%	1%			
	Single-Unit Trucks	21	0	0	21	48	0	0	0	0	0	0	48	0	48	21	69			
	%	2%	0%	0%	2%	3%	0%	0%	0%	0%	0%	0%	3%	0%	3%	2%	3%			
	Articulated Trucks	20	0	0	20	15	0	0	0	0	1	1	15	0	16	20	36			
	%	2%	0%	0%	2%	1%	0%	0%	0%	0%	8%	14%	1%	0%	1%	2%	1%			
	Bicycles on Road	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	1090	6	0	1096	1490	0	0	0	0	13	7	1490	0	1497	1090	2593			
	PHF	0.94	0.75	0	0.94	0.96	0	0	0	0	0.65	0.58	0.96	0	0.96	0.94	0.95			
	Approach %				42%	57%				0%	1%				58%	42%				
Peak 2	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	0	
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
3:00 PM - 6:00 PM	Cars	1127	4	0	1131	1069	0	0	0	0	10	6	1069	0	1075	1127	2206	E	0	
One Hour Peak	%	88%	100%	0%	88%	92%	0%	0%	0%	0%	91%	86%	92%	0%	92%	88%	90%		0%	
5:00 PM - 6:00 PM	Light Goods Vehicles	123	0	0	123	57	0	0	0	0	1	1	57	0	58	123	181	S	0	
	%	10%	0%	0%	10%	5%	0%	0%	0%	0%	9%	14%	5%	0%	5%	10%	7%		0%	
	Buses	11	0	0	11	15	0	0	0	0	0	0	15	0	15	11	26		0	
	%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	1%	0%	1%	1%	1%			
	Single-Unit Trucks	18	0	0	18	15	0	0	0	0	0	0	15	0	15	18	33			
	%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	1%	0%	1%	1%	1%			
	Articulated Trucks	3	0	0	3	5	0	0	0	0	0	0	5	0	5	3	8			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Bicycles on Road	1	0	0	1	2	0	0	0	0	0	0	2	0	2	1	3			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	1283	4	0	1287	1163	0	0	0	0	11	7	1163	0	1170	1283	2457			
	PHF	0.96	1	0	0.96	0.95	0	0	0	0	0.55	0.44	0.95	0	0.95	0.96	0.98			
	Approach %				52%	47%				0%	0%	1 3		FULL	48%	52%				

Study Name Milton - Route 138 and Blue Hills North Parking Lot Entrance TM10 TMC
Start Date Thursday, March 29, 2018 6:00 AM
Saturday, March 31, 2018 5:00 PM
Site Code

			So	uthbou	ınd			V	Vestbou	ınd			No	orthbou	ınd				Crosswa	alk
Time Period	Class.	T	L	U	1	0	R	Ľ	U		0	R	i	U		0	Total		Pedestrians	1
Peak 1	Motorcycles	1	0	0	1	1	0	0	0	0	0	0	1	0	1	1	2	N	0	
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
3:00 AM - 11:00 AM	Cars	905	13	0	918	841	0	0	0	0	50	37	841	0	878	905	1796	E	1	
One Hour Peak	%	87%	87%	0%	87%	88%	0%	0%	0%	0%	91%	93%	88%	0%	88%	87%	88%		100%	
0:00 AM - 11:00 AM	Light Goods Vehicles	97	2	0	99	75	0	0	0	0	4	2	75	0	77	97	176	S	0	
	%	9%	13%	0%	9%	8%	0%	0%	0%	0%	7%	5%	8%	0%	8%	9%	9%		0%	
	Buses	6	0	0	6	4	0	0	0	0	0	0	4	0	4	6	10		1	
	%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%			
	Single-Unit Trucks	26	0	0	26	18	0	0	0	0	0	0	18	0	18	26	44			
	%	3%	0%	0%	2%	2%	0%	0%	0%	0%	0%	0%	2%	0%	2%	3%	2%			
	Articulated Trucks	4	0	0	4	6	0	0	0	0	0	0	6	0	6	4	10			
	%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	1%	0%	1%	0%	0%			
	Bicycles on Road	1	0	0	1	8	0	0	0	0	1	1	8	0	9	1	10			
	%	0%	0%	0%	0%	1%	0%	0%	0%	0%	2%	3%	1%	0%	1%	0%	0%			
	Total	1040	15	0	1055	953	0	0	0	0	55	40	953	0	993	1040	2048			
	PHF	0.93	0.47	0	0.91	0.92	0	0	0	0	0.69	0.83	0.92	0	0.92	0.93	0.95			
	Approach %				52%	47%				0%	3%				48%	51%				
Peak 2	Motorcycles	3	0	0	3	6	0	0	0	0	0	0	6	0	6	3	9	N	0	
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
2:00 PM - 5:00 PM	Cars	1152	22	0	1174	1105	0	0	0	0	57	35	1105	0	1140	1152	2314	E	0	
One Hour Peak	%	89%	88%	0%	89%	90%	0%	0%	0%	0%	93%	97%	90%	0%	90%	89%	90%		0%	
2:00 PM - 3:00 PM	Light Goods Vehicles	114	0	0	114	81	0	0	0	0	0	0	81	0	81	114	195	S	0	
	%	9%	0%	0%	9%	7%	0%	0%	0%	0%	0%	0%	7%	0%	6%	9%	8%		0%	
	Buses	1	0	0	1	5	0	0	0	0	0	0	5	0	5	1	6		0	
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Single-Unit Trucks	19	0	0	19	18	0	0	0	0	0	0	18	0	18	19	37			
	%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	1%	0%	1%	1%	1%			
	Articulated Trucks	2	0	0	2	3	0	0	0	0	0	0	3	0	3	2	5			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Bicycles on Road	2	3	0	5	7	0	0	0	0	4	1	7	0	8	2	13			
	%	0%	12%	0%	0%	1%	0%	0%	0%	0%	7%	3%	1%	0%	1%	0%	1%			
	Total	1293	25	0	1318	1225	0	0	0	0	61	36	1225	0	1261	1293	2579			
	PHF	0.94	0.69	0	0.95	0.92	0	0	0	0	0.66	0.6	0.92	0	0.92	0.94	0.96			
	Approach %				51%	47%				0%	2%				49%	50%				

Thursday

Study Name Milton - Route 138 at Blue Hills North Parking Lot Exit and Green Street TM11 TMC
Start Date Thursday, March 29, 2018 6:00 AM
Saturday, March 31, 2018 5:00 PM
Site Code

				South	bound					West	bound					North	bound					Eastk	bound					Crossw	valk
Time Period	Class.	R	T	L.	U	- 1	0	R	T	L	U		0	R	T	L	U	1	0	R	T	L	U		0	Total		Pedestrians	s To
Peak 1	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	. 0	0
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
6:00 AM - 9:00 AM	Cars	6	925	0	0	931	1277	8	0	0	0	8	0	0	1264	0	0	1264	932	7	0	5	0	12	6	2215	E	0	0
One Hour Peak	%	86%	85%	0%	0%	85%	85%	80%	0%	0%	0%	80%	0%	0%	85%	0%	0%	85%	85%	100%	0%	71%	0%	86%	75%	85%		0%	
7:30 AM - 8:30 AM	Light Goods Vehicles	1	92	0	0	93	160	1	0	0	0	1	0	0	158	1	0	159	92	0	0	1	0	1	2	254	S	0	0
	%	14%	8%	0%	0%	9%	11%	10%	0%	0%	0%	10%	0%	0%	11%	100%	0%	11%	8%	0%	0%	14%	0%	7%	25%	10%		0%	
	Buses	0	26	0	0	26	4	0	0	0	0	0	0	0	4	0	0	4	26	0	0	0	0	0	0	30	W	0	0
	%	0%	2%	0%	0%	2%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	2%	0%	0%	0%	0%	0%	0%	1%		0%	
	Single-Unit Trucks	0	23	0	0	23	44	1	0	0	0	1	0	0	43	0	0	43	23	0	0	0	0	0	0	67		0	0
	%	0%	2%	0%	0%	2%	3%	10%	0%	0%	0%	10%	0%	0%	3%	0%	0%	3%	2%	0%	0%	0%	0%	0%	0%	3%			
	Articulated Trucks	0	19	0	0	19	13	0	0	0	0	0	0	0	13	0	0	13	19	0	0	0	0	0	0	32			
	%	0%	2%	0%	0%	2%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	2%	0%	0%	0%	0%	0%	0%	1%			
	Bicycles on Road	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0	1			
	. %	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	14%	0%	7%	0%	0%			
	Total	7	1085	0	0	1092	1499	10	0	0	0	10	0	0	1482	1	0	1483	1092	7	0	7	0	14	8	2599			
	PHF	0.44	0.96	0	0	0.96	0.96	0.62	0	0	0	0.62	0	0	0.95	0.25	0	0.95	0.95	0.44	0	0.35	0	0.7	0.5	0.95			
	Approach %					42%	58%					0%	0%					57%	42%					1%	0%				
Peak 2	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	0	0
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
3:00 PM - 6:00 PM	Cars	15	1116	0	0	1131	1046	13	0	1	0	14	0	0	1027	6	0	1033	1126	9	0	6	0	15	21	2193	E	0	0
One Hour Peak	%	83%	88%	0%	0%	87%	90%	93%	0%	100%	0%	93%	0%	0%	90%	100%	0%	90%	88%	100%	0%	86%	0%	94%	88%	89%		0%	
5:00 PM - 6:00 PM	Light Goods Vehicles	3	128	0	0	131	86	1	0	0	0	1	0	0	84	0	0	84	128	0	0	1	0	1	3	217	S	0	0
	%	17%	10%	0%	0%	10%	7%	7%	0%	0%	0%	7%	0%	0%	7%	0%	0%	7%	10%	0%	0%	14%	0%	6%	13%	9%		0%	
	Buses	0	11	0	0	11	15	0	0	0	0	0	0	0	15	0	0	15	11	0	0	0	0	0	0	26	W	1	1
	%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	1%		100%	
	Single-Unit Trucks	0	16	0	0	16	14	0	0	0	0	0	0	0	14	0	0	14	16	0	0	0	0	0	0	30		1	1
	%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	1%			
	Articulated Trucks	0	3	0	0	3	5	0	0	0	0	0	0	0	5	0	0	5	3	0	0	0	0	0	0	8			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Bicycles on Road	0	1	0	0	1	2	0	0	0	0	0	0	0	2	0	0	2	1	0	0	0	0	0	0	3			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	18	1275	0	0	1293	1168	14	0	1	0	15	0	0	1147	6	0	1153	1285	9	0	7	0	16	24	2477			
	PHF	0.64	0.95	0	0	0.94	0.95	0.44	0	0.25	0	0.47	0	0	0.96	0.5	0	0.95	0.95	0.75	0	0.58	0	0.8	0.6	0.97			
	Approach %					52%	47%					1%	0%				No.	47%	52%					1%	1%				

Site Code

Study Name Milton - Route 138 at Blue Hills North Parking Lot Exit and Green Street TM11 TMC

Start Date Thursday, March 29, 2018 6:00 AM End Date Saturday, March 31, 2018 5:00 PM

				South	bound					West	bound	al ear				North	bound			L TEX		Eastl	oound	W				Crossv	wall
Time Period	Class.	R	T	L	U		0	R	T	L,	U		0	R	T	L	U		0	R	T.	L	U		0	Total		Pedestrian	1S
Peak 1	Motorcycles	0	1	0	0	1	1	0	0	0	0	0	0	0	1	0	0	1	1	0	0	0	0	0	0	2	N	0	
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
3:00 AM - 11:00 AM	Cars	2	905	0	0	907	826	16	2	3	0	21	0	0	807	4	0	811	918	10	0	3	0	13	8	1752	Е	5	
One Hour Peak	%	40%	87%	0%	0%	87%	85%	84%	100%	100%	0%	88%	0%	0%	85%	67%	0%	85%	87%	83%	0%	100%	0%	87%	62%	86%		100%	
.0:00 AM - 11:00 AM	Light Goods Vehicles	1	103	0	0	104	108	2	0	0	0	2	0	0	106	2	0	108	105	2	0	0	0	2	3	216	S	0	
	%	20%	10%	0%	0%	10%	11%	11%	0%	0%	0%	8%	0%	0%	11%	33%	0%	11%	10%	17%	0%	0%	0%	13%	23%	11%		0%	
	Buses	0	6	0	0	6	4	0	0	0	0	0	0	0	4	0	0	4	6	0	0	0	0	0	0	10	W	0	
	%	0%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%		0%	
	Single-Unit Trucks	0	22	0	0	22	18	. 0	0	0	0	0	0	0	18	0	0	18	22	0	0	0	0	0	0	40		5	
	%	0%	2%	0%	0%	2%	2%	0%	0%	0%	0%	0%	0%	0%	2%	0%	0%	2%	2%	0%	0%	0%	0%	0%	0%	2%			
	Articulated Trucks	0	4	0	0	4	5	0	0	0	0	0	0	0	5	0	0	5	4	0	0	0	0	0	0	9			
	%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%			
	Bicycles on Road	2	1	0	0	3	9	1	0	0	0	1	0	0	8	0	0	8	1	0	0	0	0	0	2	12			
	%	40%	0%	0%	0%	0%	1%	5%	0%	0%	0%	4%	0%	0%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	15%	1%			
	Total	5	1042	0	0	1047	971	19	2	3	0	24	0	0	949	6	0	955	1057	12	0	3	0	15	13	2041			
	PHF	0.42	0.93	0	0	0.92	0.92	0.53	0.25	0.38	0	0.67	0	0	0.92	0.5	0	0.91	0.92	0.43	0	0.38	0	0.54	0.65	0.95			
	Approach %					51%	48%					1%	0%					47%	52%					1%	1%				
Peak 2	Motorcycles	0	3	0	0	3	6	0	0	0	0	0	0	0	6	0	0	6	3	0	0	0	0	0	0	9	N	0	
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
2:00 PM - 5:00 PM	Cars	5	1157	0	0	1162	1178	76	0	2	0	78	0	0	1099	6	2	1107	1171	10	0	3	0	13	11	2360	E	5	
One Hour Peak	%	71%	89%	0%	0%	89%	91%	95%	0%	100%	0%	95%	0%	0%	90%	86%	100%	90%	89%	83%	0%	100%	0%	87%	79%	90%		100%	
2:00 PM - 3:00 PM	Light Goods Vehicles	1	115	0	0	116	79	3	0	0	0	3	0	0	76	1	0	77	117	2	0	0	0	2	2	198	S	0	
	%	14%	9%	0%	0%	9%	6%	4%	0%	0%	0%	4%	0%	0%	6%	14%	0%	6%	9%	17%	0%	0%	0%	13%	14%	8%		0%	
	Buses	0	4	0	0	4	5	0	0	0	0	0	0	0	5	0	0	5	4	0	0	0	0	0	0	9	W	0	
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
	Single-Unit Trucks	0	15	0	0	15	18	0	0	0	0	0	0	0	18	0	0	18	15	0	0	0	0	0	0	33		5	
	%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	1%			
	Articulated Trucks	0	3	0	0	3	3	0	0	0	0	0	0	0	3	0	0	3	3	0	0	0	0	0	0	6			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Bicycles on Road	1	4	0	0	5	9	1	0	0	0	1	0	0	8	0	0	8	4	0	0	0	0	0	1	14			
	%	14%	0%	0%	0%	0%	1%	1%	0%	0%	0%	1%	0%	0%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	7%	1%			
	Total	7	1301	0	0	1308	1298	80	0	2	0	82	0	0	1215	7	2	1224	1317	12	0	3	0	15	14	2629			
	PHF	0.58	0.95	0	0	0.95	0.94	0.65	0	0.25	0	0.66	0	0	0.92	0.58	0.5	0.92	0.95	0.75	0	0.25	0	0.62	0.88	0.97			
	Approach %					50%	49%					3%	0%				THE PARTY	47%	50%					1%	1%				
						The second						N. S.								100									

Study Name Milton - Route 138 at Canton Avenue and Summit Road TM12 TMC

Start Date Thursday, March 29, 2018 6:00 AM End Date Saturday, March 31, 2018 5:00 PM

Site Code

				South	bound					Southw	estbour	nd			1	West	bound					North	bound					Crossw	val
Time Period	Class.	T	L	HL	U	1	0	HR	BL	HL	U	1	0	HR	R	L	U	Î	0	R	BR	T	U		0	Total		Pedestrians	S
Peak 1	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	0	
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
6:00 AM - 9:00 AM	Cars	945	0	5	0	950	824	0	0	0	4	4	484	0	0	0	0	0	0	0	475	824	0	1299	945	2253	NE	0	
One Hour Peak	%	87%	0%	71%	0%	86%	85%	0%	0%	0%	100%	100%	89%	0%	0%	0%	0%	0%	0%	0%	89%	85%	0%	87%	87%	86%		0%	
7:30 AM - 8:30 AM	Light Goods Vehicles	76	0	0	0	76	79	0	0	0	0	0	55	1	0	0	0	1	0	0	54	79	0	133	76	210	E	0	
	%	7%	0%	0%	0%	7%	8%	0%	0%	0%	0%	0%	10%	100%	0%	0%	0%	100%	0%	0%	10%	8%	0%	9%	7%	8%		0%	
	Buses	25	0	0	0	25	5	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5	0	5	25	30	S	0	
	%	2%	0%	0%	0%	2%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	2%	1%		0%	
	Single-Unit Trucks	27	0	2	0	29	46	0	0	0	0	0	5	0	0	0	0	0	0	0	3	46	0	49	27	78		0	
	%	2%	0%	29%	0%	3%	5%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	1%	5%	0%	3%	2%	3%			
	Articulated Trucks	19	0	0	0	19	12	0	0	0	0	0	2	0	0	0	0	0	0	0	2	12	0	14	19	33			
	%	2%	0%	0%	0%	2%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%	1%	2%	1%			
	Bicycles on Road	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	1	0	1			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	100%	100%	0%	0%	0%	0%	0%	0%			
	Total	1092	0	7	0	1099	966	0	0	0	4	4	546	1	0	0	0	1	1	1	534	966	0	1501	1092	2605			
	PHF	0.95	0	0.88	0	0.95	0.97	0	0	0	0.5	0.5	0.84	0.25	0	0	0	0.25	0.25	0.25	0.83	0.97	0	0.96	0.95	0.95			
	Approach %					42%	37%					0%	21%					0%	0%					58%	42%				
Peak 2	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	0	
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
3:00 PM - 6:00 PM	Cars	1144	0	2	0	1146	689	0	0	0	0	0	373	1	0	0	0	1	0	0	370	689	0	1059	1144	2206	NE	0	
One Hour Peak	%	88%	0%	100%	0%	88%	90%	0%	0%	0%	0%	0%	91%	50%	0%	0%	0%	33%	0%	0%	91%	90%	0%	90%	88%	89%		0%	
5:00 PM - 6:00 PM	Light Goods Vehicles	119	0	0	0	119	49	0	0	0	0	0	29	0	0	0	0	0	0	0	29	49	0	78	119	197	E	0	
	%	9%	0%	0%	0%	9%	6%	0%	0%	0%	0%	0%	7%	0%	0%	0%	0%	0%	0%	0%	7%	6%	0%	7%	9%	8%		0%	
	Buses	11	0	0	0	11	11	0	0	0	0	0	4	0	0	0	0	0	0	0	4	11	0	15	11	26	S	0	
	%	1%	0%	0%	0%	1%	1%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	1%	1%	0%	1%	1%	1%		0%	
	Single-Unit Trucks	16	0	0	. 0	16	13	0	0	0	0	0	1	0	0	0	0	0	0	0	1	13	0	14	16	30		0	
	%	1%	0%	0%	0%	1%	2%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	2%	0%	1%	1%	1%			
	Articulated Trucks	3	0	0	0	3	4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	4	3	7			
	%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%			
	Bicycles on Road	1	0	0	0	1	0	0	0	0	0	0	3	1	0	0	1	2	1	0	2	0	0	2	1	5			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	50%	0%	0%	100%	67%	100%	0%	0%	0%	0%	0%	0%	0%			
	Total	1294	0	2	0	1296	766	0	0	0	0	0	410	2	0	0	1	3	1	0	406	766	0	1172	1294	2471			
	PHF	0.93	0	0.5	0	0.93	0.95	0	0	0	0	0	0.92	0.5	0	0	0.25	0.38	0.25	0	0.91	0.95	0	0.94	0.93	0.97			
	Approach %					52%	31%					0%	17%					0%	0%					47%	52%				

Study Name Milton - Route 138 at Canton Avenue and Summit Road TM12 TMC
Start Date Thursday, March 29, 2018 6:00 AM
End Date Saturday, March 31, 2018 5:00 PM

Site Code

				South	bound			医复测器	P. Turk	Southw	estboui	nd				West	bound		1 2 2 1			North	bound			100		Crossv	wall
Time Period	Class.	I	L	HL	U	1	0	HR	BL	HL	U	I	0	HR	R	L	U		0	R	BR	T	U		0	Total		Pedestrian	ns
Peak 1	Motorcycles	1	0	0	0	1	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	1	3	N	0	
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
:00 AM - 11:00 AM	Cars	906	0	2	0	908	628	0	0	0	0	0	255	0	0	0	0	0	3	3	253	628	0	884	906	1792	NE	0	
One Hour Peak	%	87%	0%	100%	0%	87%	92%	0%	0%	0%	0%	0%	94%	0%	0%	0%	0%	0%	43%	50%	94%	92%	0%	92%	87%	89%		0%	
0:00 AM - 11:00 AM	Light Goods Vehicles	105	0	0	0	105	26	0	0	0	0	0	10	0	0	0	0	0	0	0	10	26	0	36	105	141	E	13	
	%	10%	0%	0%	0%	10%	4%	0%	0%	0%	0%	0%	4%	0%	0%	0%	0%	0%	0%	0%	4%	4%	0%	4%	10%	7%		100%	
	Buses	6	0	0	0	6	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	6	7	S	0	
	7%	1%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%		0%	
	Single-Unit Trucks	21	0	0	0	21	19	0	0	0	0	0	2	0	0	0	0	0	0	0	2	19	0	21	21	42		13	
	%	2%	0%	0%	0%	2%	3%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	1%	3%	0%	2%	2%	2%			
	Articulated Trucks	4	0	0	0	4	5	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5	0	5	4	9			
	%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%	1%	0%	0%			
	Bicycles on Road	3	1	0	0	4	1	0	0	0	0	0	5	0	0	0	0	0	4	3	5	1	0	9	3	13			
	%	0%	100%	0%	0%	0%	0%	0%	0%	0%	0%	0%	2%	0%	0%	0%	0%	0%	57%	50%	2%	0%	0%	1%	0%	1%			
	Total	1046	1	2	0	1049	682	0	0	0	0	0	272	0	0	0	0	0	7	6	270	682	0	958	1046	2007			
	PHF	0.92	0.25	0.25	0	0.92	0.96	0	0	0	0	0	0.83	0	0	0	0	0	0.44	0.38	0.82	0.96	0	0.91	0.92	0.94			
	Approach %					52%	34%					0%	14%					0%	0%					48%	52%				
Peak 2	Motorcycles	3	0	. 0	0	3	3	0	0	0	0	0	4	1	0	0	0	1	0	0	3	3	0	6	3	10	N	0	
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	25%	0%	0%	0%	25%	0%	0%	1%	0%	0%	0%	0%	0%		0%	
:00 PM - 5:00 PM	Cars	1211	0	3	0	1214	756	0	0	0	0	0	430	2	0	0	0	2	1	1	425	756	0	1182	1211	2398	NE	0	
One Hour Peak	%	93%	0%	75%	0%	93%	91%	0%	0%	0%	0%	0%	92%	50%	0%	0%	0%	50%	25%	33%	92%	91%	0%	91%	93%	92%		0%	
2:00 PM - 3:00 PM	Light Goods Vehicles	63	0	0	0	63	44	0	0	0	0	0	27	0	0	0	0	0	0	0	27	44	0	71	63	134	E	1	
	%	5%	0%	0%	0%	5%	5%	0%	0%	0%	0%	0%	6%	0%	0%	0%	0%	0%	0%	0%	6%	5%	0%	5%	5%	5%		100%	
	Buses	4	0	1	0	5	5	0	0	0	0	0	1	0	0	0	0	0	0	0	0	5	0	5	4	10	S	0	
	%	0%	0%	25%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%		0%	
	Single-Unit Trucks	18	0	0	0	18	21	0	0	0	0	0	0	0	0	0	0	0	0	0	0	21	0	21	18	39		1	
	%	1%	0%	0%	0%	1%	3%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	3%	0%	2%	1%	1%			
	Articulated Trucks	2	0	0	0	2	3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	3	2	5			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Bicycles on Road	0	0	0	0	0	1	0	0	1	0	1	7	1	0	0	0	1	3	2	6	1	0	9	0	11			
	%	0%	0%	0%	0%	0%	0%	0%	0%	100%	0%	100%	1%	25%	0%	0%	0%	25%	75%	67%	1%	0%	0%	1%	0%	0%			
	Total	1301	0	4	0	1305	833	0	0	1	0	1	469	4	0	0	0	4	4	3	461	833	0	1297	1301	2607			
	PHF	0.97	0	0.5	0	0.97	0.95	0	0	0.25	0	0.25	0.9	0.33	0	0	0	0.33	0.5	0.38	0.89	0.95	0	0.93	0.97	0.97			
	Approach %					50%	32%					0%	18%					0%	0%				Can ye	50%	50%				

Study Name Milton - Route 138 at Thacher Montessori School TM13 TMC
Start Date Thursday, March 29, 2018 6:00 AM
Saturday, March 31, 2018 5:00 PM
Site Code

				South	bound	V				North	bound					Northe	astbo <u>u</u> r	nd				East	bound					Crossw	valk
Time Period	Class.	R	BR	T	U	1	0	T	L	HL	U		0	HR	BL	HL	U		0	HR	R	L	U		0	Total		Pedestrians	s Tota
Peak 1	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	0	0
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
6:00 AM - 9:00 AM	Cars	46	0	910	0	956	802	802	3	0	0	805	919	9	0	0	0	9	0	0	0	0	0	0	49	1770	S	0	0
One Hour Peak	%	100%	0%	85%	0%	85%	84%	84%	100%	0%	0%	84%	85%	100%	0%	0%	0%	100%	0%	0%	0%	0%	0%	0%	100%	85%		0%	
7:30 AM - 8:30 AM	Light Goods Vehicles	0	0	97	0	97	99	99	0	0	0	99	97	0	0	0	0	0	0	0	0	0	0	0	0	196	SW	0	0
	%	0%	0%	9%	0%	9%	10%	10%	0%	0%	0%	10%	9%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	9%		0%	
	Buses	0	0	26	0	26	5	5	0	0	0	5	26	0	0	0	0	0	0	0	0	0	0	0	0	31	W	0	0
	%	0%	0%	2%	0%	2%	1%	1%	0%	0%	0%	1%	2%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%		0%	
	Single-Unit Trucks	0	0	24	0	24	42	42	0	0	0	42	24	0	0	0	0	0	0	0	0	0	0	0	0	66		0	0
	%	0%	0%	2%	0%	2%	4%	4%	0%	0%	0%	4%	2%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	3%			
	Articulated Trucks	0	0	18	0	18	12	12	0	0	0	12	18	0	0	0	0	0	0	0	0	0	0	0	0	30			
	%	0%	0%	2%	0%	2%	1%	1%	0%	0%	0%	1%	2%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%			
	Bicycles on Road	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	46	0	1075	0	1121	960	960	3	0	0	963	1084	9	0	0	0	9	0	0	0	0	0	0	49	2093			
	PHF	0.77	0	0.92	0	0.92	0.98	0.98	0.75	0	0	0.98	0.93	0.56	0	0	0	0.56	0	0	0	0	0	0	0.77	0.97			
	Approach %					54%	46%					46%	52%					0%	0%					0%	2%				
Peak 2	Motorcycles	0	0	0	0	0	1	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1	N	0	0
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
3:00 PM - 6:00 PM	Cars	3	0	1067	0	1070	680	680	0	0	0	680	1086	19	0	0	0	19	0	0	0	0	0	0	3	1769	S	0	0
One Hour Peak	%	100%	0%	82%	0%	82%	87%	87%	0%	0%	0%	87%	82%	90%	0%	0%	0%	90%	0%	0%	0%	0%	0%	0%	100%	84%		0%	
3:30 PM - 4:30 PM	Light Goods Vehicles	0	0	191	0	191	74	74	0	0	0	74	193	2	0	0	0	2	0	0	0	0	0	0	0	267	SW	0	0
	%	0%	0%	15%	0%	15%	9%	9%	0%	0%	0%	9%	15%	10%	0%	0%	0%	10%	0%	0%	0%	0%	0%	0%	0%	13%		0%	
	Buses	0	0	11	0	11	7	7	0	0	0	7	11	0	0	0	0	0	0	0	0	0	0	0	0	18	W	0	0
	%	0%	0%	1%	0%	1%	1%	1%	0%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%		0%	
	Single-Unit Trucks	0	0	24	0	24	15	15	0	0	0	15	24	0	0	0	0	0	0	0	0	0	0	0	0	39		0	0
	%	0%	0%	2%	0%	2%	2%	2%	0%	0%	0%	2%	2%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	2%			
	Articulated Trucks	0	0	6	0	6	9	9	0	0	0	9	6	0	0	0	0	0	0	0	0	0	0	0	0	15			
	%	0%	0%	0%	0%	0%	1%	1%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%			
	Bicycles on Road	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	3	0	1299	0	1302	786	786	0	0	0	786	1320	21	0	0	0	21	0	0	0	0	0	0	3	2109			
	PHF	0.75	0	0.96	0	0.96	0.95	0.95	0	0	0	0.95	0.96	0.75	0	0	0	0.75	0	0	0	0	0	0	0.75	0.96			
	Approach %					62%	37%					37%	63%					1%	0%					0%	0%				
								E //E						THE STATE OF															

Start Date
Start Date
End Date
Site Code

Milton - Route 138 at Thacher Montessori School TM13 TMC
Thursday, March 29, 2018 6:00 AM
Saturday, March 31, 2018 5:00 PM

				South	bound	and the second				North	bound					Northea	astbour	nd				East	oound			A Property	1 8 9	Crossw	valk
Time Period	Class.	R	BR	Т	U	1	0	Т	L	HL	U	1	0	HR	BL	FIL	U		0	HR	R	L	U	97.14	0	Total		Pedestrians	s To
Peak 1	Motorcycles	0	0	1	0	1	1	1	0	0	0	1	1	0	0	0	0	0	0	0	0	0	0	0	0	2	N	0	(
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
8:00 AM - 11:00 AM	Cars	0	0	907	0	907	570	570	1	0	0	571	908	1	0	0	0	1	0	0	0	0	0	0	1	1479	S	0	(
One Hour Peak	% .	0%	0%	87%	0%	87%	83%	83%	100%	0%	0%	83%	87%	100%	0%	0%	0%	100%	0%	0%	0%	0%	0%	0%	100%	85%		0%	
10:00 AM - 11:00 AM	Light Goods Vehicles	0	0	102	0	102	87	87	0	0	0	87	102	. 0	0	0	0	0	0	0	0	0	0	0	0	189	SW	0	(
	%	0%	0%	10%	0%	10%	13%	13%	0%	0%	0%	13%	10%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	11%		0%	
	Buses	0	0	5	0	5	4	4	0	0	0	4	5	0	0	0	0	0	0	0	0	0	0	0	0	9	W	0	(
	%	0%	0%	0%	0%	0%	1%	1%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%		0%	
	Single-Unit Trucks	0	0	23	0	23	16	16	0	0	0	16	23	0	0	0	0	0	0	0	0	0	0	0	0	39		0	(
	%	0%	0%	2%	0%	2%	2%	2%	0%	0%	0%	2%	2%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	2%			
	Articulated Trucks	0	0	4	0	4	5	5	0	0	0	5	4	0	0	0	0	0	0	0	0	0	0	0	0	9			
	%	0%	0%	0%	0%	0%	1%	1%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%			
	Bicycles on Road	0	0	4	0	4	2	2	0	0	0	2	4	0	0	0	0	0	0	0	0	0	0	0	0	6			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	0	0	1046	0	1046	685	685	1	0	0	686	1047	1	0	0	0	1	0	0	0	0	0	0	1	1733			
	PHF	0	0	0.91	0	0.91	0.96	0.96	0.25	0	0	0.96	0.91	0.25	0	0	0	0.25	0	0	0	0	0	0	0.25	0.94			
	Approach %					60%	40%					40%	60%					0%	0%					0%	0%				
Peak 2	Motorcycles	0	0	8	0	8	5	5	0	0	0	5	8	0	0	0	0	0	0	0	0	0	0	0	0	13	N	0	C
Specified Period	%	0%	0%	1%	0%	1%	1%	1%	0%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%		0%	
2:00 PM - 5:00 PM	Cars	0	0	1138	0	1138	777	777	5	0	0	782	1143	5	0	0	0	5	0	0	0	0	0	0	5	1925	S	0	0
One Hour Peak	%	0%	0%	89%	0%	89%	91%	91%	100%	0%	0%	91%	89%	100%	0%	0%	0%	100%	0%	0%	0%	0%	0%	0%	100%	90%		0%	
2:45 PM - 3:45 PM	Light Goods Vehicles	0	0	103	0	103	53	53	0	0	0	53	103	0	0	0	0	0	0	0	0	0	0	0	0	156	SW	0	0
	%	0%	0%	8%	0%	8%	6%	6%	0%	0%	0%	6%	8%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	7%		0%	
	Buses	0	0	3	0	3	3	3	0	0	0	3	3	0	0	0	0	0	0	0	0	0	0	0	0	6	W	0	0
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
	Single-Unit Trucks	0	0	13	0	13	11	11	0	0	0	11	13	0	0	0	0	0	0	0	0	0	0	0	0	24		0	0
	%	0%	0%	1%	0%	1%	1%	1%	0%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%			
	Articulated Trucks	0	0	4	0	4	0	0	0	0	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	4			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Bicycles on Road	0	0	8	0	8	3	3	0	0	0	3	8	0	0	0	0	0	0	0	0	0	0	0	0	11			
	%	0%	0%	1%	0%	1%	0%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%			
	Total	0	0	1277	0	1277	852	852	5	0	0	857	1282	5	0	0	0	5	0	0	0	0	0	0	5	2139			
	PHF.	0	0	0.96	0	0.96	0.94	0.94	0.42	0	0	0.95	0.97	0.42	0	0	0	0.42	0	0	0	0	0	0	0.42	0.96			
	Approach %					60%	40%					40%	60%					0%	0%					0%	0%				
								Program.											het hire										

Study Name Milton - Route 138 and Brush Hill Road TM14 TMC
Start Date Thursday, March 29, 2018 6:00 AM
Saturday, March 31, 2018 5:00 PM
Site Code

Peak 1 Specified Period :00 AM - 9:00 AM One Hour Peak :30 AM - 8:30 AM	Class. Motorcycles % Cars % Light Goods Vehicles % Buses %	R 0 0% 5 71% 1 14%	T 0 0% 561 85% 44 7%	0 0% 0 0 0% 0	0 0% 566 84% 45	0 0 0% 822 83%	R 0 0% 13 81%	T 0 0% 89	0 0% 152	0 0%	0	T 0	L O	0	0	0	R	L 0	U	0	0	Total 0	N	Pedestrians 0	s 1
Specified Period :00 AM - 9:00 AM One Hour Peak	% Cars % Light Goods Vehicles % Buses %	0% 5 71% 1 14%	0% 561 85% 44 7%	0% 0 0% 0	0% 566 84%	0% 822 83%	0% 13	0%	0%			0	0	0	0	0	0	0	0	0	0	0	N	0	
:00 AM - 9:00 AM One Hour Peak	Cars % Light Goods Vehicles % Buses %	5 71% 1 14%	561 85% 44 7%	0 0% 0	566 84%	822 83%	13			0%	001					100		- 0				-			
One Hour Peak	% Light Goods Vehicles % Buses %	71% 1 14%	85% 44 7%	0% 0	84%	83%		89	152		0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
	Light Goods Vehicles % Buses %	1 14%	44 7%	0			81%		102	254	0	805	0	0	805	973	260	4	0	264	94	1889	E	0	
:30 AM - 8:30 AM	% Buses %	14%	7%		45		0270	82%	94%	89%	0%	83%	0%	0%	83%	86%	85%	67%	0%	85%	81%	84%		0%	
	Buses %			00/		105	1	17	8	26	0	102	1	0	103	88	36	2	0	38	19	212	S	0	
	%	0		0%	7%	11%	6%	16%	5%	9%	0%	11%	100%	0%	11%	8%	12%	33%	0%	12%	16%	9%		0%	
			17	0	17	5	0	0	0	0	0	5	0	0	5	26	9	0	0	9	0	31	W	0	
	Charle Unit Tavels	0%	3%	0%	3%	1%	0%	0%	0%	0%	0%	1%	0%	0%	1%	2%	3%	0%	0%	3%	0%	1%		0%	
	Single-Unit Trucks	1	22	0	23	49	2	1	1	4	0	47	0	0	47	24	1	0	0	1	2	75		0	
	%	14%	3%	0%	3%	5%	13%	1%	1%	1%	0%	5%	0%	0%	5%	2%	0%	0%	0%	0%	2%	3%			
	Articulated Trucks	0	19	0	19	11	0	0	0	0	0	11	0	0	11	19	0	0	0	0	0	30			
	%	0%	3%	0%	3%	1%	0%	0%	0%	0%	0%	1%	0%	0%	1%	2%	0%	0%	0%	0%	0%	1%			
	Bicycles on Road	0	0	0	0	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	1	1			
	%	0%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%			
	Total	7	663	0	670	992	16	108	161	285	0	970	1	0	971	1130	306	6	0	312	116	2238			
	PHF	0.88	0.93	0	0.94	0.98	0.8	0.9	0.89	0.9	0	0.97	0.25	0	0.96	0.93	0.92	0.5	0	0.92	0.94	0.96			
	Approach %				30%	44%				13%	0%				43%	50%				14%	5%				
Peak 2	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	0	
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
:00 PM - 6:00 PM	Cars	2	587	0	589	720	11	65	117	193	0	694	2	0	696	990	286	15	0	301	69	1779	E	0	
One Hour Peak	%	100%	77%	0%	77%	88%	92%	89%	85%	87%	0%	88%	100%	0%	88%	78%	78%	94%	0%	79%	90%	82%		0%	
:00 PM - 4:00 PM	Light Goods Vehicles	0	138	0	138	59	0	6	18	24	0	58	0	0	58	230	74	1	0	75	6	295	S	0	
	%	0%	18%	0%	18%	7%	0%	8%	13%	11%	0%	7%	0%	0%	7%	18%	20%	6%	0%	20%	8%	14%		0%	
	Buses	0	5	0	5	12	0	1	0	1	0	12	0	0	12	6	1	0	0	1	1	19	W	0	
	%	0%	1%	0%	1%	1%	0%	1%	0%	0%	0%	2%	0%	0%	2%	0%	0%	0%	0%	0%	1%	1%		0%	
	Single-Unit Trucks	0	26	0	26	17	1	1	2	4	0	16	0	0	16	31	3	0	0	3	1	49		0	
	%	0%	3%	0%	3%	2%	8%	1%	1%	2%	0%	2%	0%	0%	2%	2%	1%	0%	0%	1%	1%	2%			
	Articulated Trucks	0	10	0	10	7	0	0	0	0	0	7	0	0	7	10	0	0	0	0	0	17			
	%	0%	1%	0%	1%	1%	0%	0%	0%	0%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	1%			
	Bicycles on Road	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	0	0	1	0	1			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	2	766	0	768	815	12	73	137	222	0	787	2	0	789	1268	365	16	0	381	77	2160			
	PHF	0.5	0.95	0	0.95	0.95	1	0.91	0.93	0.96	0	0.94	0.5	0	0.94	0.96	0.94	0.57	0	0.94	0.92	0.97			
	Approach %				36%	38%				10%	0%				37%	59%				18%	4%	0.07			

Study Name Milton - Route 138 and Brush Hill Road TM14 TMC
Start Date Thursday, March 29, 2018 6:00 AM
End Date Saturday, March 31, 2018 5:00 PM
Site Code

		71	So	uthbou	ınd	一位是	ne de la	V	/estbou	ınd			No	orthbou	und			2	astbou	nd				Crossw	/al
Time Period	Class.	R	T	U		0	R	Т	L	4-1	0	T	L	U		0	R	L	U	Ju	0	Total		Pedestrians	5
Peak 1	Motorcycles	0	1	0	1	1	0	0	0	0	0	1	0	0	1	1	0	0	0	0	0	2	N	1	
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		100%	
8:00 AM - 11:00 AM	Cars	0	543	0	543	579	4	69	123	196	0	567	1	0	568	903	237	8	0	245	70	1552	E	0	
One Hour Peak	%	0%	87%	0%	87%	84%	100%	88%	88%	88%	0%	84%	50%	0%	84%	87%	86%	89%	0%	87%	88%	86%		0%	
0:00 AM - 11:00 AM	Light Goods Vehicles	0	55	0	55	84	0	7	14	21	0	83	0	0	83	101	32	1	0	33	7	192	S	0	
	%	0%	9%	0%	9%	12%	0%	9%	10%	9%	0%	12%	0%	0%	12%	10%	12%	11%	0%	12%	9%	11%		0%	
	Buses	0	5	0	5	4	0	0	0	0	0	4	0	0	4	5	0	0	0	0	0	9	W	0	
	%	0%	1%	0%	1%	1%	0%	0%	0%	0%	0%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%		0%	
	Single-Unit Trucks	0	19	0	19	16	0	1	0	1	0	16	0	0	16	22	3	0	0	3	1	39		1	
	%	0%	3%	0%	3%	2%	0%	1%	0%	0%	0%	2%	0%	0%	2%	2%	1%	0%	0%	1%	1%	2%			
	Articulated Trucks	0	3	0	3	5	0	0	1	1	0	5	0	0	5	4	0	0	0	0	0	9			
	%	0%	0%	0%	0%	1%	0%	0%	1%	0%	0%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%			
	Bicycles on Road	0	0	0	0	2	0	1	2	3	0	2	1	0	3	4	2	0	0	2	2	8			
	%	0%	0%	0%	0%	0%	0%	1%	1%	1%	0%	0%	50%	0%	0%	0%	1%	0%	0%	1%	3%	0%			
	Total	0	626	0	626	691	4	78	140	222	0	678	2	0	680	1040	274	9	0	283	80	1811			
	PHF	0	0.94	0	0.94	0.95	0.5	0.78	0.88	0.84	0	0.95	0.25	0	0.95	0.92	0.84	0.38	0	0.85	0.8	0.96			
	Approach %				35%	38%				12%	0%				38%	57%				16%	4%				
Peak 2	Motorcycles	0	3	0	3	7	0	1	0	1	0	7	0	0	7	7	4	0	0	4	1	15	N	0	
Specified Period	%	0%	0%	0%	0%	1%	0%	1%	0%	0%	0%	1%	0%	0%	1%	1%	1%	0%	0%	1%	1%	1%		0%	
2:00 PM - 5:00 PM	Cars	4	702	0	706	775	0	108	152	260	0	770	1	0	771	1156	302	5	0	307	113	2044	Е	0	
One Hour Peak	%	80%	91%	0%	91%	90%	0%	90%	88%	89%	0%	90%	100%	0%	90%	91%	92%	100%	0%	92%	90%	91%		0%	
3:00 PM - 4:00 PM	Light Goods Vehicles	1	54	0	55	58	0	11	18	29	0	58	0	0	58	92	20	0	0	20	12	162	S	0	
	%	20%	7%	0%	7%	7%	0%	9%	10%	10%	0%	7%	0%	0%	7%	7%	6%	0%	0%	6%	10%	7%		0%	
	Buses	0	4	0 .	4	4	0	0	0	0	0	4	0	0	4	4	0	0	0	0	0	8	W	0	
	%	0%	1%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
	Single-Unit Trucks	0	7	0	7	12	0	0	1	1	0	12	0	0	12	8	0	0	0	0	0	20		0	
	%	0%	1%	0%	1%	1%	0%	0%	1%	0%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	1%			
	Articulated Trucks	0	2	0	2	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0	2			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Bicycles on Road	0	1	0	1	2	0	0	1	1	0	2	0	0	2	5	3	0	0	3	0	7			
	%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	0%	0%			
	Total	5	773	0	778	858	0	120	172	292	0	853	1	0	854	1274	329	5	0	334	126	2258			
	PHF	0.42	0.96	0	0.95	0.96	0	0.77	0.98	0.88	0	0.96	0.25	0	0.96	0.97	0.87	0.42	0	0.88	0.79	0.95			
	Approach %				34%	38%				13%	0%				38%	56%				15%	6%				
										- Harrison	FIRM LAV			MINISTER					H KAN						

Study Name Milton - Canton Avenue and Brush Hill Road TM15 TMC
Start Date Thursday, March 29, 2018 6:00 AM
Saturday, March 31, 2018 5:00 PM
Site Code

		5 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -	S	outhbou	ınd			No	orthbou	ınd		East	bound		E ALL	Crosswa	
Time Period	Class.	R	T	U	1	0	Т	L	U	1	0	T	0	Total		Pedestrians	To
Peak 1	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	N	0	
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
6:00 AM - 9:00 AM	Cars	122	0	0	122	346	346	134	0	480	0	0	256	602	S	0	
One Hour Peak	- %	92%	0%	0%	92%	88%	88%	86%	0%	88%	0%	0%	89%	88%		0%	
7:30 AM - 8:30 AM	Light Goods Vehicles	10	0	0	10	41	41	17	0	58	0	0	27	68	W	0	
	%	8%	0%	0%	8%	10%	10%	11%	0%	11%	0%	0%	9%	10%		0%	
	Buses	0	0	0	0	0	0	0	0	0	0	0	0	0		0	
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Single-Unit Trucks	1	0	0	1	5	5	3	0	8	0	0	4	9			
	%	1%	0%	0%	1%	1%	1%	2%	0%	1%	0%	0%	1%	1%			
	Articulated Trucks	0	0	0	0	1	1	0	0	1	0	0	0	1			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Bicycles on Road	0	0	0	0	0	0	1	0	1	0	0	1	1			
	%	0%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%			
	Total	133	0	0	133	393	393	155	0	548	0	0	288	681			
	PHF	0.85	0	0	0.85	0.86	0.86	0.84	0	0.86	0	0	0.89	0.87			
	Approach %				20%	58%				80%	0%	0%	42%			*	
Peak 2	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	N	0	
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
3:00 PM - 6:00 PM	Cars	181	0	0	181	305	305	64	0	369	0	0	245	550	S	0	
One Hour Peak	%	91%	0%	0%	91%	91%	91%	88%	0%	90%	0%	0%	90%	90%		0%	
5:00 PM - 6:00 PM	Light Goods Vehicles	17	0	0	17	24	24	8	0	32	0	0	25	49	W	0	
3.00 1 141 0.00 1 141	%	9%	0%	0%	9%	7%	7%	11%	0%	8%	0%	0%	9%	8%	- "	0%	
	Buses	0	0	0	0	3	3	1	0	4	0	0	1	4		0	
	%	0%	0%	0%	0%	1%	1%	1%	0%	1%	0%	0%	0%	1%			
	Single-Unit Trucks	0	0	0	0	1	1	0	0	1	0	0	0	1			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Bicycles on Road	1	0	1	2	4	3	0	0	3	0	0	1	5			
	% Total	1% 199	0% 0	100%	1% 200	1% 337	1% 336	0% 73	0%	1% 409	0% 0	0%	0%	1% 609			
	PHF		0		200-27-9			0.87	0	2.22			272	and the same of			
		0.9	U	0.25	0.89	0.86	0.87	0.87	U	0.92	0	- 0-	0.93	0.91			
	Approach %				33%	55%				67%	0%	0%	45%				

Study Name Milton - Canton Avenue and Brush Hill Road TM15 TMC

Start Date Thursday, March 29, 2018 6:00 AM

End Date Site Code Saturday, March 31, 2018 5:00 PM

	Class. Motorcycles % Cars % Light Goods Vehicles % Buses % Single-Unit Trucks	R 0 0% 118 89% 11 8% 1 1%	0 0% 0 0% 0 0% 0	0 0% 0 0% 0 0% 0	0 0% 118 89% 11	0 0 0% 176 89% 13	T 0 0% 176 89% 13	0 0% 82 92%	0 0% 0	0 0% 258 90%	0 0 0% 0	0 0% 0	0 0 0% 200	0 0% 376	N S	Pedestrians 1 100% 2	Tota 1
Specified Period :00 AM - 11:00 AM One Hour Peak :45 AM - 10:45 AM L	% Cars % Light Goods Vehicles % Buses % Single-Unit Trucks	0% 118 89% 11 8% 1	0% 0 0% 0 0% 0	0% 0 0% 0 0	0% 118 89% 11	0% 176 89%	0% 176 89%	0% 82 92%	0% 0	0% 258	0% 0	0% 0	0% 200	0% 376		100%	1
:00 AM - 11:00 AM One Hour Peak :45 AM - 10:45 AM L	Cars % ight Goods Vehicles % Buses % Single-Unit Trucks	118 89% 11 8% 1	0 0% 0 0% 0	0 0% 0 0%	118 89% 11	176 89%	176 89%	82 92%	0	258	0	0	200	376	S		
One Hour Peak :45 AM - 10:45 AM L	% Light Goods Vehicles % Buses % Single-Unit Trucks	89% 11 8% 1 1%	0% 0 0% 0	0% 0 0%	89% 11	89%	89%	92%	- "-						S	2	
:45 AM - 10:45 AM L	ight Goods Vehicles	11 8% 1 1%	0 0% 0	0 0%	11				0%	90%		-		10000		-	2
	% Buses % Single-Unit Trucks	8% 1 1%	0% O	0%		13	13	100		3070	0%	0%	90%	90%		100%	
	Buses % Single-Unit Trucks	1 1%	0		8%		10000	6	0	19	0	0	17	30	W	1	1
	% Single-Unit Trucks	1%		0		7%	7%	7%	0%	7%	0%	0%	8%	7%		100%	
	Single-Unit Trucks		0%	9	1	0	0	0	0	0	0	0	1	1		4	4
		0	0/0	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	%		0	0	0	1	1	1	0	2	0	0	1	2			
		0%	0%	0%	0%	1%	1%	1%	0%	1%	0%	0%	0%	0%			
	Articulated Trucks	1	0	0	1	0	0	0	0	0	0	0	1	1			
	%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Bicycles on Road	2	0	0	2	8	8	0	0	8	0	0	2	10			
	%	2%	0%	0%	2%	4%	4%	0%	0%	3%	0%	0%	1%	2%			
	Total	133	0	0	133	198	198	89	0	287	0	0	222	420			
	PHF	0.85	0	0	0.85	0.88	0.88	0.86	0	0.88	0	0	0.85	0.87			
	Approach %				32%	47%				68%	0%	0%	53%				
									- *								
Peak 2	Motorcycles	1	0	0	1	6	6	1	0	7	0	0	2	8	N	0	0
Specified Period	%	1%	0%	0%	1%	2%	2%	1%	0%	2%	0%	0%	1%	1%		0%	
2:00 PM - 5:00 PM	Cars	117	0	0	117	252	252	160	0	412	0	0	277	529	S	0	0
One Hour Peak	%	87%	0%	0%	87%	89%	89%	91%	0%	90%	0%	0%	90%	90%		0%	
3:15 PM - 4:15 PM L	ight Goods Vehicles	13	0	0	13	16	16	13	0	29	0	. 0	26	42	W	0	0
	%	10%	0%	0%	10%	6%	6%	7%	0%	6%	0%	0%	8%	7%		0%	
	Buses	0	0	0	0	0	0	0	0	0	0	0	0	0		0	0
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Single-Unit Trucks	2	0	0	2	0	0	0	0	0	0	0	2	2			
	%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%			
	Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Bicycles on Road	1	0	0	1	8	8	1	0	9	0	0	2	10			
	%	1%	0%	0%	1%	3%	3%	1%	0%	2%	0%	0%	1%	2%			
	Total	134	0	0	134	282	282	175	0	457	0	0	309	591			
	PHF	0.93	0	0	0.93	0.79	0.79	0.88	0	0.82	0	0	0.92	0.85			
	Approach %				23%	48%				77%	0%	0%	52%				

Study Name Milton - Route 138 and Neponset Valley Parkway TM16 TMC
Start Date Thursday, March 29, 2018 6:00 AM
End Date Saturday, March 31, 2018 5:00 PM
Site Code

			So	uthbou	ınd			No	orthbou	ınd			E	astboui	nd				Crosswa	alk
Time Period	Class.	R	Т	U	1	0	Т	L	U	1	0	R	L	U	- 1	0	Total	F	Pedestrians	Т
Peak 1	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	0	
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
6:00 AM - 9:00 AM	Cars	2	432	0	434	460	458	364	0	822	548	116	2	0	118	366	1374	S	0	
One Hour Peak	%	67%	88%	0%	88%	84%	84%	83%	0%	83%	83%	69%	50%	0%	69%	82%	83%		0%	
7:30 AM - 8:30 AM	Light Goods Vehicles	0	33	0	33	51	50	54	0	104	54	21	1	0	22	54	159	W	0	
	%	0%	7%	0%	7%	9%	9%	12%	0%	11%	8%	13%	25%	0%	13%	12%	10%		0%	
	Buses	0	5	0	5	3	3	2	0	5	17	12	0	0	12	2	22		0	
	%	0%	1%	0%	1%	1%	1%	0%	0%	1%	3%	7%	0%	0%	7%	0%	1%			
	Single-Unit Trucks	1	8	0	9	31	30	13	0	43	22	14	1	0	15	14	67			
	%	33%	2%	0%	2%	6%	6%	3%	0%	4%	3%	8%	25%	0%	9%	3%	4%			
	Articulated Trucks	0	14	0	14	4	4	8	0	12	19	5	0	0	5	8	31			
	%	0%	3%	0%	3%	1%	1%	2%	0%	1%	3%	3%	0%	0%	3%	2%	2%			
	Bicycles on Road	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	3	492	0	495	549	545	441	0	986	660	168	4	0	172	444	1653			
	PHF	0.38	0.98	0	0.98	0.93	0.93	0.93	0	0.98	0.94	0.86	0.5	0	0.86	0.93	0.98			
	Approach %				30%	33%				60%	40%				10%	27%				
Peak 2	Mataravalos	0	0	0		0		0	0	0	0		0	0		0	0	N	0	
	Motorcycles	0	0		0	0	0					0	0	0	0	0		IN		
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
3:00 PM - 6:00 PM One Hour Peak	Cars	3	424	0	427	451	445	258	0	703	583	159	6	0	165	261		S	0	
	%	100%	80%	0%	80%	90%	90%	82%	0%	87%	76%	69%	86%	0%	69%	82%	82%	14/	0%	
3:00 PM - 4:00 PM	Light Goods Vehicles	0	81	0	81	35	35	36	0	71	140	59	0	0	59	36		W	0	
	%	0%	15%	0%	15%	7% -	7%	11%	0%	9%	18%	25%	0%	0%	25%	11%	13%		0%	
	Buses	0	4	0	4	7	6	6	0	12	5	1	1	0	2	6	18		0	
	%	0%	1%	0%	1%	1%	1%	2%	0%	1%	1%	0%	14%	0%	1%	2%	1%			
	Single-Unit Trucks	0	15	0	15	5	5	11	0	16	26	11	0	0	11	11	42			
	% • • • • • • • • • • • • • • • • • • •	0%	3%	0%	3%	1%	1%	4%	0%	2%	3%	5%	0%	0%	5%	3%	3%			
	Articulated Trucks	0	8	0	8	3	3	3	0	6	10	2	0	0	2	3	16			
	%	0%	2%	0%	1%	1%	1%	1%	0%	1%	1%	1%	0%	0%	1%	1%	1%			
	Bicycles on Road	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	% 	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	3	532	0	535	501	494	314	0	808	764	232	7	0	239	317	1582			
	PHF	0.75	0.92	0	0.92	0.97	0.97	0.88	0	0.95	0.93	0.87	0.44	0	0.88	0.88	0.97			
	Approach %				34%	32%				51%	48%				15%	20%				

			Sou	thbo	ound			Nor	thbo	ound			Eas	tbo	und				Crosswa	lk
Time Period	Class.	R	Т	U	- 1	0	Т	L	U	- 1	0	R	L	U	- 1	0	Total		Pedestrians	Total
Peak 1	Motorcycles	0	1	0	1	0	0	1	0	1	1	0	0	0	0	1	2	N	0	0
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
8:00 AM - 11:00 AM	Cars	1	400	0	401	365	364	229	0	593	549	149	1	0	150	230	1144	S	1	1
One Hour Peak	%	50%	89%	0%	89%	89%	89%	78%	0%	85%	87%	81%	100%	0%	81%	78%	86%		100%	
10:00 AM - 11:00 AM	Light Goods Vehicles	1	36	0	37	26	26	54	0	80	57	21	0	0	21	55	138	W	1	1
	%	50%	8%	0%	8%	6%	6%	18%	0%	11%	9%	11%	0%	0%	11%	19%	10%		100%	
	Buses	0	3	0	3	3	3	0	0	3	5	2	0	0	2	0	8		2	2
	%	0%	1%	0%	1%	1%	1%	0%	0%	0%	1%	1%	0%	0%	1%	0%	1%			
	Single-Unit Trucks	0	7	0	7	11	11	4	0	15	18	11	0	0	11	4	33			
	%	0%	2%	0%	2%	3%	3%	1%	0%	2%	3%	6%	0%	0%	6%	1%	2%			
	Articulated Trucks	0	2	0	2	2	2	3	0	5	3	1	0	0	1	3	8			
	%	0%	0%	0%	0%	0%	0%	1%	0%	1%	0%	1%	0%	0%	1%	1%	1%			
	Bicycles on Road	0	0	0	0	1	1	1	0	2	0	0	0	0	0	1	2			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	2	449	0	451	408	407	292	0	699	633	184	1	0	185	294	1335			
	PHF	0.25	0.94	0	0.93	0.95	0.96	0.95	0	0.99	0.96	0.82	0.25	0	0.81	0.93	0.98			
	Approach %				34%	31%				52%	47%				14%	22%				
Peak 2	Motorcycles	0	2	0	2	1	1	4	0	5	4	2	0	0	2	4	9	N	0	0
Specified Period	%	0%	0%	0%	0%	0%	0%	1%	0%	1%	1%	1%	0%	0%	1%	1%	1%		0%	
2:00 PM - 5:00 PM	Cars	2	519	0	521	429	427	357	0	784	705	186	2	0	188	359	1493	S	0	0
One Hour Peak	%	100%	90%	0%	90%	93%	93%	90%	0%	91%	89%	86%	100%	0%	86%	90%	90%		0%	
2:45 PM - 3:45 PM	Light Goods Vehicles	0	39	0	39	26	26	22	0	48	61	22	0	0	22	22	109	W	0	0
	%	0%	7%	0%	7%	6%	6%	6%	0%	6%	8%	10%	0%	0%	10%	6%	7%		0%	
	Buses	0	2	0	2	1	1	2	0	3	3	1	0	0	1	2	6		0	0
	%	0%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%			
	Single-Unit Trucks	0	10	0	10	2	2	10	0	12	13	3	0	0	3	10	25			
	%	0%	2%	0%	2%	0%	0%	3%	0%	1%	2%	1%	0%	0%	1%	3%	2%			
	Articulated Trucks	0	2	0	2	0	0	0	0	0	4	2	0	0	2	0	4			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	1%	0%	0%	1%	0%	0%			
	Bicycles on Road	0	3	0	3	4	4	1	0	5	4	1	0	0	1	1	9			
	%	0%	1%	0%	1%	1%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	1%			
	Total	2	577	0	579	463	461	396	0	857	794	217	2	0	219	398	1655			
	PHF	0.5	0.91	0	0.92	0.86	0.87	0.87	0	0.94	0.95	0.9	0.5	0	0.9	0.87	0.97			
	Approach %				35%	28%				52%	48%				13%	24%				

Study Name Milton - Route 138 at Dollar Lane and Milton Street TM17 TMC
Start Date Thursday, March 29, 2018 6:00 AM
Saturday, March 31, 2018 5:00 PM
Site Code

				South	bound		11111	Pipe		West	bound					North	bound					East	bound					Crossw	wal
Time Period	Class.	R	T	L	U	T. IL	0	R	T	L	U	1	0	R	ī	L	U	i	0	R	T	L	U	1	0	Total		Pedestrians	15
Peak 1	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	0	
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
00 AM - 9:00 AM	Cars	24	404	0	0	428	488	34	166	23	0	223	107	7	423	9	0	439	439	12	100	31	0	143	199	1233	E	0	
One Hour Peak	%	86%	88%	0%	0%	88%	82%	92%	89%	92%	0%	90%	88%	88%	80%	82%	0%	81%	89%	100%	88%	91%	0%	90%	88%	86%		0%	
':15 AM - 8:15 AM	Light Goods Vehicles	3	26	1	0	30	64	2	19	2	0	23	10	1	61	2	0	64	28	0	8	1	0	9	24	126	S	0	
	%	11%	6%	100%	0%	6%	11%	5%	10%	8%	0%	9%	8%	13%	12%	18%	0%	12%	6%	0%	7%	3%	0%	6%	11%	9%		0%	
	Buses	0	4	0	0	4	4	0	0	0	0	0	2	0	3	0	0	3	4	0	2	1	0	3	0	10	W	0	
	%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	2%	0%	1%	0%	0%	1%	1%	0%	2%	3%	0%	2%	0%	1%		0%	
	Single-Unit Trucks	1	9	0	0	10	36	1	2	0	0	3	3	0	34	0	0	34	9	0	3	1	0	. 4	3	51		0	
	%	4%	2%	0%	0%	2%	6%	3%	1%	0%	0%	1%	2%	0%	6%	0%	0%	6%	2%	0%	3%	3%	0%	3%	1%	4%			
	Articulated Trucks	0	15	0	0	15	5	0	0	0	0	0	0	0	5	0	0	5	15	0	0	0	0	0	0	20			
	%	0%	3%	0%	0%	3%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	3%	0%	0%	0%	0%	0%	0%	1%			
	Bicycles on Road	0	0	0	0	0	0	0	0	0	0	0	. 0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	28	458	1	0	487	597	37	187	25	0	249	122	8	526	11	0	545	495	12	113	34	0	159	226	1440			
	PHF	0.64	0.96	0.25	0	0.94	0.93	0.77	0.88	0.69	0	0.92	0.87	0.67	0.91	0.46	0	0.91	0.98	0.6	0.88	0.85	0	0.85	0.86	0.97			
	Approach %					34%	41%					17%	8%					38%	34%					11%	16%				
Peak 2	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	1	
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		100%	
3:00 PM - 6:00 PM	Cars	20	414	5	0	439	476	26	93	9	0	128	120	9	428	15	0	452	437	14	106	22	0	142	128	1161	E	0	
One Hour Peak	%	80%	81%	63%	0%	81%	90%	96%	82%	82%	0%	84%	90%	90%	90%	88%	0%	90%	82%	93%	91%	79%	0%	89%	82%	86%		0%	
3:00 PM - 4:00 PM	Light Goods Vehicles	4	71	3	0	78	35	1	15	1	0	17	12	0	30	2	0	32	73	1	9	4	0	14	21	141	S	0	
	%	16%	14%	38%	0%	14%	7%	4%	13%	9%	0%	11%	9%	0%	6%	12%	0%	6%	14%	7%	8%	14%	0%	9%	13%	10%		0%	
	Buses	1	4	0	0	5	8	0	4	0	0	4	1	1	6	0	0	7	4	0	0	2	0	2	5	18	W	0	
	%	4%	1%	0%	0%	1%	2%	0%	4%	0%	0%	3%	1%	10%	1%	0%	0%	1%	1%	0%	0%	7%	0%	1%	3%	1%		0%	
	Single-Unit Trucks	0	16	0	0	16	6	0	2	0	0	2	1	0	6	0	0	6	16	0	1	0	0	1	2 .	25		1	
	%	0%	3%	0%	0%	3%	1%	0%	2%	0%	0%	1%	1%	0%	1%	0%	0%	1%	3%	0%	1%	0%	0%	1%	1%	2%			
	Articulated Trucks	0	5	0	0	5	3	0	0	1	0	1	0	0	3	0	0	3	6	0	0	0	0	0	0	9			
	%	0%	1%	0%	0%	1%	1%	0%	0%	9%	0%	1%	0%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	1%			
	Bicycles on Road	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	25	510	8	0	543	528	27	114	11	0	152	134	10	473	17	0	500	536	15	116	28	0	159	156	1354			
	PHF	0.78	0.9	0.33	- 0	0.94	0.93	0.52	0.89	0.55	0	0.76	0.86	0.83	0.97	0.71	0	0.95	0.92	0.62	0.85	0.7	0	0.92	0.91	0.95			
	Approach %					40%	39%	10.				11%	10%					37%	40%					12%	12%				

Start Date
Start Date
End Date
Milton - Route 138 at Dollar Lane and Milton Street TM17 TMC
Thursday, March 29, 2018 6:00 AM
Saturday, March 31, 2018 5:00 PM

Site Code

				South	bound	The state of		× 11.53		West	bound					North	bound					Eastl	bound		Pleas		120	Crossw	
Time Period	Class.	R	T	L	U		0	R	Time	L	U		0	R	T	L	U	Seattle of	0	R	T	L	U		0	Total		Pedestrians	To
Peak 1	Motorcycles	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	N	1	1
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		100%	
8:00 AM - 11:00 AM	Cars	15	378	0	0	393	365	8	63	11	. 0	82	80	8	336	8	0	352	401	12	72	21	0	105	86	932	E	0	O
One Hour Peak	%	88%	90%	0%	0%	90%	87%	100%	89%	73%	0%	87%	90%	89%	88%	100%	0%	88%	89%	92%	91%	81%	0%	89%	90%	89%		0%	
10:00 AM - 11:00 AM	Light Goods Vehicles	2	32	1	0	35	37	0	7	3	0	10	8	0	33	0	0	33	36	1	7	4	0	12	9	90	S	0	0
	%	12%	8%	100%	0%	8%	9%	0%	10%	20%	0%	11%	9%	0%	9%	0%	0%	8%	8%	8%	9%	15%	0%	10%	9%	9%		0%	
	Buses	0	3	0	0	3	4	0	0	0	0	0	0	0	3	0	0	3	3	0	0	1	0	1	0	7	W	0	0
	%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	1%	0%	0%	4%	0%	1%	0%	1%		0%	
	Single-Unit Trucks	0	5	0	0	5	9	0	0	1	0	1	1	1	9	0	0	10	6	0	0	0	0	0	0	16		1	1
	%	0%	1%	0%	0%	1%	2%	0%	0%	7%	0%	1%	1%	11%	2%	0%	0%	2%	1%	0%	0%	0%	0%	0%	0%	2%			
	Articulated Trucks	0	2	0	0	2	2	0	0	0	0	0	0	0	2	0	0	2	2	0	0	0	0	0	0	4			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Bicycles on Road	0	0	0	0	0	1	0	1	0	0	1	0	0	1	0	0	1	0	0	0	0	0	0	1	2			
	%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%			
	Total	17	421	1	0	439	418	8	71	15	0	94	89	9	384	8	0	401	449	13	79	26	0	118	96	1052			
	PHF	0.47	0.89	0.25	0	0.9	0.91	1	0.77	0.54	0	0.81	0.72	0.56	0.91	0.67	0	0.93	0.91	0.65	0.66	0.72	0	0.69	0.89	0.97			
	Approach %					42%	40%					9%	8%					38%	43%					11%	9%				
Peak 2	Motorcycles	0	2	0	0	2	2	0	2	0	0	2	0	0	2	0	0	2	2	0	0	0	0	0	2	6	N	0	0
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	3%	0%	0%	2%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	2%	0%		0%	
2:00 PM - 5:00 PM	Cars	24	505	1	0	530	480	17	65	17	0	99	95	9	435	15	0	459	529	7	85	28	0	120	104	1208	Е	0	0
One Hour Peak	%	89%	93%	100%	0%	93%	94%	89%	89%	100%	0%	91%	90%	100%	94%	88%	0%	94%	93%	88%	89%	93%	0%	90%	89%	93%		0%	
3:00 PM - 4:00 PM	Light Goods Vehicles	2	27	0	0	29	26	2	6	0	0	8	7	0	22	2	0	24	28	1	7	2	0	10	10	71	S	1	1
	%	7%	5%	0%	0%	5%	5%	11%	8%	0%	0%	7%	7%	0%	5%	12%	0%	5%	5%	13%	7%	7%	0%	7%	9%	5%		100%	
	Buses	0	2	0	0	2	1	0	0	0	0	0	2	0	1	0	0	1	2	0	2	0	0	2	0	5	W	0	0
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	2%	0%	0%	0%	0%	0%	0%	0%	2%	0%	0%	1%	0%	0%		0%	
	Single-Unit Trucks	0	5	0	0	5	2	0	0	0	0	0	2	0	2	0	0	2	5	0	2	0	0	2	0	9		1	1
	%	0%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	2%	0%	0%	0%	0%	0%	1%	0%	2%	0%	0%	1%	0%	1%			
	Articulated Trucks	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Bicycles on Road	1	0	0	0	1	1	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	1	2			
	%	4%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%			
	Total	27	542	1	0	570	512	19	73	17	0	109	106	9	463	17	0	489	567	8	96	30	0	134	117	1302			
	PHF	0.75	0.91	0.25	0	0.93	0.93	0.79	0.96	0.61	0	0.91	0.76	0.56	0.94	0.85	0	0.93	0.89	0.67	0.75	0.83	0	0.84	0.89	0.95			
	Approach %					44%	39%					8%	8%					38%	44%					10%	9%				

Study Name Milton - Route 138 and Blue Jay Way/Curry College TM18 TMC
Start Date Thursday, March 29, 2018 6:00 AM
Thursday, March 29, 2018 6:00 PM
Site Code

			So	uthbou	ınd		-	No	orthbou	ınd				astbour	nd				Crosswa	alk
Time Period	Class.	R	T	U		0	T	L	U		0	R	V.L	U		0	Total		Pedestrians	1
Peak 1	Motorcycles	1	0	0	1	0	0	0	0	0	0	0	0	0	0	1	1	N	1	(i :
Specified Period	%	2%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%		100%	
6:00 AM - 9:00 AM	Cars	58	419	0	477	393	388	126	0	514	431	12	5	0	17	184	1008	S	0	
One Hour Peak	%	95%	87%	0%	88%	81%	80%	96%	0%	84%	87%	80%	100%	0%	85%	96%	86%		0%	
7:30 AM - 8:30 AM	Light Goods Vehicles	2	32	0	34	54	54	4	0	58	34	2	0	0	2	6	94	W	16	
	%	3%	7%	0%	6%	11%	11%	3%	0%	9%	7%	13%	0%	0%	10%	3%	8%		100%	
	Buses	0	4	0	4	3	3	0	0	3	4	0	0	0	0	0	7		17	
	%	0%	1%	0%	1%	1%	1%	0%	0%	0%	1%	0%	0%	0%	0%	0%	1%			
	Single-Unit Trucks	0	11	0	11	34	34	1	0	35	12	1	0	0	1	1	47			
	%	0%	2%	0%	2%	7%	7%	1%	0%	6%	2%	7%	0%	0%	5%	1%	4%			
	Articulated Trucks	0	14	0	14	4	4	0	0	4	14	0	0	0	0	0	18			
	%	0%	3%	0%	3%	1%	1%	0%	0%	1%	3%	0%	0%	0%	0%	0%	2%			
	Bicycles on Road	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	61	480	0	541	488	483	131	0	614	495	15	5	0	20	192	1175			
	PHF	0.64	0.94	0	0.93	0.91	0.91	0.61	0	0.95	0.95	0.42	0.42	0	0.42	0.62	0.92			
	Approach %				46%	42%				52%	42%				2%	16%				
Peak 2	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	0	
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
3:00 PM - 6:00 PM	Cars	23	349	0	372	460	409	71	0	480	441	92	51	0	143	94	995	S	0	
One Hour Peak	%	96%	80%	0%	81%	89%	88%	95%	0%	89%	82%	91%	94%	0%	92%	95%	86%		0%	
3:00 PM - 4:00 PM	Light Goods Vehicles	1	63	0	64	42	39	3	0	42	72	9	3	0	12	4	118	W	1	
	%	4%	14%	0%	14%	8%	8%	4%	0%	8%	13%	9%	6%	0%	8%	4%	10%		100%	
	Buses	0	5	0	5	8	8	0	0	8	5	0	0	0	0	0	13		1	
	%	0%	1%	0%	1%	2%	2%	0%	0%	1%	1%	0%	0%	0%	0%	0%	1%			
	Single-Unit Trucks	0	15	0	15	4	4	1	0	5	15	0	0	0	0	1	20			
	%	0%	3%	0%	3%	1%	1%	1%	0%	1%	3%	0%	0%	0%	0%	1%	2%			
	Articulated Trucks	0	6	0	6	3	3	0	0	3	6	0	0	0	0	0	9			
	%	0%	1%	0%	1%	1%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	1%			
	Bicycles on Road	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	24	438	0	462	517	463	75	0	538	539	101	54	0	155	99	1155			
	PHF	0.75	0.89	0	0.88	0.94	0.93	0.78	0	0.91	0.93	0.7	0.68	0	0.69	0.85	0.93			
	Approach %				40%	45%	-			47%	47%	Live Co.			13%	9%	100			

Study Name Milton - Route 138 at Bradlee Road and Atherton Street TM19 TMC

Start Date Thursday, March 29, 2018 6:00 AM End Date Thursday, March 29, 2018 6:00 PM

Report Summary

Site Code

		(P)	W.E.	S	outhbou	ind					1	Vestbo	und						North	bound		T A				No	rtheas	tbound	1000					F	astbou	nd					Cross	swalk
Time Period	Class.	R	BR		1			0	R	П		1	U		0	R	Т	-	1	METERS WHICH STREET STREET			0	HR	BR	and the second	HL	ALC: UNKNOWN	-	nation of	0	HR	R	TE		U		0	Tota		Pedestria	
Peak 1	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	-0	0	0) (0	0	0	0	0	0	0			0	0	0	0	0	0	0	0	0	N	0	0
Specified Period	*	0%	0%	0%	0%	0%	0%	0%	0%	0%	096	0%	0%	0%	0%	0%	0%	00	6 07			0%	0%	0%	0%	0%	0%	09	6 02			0%	0%	0%	0%	0%	0%	094	0%		0%	
6:00 AM - 9:00 AM	Cars	2	0	403	3	0	408	412	27	41	0	38	0	106	34	6	38	1 2) (10	469	1	2	0	0	0	7			0	27	23	4	0	54	66	981	F	0	0
One Hour Peak	%	100%	0%	90%	100%	0%	90%	81%	96%	89%	0%	90%	0%	91%	92%	86%	809	6 79	6 09	% O	×	30%	89%	100%	100%	0%	0%	05	100	7% (0%	0%	84%	92%	57%	0%	84%	86%	85%		0%	
7:15 AM - 8:15 AM	Light Goods Vehicles	0	0	20	0	0	20	56	1	5	1	4	0	11	1	0	53	4	1	L (0	58	28	0	0	0	0	0	0		2	0	4	1	2	0	7	9	96	S	0	0
	×	0%	0%	4%	0%	0%	4%	11%	4%	11%	100%	10%	0%	9%	3%	0%	119	6 14:	6 100	0% 0%	96	1%	5%	0%	0%	0%	0%	0)	02	6 10	00%	0%	13%	4%	29%	0%	11%	12%	8%		0%	
	Buses	0	0	3	0	0	3	3	0	0	0	0	0	0	1	0	3	1	0) (0	4	4	0	0	0	0	0	0		0	0	1	1	0	0	2	1	9	sw	1	1
	%	0%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	3%	0%	1%	39	. 09	6 0	ж	196	1%	0%	0%	0%	0%	09	0)		%	0%	3%	4%	0%	0%	3%	1%	1%		100%	
	Single-Unit Trucks	0	0	10	0	0	10	31	0	0	0	0	0	0	1	1	31	1	0) (0	33	10	0	0	0	0	0	0		0	0	0	0	0	0	0	1	43	w	1	1
	%	0%	0%	2%	0%	0%	2%	6%	0%	0%	096	0%	0%	0%	3%	14%	7%	39	09	6 0	96	6%	2%	0%	0%	0%	0%	0)	07		%	0%	0%	0%	0%	0%	0%	1%	4%		100%	
	Articulated Trucks	0	0	14	0	0	14	6	0	0	0	0	0	0	0	0	6	0	0) (0	6	14	0	0	0	0	0	0		0	0	0	0	0	0	0	0	20		2	2
	%	0%	096	3%	0%	0%	3%	1%	096	0%	0%	0%	0%	0%	0%	0%	1%	09	09	6 0	%	196	3%	0%	0%	0%	0%	09	09		96	0%	0%	0%	0%	0%	0%	0%	2%			
	Bicycles on Road	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0) C	0	0	0	0	0	0	0	0	0		0	0	0	0	1	0	1	0	1			
	*	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	6 02	%	096	0%	0%	0%	0%	0%	09	0)		96	0%	0%	0%	14%	0%	2%	0%	0%			
	Total	2	0	450	3	0	455	509	28	46	1	42	0	117	37	7	474	29	1	. 0) !	11	525	1	2	0	0	0	3		2	0	32	25	7	0	64	77	1150			
	PHF	0.25	0	0.95	0.38	0	0.96	0.96	0.58	0.77	0.25	0.95	0	0.98	0.71	0.44	0.9	5 0.7	2 0.2	25 0	0 0	.96	0.94	0.25	0.25	0	0	0	0.3	8 0	.5	0	0.67	0.69	0.58	0 .	0.67	0.77	0.96			
	Approach %						40%	44%	100					10%	3%						4	4%	46%						0%	. 0	16						6%	7%	la an			
Peak 2	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	0	0			0	0	0	0	0	0	0	0	N	0	0
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	6 0)	×	996	0%	096	0%	096	0%	096	0%	0	%	0%	0%	0%	0%	096	0%	0%	0%		0%	
3:00 PM - 6:00 PM	Cars	4	0	323	6	0	333	413	10	33	2	24	0	69	64	29	396	31	1	. 0	137		361	0	1	2	0	0	3			0	14	28	5	0	47	68	909	E	0	0
One Hour Peak	*	67%	0%	78%	86%	0%	78%	90%	91%	77%	100%	92%	0%	84%	85%	85%	90%	919	507	% 09	% 8	9%	79%	0%	50%	100%	0%	0%	759	6 75	%	0%	82%	83%	83%	0%	85%	82%	84%		0%	
3:00 PM - 4:00 PM	Light Goods Vehicles	2	0	65	0	0	67	33	1	8	0	2	0	11	7	4	31	3	1	. 0)	39	70	0	1	0	0	0	1		1	0	3	2	1	0	6	13	124	S	0	0
	%	33%	0%	16%	0%	0%	16%	7%	9%	19%	0%	8%	0%	13%	9%	12%	7%	9%	509	16 09	κ.	96	15%	0%	50%	0%	0%	0%	259	6 25	%	0%	18%	6%	17%	0%	11%	16%	12%		0%	
	Buses	0	0	5	0	0	5	7	0	1	0	0	0	1	2	1	7	0	0	0)	8	5	0	0	0	0	0	0	(0	0 .	1	0	0	1	1	15	sw	0	0
	*	0%	0%	1%	0%	0%	1%	2%	096	2%	0%	0%	0%	1%	3%	3%	2%	0%	0%	6 0)	16	%	1%	0%	0%	0%	0%	0%	0%	0	76 (0%	0%	3%	0%	0%	2%	1%	196		0%	
	Single-Unit Trucks	0	0	13	1	0	14	4	0	1	0	0	0	1	2	0	4	0	0	0)	4	13	0	0	0	0	0	0	()	0	0	1	0	0	1	1	20	W	1	1
	%	0%	0%	3%	14%	0%	3%	1%	0%	2%	0%	0%	0%	1%	3%	0%	1%	0%	096	6 0)	K .	96	3%	0%	0%	0%	0%	0%	0%	0	16 () %	0%	3%	096	0%	2%	1%	2%		100%	
	Articulated Trucks	0	0	6	0	0	6	3	0	0	0	0	0	0	0	0	3	0	0	0)	3	6	0	0	0	0	0	0	()	0	0	0	0	0	0	0	9		1	1
	%	0%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	6 09	6	%	1%	0%	0%	0%	0%	0%	0%	0	6 (0%	0%	0%	0%	0%	0%	0%	1%			
	Bicycles on Road	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	0	0	()	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	0%	6	196	0%	0%	0%	0%	0%	0%	0%	0	6 (0%	0%	0%	0%	0%	0%	ο×	0%			
	Total	6	0 ,	412	7	0	425	460	11	43	2	26	0	82	75	34	441		2	0	100		455	0	2	2	0	0	4	-	- FN	0	17	32	6	0	55	83	1077			
	PHF	0.75	0	0.87	0.44	0	0.87	0,96	0,69	0.83	0,5	0.81	0	0.89	0.78	0.77	0.95	0.7	1 0.5	5 0	0	95	0.87	0	0.25	0.5	0	0	0.5	0.	5	0 (0.71	0.89	0.5	0	0.92	0.83	0.93			
	Approach %						39%	43%						8%	7%						4	7%	42%						0%	0	6						5%	8%				
						0.37						136					II II	THE TA	100			DO.		10.5	LUM		11.70					IL TUY		74.0				HIMIT				

Study Name Milton - Route 138 and Robbins Street TM20 TMC
Start Date Thursday, March 29, 2018 6:00 AM
Thursday, March 29, 2018 6:00 PM
Site Code

Time Period Peak 1 Specified Period 6:00 AM - 9:00 AM	Class. Motorcycles	R	-													North							bound					Crossw	KIIIN
Specified Period	Motorcycles			L	U	T I	0	R	J	L	U	1	0	R	T	La	U		0	R	ī	L	U		0	Total		Pedestrians	THE RESERVE OF THE PARTY OF
		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	0	0
6:00 AM - 9:00 AM	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	. 0%	0%	0%	0%	0%		0%	
	Cars	7	415	0	0	422	426	12	26	25	0	63	23	10	404	3	1	418	444	3	13	10	0	26	36	929	E	0	0
One Hour Peak	%	100%	88%	0%	0%	89%	83%	80%	90%	81%	0%	84%	88%	83%	83%	75%	100%	83%	88%	100%	93%	100%	0%	96%	90%	86%		0%	
7:30 AM - 8:30 AM L	Light Goods Vehicles	0	22	0	0	22	44	1	3	5	0	9	2	1	43	1	0	45	27	0	1	0	0	1	4	77	S	0	0
	%	0%	5%	0%	0%	5%	9%	7%	10%	16%	0%	12%	8%	8%	9%	25%	0%	9%	5%	0%	7%	0%	0%	4%	10%	7%		0%	
	Buses	0	6	0	0	6	4	0	0	0	0	0	. 1	1	4	0	0	5	6	0	0	0	0	0	0	11	W	0	0
	%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	4%	8%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	1%		0%	
	Single-Unit Trucks	0	12	0	0	12	32	2	0	1	0	3	0	0	30	0	0	30	13	0	0	0	0	0	0	45		0	0
	%	0%	3%	0%	0%	3%	6%	13%	0%	3%	0%	4%	0%	0%	6%	0%	0%	6%	3%	0%	0%	0%	0%	0%	0%	4%			
	Articulated Trucks	0	14	0	0	14	4	0	0	0	0	0	0	0	4	0	0	4	14	0	0	0	0	0	0	18			
	%	0%	3%	0%	0%	3%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	3%	0%	0%	0%	0%	0%	0%	2%			
	Bicycles on Road	0	0	0	0	0	1	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	1			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	7	469	0	0	476	511	15	29	31	0	75	26	12	486	4	1	503	504	3	14	10	0	27	40	1081			
	PHF	0.44	0.95	0	0	0.96	0.9	0.62	0.72	0.78	0	0.82	0.54	0.5	0.91	0.5	0.25	0.9	0.94	0.38	0.58	0.5	0	0.56	0.91	0.92			
	Approach %					44%	47%					7%	2%					47%	47%					2%	4%				
Peak 2	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	0	0
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
3:00 PM - 6:00 PM	Cars	9	328	3	0	340	418	4	31	6	0	41	50	16	409	0	0	425	337	3	31	5	0	39	40	845	E	1	1
One Hour Peak	%	100%	81%	60%	0%	81%	90%	80%	86%	86%	0%	85%	88%	89%	90%	0%	0%	90%	81%	100%	91%	100%	0%	93%	89%	86%		100%	
3:00 PM - 4:00 PM Li	ight Goods Vehicles	0	56	2	0	58	31	1	4	1	0	6	6	2	30	0	0	32	57	0	2	0	0	2	4	98	S	0	0
	%	0%	14%	40%	0%	14%	7%	20%	11%	14%	0%	13%	11%	11%	7%	0%	0%	7%	14%	0%	6%	0%	0%	5%	9%	10%		0%	
	Buses	0	5	0	0	5	7	0	0	0	0	0	0	0	7	0	0	7	5	0	0	0	0	0	0	12	W	2	2
	%	0%	1%	0%	0%	1%	2%	0%	0%	0%	0%	0%	0%	0%	2%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	1%		100%	
	Single-Unit Trucks	0	10	0	0	10	4	0	1	0	0	1	1	0	4	0	0	4	10	0	1	0	0	1	1	16		3	3
	%	0%	2%	0%	0%	2%	1%	0%	3%	0%	0%	2%	2%	0%	1%	0%	0%	1%	2%	0%	3%	0%	0%	2%	2%	2%			
	Articulated Trucks	0	7	0	0	7	3	0	0	0	0	0	0	0	3	0	0	3	7	0	0	0	0	0	0	10			
	%	0%	2%	0%	0%	2%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	2%	0%	0%	0%	0%	0%	0%	1%			
	Bicycles on Road	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	9	406	5	0	420	463	5	36	7	0	48	57	18	453	0	0	471	416	3	34	5	0	42	45	981			
	PHF	0.56	0.85	0.42	0	0.88	0.96	0.42	0.9	0.44	0	0.86	0.71	0.75	0.95	0	0	0.95	0.87	0.38	0.77	0.31	0	0.88	0.87	0.93			
	Approach %					43%	47%					5%	6%					48%	42%					4%	5%				
						Sand.																							

Study Name Milton - Route 138 at Blue HIII Terrace Street and Cheever Street TM21 TMC
Start Date Thursday, March 29, 2018 6:00 AM
Thursday, March 29, 2018 6:00 PM
Site Code

	HO SEAMS			South	bound					West	bound				de l'esp	North	bound			1 114		East	oound					Crosswa	alk
Time Period	Class.	R	Т		U		0	R	T	L	U		0	R	T	1	U		0	R	T		U		0	Total		Pedestrians	Total
Peak 1	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	1	1
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		100%	
6:00 AM - 9:00 AM	Cars	7	356	10	0	373	453	14	33	97	0	144	70	46	437	1	0	484	456	3	14	2	0	19	41	1020	E	4	4
One Hour Peak	%	100%	87%	100%	0%	88%	83%	100%	94%	94%	0%	95%	91%	94%	83%	100%	0%	84%	89%	100%	78%	100%	0%	83%	95%	87%		100%	
7:15 AM - 8:15 AM	Light Goods Vehicles	0	23	0	0	23	.53	0	2	4	0	6	7	3	53	0	0	56	27	0	4	0	0	4	2	89	S	1	1
	%	0%	6%	0%	0%	5%	10%	0%	6%	4%	0%	4%	9%	6%	10%	0%	0%	10%	5%	0%	22%	0%	0%	17%	5%	8%		100%	
	Buses	0	4	0	0	4	4	0	0	2	0	2	0	0	4	0	0	4	6	0	0	0	0	0	0	10	W	0	0
	%	0%	1%	0%	0%	1%	1%	0%	0%	2%	0%	1%	0%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	1%		0%	
	Single-Unit Trucks	0	9	0	0	9	29	0	0	0	0	0	0	0	29	0	0	29	9	0	0	0	0	0	0	38		6	6
	%	0%	2%	0%	0%	2%	5%	0%	0%	0%	0%	0%	0%	0%	5%	0%	0%	5%	2%	0%	0%	0%	0%	0%	0%	3%			
	Articulated Trucks	0	15	0	0	15	5	0	0	0	0	0	0	0	5	0	0	5	15	0	0	0	0	0	0 .	- 20			
	%	0%	4%	0%	0%	4%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	3%	0%	0%	0%	0%	0%	0%	2%			
	Bicycles on Road	0	0	0	0	0	1	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	1			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	7	407	10	0	424	545	14	35	103	0	152	77	49	529	1	0	579	513	3	18	2	0	23	43	1178			
	PHF	0.35	0.86	0.62	0	0.86	0.86	0.44	0.62	0.76	0	0.75	0.62	0.68	0.86	0.25	0	0.84	0.84	0.38	0.41	0.5	0	0.48	0.67	0.82			
	Approach %					36%	46%					13%	7%					49%	44%					2%	4%				
Peak 2	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	6	6
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		100%	
3:00 PM - 6:00 PM	Cars	6	330	16	1	353	414	14	8	31	0	53	58	37	397	3	0	437	363	2	5	2	0	9	17	852	E	5	5
One Hour Peak	%	86%	79%	94%	100%	80%	89%	88%	89%	91%	0%	90%	84%	80%	89%	100%	0%	88%	80%	67%	83%	67%	0%	75%	89%	85%		100%	
3:00 PM - 4:00 PM	Light Goods Vehicles	1	61	1	0	63	36	1	1	0	0	2	9	7	34	0	0	41	61	0	1	1	0	2	2	108	S	1	1
	%	14%	15%	6%	0%	14%	8%	6%	11%	0%	0%	3%	13%	15%	8%	0%	0%	8%	13%	0%	17%	33%	0%	17%	11%	11%		100%	
	Buses	0	6	0	0	6	7	0	0	2	0	2	2	2	7	0	0	9	8	0	0	0	0	0	0	17	W	3	3
	%	0%	1%	0%	0%	1%	2%	0%	0%	6%	0%	3%	3%	4%	2%	0%	0%	2%	2%	0%	0%	0%	0%	0%	0%	2%		100%	
	Single-Unit Trucks	0	15	0	0	15	4	0	0	1	0	1	0	0	4	0	0	4	17	1	0	0	0	1	0	21		15	15
	%	0%	4%	0%	0%	3%	1%	0%	0%	3%	0%	2%	0%	0%	1%	0%	0%	1%	4%	33%	0%	0%	0%	8%	0%	2%			
	Articulated Trucks	0	6	0	0	6	4	1	0	0	0	1	0	0	3	0	0	3	6	0	0	0	0	0	0	10			
	%	0%	1%	0%	0%	1%	1%	6%	0%	0%	0%	2%	0%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	1%			
	Bicycles on Road	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	7	418	17	1	443	465	16	9	34	0	59	69	46	445	3	0	494	455	3	6	3	0	12	19	1008			
	PHF	0.58	0.88	0.61	0.25	0.88	0.91	0.67	0.75	0.71	0	0.74	0.64	0.64	0.93	0.38	. 0	0.91	0.86	0.38	0.5	0.38	0	0.6	0.79	0.94			
	Approach %					44%	46%	1200				6%	7%					49%	45%					1%	2%				
		233	727					146		THE ST		Legal)											O ALCOHOL						

Study Name Milton - Route 138 at Aberdeen Road and Concord Baptist Church TM22 TMC

Start Date Thursday, March 29, 2018 6:00 AM

Sunday, April 01, 2018 12:00 PM

Site Code

				South	bound					West	bound	100				North	bound					East	bound			Various I		Crosswa	alk
Time Period	Class.	R	T	L	U		0	R	T	L	U		0	R	T	L	U	- 1	0	R	T	L	U		0	Total		Pedestrians	Total
Peak 1	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	3	3
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		100%	
6:00 AM - 9:00 AM	Cars	32	361	1	0	394	460	0	0	0	0	0	3	2	451	2	1	456	367	5	0	9	0	14	34	864	E	3	3
One Hour Peak	%	94%	88%	100%	0%	88%	82%	0%	0%	0%	0%	0%	100%	100%	82%	100%	100%	82%	88%	100%	0%	100%	0%	100%	94%	85%		100%	
7:15 AM - 8:15 AM	Light Goods Vehicles	2	21	0	0	23	60	0	0	0	0	0	0	0	60	0	0	60	21	0	0	0	0	0	2	83	S	2	2
	%	6%	5%	0%	0%	5%	11%	0%	0%	0%	0%	0%	0%	0%	11%	0%	0%	11%	5%	0%	0%	0%	0%	0%	6%	8%		100%	
	Buses	0	4	0	1	5	5	0	0	0	0	0	0	0	4	0	0	4	4	0	0	0	0	. 0	0	9	W	2	2
	%	0%	1%	0%	100%	1%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	1%		100%	
	Single-Unit Trucks	0	10	0	0	10	28	0	0	0	0	0	0	0	28	0	0	28	10	0	0	0	0	0	0	38		10	10
	%	0%	2%	0%	0%	2%	5%	0%	0%	0%	0%	0%	0%	0%	5%	0%	0%	5%	2%	0%	0%	0%	0%	0%	0%	4%			
	Articulated Trucks	0	14	0	0	14	4	0	0	0	0	0	0	0	4	0	0	4	14	0	0	0	0	0	0	18			
	%	0%	3%	0%	0%	3%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	3%	0%	0%	0%	0%	0%	0%	2%			
	Bicycles on Road	0	0	0	0	0	1	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	1			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	34	410	1	1	446	558	0	0	0	0	0	3	2	548	2	1	553	416	5	0	9	0	14	36	1013			
	PHF	0.65	0.91	0.25	0.25	0.88	0.87	0	0	0	0	0	0.38	0.5	0.86	0.5	0.25	0.86	0.9	0.62	0	0.45	0	0.58	0.69	0.88			
	Approach %					44%	55%					0%	0%					55%	41%					1%	4%				
Peak 2	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	3	3
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		100%	
3:00 PM - 6:00 PM	Cars	14	359	4	0	377	420	1	0	1	0	2	5	1	411	4	0	416	362	2	0	8	0	10	18	805	E	11 .	11
One Hour Peak	%	93%	81%	80%	0%	81%	89%	100%	0%	100%	0%	100%	83%	100%	89%	100%	0%	89%	81%	67%	0%	89%	0%	77%	90%	85%		100%	
3:00 PM - 4:00 PM	Light Goods Vehicles	1	60	1	0	62	34	0	0	0	0	0	1	0	34	0	0	34	61	1	0	0	1	2	2	98	S	3	3
	%	7%	14%	20%	0%	13%	7%	0%	0%	0%	0%	0%	17%	0%	7%	0%	0%	7%	14%	33%	0%	0%	100%	15%	10%	10%		100%	
	Buses	0	6	0	0	6	- 7	0	0	0	0	0	0	0	7	0	0	7	6	0	0	0	0	0	0	13	W	5	5
	%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	0%	2%	0%	0%	2%	1%	0%	0%	0%	0%	0%	0%	1%		100%	
	Single-Unit Trucks	0	12	0	0	12	5	0	0	0	0	0	0	0	4	0	0	4	12	0	0	1	0	1	0	17		22	22
	%	0%	3%	0%	0%	3%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	3%	0%	0%	11%	0%	8%	0%	2%			
	Articulated Trucks	0	6	0	0	6	4	0	0	0	0	0	0	0	4	0	0	4	6	0	0	0	0	0	0	10			
	%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	1%			
	Bicycles on Road	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	15	443	5	0	463	470	1	0	1	0	2	6	1	460	4	0	465	447	3	0	9	1	13	20	943			
	PHF	0.62	0.89	0.62	0	0.9	0.92	0.25	0	0.25	0	0.25	0.5	0.25	0.92	0.5	0	0.92	0.89	0.75	0	0.75	0.25	0.81	0.62	0.92			
	Approach %					49%	50%					0%	1%	111-				49%	47%					1%	2%				
								Walking.													1124								AIST

Sunday

Study Name Milton - Route 138 at Aberdeen Road and Concord Baptist Church TM22 TMC

Start Date Thursday, March 29, 2018 6:00 AM End Date Sunday, April 01, 2018 12:00 PM

Site Code

And the second section	deministration of the		THE W	South	bound			B/AUS		West	bound			0.0	MANAGE &	North	bound					Eastl	ound	attended	100			Crossw	walk
Time Period	Class.	R	T	L	U		0	R	T	L	U	- 1	0	R	Ţ	L	U	1.	0	R	T	L	U		0	Total		Pedestrian	ıs To
Peak 1	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	16	1
Specified Period	- %	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	10.1	100%	
3:00 AM - 12:00 PM	Cars	28	305	66	4	403	365	0	0	0	0	0	80	14	357	7	1	379	309	3	0	4	0	7	35	789	E	91	
One Hour Peak	%	97%	95%	100%	100%	96%	94%	0%	0%	0%	0%	0%	100%	100%	94%	100%	100%	94%	95%	100%	0%	100%	0%	100%	97%	95%		100%	
9:15 AM - 10:15 AM	Light Goods Vehicles	1	10	0	0	11	20	0	0	0	0	0	0	0	20	0	0	20	10	0	0	0	0	0	1	31	S	54	
	%	3%	3%	0%	0%	3%	5%	0%	0%	0%	0%	0%	0%	0%	5%	0%	0%	5%	3%	0%	0%	0%	0%	0%	3%	4%		100%	
	Buses	0	1	0	0	1	1	0	0	0	0	0	0	0	1	0	0	1	1	0	0	0	0	0	0	2	W	6	
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		100%	
	Single-Unit Trucks	0	4	0	0	4	2	0	0	0	0	0	0	0	2	0	0	2	4	0	0	0	0	0	0	6		167	
	%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	1%			
	Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Bicycles on Road	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	29	320	66	4	419	388	0	0	0	0	0	80	14	380	7	1	402	324	3	0	4	0	7	36	828			
	PHF	0.66	0.89	0.49	0.5	0.88	0.77	0	0	0	0	0	0.47	0.39	0.77	0.58	0.25	0.8	0.89	0.38	0	0.33	0	0.44	0.64	0.93			
	Approach %					51%	47%					0%	10%					49%	39%					1%	4%				

Study Name Milton - Route 138 and Oak Street TM23 TMC
Start Date Thursday, March 29, 2018 6:00 AM
Thursday, March 29, 2018 6:00 PM
Site Code

		per less		South	nbound					West	bound					North	bound	拉拉是现		riner)		Eastl	oound					Crossw	valk
Time Period	Class.	R	Ť		U		0	R	Т	L	U		0	R	T	L	U		0	R	T	L	U		0	Total		Pedestrians	s Tota
Peak 1	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	7	7
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		100%	
6:00 AM - 9:00 AM	Cars	1	337	8	0	346	446	7	6	3	0	16	46	32	437	2	0	471	342	2	6	2	0	10	9	843	E	16	16
One Hour Peak	%	100%	85%	80%	0%	85%	82%	100%	100%	75%	0%	94%	94%	97%	82%	67%	0%	82%	85%	100%	100%	100%	0%	100%	90%	84%		100%	
7:30 AM - 8:30 AM	Light Goods Vehicles	0	25	2	0	27	61	0	0	0	0	0	3	1	61	1	0	63	25	0	0	0	0	0	1	90	S	0	0
	%	0%	6%	20%	0%	7%	11%	0%	0%	0%	0%	0%	6%	3%	11%	33%	0%	11%	6%	0%	0%	0%	0%	0%	10%	9%		0%	
	Buses	0	6	0	0	6	4	0	0	0	0	0	0	0	4	0	0	4	6	0	0	0	0	0	0	10	W	1	1
	%	0%	2%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	1%		100%	
	Single-Unit Trucks	0	14	0	0	14	29	0	0	1	0	1	0	0	29	0	0	29	15	0	0	0	0	0	0	44		24	24
	%	0%	4%	0%	0%	3%	5%	0%	0%	25%	0%	6%	0%	0%	5%	0%	0%	5%	4%	0%	0%	0%	0%	0%	0%	4%			
	Articulated Trucks	0	13	0	0	13	4	0	0	0	0	0	0	0	4	0	0	4	13	0	0	0	0	0	0	17			
	%	0%	3%	0%	0%	3%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	3%	0%	0%	0%	0%	0%	0%	2%			
	Bicycles on Road	0	0	0	0	0	1	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	1			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	1	395	10	0	406	545	7	6	4	0	17	49	33	536	3	0	572	401	2	6	2	0	10	10	1005			
	PHF	0.25	0.94	0.62	0	0.93	0.84	0.58	0.38	0.5	0	0.47	0.64	0.59	0.84	0.38	0	0.85	0.94	0.5	0.38	0.5	0	0.5	0.5	0.87			
	Approach %					40%	54%					2%	5%					57%	40%					1%	1%				
Peak 2	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	66	66
Specified Period .	%	0%	0%	0%	0%	0%	0%.	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		100%	
3:00 PM - 6:00 PM	Cars	1	361	7	1	370	424	5	1	3	0	9	19	9	415	10	0	434	372	8	3	3	0	14	12	827	Ε	66	66
One Hour Peak	%	100%	82%	100%	100%	83%	90%	100%	100%	100%	0%	100%	100%	100%	90%	91%	0%	90%	82%	73%	100%	100%	0%	82%	92%	86%		100%	
3:00 PM - 4:00 PM	Light Goods Vehicles	0	56	0	0	56	31	0	0	0	0	0	0	0	31	1	0	32	58	2	0	0	0	2	1	90	S	0	0
	%	0%	13%	0%	0%	13%	7%	0%	0%	0%	0%	0%	0%	0%	7%	9%	0%	7%	13%	18%	0%	0%	0%	12%	8%	9%		0%	
	Buses	0	3	0	0	3	7	0	0	0	0	0	0	0	7	0	0	7	4	1	0	0	0	1	0	11	W	23	23
	%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	0%	2%	0%	0%	1%	1%	9%	0%	0%	0%	6%	0%	1%		100%	
	Single-Unit Trucks	0	13	0	0	13	5	0	0	0	0	0	0	0	5	0	0	5	13	0	0	0	0	0	0	18		155	155
	%	0%	3%	0%	0%	3%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	3%	0%	0%	0%	0%	0%	0%	2%			
	Articulated Trucks	0	6	0	0	6	4	0	0	0	0	0	0	0	4	0	0	4	6	0	0	0	0	0	0	10			
	%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	1%			
	Bicycles on Road	0	0	0	0	0	1	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	1			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	1	439	7	1	448	472	5	1	3	0	9	19	9	463	11	0	483	453	11	3	3	0	17	13	957			
	PHF	0.25	0.81	0.44	0.25	0.8	0.89	0.62	0.25	0.25	0	0.38	0.79	0.56	0.89	0.69	0	0.89	0.83	0.55	0.75	0.75	0	0.71	0.81	0.97			
	Approach %					47%	49%					1%	2%	The same				50%	47%					2%	1%				
														-739					1181										

Start Date
Start Date
End Date
Site Code

Milton - Route 138 and Brook Road TM24 TMC
Thursday, March 29, 2018 6:00 AM
Thursday, March 29, 2018 6:00 PM

				South	bound			West	bound	III A		North	bound	1 2				East	oound		118			Crosswa	alk
Time Period	Class.	R	T	L	U		0		0	R	T	L	U		0	R	T	L	U		0	Total		Pedestrians	Tota
Peak 1	Motorcycles	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	0	0	1	0	1	N	1	1
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	0%	0%		100%	
6:00 AM - 9:00 AM	Cars	17	345	9	0	371	407	0	219	40	402	4	0	446	349	4	170	5	0	179	21	996	E	7	7
One Hour Peak	%	89%	85%	100%	0%	86%	81%	0%	91%	85%	81%	80%	0%	82%	86%	100%	92%	63%	0%	91%	88%	85%		100%	
7:30 AM - 8:30 AM	Light Goods Vehicles	2	26	0	0	28	60	0	14	3	58	0	0	61	26	0	11	2	0	13	2	102	S	1	1
	%	11%	6%	0%	0%	6%	12%	0%	6%	6%	12%	0%	0%	11%	6%	0%	6%	25%	0%	7%	8%	9%		100%	
	Buses	0	6	0	0	6	3	0	0	0	3	0	0	3	6	0	0	0	0	0	0	9	W	1	1
	%	0%	1%	0%	0%	1%	1%	0%	0%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	1%		100%	
	Single-Unit Trucks	0	14	0	0	14	28	0	5	4	27	1	0	32	14	0	1	1	0	2	1	48		10	10
	%	0%	3%	0%	0%	3%	6%	0%	2%	9%	5%	20%	0%	6%	3%	0%	1%	13%	0%	1%	4%	4%			
	Articulated Trucks	0	13	0	0	13	4	0	1	0	4	0	0	4	13	0	1	0	0	1	0	18			
	%	0%	3%	0%	0%	3%	1%	0%	0%	0%	1%	0%	0%	1%	3%	0%	1%	0%	0%	1%	0%	2%			
	Bicycles on Road	0	0	0	0	0	1	0	1	0	1	0	0	1	0	0	1	0	0	1	0	2			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	0%	0%			
	`Total	19	404	9	0	432	503	0	241	47	495	5	0	547	408	4	185	8	0	197	24	1176			
	PHF	0.59	0.93	0.45	0	0.91	0.91	0	0.84	0.65	0.9	0.42	0	0.88	0.92	0.5	0.94	0.67	0	0.93	0.75	0.9			
	Approach %					37%	43%	0%	20%					47%	35%					17%	2%				
Peak 2	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	2	2
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		100%	
3:00 PM - 6:00 PM	Cars	33	379	14	0	426	378	0	232	39	373	6	0	418	383	4	179	5	0	188	39	1032	E	2	2
One Hour Peak	%	92%	87%	100%	0%	88%	94%	0%	91%	87%	94%	100%	0%	94%	87%	80%	92%	100%	0%	92%	93%	91%		100%	
4:45 PM - 5:45 PM	Light Goods Vehicles	3	38	0	0	41	19	0	16	5	19	0	0	24	39	1	11	0	0	12	3	77	S	4	4
	%	8%	9%	0%	0%	8%	5%	0%	6%	11%	5%	0%	0%	5%	9%	20%	6%	0%	0%	6%	7%	7%		100%	
	Buses	0	7	0	0	7	2	0	1	1	2	0	0	3	7	0	0	0	0	0	0	10	W	3	3
	%	0%	2%	0%	0%	1%	0%	0%	0%	2%	1%	0%	0%	1%	2%	0%	0%	0%	0%	0%	0%	1%		100%	
	Single-Unit Trucks	0	8	0	0	8	1	0	4	0	1	0	0	1	8	0	4	0	0	4	0	13		11	11
	%	0%	2%	0%	0%	2%	0%	0%	2%	0%	0%	0%	0%	0%	2%	0%	2%	0%	0%	2%	0%	1%			
	Articulated Trucks	0	2	0	0	2	1	0	0	0	1	0	0	1	2	0	0	0	0	0	0	3			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Bicycles on Road	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	0	0	1	0	1			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%			
	Total	36	434	14	0	484	401	0	254	45	396	6	0	447	439	5	195	5	0	205	42	1136			
	PHF	0.75	0.94	0.58	0	0.93	0.93	0	0.93	0.75	0.92	0.5	0	0.9	0.94	0.42	0.9	0.42	0	0.88	0.7	0.96			
	Approach %					43%	35%	0%	22%					39%	39%					18%	4%				
																							11		

Part 2: Automatic Traffic Recorder (ATR) Data

MassDOT Highway Division WEEKLY SUMMARY FOR LANE 1 Starting: 3/27/2018

STA.INB

Site Reference: 180060000594 Site ID: 000000000101

Location: RTE.138 NO.OF PARK AND RIDE LOT

Direction: NORTH

File: SPD6687_NB.prn

Page: 1

City: CANTON County: SPEED NB

TIME	MON 2	27	28	29	FRI 30	AVG 🔻	SAT 31	1	WEEK AVG	
	231					220			259	
	124		114	110	148	124	162	269		927
03:00	85 98 245 713		63	71	79	74	140	219	109	657
04:00	98		72	75	92	84 254 629	128	61	87	526
05:00	245		269	269	235	254	114	126	209	1258
06:00	713		670	618	518			173	485	2915
	815		940	772			404	320	640	3843
08:00	860		1058	760	602	820	478	388	691 694 644	4146
09:00	759 715 620		1039	650	617	766 670	611 602	493	694	4169
10:00	715		771	620	576	670	602	585	593	3869
11:00	620	666	597	565	581		577			
12:00	495	569	515	563		547	616	505		
13:00	510	581	524	645	746	601	788	90B	628	4402
14:00	620 495 510 581 588	577 652	524 523 592	605	802	617 672	984	817 806	698	4889
15:00	288	652	592	594	936	672	894	806	723	5062
	667					738			752	5270
		659	503	620		649		520	649	4547
18:00	418	608	546	534	890	599	750	503	607 694 603	4249
19:00	446 468	764	523 537	640 514	920	658	845 612	722	694 603	4860
20:00	468	552	537	514	763	566	612	775	603	4221
21:00	528	555	568	579	682	582	586	679	596	4177
22:00	497	523	540	504	664	545	631	592	564	3951
23:00	353 250	480	456	432	553	454	562	512	478	3348
24:00	250	316	339	373	410	337	517	391	370	2596
TOTALS										
% AVG WKDY % AVG WEEK	92.1	65.5	100.1	95.1	112.2		104.2	93.2		
% AVG WEEK	93	66.1	101	96	113.2		105.1	94.1		
AM Times	08:00	11:00	08:00			08:00				
AM Peaks	860	666	1058	772	617	820	616	585	694	
PM Times	16:00					16:00				
PM Peaks	667	764	658	645	981	738	984	817	752	

42 AWD 27291 FAC ,97 (,95) ADT 25,100

MassDOT Highway Division WEEKLY SUMMARY FOR LANE 1 Starting: 3/27/2018

Page: 1

STA. 1 SB

Site Reference: 180060000153 Site ID: 00000000102 Location: RTE.138 NO.OF PARK AND RIDE LOT

Direction: SOUTH

File: SPD6687_SB.prn City: CANTON County: SPEED SB

TIME	MON 2	TUE 27	WED 28	THU 29	FRI 30	WKDAY AVG		SUN 1	WEEK AVG	TOTAL
500mm										
	241		160			207				
02:00	120		85	85	122	103	208	258		878
03:00	106		53	50	96 76	76 77	120	214	106	639
04:00	101		68	66	76	125	113	161	97	585
	150		118	132	143		110	98	125	751
	382	27		314	306	337				
07:00	557		598	565	494	553	338	224	462	2776
08:00	719		917	822	740	799	556	359	685	4113
09:00	779		830	874	833	829		460	741	4448
	663	455	754	681	791	722		584		4292
11:00	653	675		705	770	704		695	717	
12:00	653	715	753	699	759	715	828	724 783 777	733	5131
13:00	670	693	742	790	920	763	960	783	794	5558
14:00	747	770	790	798	971	815	964	777	831	5817
			899	913	1016	901	937	780		6225
16:00	814	1014	1005	1074		996	987	749	959	6717
17:00	772	970	946	1043	1101	966	786	741 769	908	6359
18:00	772	1009	1033	1065	1073	990	776	769	928	6497
19:00			1112			989				6512
	708	881	909	1004	892		758	772	B46	5924
21:00	596	726 618	747 642	725 622	730	704	680	738	706	4942
22:00	534	618	642	622	674	618	601	738 642 495	619	4333
23:00	433	437	429		609	471	581	. 495	490	3435
24:00	329	299	311	390	437	353	518	360	377	2644
	13025								14321	
% AVG WKDY	88.5	73.4	101.8	102.6	107.7		97.8	85.7		
% AVG WEEK	90.9	75.3	104.5	105.3	110.5		100.4	88		
	09:00	12:00	08:00	09:00	09:00			12:00	09:00	
AM Peaks	779	715	917	874	833	829	828	724	741	
PM Times	16:00	19:00	19:00			16:00				
PM Peaks	814	1072	1112	1074	1101	996	987	783	959	

5TA, 2

Site Reference: 180060000766 Site ID: 000000000201

Location: RTE.138 SO. OF BRUSH HILL RD.

Direction: ROAD TOTAL

File: V2.prn City: MILTON TOTAL

County: VOLUME NB&SB

								175		
TIME		TUE			FRI	WKDAY AVG	SAT			TOTAL
<u> </u>	2	27	28	29 :	30	AVG	31	1	AVG	
01:00	338		279	350	409	344	534	623	422	2533
02:00	158	225	151	167	247	180	344	414	246	1481
03:00	143		118	107			254			1084
04:00	160		134	142	132		226	212	167	1006
05:00	322		344 1234	366	335	341	234 416	149	291 900	1750
06:00	322 1302 1971		1234	1272	1000	1202	416	180	900	5404
07:00	1971		2016	2036	1755		788			8932
08:00	2498		2618	2527		2414	1136	577	1895	11372
09:00	2697		2710	2599	2142			825	2085	12511
10:00	2150 1950		2033	2073 1861	1960	2054	1826		1885	11315
11:00	1950		1857	1861	1904	1893	1906		1869	11215
12:00		1629	1654	1824	1938	1750		1785	1804	
13:00				1956					2005	
14:00	1800		1938	1959	2489	2012	2463		2126	14886
15:00	2112	2188		2179	2513	2235	2533	2255	2280	15966
16:00	2575	2673	2446	2604	2650	2589	2450	2172	2510	17570
	2321	2610	2436	2589	2577	2506			2378	16649
18:00	2193		2449		2737	2557	1959		2397	16785
19:00	1853	2311	2461	2479	2329	2286	2027	2173		15633
20:00	1526 1296 1076	1663	1836	1997	1787	1761 1429	1768	2398	1853	12975
21:00	1296	1399	1477	1479	1495	1429	1571	1980	1528	10697
22:00	1076	1147		1260	1432	1234	1325	1396	1270	8894
23:00	793	844	906	918	1192	930	1213	924	970	6790
24:00	560	538	585	716	839	647	959	652	692	4849
TOTALS						37037				236964
% AVG WKDY	95.2	62.9	99.9	102.9	103.5		92.1	83.1		
% AVG WEEK	99.4	65.7	104.3	107.4	108		96.2	86.7		
	09:00			09:00		09:00				
AM Peaks	2697	1629	2710	2599	2142	2537	2093	1785	2085	
PM Times	16:00					16:00				
PM Peaks	2575	2745	2461	2661	2/3/	2589	2533	2398	2510	

AWD 37037 FAC .97 (.95) ADT 34,100

MassDOT Highway Division WEEKLY SUMMARY FOR LANE 1 Starting: 3/27/2018

STA. 2 NB

Site Reference: 180060000766 Site ID: 000000000201 Location: RTE.138 SO. OF BRUSH HILL RD. Direction: NORTH

File: V2.prn City: MILTON County: VOLUME NB&SB

Page: 1

TIME	MON 2	TUE 27	WED 28	THU 29	FRI 30	WKDAY AVG	SAT 31	SUN 1	WEEK AVG	TOTAL
		: 5								
01:00	139		120	144	169	143	223	288	180	= 1083
02:00	64		66	68	131	82	135	198	110	662
03:00	70		57	45	61	58	131	129	82	493
04:00		1.6	65	75	59	70	115	103	83	499
05:00	198		208	210	190	201	127		169	1011
06:00	964		829	870	639	800	214		584	3508
07:00	1257	4	1207	1251	1088	1200	387	184	895	5374
08:00	1304	*	1352	1287	965	1227	499	260	944	5667
09:00	1128		1223	1230	985	1141	690	376	938	5632
10:00	1027		1069	1052	890	1009	739	575	892	5352
11:00	836 742		868	866	766	834	768	805	818	4909
12:00	742	683	652	775	808	732	800	776	748	5236
13:00	751	735	767	702	902	771	855	823	790	5535
14:00	715	733	763	770	881	772		948	816	5717
15:00	750	778	738	784	876	785	933	886	820	5745
16:00	813	837	839	923	905	863	941	838	870	6096
17:00	813 800	788	805	847	871	822	889	758	822	5758
18:00	789	857	788	897	977	861	843	898	864	6049
19:00	693	761	724	815	940	786	887	1018	834	5838
20:00	565	587	637	666	672	625	718	1144	712	4989
21:00	530	531	553	564	622	560	697	958	636	4455
22:00	461	439	514	533	610	511	588	1144 958 669	544	3814
23:00	328	363	420	398	515	404	542	425	427	2991
24:00	224	217	248	288	354	266	427	303	294	2061
TOTALS	15130	8309	15512	16060	15876	15523	14055	13532	14871	98474
% AVG WKDY	97.4	53.5	99.9	103.4	102.2		90.5	87.1		
% AVG WEEK	101.7	55.8	104.3	107.9	106.7		94.5	90.9		
AM Times	08:00	12:00	08:00	08:00	07:00	08:00	12:00	11:00	08:00	
AM Peaks		683	1352	1287	1088	1227	800	805	944	
PM Times	16:00	18:00	16:00	16:00	18:00	16:00	16:00	20:00	16:00	
PM Peaks	813		839	923	977	863	941		870	
									4	

MassDOT Highway Division WEEKLY SUMMARY FOR LANE 2 Starting: 3/27/2018

STA . 2 SB

Site Reference: 180060000766 Site ID: 000000000201 Location: RTE.138 SO. OF BRUSH HILL RD. Direction: SOUTH

File: V2.prn City: MILTON County: VOLUME NB&SB

Page: 2

			<u> </u>							
TIME	MON 2	TUE 27	WED 28	THU 29	FRI 30	WKDAY AVG		SUN 1	WEEK AVG	TOTAI
01:00	199		159	206	240	201	311	335	241	1450
02:00	94		85	- 99	116	98	209			819
03:00	94 73		61	62	101	98 74	123	171	136 98	819 591
04:00	78		69	67	73	71	111	109	84	507
05:00	124		136	156	145	140	107	71		739
06:00	438		405	402	361	401	202	88	316	1896
07:00	714		809	785	667	743	401	182	593	3558
08:00	1194		1266	1240	1051	1187	202 401 637	317	950	5705
09:00	1569		1487	1369	1157		848	449	1146	
10:00	1123		964	1021	1070	1044	1087		993	5963
	1114		989	995	1138	1059	1138	932	1051	6306
12:00	963	946	1002	1049	1130	1018	1293	1009	1056	7392
13:00	963 1015	949	1115	1254	1403	1147	1515	1253	1214	8504
14:00	1085	1143	1175		1608	1240	1556	1413		9169
15:00	1362	1410	1448		1637	1450		1369	1460	10221
		1836	1607	1681	1745	1726		1334	1639	11474
17.00	1521	1822	1631	1742	1706	1684		1159		10891
18:00	1404	1888		1764	1760	1695	1116	1143	1533	
	1160		1737		1389	1500		1155		9795
20:00	961	1076	1199	1331	1115	1136	1050	1254	1140	7986
21:00	766	868	924	915	873	869	874	1022	891	6242
22:00	766 615 465	708	924 744	915 727	873 822	869 723	874 737	1022 727	725	5080
23:00	465	481	486	520	677	525	671	499	542	3799
24:00	336	321	337			381		349		2788
TOTALS	20135	14998		22061		21507				138490
% AVG WKDY	93.6	69.7	99.9	102.5	104.4		93.3	80.2		
	97.7			107.1	109.1		97.4	83.7		
AM Times	03100	12:00	09:00	09:00	09:00			12:00		8.4
AM Peaks	1569	946	1487	1369	1157	1395	1293	1009	1146	
PM Times	16:00	18:00	19:00	18:00	18:00	16:00	15:00	14:00	16:00	
PM Peaks			1737						1639	

5TA.3

Site Reference: 180060000534 Site ID: 00000000303 Location: BRUSH HILL RD. WEST OF RTE.138

Direction: ROAD TOTAL

TOTAL

File: V3.prn City: MILTON County: VOLUME EB&WB

TIME	MON	TUE	WED		FRI		SAT			TOTAL
4	2	21	28	29	30	AVG	31	1	AVG	
	42			37	49	43	73	95	56	340
02:00	16		26	25	30	24	48	54	33	199
03:00	22		12	12	19	16	30 32 32	40	22	135
04:00	9		25	14	14	15	32	21	19	115
05:00	27		29	33	23	28	32	20	27	164
	113			102	105	108	42	27	84	504
07:00	232		236	233	203	226	103	50	176	1057
08:00	352		388	412	321	368	181 282	100	292	1754
09:00	377		411	406	350	386	282	100 173	333	1999
10:00	282		360	347	339	332	319	281	321	1928
11:00	301		298	344	365	327		346	339	2034
12:00	269	326	304	343	361	320	452	371	346	2426
13:00	305	345 339	346	360	426	356	466 495	466	. 387	2714
14:00	312	339	378	406	406	468	495	472	401	2808
15:00	378	415	440	415	517	433	423	440	432	3028
16:00	481	483	494	473	618	509	469	394	487	
	413	465	514	514	620	505	406	373		3305
18:00	420	452	596	461	549	495 411	359 355	386	460	3223
19.00	420 309	423	596 477	463	386		355	386 348	394	2761
20:00	291	294	327	301	320	306	326	381	320	2240
21:00	180	217				226	257	295	240	1684
22:00	153	188	188	196	215	188	245	207	198	1392
23:00	101	128	153	137	180	139	187	130	145	
24:00	69	188 128 86	84	114	126	95	138	89	100	1016 706
TOTALS		4161					6100			40944
% AVG WKDY				102.5	109.2		98	89.3		
% AVG WEEK	89.6	68.3	106.6	104.9	111.7		100.2	91.3		
AM Times	09:00	12:00	09:00	08:00	11:00	09:00	12:00	12:00	12:00	
AM Peaks	377	326	411							
PM Times	16:00	16:00	18:00	17:00	17:00	16:00	14:00	14:00	16:00	
PM Peaks	481	483	596	514	620	509	495	472	487	

46 AWD 6224 FAC , 97 (.93) ADT 5,600

MassDOT Highway Division WEEKLY SUMMARY FOR LANE 1 Starting: 3/27/2018

STA.3 EB

Site Reference: 180060000534 Site ID: 00000000303 Location: BRUSH HILL RD. WEST OF RTE.138

Direction: EAST

File: V3.prn City: MILTON County: VOLUME EB&WB

Page: 1

TIME	MON 2	TUE 27	WED 28	THU 29	FRI 30	WKDAY AVG	SAT	SUN 1	WEEK AVG	TOTAL
01:00	14		10	11	12	11	19		15	91
02:00	6		9	7	7	7	12	20	10	61
03:00	6		3	4	6	4	8		6	40
04:00	1		10	4	3	4			5	32
05:00	8		10	8	6	8		10	8	51
06:00	36		38	32	29	33			26	160
07:00	84	323	80	85	62	77	28		58	353
08:00	100		109	112	72	98	37		77	462
09:00	104		108	100	68	95	57		79	476
10:00	79		89	103	90	90	52	64	79	477
11:00	75		93	91	102	90	96	77		534
12:00	65	84	60	88	95	78	106	80	82	578
13:00	72	101	88	94	118	94	113		102	715
14:00	78	78	83	111	95	89	115	134	99	694
15:00	83	95	109	85	103	95	101		97	683
16:00	94	102	96	95	78	93	123		97	681
17:00	82	85	90	65	87	81	103		86	604
18:00	76	75	69	82	65	73	98	104	81	569
19:00	49	87	69	74	70	69	89		76	534
20:00	55	52	70	47	58		69		68	480
21:00	40		56	57	65	53	63	93	60	422
22:00	33	44	54	46	51	45	65	54	49	347
23:00	22	29	39	30	44	32	51	27	36	252
24:00	18	29	24	27	36	26	37	32	29	203
TOTALS	1280	909	1466	1458	1422	1401	1472	1492	1414	9499
% AVG WKDY	91 3	64.8	104.6	104	101.4		105	106.4		
% AVG WEEK		64.2	103.6	103.1	100.5		104.1	105.5		
AM Times	09:00	12:00	08:00	08:00	11:00	08:00	12:00	12:00	11:00	
AM Peaks	104	84	109	112	102	98	106	80	89	
					10.00	45.05	46.00		10.00	
	16:00	16:00	15:00	14:00	13:00	15:00		14:00	13:00	
PM Peaks	94	102	109	111	118	95	123	134	102	

STA.3 WB

Site Reference: 180060000534 Site ID: 00000000303 Location: BRUSH HILL RD. WEST OF RTE.138 Direction: WEST

File: V3.prn City: MILTON County: VOLUME EB&WB

TIME		27	28	29		AVG	31			
	9	8								
01:00									41	
02:00	10		17	18	23	17	36	34	23	138
03:00	16 8		9	8 10 25	. 13	11 11 20	22	27	15 13 18	95
04:00	8		15	10	11	11	22	17	13	83
05:00	19		19	25	17	20	23	10	18	113
06:00	77			70	76	75	31	13	57	344
	148		156 279	148	141	148	75	36	117	704
08:00	252		279	300	249	270	144	68 134	215	1292
09:00	213		303	306	282	291	225	134	253	
	203		271	244		241		217		
11:00	226		205	253		236				
12:00	204	242	244	255	266	242	346	291	264	
13:00	233 234	244 261	258 295	266	308	261 279	353 380	337	285	1999
14:00	234	261		295	311	279	380	338	302	2114
15:00	295	320	331	330		338		333	335	2345
16:00	387		398	378		416		301	3 9 0	2731
17:00	331	380	424	449	533	423	303	281	385	2701
18:00	344 260	377 336	527	379	484	422	261	282 252	379	2654
19:00	260	336	408	389	316	341	266	252	318	2227
20:00	236	242	257	254	262	250	257	252	251	1760
21:00	140	169	185	180	192	173	194	202	180	1262
22:00	120	144	134	150	164	142	180	153	149	1045
23:00	79	99	114	107	136	107	136	93	109	764
24:00	79 51	57	60		90	69	101	57	149 109 71	503
TOTALS	4174								4661	
% AVG WKDY % AVG WEEK	86.7 89.5	67.5 69.7	104.2 107.7	102.3 105.7	111.6 115.3		96.1 99.2	84.4 87.2		
AM Times	09:00	12:00	09:00	09:00	09:00	09:00	12:00	12:00	12:00	
AM Peaks	273	242	303	306	282	291	346	291	264	
	16:00					17:00				
PM Peaks	387	381	527	449	540	423	380	338	390	

Page: 1

Site Reference: 180060000016 Site ID: 000000000404

Location: BRUSH HILL RD. EAST OF RTE. 138

Direction: WEST

STA, 4 WB File: V4.prn City: MILTON County: VOLUME WB

TIME	MON 2			THU 29	FRI 30	WKDAY	SAT -31	SUN 1	WEEK AVG	TOTAL
	141									
01:00	17		13	19	17	16 8	29	30	20 12	125
02:00	5		7	6 2 5 11	14	8 5 3 13 68 160	17	24	12	73
	10		4	2	5	5	12	13	7	46
04:00	2		3	5	2	3	10	5	· 4	27
05:00	17 ି8େ				8	13	8	9	11	71
06:00	68		70	80	57	68	18	16	51	309
07:00	171		176		127	160	54	25	120	722
08:00	310		273	299	203	211	× 4	n n	2015	1744
09:00	315		313	247	212	271	159	77	220	1323
10:00	183		313 213 230	224	257	219	194	135	220 201 213	1206
11:00	156		230	215	254	213	228	197	213	1280
12:00	159	223			237	197	342	222	221	1551
13:00	149	174	183	239	287	206	327	326	240	1685
14:00	176 232	212 227	202	193	231	202	407 325	333 257	250	1754
15:00	232	227	300	214	337	262	325	257	270	1892
16:00	287	310	274	249		305	221	204	30 9	
17:00	243	301	340	302		317	288	213	298	2088
18:00	210 162 127	294	327	346	361	307	242	254 262 278	290	
19:00	162	230	333	318	193	247	220	262	245	1718
20:00	127	131	148	126	155	137	141	278	158	1106
21:00	94	104	103	10 9	140	110	116	177	120	843
22:00	57	100	85	76	111	85	112	99	91	640
23:00	48	42	47	46	71	50	67	57	54	378
24:00	18	41	38	41	50	85 50 37	52	32	38	272
TOTALS	3216	2389	3876	3728	4135	3709	3788	3411	3648	24543
% AVG WKDY	96 7	61 1	104 5	100 5	111 /		102 1	91.9		
% AVG WEEK								93.5		
AM Times	09:00	12:00	09:00	08:00	10:00	08:00	12:00	12:00	12:00	
AM Peaks	315	223	313			271				
PM Times	16:00	16:00	17:00	18:00	16:00	17:00	14:00	14:00	16:00	
	287		340		405	317	407	333	309	

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AWD 3709 FAC .97 (.93) ADT 3,300

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STA.5 NB

Site Reference: 180060000535

Site ID: 00000000501 Location: RTE.138 NO. OF BRUSH HILL RD.

Direction: NORTH

File: SPD501.prn City: MILTON County: SPEED NB

TIME	MON 2	TUE 27	WED 28	THU 29	FRI 30	WKDAY AVG		SUN 1	WEEK AVG	TOTAL
01:00	129		112		157	133	209			
02:00	60		63	59	99	70	131	185	99	597
03:00	65		48	42	57	53	123	121	76	456
04:00	71		55	64	54	61	104	91	73	439
05:00	171		183	184	168		101	71	146	878
06:00	699		665	678	525	641	189	89	474	2845
07:00	925		869	907	844	886	328	176	674	4049
08:00	963		954	939		897	433	256	713	4279
09:00	837		907	849	795	847	603	367	726	4358
10:00	728		798	769	728	755	636	567	704	4226
11:00	644		679	672	664	664	681	737	679	4077
12:00	590	567	567	619	700	608	721	739	643	4503
13:00	595	618	609	621	761	640	762	765	675	4731
14:00	610	614	670	652	747	658	808	839	705	4940
15:00	654	700	675	666	747	688	817	817	725	5076
16:00	682	729	713	779	742	729	826	749	745	5220
17:00	667	654	692	740	709	692	815	719	713	4996
18:00	699	687	684	729	807	721	754	800	737	5160
19:00	583		629	681	789	667	779	876	713	4991
20:00	476	507	566	538	587	534	652	941	609	4267
21:00	458	471	514	511	556	502	609	789	558	3908
22:00	403	396	477	487	558	464	529	580	490	3430
23:00	293	325	363	374	464	363	482	391		2692
24:00	189	204	228	265	332	243	390	261	267	1869
TOTALS	12191	7126	12720	12962	13324	12692	12482	12194	12496	82999
% AVG WKDY	96			102.1	104.9		98.3			
% AVG WEEK	97.5	57	101.7	103.7	106.6	.83	99.8	97.5		
AM Times	08:00	12:00	08:00	08:00		08:00			09:00	
AM Peaks	963	567	954	939	844	897	721	739	726	
PM Times			16:00			16:00				
PM Peaks	699	729	713	779	807	729	826	941	745	

42 AWD 22817 FAC .97(.95) ADT 21,000

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STA.55B

Site Reference: 180060000461

Site ID: 00000000502

Location: RTE.138 NO. OF BRUSH HILL RD. Direction: SOUTH

File: SPD502.prn City: MILTON County: SPEED SB

TIME	MON 2	TUE 27	WED 28	THU 29	FRI 30	WKDAY AVG	SAT 31	SUN 1	WEEK AVG	TOTAL
01:00	130	8	117		160		213			976
02:00	64 46		64	62	83	68 53	151	144	94	568
03:00			43	50	73	53	84	116	68	412
04:00	52		54	42					60	364
05:00	75 226		96	109		981	76	45	85	515
06:00 07:00	226 346		243	218	219	226		66	183	1102
08:00	540 542		382 542	404	377			121	310	1865
09:00	495		542 585	515 565	515	528	356	209	446	2679
10:00	432		458	515	540 516	546 480	445 590	249	479	2879
11:00	426		508	470	516 597	500	587	390 500	483	2901 3088
12:00	436	454		568	604	519	591	560	514 535	
13:00	492	482	609	613	707	580	704	681	612	3749 4288
14:00	564	549	591	604	707	602	628	732		
15:00	672	681	647	707	613	664	682	715	624 673	4373 4717
16:00	676	687	665	670	609	661	707	742	679	4717
17:00	656	637	632	624	610	631	707	665		
18:00	709	627	615	642	620	642	628		648	4541
19:00	581	681	667	607	689	645	663	642 654	640	4483 4542
20:00	455	550	650	664	631	590	596	650	648 599	4542
21:00	406	488	575	550	510	505	507	574	515	3610
22:00		415	476	464	507	442	393	717		3033
23:00	252	300	299	342	453	329			333	2331
	205	217	223	282		251	373	211	257	
24:00	203	211	223	202	330	231	336	211	251	1004
TOTALS	9287	6768	10277	10432	10836	10125	10468	9704	10080	67772
% AVG WKDY		66.8	101.5	103	107		103.3	95.8		
% AVG WEEK	92.1	67.1	101.9	103.4	107.5		103.8	96.2		
	08:00	12:00		12:00	12:00				12:00	
AM Peaks	542	454	585	568	604	546	591	560	535	
PM Times	18:00	16:00	19:00	15:00	13:00	15:00	17:00	16:00	16:00	2.5
PM Peaks	709	687	667	707	707	664	717	742	679	

STA.6

Site Reference: 180060000494 Site ID: 000000000603

TOTAL

Location: NEPONSET VALLEY PKWY WEST OF RTE. 138

File: V6.prn City: MILTON County: VOLUME EB&WB

Direction: ROAD TOTAL

	TIME	MON 2	TUE 27	WED 28	THU 29	FRI 30	WKDAY AVG		SUN 1		TOTAL
200	01:00	87		83	105	121	99	155	178	121	729
	02:00	59			42	78	58	104		.75	
	03:00	46		57			45			54	329
	04:00	60		51	63	48	55	57	61	56	340
	05:00	137		155	178	149	55 154	73	52	124	744
	06:00	492		466	487	384	457	184	68	346	2081
	07:00	626		627			621		126	481	2890
	08:00	621		634	669		611		191	499	2995
		515		616	548	496	543	402	191 215 319	465	2792
	10:00	457		500	536	490	495	472	319	462	2774
	11:00	481		495	484	535	498	506	522	503	
	12:00	395	447	434	493	528	459	568	510	482	3375
	13:00	500	467	472	510	577	505	537	612	525	3675
	14:00	494	555	472 521	527	604	540	546	650	556	
	15:00	524	582	583	575	599	572	604	586	579	4053
		549	617	608		639	597	602	563	593	4154
		534	556	625	610	620	589		485 575 570	573	4017
	18:00	563	573	571	568	595	574	513	575	565	3958
	19:00	452	573 524	539	481	578	514	52 9	570	524	3673
	20:00	364	409	402		458	416	469	570	446	
		334	323	374	348	378	351	434	492	383	2683
	22:00	272	275	322	338	404	322	352	344 206	329	2307
	23:00	212	233	231	242	312	246	352 293	206	247	1729
	24:00	148	275 233 152				322 246 187	272	161	195	1369
TC	TALS	8922	5713		9719		9508	8973	8242	9183	61163
Q.	AVG WKDY	93.8	60	101	102.2	105		94.3	86.6		2.
8	AVG WEEK	97.1	62.2	104.6	105.8	108.7	11.01	97.7	89.7		
AM.	Times	07:00	12:00		08:00	07:00					
ΑM	l Peaks	626	447	634	669	578	621	568	522	503	
		18:00				16:00		15:00			
PM	I Peaks	563	617	625	610	639	597	604	650	593	

UB AWD 9508 FAC ,96 (.96) ADT 8,800

STA. 6 EB

Site Reference: 180060000494 Site ID: 00000000603 Location: NEPONSET VALLEY PKWY WEST OF RTE. 138 Direction: EAST

File: V6.prn City: MILTON County: VOLUME EB&WB

TIME	MON 2	TUE 27	WED 28	THU 29	FRI 30	WKDAY AVG	31			TOTAL
01:00	51		42		ii 73	57	00	125	74	446
	32		35		49					288
03:00	28		33	18	27	26	48	30	30	193
04 • 00	28 40		22	41	27 -30	36	38	41	32 37 90	223
05:00	104		115	132	102	117	48	41	90	542
06:00	394		372	390	300	364	125	47	271	1628
	511		494	515	440	402	107	07		2243
08:00	447		477	489	347	440 385	242	128	355	2130
00.00			441	400	341	385	271	128 148 218	327	1962
10:00	294		347	380	334	338	287	218	310	1860
11:00	304		319		313	311		354	317	
12:00	259	279	251	307	321	283	354	342	301	2113
13:00	304	292	293	304	355	309	343	393	326	2284
14:00	304 291	292 342	315	314	333	323	343 343	415	339	2375
15:00	322	362	345	338	344	342	372	372	350	2455
16.00	312	352	341	329		336	391	345	345	2419
17:00	284	275	329	302	325	303	354	307	310	2176
18:00	300		280	300	341	300	333 356	385 353	317	2222
19:00	300 257	289	286	273	317	284	356	353	304	2131
20:00	196	236	217	240	272	232	303	400	266	1864
	212		218	213	233	214	294	354	246	1722
22:00	175	168	203	224	255	205	232	225	211	1482
23:00	139	155	150	159	193	159	190	139	160	1125
24:00	139 91	94	120	122	154	116	172	112	160 123	865
TOTALS										
% AVG WKDY % AVG WEEK	95 97.8	55.3 57	100.8	103 106.1	102.9 105.9		95.7 98.5	90.7 93.4		
	2		-							
AM Times	07:00	12:00	07:00		07:00	07:00	12:00	11:00	07:00	
AM Peaks	511	279	494		449	492	354	354	373	
PM Times	15:00	15:00	15:00	15:00	13:00	15:00	16:00	14:00	15:00	
			345			342				

STA, 6 WB

Site Reference: 180060000494 Site ID: 00000000603 Location: NEPONSET VALLEY PKWY WEST OF RTE. 138

Direction: WEST

File: V6.prn City: MILTON County: VOLUME EB&WB

TIME	2	TUE 27	28	29		WKDAY AVG		1	WEEK AVG	
	: 36		40	41	48	41	65	53	47	283
02:00	27		21	16	29	23	37	36	27	166
03:00	18		24 18	14	21	19 19 41	27	32	22	136
04:00	20 33			22 46	18	19	19	20	19	117
05:00 06:00	33		94	97	4.7	93	25	11	33	202
	98				120	129	39	20	107	453 647
	115 174		133 157 175	180	123	123	110	53	144	
08:00	154		175	148	173 155	171 158	121	63 67 101	138	
10:00	163			156	156	157	105	101	152	914
11:00	177					187	199	160	186	
	136			186		176	214	168	180	
13:00	196	175	179	200	222	195	214 194 203	168 219 235	198	1391
14:00	196 203	213	206	213	249	216	203	235	217	1522
15:00	202	220	238	237	255	230	232	214	228	1598
16:00	237	265		247	290	261		218		
17:00	250	281	296	308	295	286	233	178	263	1841
18:00	263 195	281 290 235	291	268	254	273	180	190	263 248 220	
19:00	195	235	253	268 208	254 261	230	173	217	220	
20:00	195 168	173	185	210		184	166	170	179	1258
21:00	122	125	156	135	145	136	140	138	137	961
22:00	97	107	119	114	149	117	120	119	117	825
23:00	73	78	81	83	119	86	103	67	86	604
24:00	57	58	66	81	93	117 86 71	100	49	72	504
+										
TOTALS	3214					3499				22509
% AVG WKDY	91.8	68.2	101.4	100.8	108.8		92.1	79.8		
% AVG WEEK	96.1	71.4	106.2	105.6	113.9		96.5	83.5		
AM Times	11:00	12:00	12:00	12:00	11:00	11:00	12:00	11:00	11:00	
AM Peaks	177	168	183	186	222	187	214	168	186	
PM Times	18:00	18:00	17:00	17:00	17:00	17:00	17:00	14:00	17:00	
	263	290	296	308		286				

STA, 7NB

Site Reference: 180060000532

Site ID: 000000000701

Location: RTE. 138 NO. OF NEPONSET VALLEY PKWY

Direction: NORTH

File: SPD8028 NB.prn

Page: 1

City: MILTON County: SPEED NB

TIME	MON 2	TUE	WED	THU	FRI	WKDAY	SAT	SUN	WEEK AVG	TOTAL
	2	27	28					1	AVG	
01:00	91		79	84 35	100	88 43 34	152	164	111	670
02:00	91 40		30	35	67	43	78	118	61	368
03:00	35		30	33	38	34	78	84	49	298
04:00	46		28	30	38	35	74	57	45	273
- 05:00	74		80	63	79	74	58	39	65	393
06:00	74 311 450		322	ี จกจ	270	74 301 446	79	47	222	1222
07:00	450		433	471	431	446	161	95	340	2041
	543		518	486	461	502	203	150	sy 393	2361
9:00	507		519	544	475	511	346	218	434	2609
10:00	477		504	486	439	476	356	339 438	433	2601
11:00	391		410	486 449	406	476 414	397	438	415	2491
12:00	222		323	371	427	369	402	399	379	
13:00	329	377	351	346			442	389	383	
14:00	346	292	386	373	448		491	455	398	2791
15:00			344	373 391	455	200	445	458	406	2844
16:00	408	384	425	480	446	428	493	423	437	3059
17:00	408	415	393	458	462	427	468	432	433	3036
18:00	415		407		502	442	433	432		3076
19:00	359	368	393	447	505	414	456	529	436	3057
20:00	359 306	282	393 358	447 337	346	414 325	380	557	366	2566
21:00	254	304	333	330	328	309	329	474	336	2352
22:00	261	248				287			302	
23:00	168	206	236	227	310	229	306	259	244	1712
24:00	168 122	125	128	168	183	145	230	165	160	1121
							20			
TOTALS						7426		7104	7287	48125
% AVG WKDY	95.2	51	98.8	103.3	107.6		96.2	95.6		
% AVG WKDY % AVG WEEK	97	52	100.6	105.3	109.6		98.1	97.4	9	
AM Times	08:00		09:00	09:00		09:00				
AM Peaks	543		519	544	475	511	402	438	434	
PM Times	18:00	18:00	16:00	16:00		18:00			18:00	**
PM Peaks			425	480	505	442	493	557	439	

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AWD 15496 FAC .97 (.95) ADT 14,300

STA. 7SB

Site Reference: 180060000822

Site ID: 000000000702

Location: RTE. 138 NO. OF NEPONSET VALLEY PKWY Direction: SOUTH

File: SPD8028.prn City: MILTON -County: SPEED SB

10										
TIME	MON	TUE	WED	THU	FRI	WKDAY	SAT	SUN	WEEK	TOTAL
	2									
01:00	106		74	108	129	104 45 36	161	174	125	752
02:00	45		41	46	51	45	124	110	69	417
03:00	45 30		22	34	61	36	60	91	49	298
04:00	38		35	28	40	25	59	64	44	264
05:00	42		66	63	71	60	51	42	55	335
06:00	159		171 339	158	148	60 159 317 465	82	48	127	766 1515
07:00	159 295		339	335	299	317	164	83	252	1515
08:00	476		500	470	417	465	275	156	382	2294
09:00	482		520	476	467	486	346	198	414	2489
10:00	367		364	408	403	385	429	302	378	2273
11:00	338		386	355	456	383	448	371	392	2354
12:00	375		364 386 392	443	478	385 383 422	437	424	424	2549
13:00	383	383	520	517	575	475	550	525	493	
		442				479		587		
15:00	566	595	520	563	518	552	523	601	555	3886
16.00	553	595 563	506	536	518 484	552 528	563	601 571	539	3776
17:00	471	462	456	450	436	455	550	526	478	3351
18:00	497	483	452	482	428	468	491	482	473	3315
19:00	456	508	512	497	482	491	508	491	493	3454
20:00	389	508 449 425	516	522	475	491 470 422	483	531	480	
21:00	356	425	468	463	399	422	409	498	431	3018
22:00	298	354	407	381	413	370	311	358	360	2522
23:00	225	249	241	275	358	269	326	266	277	1940
24:00	168	249 161	172	213	257	194	274	183	204	1428
TOTALS										
% AVG WKDY % AVG WEEK	93.7	62.8	100.6	102.5	104.6		101.2	95.1		
% AVG WEEK	94.6	63.4	101.6	103.4	105.6		102.1	96	G.	
AM Times	09:00		09:00	09:00	12:00	09:00	11:00	12:00	12:00	
AM Peaks	482		520	476	478	486	448	424	424	
PM Times	15:00	15:00	13:00	15:00	14:00	15:00	16:00	15:00	15:00	
		595								
					-		A		-	

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STA . 8 E.B

Site Reference: 180060000108

Site ID: 000000000803

Location: MILTON ST. WEST OF RTE. 138

Direction: EAST

File: V8.prn City: MILTON

County: VOLUME EB&WB

TIME		TUE 27		THU 29	FRI 30	WKDAY AVG		SUN 1		TOTAL
	2	21	28			AVG	31	 1	AVG	
01:00	16		9	17	15	14	16	12	14	
02:00	3		. 23 5	В	8	5	8	14	7	44
03:00	3		∜ 5	2	3 3 11 25	3	5	7	4	25
	10		5	10	3	7	4	5 =	6	37
05:00			18	15	11	13	10	2	11	67
06:00	26		36	28	25	28	14	5	22	134
07:00	79		76	78	60	73	21	7 :	53	321
	160		183	168	88	149	30	23	108	652
	108		132	142	90	118	70	40	97 95	582
10:00	86 89		129	106	82	100	307	- 61	95 92	
11:00	89		87	80	115	92	116	67	92	554
12:00	75	3.0	82	116		95		65		
13:00	94	89		116	115	104	112	87	103	721
	110	127	101	107	140	117	127 112	89	114	801
15:00	130	141	139	141	153	140	112	87	129	
16:00	150	187	186	170	167	172	135	53		
17:00	193	206	216	192		211	111	77	178	1247
	179				169	208	88	59	169	1189
19:00	137	189	169	188	136	· 163 94	89	73	140 88	981 616
20:00	95 45 54	87	96	102	92 57	94	71	73	88	616
21:00	45	76	74	78	57	66	59	45		434
22:00	54	59	61	61	55			33	54	378
23:00	24	19	22	36	47	29	33	20	28	201
24:00	13	17	21	31	26	21		15	20	145
TOTALS	1890	1423	2232	2186	2018	2080	1527	1019	1836	12295
<pre>% AVG WKDY % AVG WEEK</pre>	90.8	68.4	107.3	105	97		73.4	48.9		
% AVG WEEK					109.9		83.1	55.5		
AM Times			08:00	08:00	11:00		11:00			
AM Peaks	160	*:	183	168	115	149	116	67	108	
PM Times	17:00	18:00	18:00	18:00	17:00	17:00				¥
PM Peaks	193	226	274	194	252	211	135	89	178	
				6.7.7						

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AWD 2080 FAC .97(.93) ADT 1,900

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STABWB

Site Reference: 180060000108 Site ID: 000000000803 Location: MILTON ST. WEST OF RTE. 138

Direction: WEST

File: V8.prn City: MILTON County: VOLUME EB&WB

TIME	MON 2	TUE 27	WED 28	THU 29	FRI 30	WKDAY AVG	SAT 31	SUN 1	WEEK AVG	TOTAL
01:00	10		11	12	17	12	17	13	13	80
02:00	6		4			7		14	9	54
03:00	12		5	8	-8	8	12	∷ 9	9	54
04:00	7		10	11	= 8	9	7	12	9 9	55
05:00	15		21	21	13	17	9	5	9 14	84
06:00	50		51	61	44		26	7		239
07:00	172		165	167	132	159	55	25	119	716
08:00	209		257	229	121	204			149	898
09:00	159		161	181	140	160	97	45	130	783
10:00	102		118	121	131	118	91	58	103	621
11:00	81		105	104	106	99			93	559
12:00	80		85	112	124	100		73	102	617
13:00	100	95	125		137	117	112	84	111	783
14:00	115	97	102	120	124	111	112 123	89	110	770
15:00	102	106	116	117	141	116	111	91	112	784
16:00	154	160	144	162	177	159	108		142	995
17:00	153	185	158	196	186	175	119	70	152	1067
18:00	163	202	170	190	187	182	108	78 96	156	1098
19:00		151		190	119	155	99	% 96	138	970
20:00		89	112	122	103		78		96	672
21:00	81	90	98	79					76	
22:00	57	62	64	74	67	64	53	44		421
23:00	28 20	23	29	48	48	35	41	27	34	244
24:00	20	21	19	26	29	23	29	19	34 23	163
TOTALS	2102	1281	2305	2490	2234	2264	1654	1199	1999	13265
% AVG WKDY	92.8	56.5	101.8		98.6		73			60
% AVG WEEK	105.1	64	115.3	124.5	111.7		82.7	59.9		
AM Times	08:00		08:00	08:00	09:00	08:00	12:00	12:00	08:00	
AM Peaks	209		257			204			149	
PM Times	18:00	18:00	19:00	17:00	18:00	18:00	14:00	19:00	18:00	224
PM Peaks		202	175	196		182	123	96	156	
III IGUNU	100	202	2,5	× 130	(2)				¥0.	

STA. 9 WB

Site Reference: 180060000151

Site ID: 000000000904

Location: DOLLAR LN. EAST OF RTE. 138

Direction: WEST

File: V904.prn City: MILTON County: VOLUME WB

		114								
TIME	MON 2	TUE 27	WED 28	THU 29	FRI 30		SAT 31	SUN 1		TOTAL
01:00	10		6	13	0	9	1.4	12	10	64
02:00	5		5	8	9	6	14	14	8	49
03:00	10		ĭ	2	6	Ā	6	6	5	31
04:00	4		B	9	ă	7	ă	B	5 7 12 43	45
05:00	4 14		16	21	14	16	ă	2	12	75
06:00	53		49	70	53	56	31	6	43	262
	197			164	138	170	49	22	125	754
08:00	292		280	279	131	245	50	32	177	1064
	203		216	243	163	206	99 50 83	36	157	944
10:00	203 110		137	132	109	122	95	50	105	633
11:00	97		108	102	97	101	108	66	96	578
12:00	91		81	125	131	107	144	69 104	106	641
13:00		92	129	124	140	116	119	104	115	805
14:00	124 124	104	96	126	126	115	121	91	112	788
15:00	114	118		123	155	129				885
16:00	185	158		176	165	170	125	80	151	1057
17:00	189	191	182	214	195	194 195	141	78	170	1190
18:00	194		199	185	187			B1	168	1182
19:00	142	123	162	171	128	145	83	129	134	938
20:00	64	82	119	81	91	87	78	118	90	633
21:00	61	70	82	80	62	71	67	79	71	501
22:00	50 29	70 45 22	68 31	64 60	72	59	56	79 70 30	60	3 425
23:00	29	22	31	60	48	38	40	30	-37	260
24:00	26	21	18	32	25	24	31	24	25	177
TOTALS	2361	1237	2482	2604	2262	2392	1711	⁶ 1324	2110	13981
% AVG WKDY % AVG WEEK	98.7 111.8	51.7 58.6	103.7 117.6	108.8 123.4	94.5 107.2		71.5 81	55.3 62.7		
AM Times AM Peaks	08:00 292		08:00 280	08:00 279	09:00 163		12:00 144		08:00 177	***
PM Times PM Peaks	18:00 194	18:00 211		17:00 214	17:00 195			19:00 129	17:00 170	

46

WB ONLY EB NO DATA, STA. 10

Site Reference: 180060000423 Site ID: 000000001001

Location: RTE. 138 NO. OF MILTON ST.

Direction: ROAD TOTAL

File: V10.prn City: MILTON County: VOLUME NB&SB TOTAL

TIME		MON 2	TUE 27	WED 28	THU 29	FRI 30	WKDAY AVG	SAT 31			TOTAL
01:00		216		171	207	240	208	323	350	251	1507
02:00		85			91	130	94	216	242		837
03:00		70		60	73	105	77	149			65 <i>6</i>
04:00		06		67	68	78	74	143	137	109 96 130	579
05:00		130		150	146	163	147	110	137 85	130	784
06:00		546		554	524	458	520	172	93	391	2347
07:00		889		891	945	779	876		181		4040
08:00		1250				1049	1180	529 741	305	926	5557
09:00		1221		1201	1197 1149	1049	1180 1155 976	741	305 433	965	5794
10:00		1079		941	996	891	976	854	672	905	5433
11:00		953		863	878	933	906	881	817	887	5325
12:00		855	180	808 918 888	909	954	881	844	830	866	5200
13:00		796	827	918	925	1111	915	1023	930	רכם	6530
14:00		796 894	814	888	925 901	1157	930	1091			6786
15:00		1155	1064	938	1031	1291	1095	1041	1128	1092	7648
16:00		1076		1005	1140	1330	1145	1116			7912
17:00	16			915	967	1325	1030	1107	1042	1042	7300
18:00		1013	994 1019	960	1041	1160	1038	985	961	1019	
19:00		966	1073	1069	1336	1066	1102	1063	1099	1096	7672
20:00		890	892	986	1071	916	951	955	1220	990	6930
21:00		794	842	898	889	809	846	822	1135	884	6189
22:00		681	731	898 811 531	769	799	846 758 547	692	863	763	5346
23:00		472	487	531	533	716	547	698	609	578	4046
24:00		344	303	329	389	462	365	566	413	400	2806
TOTALS	*	17411	10224		18175	18971	17816	16476	15852	17233	114363
% AVG WKDY		07 7	67 3	06.0	102	106.4		92.4	00 0		
% AVG WEEK					105.4	110		95.6			
AM Times		08:00		08:00	08:00	08:00	08:00	11:00	12:00	09:00	
AM Peaks		1250		1227						965	
								80.7		303	
PM Times		15:00	16:00	19:00	19:00		16:00				
PM Peaks		1155	1178	1069	1336	1330	1145	1116	1220	1130	

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AWD 17816 FAC ,97(.95) ADT 16,400

STA.10 NB

Site Reference: 180060000423 Site ID: 000000001001 Location: RTE. 138 NO. OF MILTON ST. Direction: NORTH

File: V10.prn City: MILTON County: VOLUME NB&SB

TIME	MON 2	TUE 27	WED 28		FRI 30	WKDAY AVG		SUN 1		TOTAL
01:00	100		9.4	05	106	96	153	- 162	116 64 52	700 388
02:00	41		32	95 41	71	96 46	80	123	64	388
03:00	37		31	28	39	33	82	95	52	312
04:00	45		32	37	34	37				287
	78		75		86	75	56	33	65	392
06:00	355		366	358	299	344 522 626	89	49	252	1516
07:00	557	ž.	507	546	479	522	175	91	392	2355
08:00	704		628	627	548	626	225	139	478	2871
09:00	630	100	622	638	534	606	372	228	504	3024
10:00	617		573	566	466	555	411	359	498	2992
	509		473	518	427	481		442	467	
12:00	436 371	408 353	377	430	470	428	415	401	421	2529
13:00		408	395	. 393	478	409	451	404	414	
14:00	385	353		415	490	411	516	447	431	
		425	390	411	514	430		482	446	3124
		491	472		485	491				3408
		480	463	498	545	490	527		494	3460
18:00	464	510	502	539	543	511	477	460	499	3495
19:00	435 377 328	472	477	534	561	495	476		507	
20:00	377	369	421	411	391	393	415	641	432	3025
21:00	328	349	371	388		360		555	389	2728
22:00	301		354			330		448		
23:00	213	215	260	259		255		301	274	1918
24:00	144	132	143	175	193	157	264	206	179	1257
TOTALS	8470	4504	8461	8851	8813	8581	7753	7641	8255	54493
% AVG WKDY										
% AVG WEEK	102.6	54.5	102.4	107.2	106.7		93.9	92.5		
AM Times	08:00		08:00	09:00		08:00			09:00	
AM Peaks	704		628			626			504	
PM Times	17:00	18:00	18:00	16:00	19:00	18:00	17:00	20:00	19:00	
PM Peaks	468	510	502	544	561	511	527	641	507	

STA, 105B

Site Reference: 180060000423 Site ID: 00000001001 Location: RTE. 138 NO. OF MILTON ST.

Direction: SOUTH

File: V10.prn City: MILTON County: VOLUME NB&SB

TIME	MON	TUE	WED	THU	FRI	WKDAY	SAT	SUN		TOTAL
	2	27	28	29	30	AVG	31	1	AVG	
01:00	116		87	112	134	112	170	188	134	807
02:00	44		41			48			74	449
03:00	33		29	45	66	43	67	104	57 48 65	344
	41		35 75	31	44	37	66	75	48	292
<pre>05:00</pre>	41 52		75	82	77	71	54	52	65	392
06:00	191		188	166	159	176	83	44	138	831
07:00	332		384	399	300	353	180	90	280	1685
08:00	546		599	570	501	554	304 369 443	166	447	2686
09:00			579	511	515	549	369			2770
10:00	462		368		425	421	443	313	406	2441
11:00	444		390		506	425			420	
	419		431		484	453		429		
		419			633	506	572	526	518	3630
14:00	.509	461	475	486	667	510	575	594	538	3767
15:00	.509 743	639	548	620	667 777	665	551	646	646	4524
16:00	613	687	533	596	945	654	603	627	643	4504
17:00	482		452	469	780	539	580	563	548	3840
18:00	549	509	458		617	527	508	501	520	3644
19:00	531	601 523	592	802	505	606	587	505	589	4123
20:00	513	523	565	660	525	557	540	579	557	3905
21:00	513 466	493	527	501	445	486	449	580	494	3461
22:00	380	431	457			428	348	415	414	
			271						304	
24:00	200	171	186	214	269	208	302	207	221	1549
TOTALS	8941	5720	8793	9324	10158	9229	8723	8211	8967	59870
% AVG WKDY	96.8	61 9	95 2	: 101	110		94.5	88.9		
% AVG WEEK	99.7	63.7	98	103.9	113.2		97.2	91.5	•	
						10				
AM Times			08:00	08:00	09:00	08:00	11:00	12:00	09:00	
AM Peaks	591		599	570	515	554	448	429	461	
PM Times	15:00	16:00	19:00	19:00	16:00	15:00	16:00	15:00	15:00	
PM Peaks	743	687	592	802	845	665	603	646	646	

STA.11

Site Reference: 180060000742 Site ID: 000000110304

Location: BRADLEE RD. WEST OF RTE.138

Direction: ROAD TOTAL

TOTAL

File: V11.prn City: MILTON County: VOLUME EB&WB

TIME	MON 2	TUE 27	WED 28	THU 29	FRI 30	WKDAY AVG	SAT 31	SUN 1	WEEK AVG	TOTAL
01:00	20		26	25	38	27	33	40	30	182
02:00	10		3	6	14	8			14	87
03:00	3		6	6	8			16	10	61
04:00	9		ī	ĭ	4			10	-6	38
05:00	16		16	12	19		10	- 6	13	79
06:00	39		32	42	33	36		9		165
07:00	93		84	96	83	89	23	18	66	397
08:00	170		172	140	102	115	50	26	110	660
09:00	121		128	122	117	122	75	63	104	626
10:00	87		110	106	109	103	119	71	100	602
11:00	78		97	85	114	93	125	108	101	607
12:00	84		80	112	123	99	140	131		670
13:00	89		103	128	118	109	135	135	118	708
14:00	125	92	103	106	119	109	169	136		850
15:00	131	137	120	121	141	130	147			938
16:00	140	139	145	139	181	148	171	116	147	1031
17:00	153	155	147	173	184	162	159	134	157	1105
18:00	143	144		165		147		135	142	996
19:00	118	114	140	121	149	128		157		943
20:00	114	103	117			111	142	139		
21:00	91	90	98		97	97	118	123	103	726
22:00	56	80	91	72	98	79	80	76	79 54 34	553
23:00	35	37	46 25	71 35	70	51	- 70	55	54	384
24:00	29	29	25	35	41	31	57	27	34	243
TOTALS	1954	1120	2032	2100	2218	2048	2162	1902	2034	13488
	95.4				108.3			92.8		
% AVG WEEK	96	- 55	99.9	103.2	109		106.2	93.5		
AM Times	08:00	100	08:00	08:00	12:00			12:00	12:00	
AM Peaks	170		172	140	123	146	140	131	111	4
PM Times	17:00	17:00	17:00	17:00	17:00	17:00	16:00	19:00	17:00	
PM Peaks	153	155	147	173	184	162	171	157	157	**

46 AWD 2048 FAC .97 (,93) ADT 1,800

STA. 11 EB

Site Reference: 180060000742 Site ID: 000000110304 Location: BRADLEE RD. WEST OF RTE.138

Direction: EAST

File: V11.prn
City: MILTON
County: VOLUME EB&WB

TIME	MON 2	TUE 27	WED 28	THU 29	FRI 30	WKDAY AVG		SUN 1	WEEK AVG	TOTAL
	<u>-</u>					AVG			AVG	
01:00	12		20	19	22	18	23	27	20	123
02:00	4		2	4	13	5	14	20	. 9	57
03:00	2		. 4	2	6	3	13	10	6	37
04:00	6		* 1	O	1	2	9 5	8	4	25
05:00	7		6	3	7	5	5	4	5	32
06:00	19		18	26	20	20	5	4	15	92
07:00	57	53	48	52	50	51	10	9	37	226
08:00	101		108	79	48	84	19	11	61	366
09:00	77		81	72	67	74	32	31	60	360
10:00	47	50.00	71	55	58	57	49	41	53	321
11:00	42		59	45	50	49	65	61	53	322
12:00	52		. 45	61	70	57	72	56	59	356
13:00	50		56	70	63	59	83	75	66	397
14:00	66	51	65	68	67	63	86	80	69	483
15:00	77	76	70	68	92	76	84	86	79	553
16:00	71	79	90	82	92	82	98	69	83	581
17:00	90	79	76	98	104	89	84	67	85	598
18:00	83	64	69	84	74	74	82	74	75	530
19:00	67	50	74	62	88	68	89	79	72	509
20:00	61	56	67	66	65	63	89	81	69	485
21:00	58	61	61	75	53	61	69	67	63	444
22:00	34	48	53	43	54	46	42	49	46	323
23:00	22	20	20	38	46	29	46	33	32	225
24:00	16	16	16	25	28	20	36	13	21	150
						<u> </u>				
TOTALS	1121	600	1180	1197	1238	1155	1204	1055	1142	7595
% AVG WKDY	97	51.9	102.1	103.6	107.1		104.2	91.3		
% AVG WEEK	98.1	52.5	103.3	104.8	108.4		105.4	92.3		
AM Times	08:00		08:00	08:00	12:00	08:00	12:00	11:00	08:00	
AM Peaks	101		108	79	70	84	72	61	61	
PM Times	17:00	16:00	16:00	17:00	17:00	17:00	16:00	15:00	17:00	
PM Peaks	90	79	90	98	104	89	98	86	85	

STA. 11 WB

Site Reference: 180060000742 Site ID: 000000110304

Location: BRADLEE RD. WEST OF RTE.138 Direction: WEST

File: V11.prn City: MILTON County: VOLUME EB&WB

TIME	MON 2	TUE 27	WED 28	THU 29	FRI 30	WKDAY AVG	SAT 31	SUN 1	WEEK AVG	TOTAL
		0.0			01					
01:00	8		6	6	16	9	10	13	9	59
02:00	6		1	2	1	2	10	10	5	30
03:00	1	-3	2	<i>≅</i> 4	2 3	2	9	6	4	24
04:00	3		0	1	3	1	9 4	2	2	13
05:00	9		10	9	12	10	5 5	2 5	7	47
06:00	20		14	16	13	15	5	5	12	73
07:00	36		36	44	33	37	13	9	28	171
08:00	69		64	61	54	62	31	15	49	294
09:00	44		47	50	50	47	43	32	44	266
10:00	40		39	51	51	45	70	30	46	281
11:00	36		38	40	64	44	60	47		285
12:00	32		35	51	53	42	68	75	52	314
13:00	39		47	58	55	49	52	60	51	311
14:00	59	41	38	38	52	45	83	56	52	367
15:00	54	61	50	53	49	53	63	55	55	385
16:00	69	60	55	57	89	66	73	47	64	450
17:00	63	76	71	75	80	73	75	67	72	507
18:00	60	80	73	81	67	72	44	61	66	466
19:00	51	64	66	³ 59	61	60	55	78	62	434
20:00	53	47	50	41	· 50	48	53	58	50	352
21:00	33	29	37	34	44	35	49	56	40	282
22:00	22	32	38	29	44	33	38	27	32	230
23:00	13	17	26	33	24	22	24	22	22	159
24:00	13	13	9	10	13	11	21	14	13	93
TOTALS	833	520	852	903	980	883	958	847	884	5893
% AVG WKDY % AVG WEEK	94.3 94.2	58.8 58.8	96.4 96.3	102.2 102.1	110.9 110.8		108.4 108.3	95.9 95.8	3%	
AM Times AM Peaks	08:00 69		08:00 64	08:00 61	11:00 64	08:00 62	10:00 70	12:00 75	12:00 52	
PM Times PM Peaks	16:00 69	18:00 80	18:00 73	18:00 81	16:00 89	17:00 73	14:00 83	19:00 78	17:00 72	

STA. 12

Site Reference: 180060000863

Site ID: 000000120304

Location: ATHERTON ST. EAST OF RTE.138

Direction: ROAD TOTAL

TOTAL

File: V12.prn City: MILTON County: VOLUME EB&WB

TIME	MON 2	TUE 27	WED 28	THU 29	FRI 30	WKDAY AVG		SUN 1		TOTAL
01:00	15	F.	12	22	1.6	16	22	17	17 8 6 4 6 23 54	103
02:00	3		6	22	13	70	12	117	8 6 4	103 51
03:00	3		3	2	12	3	13	17	9	37
04:00	5		2	1	2	2	· 7	7	4	24
05:00	8		ã	4	-	6		ź	Ē	39
06:00	33		29	41	24	31	6	6	23	139
07:00	74		75	81	60	72	18	19	54	327
08:00	201		150	150	96	149	41	15	108	653
09:00	139		156	168	108		48			653
10:00	106		119	117		110	61	4.5	94	568
11:00	106 85		92	75	85	110 84	90	71	94 83	498
12:00	77		85	113	97	93	98	102	95	572
13:00	88		106	106	128	107	93	71		592
14:00	101	79	83	93	133	97	139	94	103	722
15:00	108	129	112	126	125	120		87		795
16:00	146	150	156	158	198	161	108 109	83	142	
17:00	163	191	179	193	217	188	83	72	156	1098
18:00	117	186	161	163	155	156	92	105	139	979
19:00	120	107	139	144	114	124	89	101	116	814
20:00	69 61	80	97	99	66	82	66	103	82 69	580
21:00	61	70	82	72	61	69	58	84	69	488
22:00	34	54	52	72	50	52	55	56	53	373
23:00	26	32	34	37	52	36	38	37	36	256
24:00	16	27	19	31	30	24	32	15	24	170
TOTALS	1798	1105	1958	2074	1939	1930	1398	1259	1737	11531
% AVG WKDY % AVG WEEK	93.1	57.2	101.4	107.4	100.4		72.4	65.2	=47	
% AVG WEEK	103.5	63.6	112.7	119.4	111.6		80.4			
AM Times		34	09:00	09:00		08:00				
AM Peaks	201		156	168	108	149	98	102	108	
PM Times	17:00	17:00	17:00	17:00	17:00	17:00	14:00	18:00	17:00	
PM Peaks	163						139			

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AWD 1930 FAC. 97 (.93) ADT 1,700

STA. 12 EB

Site Reference: 180060000863 Site ID: 000000120304 Location: ATHERTON ST. EAST OF RTE.138

Direction: EAST

File: V12.prn City: MILTON County: VOLUME EB&WB

						15				
TIME	MON 2	TUE 27	WED 28	THU 29	FRI 30	WKDAY AVG	SAT 31	SUN 1	WEEK AVG	TOTAL
01:00	6		3	7	1	4	6	2	4	25
02:00	1		0	0	4	1	4	4	2	13
03:00	1		0	0	2	<u>.</u> 0	3	7	2	13
04:00	1		0	0	1	0	0	2	0	4
05:00	6		3	2	4	3	3	2	3	20
06:00	13		9	13	7	10	4	3	8	49
07:00	16		19	24	14	18	8	9	15	90
08:00	56		40	44	30	42	19	7	32	196
09:00	31		32	49	30	35	18	13	28	173
10:00	30	W 14	29	33	43	33	38	21	32	194
11:00	29		38	28	-38	33	33	27	32	193
12:00	37		29	37	43	36	39	46	38	231
13:00	39		39	40	59	44	28	24	38	229
14:00	45	34	25	36	55	39	67	29	41	291
15:00	46	63	47	60	57	54	40	31	49	344
16:00	68	73	61	78	129	81	40	28	68	477
17:00	67	100	102	101	130	100	40	26	80	566
18:00	47	93	84	73	89	77	32	32	64	450
19:00	62	53	68	87	48	63	35	25	54	378
20:00	29	29	35	37	28	31	21	29	29	208
21:00		24	29	19	23	23	18		22	160
22:00	15	19	14	30	17			12	18	126
23:00	5	16	12	14	13	12	13	8		81
24:00	4	14	6	7	5	7	8	4	-6	48
24.00	3	**	e .	•	J	ŕ	Ū	-	J	
TOTALS	677	518	724	819	870	765	536	415	676	4559
% AVG WKDY	00.4	67.7	04.6	107	113.7		70	54.2		
% AVG WEEK	100.1	76.6	107.1	121.1	128.6		79.2	61.3		
AM Times	08:00		08:00	09:00	10:00	08:00	12:00	12:00	12:00	
AM Peaks	56		40	49	43		39	46	38	
PM Times	16:00	17:00	17:00	17:00	17:00	17:00	14:00	18:00	17:00	
PM Peaks	68	100	102	101	130	100	67	32	80	
		200								

Page: 2

6972

5TA. 12 WB

Site Reference: 180060000863 Site ID: 000000120304 Location: ATHERTON ST. EAST OF RTE.138

Direction: WEST

File: V12.prn City: MILTON County: VOLUME EB&WB

	TIME	300	MON 2	TUE 27	WED 28	THU 29	FRI 30	WKDAY AVG	SAT 31	SUN 1	WEEK AVG	TOTAL
_					W,							
	01:00		9		9	15	14	11	16	15	13	78
	02:00		2		6	6	8	5	9	7	6	38
	03:00		2		3	2	3	2	4	10	0C 4	24
	04:00		4	- 37	2	1	1	2 2	7	- 5	3	20
	05:00		2		6	2	2	3	2	5	3	19
	06:00		20		20	28	17	21	2	3	15	90
	07:00		58		56	57	46	54	10	10	39	237
	08:00		145		110	106	66	106	22	8	76	457
	09:00		108		124	119	78	107	30	21	80	480
	10:00		76		90	84	57	76	43	24	62	374
	11:00		56		54	47	47	51	57	44	50	305
	12:00		40		56	76	54	56	59	56	56	341
	13:00		49		67	66	69	62	65	47	60	363
	14:00		56	45	58	57	78	58	72	65	61	431
	15:00		62	66	65	66	68	65	68	56	64	451
	16:00		78	77	95	80	69	79	69	55	74	523
	17:00		96	91	77	92	87	88	43	46	76	532
			70	93	77			79	60	73		529
	18:00					90 = 57	66		54	75 76	75 62	426
	19:00		58	54	5 71 62		66	61			62	436
	20:00		40	51	62	62	38	50	45	74	53	372
	21:00		.38	46	53	53	38	45	40	60	46	328
	22:00		19	35	38	42	33	33	36	44	35	247
	23:00		21	16	22	23	39	24	25	29	25	175
	24:00	2.0	12	13	13	24	25	17	24	11	17	122

						F				
TOTALS	1121	587	1234	1255	1069	1155	862	844	1055	_
% AVG WKDY % AVG WEEK	97 106.2	50.8 55.6	106.8 116.9	108.6 118.9	92.5 101.3		74.6 81.7	73 80		
AM Times AM Peaks	08:00 145		09:00 124	09:00 119	09:00 78	09:00 107	12:00 59	12:00 56	09:00 80	
PM Times PM Peaks	17:00 96	18:00 93	16:00 95	17:00 92	17:00 87	17:00 88	14:00 72	19:00 76	17:00 76	

STAIB NB

Site Reference: 180060000787

Site ID: 00000001301 Location: RTE 138 NORTH OF BRADLEE RD.

Direction: NORTH

File: SPD1301.prn City: MILTON County: SPEED NB

				¥2						
TIME	MON	TUE 27	WED	THU 29	FRI					TOTAL
					30	AVG	 31		AVG	
01:00	75 38		77	82	85 74	79	150	142	101	611
				38	74	46	69	108	60	
	28		25	28	35	29	77	93	47	286
04:00	37		26 67	33	32 76 254	32	71	63	43	262
05:00	65		67	50	76	64	48	33	56	339
06:00	65 299		304	309	254	291	74	51	215	1291
	412		451	430		423		97		
08:00	501		488	480	423	473	204	136	372	2232
- 09:00	411		467	451	419	437 384	314	208 327 397	378	2270
10:00	376 322		427 339	392	343	384	344	327	368	2209
11:00	322	2	339	387	313	340	373	397	355	2131
12:00	291		305	341		324	353	357		
13:00	282		325	308		321	367	349	333	2002
14:00	.269	304	314	310	389	317 339	409 401	378 387	339	2373
15:00	302	369 390	328	328	372	339	401	387	355	2487
16:00	360	390	380		389	393	395	382		2746
17:00	379	400	367	410	432	397	434	373	399	
18:00	399	448	399	444	439	425	352	371 438 460	407	2852
19:00	330	394 259	379	417	448	393 297	376	438	397 328	2782
20:00	269	259	343	318	299	297	349	460	328	2297
21:00	238	282	302	297	291	282	292	392	299	2094
22:00	227	226	272	257	288	254	291	321	268	1882
23:00	164	184	216	220	276	212	288	203	221	1551
24:00	103	184 114	126	159	178	136	224	152	150	1056
TOTALS		3370								
						15				
<pre>% AVG WKDY</pre>	9 2.3	50.3	101.1	103.7	104.4		95.7	92.9		
% AVG WKDY % AVG WEEK	94.4	51.5	103.3	106.1	106.8		97.9	95		
AM Times	08:00				08:00			11:00		
AM Peaks	501	10	488	480	423	473	373	397	378	(4)
PM Times	18:00	18:00	18:00	16:00	19:00	18:00	17:00	20:00	18:00	
PM Peaks	399	448	399	450	448	425	434	460	407	

42 AWD 13297 FAC .97 ADT 12,900

STA,13 SB

Site Reference: 180060000554 Site ID: 000000001302

Location: RTE 138 NORTH OF BRADLEE RD. Direction: SOUTH

File: SPD1302.prn City: MILTON County: SPEED SB

TIME	MON 2	TUE 27	WED 28	THU 29		AVG	31			TOTAL
	79					83				639
	33		33	36	52	38	115	100	61	369
03:00	28		23	32	55	34 31 60	57	85	46	280
04:00	29 45		32	25 65	38	31	60	71	42	255
05:00	45			65	71	60	51	43	55	335
	147					143	79	38		689
	258		292	295	267	278	145	78	222	1335
00:00	422		463	442	392	429	246 305	145 179	351	2110
09:00	440		491	438	414	445	305	179	377	2267
10:00	330		294	360	325	327	353	274	322	1936
	283		291			306				
	298		329	345	364	334	321			2015
13:00	294	246	350	375	403	355	465 442	441 504	388	2328
14:00	327 423	346 473	348	337	454	362 438			394	
15:00	423		429	455	411				449	3143
			417			433		520	453	3174
	347	358	366		:351 366	359		445	385 393	2699 2752
18:00	414	398 372	374	382 415	379	386 387	419 435	399 373	393 392	2745
19:00	340 304	351	431 410	415	379 404	374	389	411:	382	2674
20:00	278	321	331		350			411		
						288			285	1995
23:00	220	203	201	377	217	200	265 304	200	203	
23:00	188 143	213	150	177	27.1	160	249	155	241	1249
				23						
TOTALS	6091					6609				
% AVG WKDY % AVG WEEK	92.1 91.9	55.9 55.7	100.3 100	102.4 102.2	106.3 106		105 104.7	98.9 98.7		
				00.00				10.00	00-00	
AM Times AM Peaks	09:00 440		09:00 491			09:00 445				
PM Times PM Peaks	15:00 423					15:00 438	16:00 488	15:00 525	16:00 453	

STA.14 EB

Site Reference: 180060000448 Site ID: 00000001403

Location: Robbins St. EB, east of Rte.138

Direction: EAST

File: D0327029.prn City: Milton

County: Volume EB

					5.5					
TIME	MON 2	TUE 27	WED 28	THU 29	FRI 30	WKDAY AVG	SAT 31	SUN 1	WEEK AVG	TOTAL
	-57.0	Ш	ĸĵ.							
01:00	3		0	4	4	2	7	4	3	22
02:00	1		1	0	0	0	2	3	1	7
03:00	1		1	0	0	0	2	1	0	5
04:00	0		1	1	1	0	0	3	1	∅ 6
05:00	1		0	0	3	1	0 "	1	0	5
06:00	4		6 7	2	6 7	4	1	3	3	22
07:00 08:00	8 38		39	18 37	31	10 36	6	3	8	49
09:00	46		42	42	47	44	18 30	10 10	28 36	173 217
10:00	26		36	43	34	34	24	22	30	185
11:00	26		17	23	39	26	37	22	27	164
12:00	24		54	27	38	35	33	33	34	209
13:00	29		46	29	30 41	36	32	27	34	204
14:00	32	34	33	29	56	36	35	34	36	253
15:00	31	36	30	33	37	33	32	26	32	225
16:00	44	46	48	57	48	48	34	24	43	301
17:00	59	57	66	54	29	53	32	32	47	329
18:00	55	82	51	60	41	57	43	22	50	354
19:00	30	52	48	48	35		26	26	37	265
20:00	20	31	32	36	23	28	22	27	27	191
21:00	10	11	15	17	21	14	19	19	16	112
22:00	9	12	17	19	17	14	16	7	13	97
23:00	12	9	6	9	13	9	9	3	8	61
24:00	1	2	3	4	- 4	2	9	6	4	29
TOTALS	510	372	599	592	575	564	469	368	518	3485
% AVG WKDY	90.4	65.9	106.2	104.9	101.9		83.1	65.2		
% AVG WEEK	98.4	71.8	115.6	114.2	111		90.5	71		
AM Times	09:00		12:00	10:00	09:00	09:00	11:00	12:00	09:00	
AM Peaks	46		54	43	47	44	37	33	36	
PM Times	17:00	18:00	17:00	18:00	14:00	18:00	18:00	14:00	18:00	
PM Peaks	59	82	66	60	56	57	43	34	50	

U0 AWD 1215 FAC .97(.93) ADT 1,100

Page: 1

STA. 14 WB

Site Reference: 180060000668

Site ID: 00000001404 Location: Robbins St. WB, east of Rte.138

Direction: WEST

File: D0327030.prn City: Milton County: Volume WB

TIME	MON 2	TUE 27	WED 28	THU 29	FRI 30	WKDAY AVG	SAT 31	SUN 1	WEEK AVG	TOTAL
21.00	%_				_	_	_			•
01:00	¹ 5		1	3	5	3	3	5	3	22
02:00 03:00	2 1		1	0	2 1	1 0	4	7 2	2 1	16
04:00	1		1 1	1	1	1	1	2	1	6 6
05:00	1		0	3	3	. 1	-	2	1	9
06:00	3		4	2	4	3	2	3	3	18
07:00	12		20	11	8	12	2	6	9	59
08:00	59		68	64	34	56	18	4	41	247
09:00	67		62	70	56	63	26	15	49	296
10:00	36		39	48	37	40	35	21	36	216
11:00	18		34	27	50	32	37	19	30	185
12:00	32		37	35	36	35	50	37	37	
13:00	29		31	32	49	35	49	46	39	236
14:00	31	28	27	42	43	34	34	32	33	237
15:00	39	35	35	40	53	40	37	29	38	268
16:00	48	47	49	59	41	48	49	31	46	324
17:00	58	66	59	74	43	60	42	33	53	375
18:00	44	53	49	57	51	50	38	33	46	325
19:00	44	33	51	49	45	44	35	32	41	289
20:00	25	36	29	30	29	29	20	22	27	o 191
21:00	16	39	39	32	26	30	27	23	28	202
22:00	12	17	18	20	25	18	24	9	17	125
23:00	11	8	9	10	20	11	10	10	11	78
24:00	4	8 3	8	8	5	5	12	9.55 4		44
£:	3			8	-	_			_	
TOTALS	598	365	672	717	667	651	555	427	598	4001
% AVG WKDY	91.8	56	103.2	110.1	102.4		85.2	65.5		
% AVG WEEK	100	61	112.3	119.8	111.5		92.8	71.4		
AM Times	09:00		08:00	09:00	09:00	09:00	12:00	12:00	09:00	
AM Peaks	67		68	70	56	63	50	37	49	
PM Times	17:00	17:00	17:00	17:00	15:00	17:00	13:00	13:00	17:00	
PM Peaks	58	66	59	74	53	60	49	46	53	

Site Reference: 180060000400

Site ID: 000000150304

Location: ROBBINS ST. WEST OF RTE. 138

Direction: ROAD TOTAL

5TA.15 TOTAL

File: V15.prn City: MILTON County: VOLUME EB&WB

TIME	MON 2	TUE 27	WED 28	THU 29	FRI 30	WKDAY AVG	SAT 31	SUN 1	WEEK AVG	TOTAL
01:00	3		1	5	7	4	٥	. 12	6	37
02:00	3 1		i	0	2	1	9 5	7	2	16
03:00	1		Ō	ŏ	1	ō	.4	2	1	8
04:00	Ō		1	ŏ	ō	0 0	3	3	i i	11.7
05:00	1		Ô	ĭ	ĭ	Ö	ő	Ö	ō	á
06:00	6		4	5	7	5	2	ŏ	4	24
07:00	19		24	29	10	20	5	7	15	94
08:00	78		78	65	60	70	25	22	54	328
09:00	65		88	,, 77	67	74	42	24		363
10:00	44		52	49	53	49	45	30	45	273
11:00	34		49	38	53	43	72	44	48	290
12:00	32		53	48	70	50	75	70	58	348
13:00	40		31	52	61	46	65	59	51	308
14:00	50	53	50	39	59	50	50	57	51	358
15:00	49	63	54	52	59	-55	44		55	388
16:00	61	65	63	85	70	68	68		67	469
17:00	77	91	83	78	61	78	64		74	520
18:00	65	100	74	74	84	79	61		71	499
19:00	71	70	80	81	61	72	60	33	65	456
20:00	31	60	56	ଂ 48	47	48	28	45	45	315
21:00	20	29	29	41	34	30	28	36	31	217
22:00		27	24	32	30		35		24	174
23:00	6		7	11	20	11	18			85
24:00	6	4	₋₁ 8	12	7	7	20		9	67
TOTALS	774	574	910	922	924	885	828	715	849	5647
TOTALO	//3	3.4	310	722	224	000	020	83	0.15	0011
% AVG WKDY	87.4	64.8	102.8	104.1	104.4		93.5	80.7		
% AVG WEEK		67.6	107.1	108.5	108.8		97.5	84.2	7 40	
AM Times	08:00		09:00	09:00	12:00	09:00	12:00	12:00	09:00	
AM Peaks	78		88	77	70	74	75	70	60	
PM Times	17:00	18:00	17:00	16:00	18:00	18:00	16:00	15:00	17:00	
PM Peaks	77	100	83	85	84	79	68	67	74	

NO

AWD 885 FAC .97(.93) ADT 800

STAILS EB

Site Reference: 180060000400 Site ID: 000000150304 Location: ROBBINS ST. WEST OF RTE. 138

Direction: EAST

File: V15.prn City: MILTON County: VOLUME EB&WB

TIME	MON 2	TUE 27	WED 28	THU 29	FRI 30	WKDAY AVG	SAT 31	SUN 1	WEEK AVG	TOTAL
01:00	1		0	2	3	1	6	5	2	17
02:00	0		0	0	0	0	2	2	0	4
03:00	1		0	0	0	0	1	1	0	3
04:00	0		0	∴ O	0	0	1	2	0	3
05:00	0		0	0	0	0	0	0	0	0
06:00	2	100	3	2	3	2	1	0	1	11
07:00	9		В	15	4	9	4	4	7	44
08:00	36		31	29	35	32	14	18	27	163
09:00	27		44	33	29	33	25	10	28	168
10:00	17		22	24	30	23	19	14	21	126
11:00	19		17	17	28	20	37	18	22	136
12:00	16		28	23	30	24	39	30	27	166
13:00	16		13	23	30	20	26	20	21	128
14:00	23	30	28	19	25	25	28	30	26	183
15:00	17	33	27	23	24	24	20	24	24	168
16:00	30	30	26	40	35	32	33	24	31	218
17:00	33	35	34	· 32	24	31	22	30	30	210
18:00	34	52	37	32	. 37	38	29	22	34	243
19:00	27	41	35	39	25	33	28	17	30	212
20:00	14	23	33	27	22	23	12	23	22	154
21:00	7	6	8	13	15	9	16	13	₀ 11	78
22:00	4	6	8	14	14	9	14	4	9	64
23:00	.3	4	3	3	4	3	7	3	3	27
24:00	1	1	1	5	4	2	10	7	4	29
TOTALS	337	261	406	415	421	393	394	321	380	2555
% AVG WKDY % AVG WEEK	85.7 88.6	66.4 68.6	103.3 106.8	105.5 109.2	107.1 110.7		100.2 103.6	B1.6 84.4		
				51.1.						
AM Times AM Peaks	08:00 36		09:00 44	09:00 33	08:00 35	09:00 33	12:00 39	12:00 30	09:00 28	
PM Times	18:00	18:00	18:00	16:00	18:00	18:00	16:00	14:00	18:00	
PM Peaks	34	52	37	40	37	38	33	30	34	

STA.15 WB

Site Reference: 180060000400 Site ID: 000000150304 Location: ROBBINS ST. WEST OF RTE. 138 Direction: WEST

File: V15.prn City: MILTON County: VOLUME EB&WB

TIME	MON 2	TUE 27	WED 28	THU 29	FRI 30	WKDAY AVG	SAT 31	SUN 1	WEEK AVG	_ TOTAL
						W _O				
01:00	2		1	3	4	2	3	7	3	20
02:00	1		1	0	2	1	3	5	2	12
03:00	0		0	0	1	0	. 3	1	0	5
04:00	0		1	0	0	0	_	· 1	0	4
05:00	1		0	1	1		0	0	0	3
06:00	4		1	3	4	•	1	0	2	13
07:00	10		16	14	6	11	1	3	8	50
08:00	42		47	36	25	37	11	.4	27	165
09:00	38		44	44	38	41	17	14	32	195
10:00	27		30	25	23	26	26	16	24	147
11:00	15		32	21	25	23	35	26	25	154
12:00	16		25	25	40	26	36	40	30	182
13:00	24		18	29	31	25	39	39	30	180
14:00	27	23	22	20	34	25	22	27	25	175
15:00	32	30	27	29	35	30	24	43	31	220
16:00	31	35	37	45	35	36	35	33	35	251
17:00	44	56	49	46	37	46	42	36	44	310
18:00	31	48	37	42	47	41	32	19	36	256
19:00	44	29	45	42	36	39	32	16	34	244
20:00	17	37	23	21	25	24		22	23	161
21:00	13	23	21	28	19	20	12	23	19	139
22:00	10	21	16	18	16	16	21	8	15	110
23:00	3	8	" 4	- 8	16	7	11	8	8	58
24:00	₀₀ 5	3	7	7	3	5	10	3	5	38
TOTALS	437	313	504	507	 503	484	434	394	458	3092
TOTALL	45,	010	501			101	55			••••
% AVG WKDY	90.2	64.6	104.1	104.7	103.9		89.6	81.4		
% AVG WEEK	95.4	68.3	110	110.6	109.8		94.7	86		
AM Times	08:00		08:00	09:00	12:00	09:00	12:00	12:00	09:00	
AM Peaks	_ 42	Oi.	47	44	40	41	36	40	32	
PM Times	17:00	17:00	17:00	17:00	18:00	17:00	17:00	15:00	17:00	10
PM Peaks	44	56	49	46	47	46	42	43	44	6.0
										0.2

STA.16

Site Reference: 180060000874

Site ID: 00000001601

Location: RTE.138 NORTH OF ROBBINS ST.

Direction: ROAD TOTAL

File: V16.prn City: MILTON County: VOLUME NB&SB TOTAL

TIME	MON 2	TUE 27	WED 28		FRI 30			SUN 1	WEEK AVG	TOTAL
01:00	151		69	112	107	100	100	205	155	933
02:00	65		48	45		57		106	87	525
03:00	51		25	34	67		83			432
04:00	66		48				90	116	66	398
05:00	66 109		70	98	43 114 222	48 97	60	73	87	
06:00	440		400	405	332	394	101	73 85	293	1763
07:00	704		673			662	190	167	501	3007
08:00	962		1020	769	731	870	270		671	4028
09:00	911			922	B23	905	475	348	740	4444
10:00			965 727	658	6B3	695	655	576	668	4013
11:00	600		623	551	697	617	693	696	643	3860
12:00	596		584	508	676	591		705	618	3711
13:00	543	22	595				830	747	666	4000
14:00	542		/ N N /	483	845	589	847 823 899	824	671	4028
15:00	599 793	484 718	504	554	789	586	823	920 883	667	4673
16:00				692	830	707	899	883	760	5321
17:00	752	760	529	479	814	666	849	810	713	4993
18:00	800	777	596	490	819	696	785	741	715	5008
19:00	605	665	458	691	850	653	804	803	696	4876
20:00	543	515	430	4/0		531 446	707	641 449	572	4007
21:00	543 494	537	326	362	514	446	621	449	471	3303
22:00	418	469			469	405		329	412	
23:00	334	350	257	256	444	328	569	222	347	
24:00	244	191	160	192	219	201		174	234	1640
TOTALS	12036	5466	10395	10368	13012			11249	11525	74803
% AVG WKDY	104.6	47.5	90.3	90.1	113.1					
* AVG WEEK	104.4	47.4	90.1	89.9	112.9		106.5	97.6		
AM Times			08:00			09:00			09:00	
AM Peaks	962		1020	922	823	905	693	705	740	
PM Times	18:00 800	18:00	18:00	16:00	19:00	16:00	16:00	15:00	16:00	
PM Peaks	800	777	596	692	850	707	899	920	760	

42

AWD 11502

FAL .97 (.95)

ADT 10,600

STA.16 NB

Site Reference: 180060000874 Site ID: 000000001601

Location: RTE.138 NORTH OF ROBBINS ST.

Direction: NORTH

File: V16.prn City: MILTON

County: VOLUME NB&SB

TIME	MON	TUE	WED		FRI	WKDAY	SAT	SUN	WEEK	TOTAL
	2	27	28	29	30	AVG	31	₃₅ 1	AVG	
									22	
01:00	83		²⁰ 0	36	24	35	6B	157	61	368
02:00	31		11	7	8	14	40	97		194
03:00	26			6	36	14 17 16	10	87	27	167
04:00	29		20	6 1	14	16	24	87 65 45	25	153
05:00	44		5	40	39	32	18	45	31	191
	138		92	101	72	100	23	38	77	464
07:00	253		215	243	160	217	43	74	164	
08:00	415		477	21.6	281	347	93	74 140 154	270	
09:00	415 450		458	414	386	427	158	154	336	2020
10:00	321		300	230	330	295	293	258	288	1732
11:00	289		266	139		263	330			1678
12:00	305		276	172	332	271	291		287	1723
13:00	277		289	168	411	286			338	2030
14:00	277 294		163	163	448	267	452 422	480	328	1970
15:00	291	136	172	229	405	246	413	545	313	2191
		323	133	222		308	481			2525
		366	133	77	375	266		444	316	2217
18:00	412	325	15/	51	372		414	380	301	2108
19:00	412 282	289	96	280	385	266	425		302	2118
20:00	300	270	100	188		251		215	264	
		279		96	249	191	340	71	195	1368
22:00	213	256	44	137	200	170	258	15	160	1123
23:00	182	174	51	51	165	124	304	31	136	958
24:00	142	174 84	42	50	55	74	246	39	160 136 94	658
TOTALS	 5861	2502	3558	3317	5940	4745	 5964	5275	4984	32417
					05.0		0,000	02,0		02127
% AVG WKDY					125.1		125.6	111.1		
% AVG WEEK	117.5	50.2	71.3	66.5	119.1		119.6	105.8		
AM Times	09:00		08:00		09:00				09:00	
AM Peaks	450		477	414	386	427	330	347	336	
PM Times	16:00	17:00	13:00	19:00	14:00	16:00	16:00	15:00	16:00	
PM Peaks			289		448	308	481	545	360	

Site Reference: 180060000874 Site ID: 000000001601 Location: RTE.138 NORTH OF ROBBINS ST. Direction: SOUTH

STA.16 SB

File: V16.prn City: MILTON County: VOLUME NB&SB

TIME	MON 2	TUE 27		THU 29	FRI 30	WKDAY AVG	SAT 31	SUN 1		, TOTAL
	68		69	76	83	74	130		94	
02:00	34		37	38	63	43	60	99	55	331
03:00	25		23	28	31	26 32 65	73	85	44	265
04:00	37		28	9.44	29	32	66	51	40	245
05:00	65		65		75	65	42	28	55	333
	302		308		260		78			1299
07:00	451		458		405	444	147	93	336	2019
08:00	547		543	553	450	523	177	136	401	2406
09:00	461		507	508	437	47R	317	194	404	2424
10:00	461 393		427		353	400		318	380	2281
11:00	2TT		357		339	354				2182
12:00	291		308		344	319	351	358	331	1988
13:00	266		306	308	398	319	378	314	328	1970
14:00	248		324	320	397	322	425	344	343	2058
15:00	308	348	332	325	384	339	410	375	354	2482
16:00	364	395	373	470	396	399	418	380	399	2796
	371	394	396	402	439	400	408	366	396	2776
18:00	388	452	442	439	447	433	371	361	414	2900
19:00	323	376	362	411	465	387	779	442	394	2758
20:00	323 243	245	330	282	300	280	330	426	308	2156
21:00	220	258	267		265	255	281	378	276	1935
	205					235				1761
	152		206	205	279	203	265	191		1474
24:00	102	107	118	142	164	126	214	135	140	982
TOTALS						6749	6313	5974	6532	42386
% AVG WKDY % AVG WEEK	91.4	43.9	101.3	104.4	104.7		93.5	88.5		
% AVG WEEK	94.5	45.3	104.6	107.9	108.2		96.6	91.4		
AM Times				08:00		08:00				
AM Peaks						523	363	400	404	
PM Times	18:00 388	18:00	18:00	16:00	19:00	18:00			18:00	
PM Peaks	388	452	442	470	465	433	425	442	414	4 7

STA. 18

Site Reference: 180060000513 Site ID: 000000180304

TOTAL

File: V18.prn City: MILTON

Location: BLUE HILL TERRACE ST. EAST OF RTE.138

County: VOLUME EB&WB

Direction: ROAD TOTAL

TIME	MON 2	TUE 27	WED 28	THU 29	FRI 30	WKDAY AVG	SAT 31	SUN 1	WEEK AVG	TOTAL
01:00	10		12	9	17	12	18	22	14	88
02:00	3				•	5	11	17	٥	40
03:00	8		2	4 4 5	4	4	-8	B	5 5 5 13	34
04:00	s 8		6	4	5	5	7	3	5	33
05:00	_		9	5	7	6	1	3	5	31
06:00	21	2.0	16	19	16	18	6	5	13	83
07:00	64		81	65	51	65	16	9	47	286
08:00	230		243	246	93	203	36	30	146	878
09:00	169		179	167	111	156	56 82	45	121	727
10:00	64		0.4	101	80	84	82	6.3	80	484
11.00	64 74		76	89	77	79	114	77	84	507
12:00	60	475	70	81		79	114	87	86	519
	70	1	79	91	101	85	100	79	86	520
14:00	84	1	82	75	104	86	112	109	0.4	566
15:00	132	124	133	108	97	118	89	83	109	766
	137	126	167	-137	119	137	105	91	126	
17:00	147	165	151	141	125	145		77	130	911
18:00	165	171 141 79			142	158	99	78 79	138	969
19:00	129	141	137	137	3 131	135	99 76	79	118	830
20:00	82	79	91	153 137 84	95	86	102	95	89	628
21 • 00	65	73	67	94	76	75	66	52	70	493
22:00	37	58	50	50	73	53	50	42	51	360
23:00	22	25	33	37	55	34	43	. 32	35	247
24:00	37 22 24	18	19	27	21	21	36	11	51 35 22	156
TOTALS	1811	980	1962	1929	1715	1849	1452	1197	1682	11046
% AVG WKDY	97.9	53	106.1	104.3			78.5 86.3		4	
% AVG WEEK	10/.6	58.2	116.6	114.6	101.9		00.3	11.1		
AM Times	08:00		08:00	08:00	09:00	08:00	11:00	12:00	08:00	
AM Peaks	230		243			203			146	
PM Times	18:00	18:00	16:00	18:00	18:00	18:00	14:00	14:00	18:00	
PM Peaks	165	171	167	153	142	158	112	109	138	

uo :

AWD 1849 FAC .97 (.93) ADT 1,700

STA. 18 EB

Site Reference: 180060000513 Site ID: 000000180304 Location: BLUE HILL TERRACE ST. EAST OF RTE.138

Direction: EAST

File: V18.prn City: MILTON County: VOLUME EB&WB

TIME	MON	TUE	WED	THU	FRI	WKDAY		SUN	WEEK	TOTAL
	2	27	28	29	30	AVG	31	1	AVG	
01:00	2		2	2	8	3	10	11	5	35
02:00	1		4	4	3	3	3	7	3	22
03:00	≅ 3		0	2	0	1	2	3	1	10
04:00	2	113	2	1	1	1		1	1	9
05:00	3		2	4	2	. 2	0	1	2	12
06:00	6		7	5	5	5	1	3	4	27
07:00	21		27	16	16	20	7	6	15	93
08:00	89		90	88	36	75	. 13	14	55	330
09:00	58		53	56	43	52	25	19	42	254
10:00	25		34	47	32	34	32	24	32	194
11:00	33		33	40	43	37	48	36	38	233
12:00	26		23	25	40	28	44	27	30	185
13:00	28	8	38	39	41	36	45	32	37	223
14:00	39	**	25	37	54	38	52	55	43	262
15:00	58	62	63	41	50	54	46	36	50	356
16:00	69	54	75	71	65	66	41	W 51	60	426
17:00	70	75	70	77	61	. 70	55	33	63	441
18:00	83	92	94	78	72	83	44	40	71	503
19:00	67	86	74	73	69	73	37	32	62	438
20:00	39	33	40	38	40	38	52	47	41	289
21:00	32	32	37	47	. 35	36	23	16	31	222
22:00	19	28	26	23	35	26	23	21	25	175
23:00	6	13	12	19	26	15	20	11	15	107
24:00	16	12	10	14	12	12	15	4	11	83
TOTALS	795	487	841	847	789	808	640	530	737	4929
% AVG WKDY	98.3	60.2	104	104.8	97.6		79.2	65.5		
% AVG WEEK	107.8	66	114.1	114.9	107		86.8	71.9		
AM Times	08:00		08:00	08:00	09:00	08:00	11:00	11:00	08:00	
AM Peaks	89		90	88	43	75	48	36	55	* 1
PM Times	18:00	18:00	18:00	18:00	18:00	18:00	17:00	14:00	18:00	
PM Peaks	83	92	94	78	72	83	55	55	71	

5TA.18 WB

Site Reference: 180060000513 Site ID: 000000180304 Location: BLUE HILL TERRACE ST. EAST OF RTE.138

Direction: WEST

File: V18.prn City: MILTON County: VOLUME EB&WB

TIME	Ģ	MON 2	TUE 27	WED 28	THU 29	FRI 30	WKDAY AVG	SAT 31	รบ ท 1	WEEK AVG	TOTAL
01:00		В		10	7	9	В	8	11	8	53
02:00		2		0	1	- 5	2	8	10	4	26
03:00		. 5		2	2	4	3	6	5	4	24
04:00		6		4	3	4	4	5 :		4	24
05:00		. 3		7	1	- 5	4	1	2	3	19
06:00		15		9	14	11	12	5	2	9	56
07:00		43		54	49	35	45	9	3	32	193
08:00		141		153	158	57	127	23	16	91	548
09:00		111		126	111	68	104	31	26	78	473
10:00		39		60	54	48	50	50	39	48	290
11:00		41		43	49	34	41	66	41	45	274
12:00		34		47	56	67	51	70		55	334
13:00		42		41	52	60	48	55		49	297
14:00		45		57	38	50	47			50	304
15:00		74	62	70	67	47	64	43	47	58	410
16:00		68	72	92	66	: 5 4	70		40	i 65	456
17:00		77	90	81	64	64	75			67	470
18:00		82	79	67	75	70	74	55	38	66	466
19:00		62	55	63	64	62	61	39		56	392
20:00		43	46	51	46	55		50		48	339
21:00		33	41	30	47	41	38		36	38	271
22:00		18		24	27	38 29	27	27	21 21	26	185
23:00		16		21	18	29	19	23	21	20	140
24:00		8	6	9	13	9	9	21	7	10	73
TOTALS	111	1016	493	1121	1082	926	1031	812	667	934	6117
% AVG WKDY % AVG WEEK			47.8 52.7	108.7 120		89.8 99.1		78.7 86.9	64.6 71.4		
AM Times		08:00		08:00	08:00	09:00		12:00		08:00	
AM Peaks		141		153	158	68	127	70	60	91	
PM Times		18:00	17:00	16:00	18:00	18:00	17:00	16:00	14:00	17:00	
PM Peaks		82	90	92	75	₆ 70	75	64	54	67	

Site Reference: 180060000756

Site ID: 000000190304

Location: OAK ST. EAST OF RTE. 138

Direction: ROAD TOTAL

STA, 19 TOTAL

File: V19.prn City: MILTON

County: VOLUME EB&WB

TIME	MON 2	TUE 27	∈ WED 28	THU 29	FRI 30	WKDAY AVG	SAT 31	SUN 1	WEEK AVG	TOTAL
									19	12.
01:00	5		2	0	2	2	2	10	3	21
02:00	1		1	0	1	0	1	3	1	7
03:00	13		0	0	0	3	1	0) 1	. 2	15
04:00	2	2070	2	1	2	1	3 0	0	×1	10
05:00	0		0	1	1	0		2	0	4
06:00	2		1	2	4	2	1	3	2	13
07:00	8		9	8	4	7	≘ 1	3	5	33
00:00	32		35	28	15	27	10	. 4 .		124
09:00	22		26	15	12	18	9	7	15	91
10:00	7		21	11	25	16	11	7	13	82
11:00	. 9		11	8	11	9	14	14	11	67
1,2:00	9		14	9	21	13	34	21	18	108
13:00	10		7	20	18	13	15	14	14	84
14:00	13		10	16	18	14	15	25	16	97
15:00	14	12	15	15	10	13	21	20	15	107
16:00	17	24	22	26	14	20	17	16	19	136
17:00	22	11	22	15	19	17	22	15	18	126
18:00	24	27	25	23	19	23	21	13	21	152
19:00	23	18	22	32	20	23	20	12	21	147
20:00	10	26	17	18	23	18	13	9	16	116
21:00	9	17	16	19	17	15	20	11	15	109
22:00	7	14	13	9	22	13	15	4	12	84
23:00	3	5	3	6	11	5	4		6	42
24:00	0	3	8	4	7	4	6	1	4	29
TOTALS	262	157	302	286	296	276	276	225	268	1804
% AVG WKDY	94.9	56.8	109.4	103.6	107.2		100	81.5	#3	
% AVG WEEK	97.7	58.5	112.6	106.7	110.4		102.9	83.9		
AM Times	08:00		08:00	08:00	10:00	08:00	12:00	12:00	08:00	
AM Peaks	32		35	28	25	27	34	21	20	
PM Times	18:00	18:00	18:00	19:00	20:00	18:00	17:00	14:00	18:00	
PM Peaks	24	27	25	32	23	23	22	25	21	

NO

AWD 276

FAC .97(.93)

ADT 250

Page: 1

STA.19 EB

Site Reference: 180060000756

Site ID: 000000190304

Location: OAK ST. EAST OF RTE. 138

Direction: EAST

File: V19.prn

City: MILTON
County: VOLUME EB&WB

TIME	MON 2	TUE 27	WED 28	THU 29	FRI 30	WKDAY AVG	SAT 31	SUN 1	WEEK AVG	TOTAL
01:00	P 1		0	0	2	0	1	1	0	5
02:00	ō		ĭ	Ō	ō	ŏ	<u>-</u>	2	ŏ	4
03:00	7		ō	Ö	ō	í	25 > 1	ō	ī	8
04:00	1		1	Ö	1	† ō	1	Ō	ō	4
05:00	0		0	1	0	0	Ö	1	Ō	2
06:00	1		1	· 1	3	1	0	1	1	7
07:00	7		6	6	3	5	1	2	4	25
08:00	17		18	15	9	14	6	1	11	66
09:00	9		9	5	9	В	4	4	6	40
10:00	6.		13	10	11	10	9	3	8	52
11:00	5		7	3	7	5	8	7	6	37
12:00	3		4	7	11	6	14	12	8	:: 51
13:00	4		6	14	11	В	9	6	8	50
14:00	9		6	4	13	В	9	16	9	57
15:00	6	6	8	3	3	5	10	13	7	49
16:00	8	12	10	10	6	9	9	9	9	64
17:00	12	6	13	9	11	10	12	6	9	69
18:00	19	14	10	15	9	13	10	4	11	81
19:00	13	11	15	11	9	11	10	8	11	77
20:00	4	12	10	11	13	10	10	6	9	66
21:00	7	10	В	6	11	8	9	6	В	57
22:00	3	5	7 :-	4	13	6	8	2	6	42
23:00	2	1	0	3	7	2	3	6	3	22
24:00	0	2	2	1	2	1	1	1	1	9
TOTALS	144	79	155	139	164	141	146	117	136	944
% AVG WKDY	102.1	56	109.9	98.5	116.3		103.5	82.9		
% AVG WEEK	105.8	58	113.9	102.2	120.5		107.3	86		
AM Times	08:00		08:00	08:00	10:00	08:00	12:00	12:00	08:00	
AM Peaks	17		18	15	11	14	14	12	11	
PM Times	18:00	18:00	19:00	18:00	14:00	18:00	17:00	14:00	18:00	
PM Peaks	19	14	15	15	13	13	12	16	11	

Page: 2

STA. 19 WB

Site Reference: 180060000756

Site ID: 000000190304

Location: OAK ST. EAST OF RTE. 138 Direction: WEST

File: V19.prn City: MILTON

County: VOLUME EB&WB

TIME	MON 2	TÜE 27	WED 28	THU 29	FRI 30	WKDAY AVG	SAT 31	SUN 1	WEEK AVG	TOTAL
01:00	4	: : ::	2	0	0	1	1	9	2	16
02:00	1		0	Ö	1	Ō	ō	1	0	3
03:00	6		ő	0	ō	1	ŏ	ī	1	s 7
04:00	, ĭ		i	1	1	ī	2	ō	1	6
05:00	Ō		ō	ō	ī	ō	ō	ĭ	ō	2
06:00	ĭ		Ö	ĭ	ī	ŏ	ĭ	2	1	6
07:00	ī		3	2	1	i	ō	ī	ī	8
08:00	15		17	13	6	12	11 4	3	وَ ا	58
09:00	13		17	10	3	10	5	3	8	51
10:00	1		8	1	14	6	2	4	5	30
11:00	4		4	5	4	4	6	7	5	30
12:00	₩ -6		10	5 2	10	7	20	9	9	57
13:00	6		1	6	7	5	6	8	5	34
14:00	4		4	12	5	6	6	9	6	40
15:00	8	6	7	12	7	8	11	7	8	58
16:00	[்] 9	12	12	16	8	11	8	7	10	72
17:00	10	5	9	6	8	7	10	g 9	8	57
18:00	∜5	13	15	· 8	10	10	11	. 9	10	71
19:00	10	7	7	21	11	11	10	4	10	70
20:00	6	14	7	7	10	8	3	3	7	50
21:00	2	7	. 8	13	6	7	11	5	7	52
22:00	4	9	6	5	9	6	7	2	6	42
23:00	1	4	3	3	4	3	1	4	2	20
24:00	0	1	6	3	5	3	5	0	2	20
TOTALS	118	78	147	147	132	128	130	108	123	960
% AVG WKDY % AVG WEEK	92.1 95.9	60.9 63.4	114.8 119.5	114.8 119.5	103.1 107.3		101.5 105.6	84.3 87.8		
	2013									
AM Times	08:00		08:00	08:00	10:00	08:00	12:00	01:00	08:00	
AM Peaks	15		17	13	14	12	20	₩ 9	9	
PM Times	17:00	20:00	18:00	19:00	19:00	16:00	15:00	14:00	16:00	
PM Peaks	10	14	15.00	21	11	11	13.00	9	10.00	
III ICUNG	10	2 1	10							

MassDOT Highway Division SPEED SUMMARY Tue 3/27/2018

STA.5 NB

Page: 1

Site Reference: 180060000535

Site ID: 000000000501

Location: RTE.138 NO. OF BRUSH HILL RD.

Direction: NORTH

Lane: 1

TIME 86+ Tota 12:00 13:00 n . 0 14:00 15:00 16:00 17:00 18:00 19:00 20:00 21:00 В 22:00 23:00 24:00 DAY TOTAL 203 1397 3860 1485 0.0% 0.0% 0.1% 2.9% 19.7% 54.2% 20.9% 2.1% 0.1% 0.0% 0.0% 0.0% 0.0% 0.0% PERCENTS

Statistical Information...

15th Percentile Speed 22.1 mph

Median Speed 26.6 mph

10 MPH Pace Speed 24 mph to 34 mph 5345 vehicles in pace Representing 75.0% of the total vehicles 85th Percentile Speed 31.0 mph

Average Speed 26.4 mph

File: SPD501.prn

County: SPEED NB

City: MILTON

Vehicles > 65 MPH 0.3%

File: SPD501.prn

County: SPEED NB

City: MILTON

Page: 2

MassDOT Highway Division SPEED SUMMARY Wed 3/28/2018

Site Reference: 180060000535

Site ID: 00000000501

Bocation: RTE.138 NO. OF BRUSH HILL RD.

Direction: NORTH

Lane: 1

TIME 86+ Tota 01:00 02:00 03:00 04:00 05:00 Λ O n 06:00 07:00 O 08:00 09:00 10:00 11:00 12:00 13:00 14:00 15:00 16:00 n 17:00 18:00 19:00 n 20:00 21:00 Ω 22:00 23:00 24:00 368 2960 6906 2197 DAY TOTAL 0.1% 0.0% 0.0% 0.0% 0.0% 0.0% 0.0% 0.0% 0.0% 100% PERCENTS 2.9% 23.3% 54.3% 17.3% 1.9% 0.2%

Statistical Information...

15th Percentile Speed 21.6 mph

Median Speed 26.2 mph

10 MPH Pace Speed
19 mph to 29 mph
9866 vehicles in pace
Representing 77.5% of the total vehicles

85th Percentile Speed 30.3 mph

Average Speed 26.0 mph

Vehicles > 65 MPH
13
0.1%

MassDOT Highway Division SPEED SUMMARY Thu 3/29/2018

File: SPD501.prn

County: SPEED NB

City: MILTON

Page: 3

Site Reference: 180060000535

Site ID: 000000000501

Location: RTE.138 NO. OF BRUSH HILL RD.

Direction: NORTH

Lane: 1

TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
		3			83											
01:00	0	3	27	76	30	1	0	0	0	0	** 0	0	0	0	.≅ o	137
02:00	0	3	19	25	11	1	0	0	0	0	- 0	0	0	0	Ó	59
03:00	0	1	14	14	9	4	0	0	0	0	0	0	. O	0	0	42
04:00	0	1	18	27	13	3	0	0	0	2	0	Ó	0	0	0	64
05:00	1	4	68	85	22	2	0	: O	0	0	0	2	0	0	0	184
06:00	14	204	292	148	16	3	0	1	0	0	0	0	0	0	0	678
07:00	23	347	450	80	5	0	1	0	1	0	0	0	0	0	0	907
08:00	35	382	429	86	5	1	1	0	0	0	0	0	0	0	0	939
09:00	60	214	492	71	· 9	0	1	0	0	- 1	0	1	0	0	0	849
10:00	61	193	394	113	6	1	0	0	1	0	Ó	0	0	0	0	769
11:00	33	119	348	157	12	- 0	0	1	2	0	0	0	0	0	0	672
12:00	23	91	359	139	7	0	0	0	0	0	0	0	0	0	0	619
13:00	6	139	339	123	12	0	2	0	0	0	0	0	0	0	0	621
14:00	18	122	369	125	9	0	1	0	0	0	4	2	0	0	2	652
15:00	× 40	130	368	111	16	0	0	0	0	0	0	0	0	1	0	666
16:00	38	239	429	66	6	0	0	0	1	0	. 0	0	0	0	0	779
17:00	44	226	372	94	3	1	0	0	0	0	0	0	0	0	0	740
18:00	29	257	358	77	6	0	0	0	0	0	0	0	0	0	2	729
19:00	55	255	288	78	0	0	1	0	0	- 2	0	0	0	0	2	681
20:00	54	219	191	63	6	0	0	1	1	0	0	1	0	0	2	538
21:00	8	106	302	83	8	2	0	0	0	0	0	0	0	0	2	511
22:00	. 7	92	. 283	98	5	0	0	0	0	1	0	0	0	0	1	487
23:00	7	67	174	103	21	2	0	0	· 0	0	0	0	0	0	0	374
24:00	5	23	126	96	10	2	0	1	0	0	0	2	0	0	0	265
DAY TOTAL	561	3437	6509	2138	247	23	7	4	6	<u>-</u> -	4	8	0	1	11	12962
PERCENTS	4.4%	26.6%	50.3%	16.5%	2.0%	0.2%	90.0	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 21.0 mph

Median Speed 25.9 mph

10 MPH Pace Speed 19 mph to 29 mph 9946 vehicles in pace Representing 76.7% of the total vehicles 85th Percentile Speed 30.2 mph

Average Speed 25.6 mph

Vehicles > 65 MPH 24 0.2%

MassDOT Highway Division SPEED SUMMARY Fri 3/30/2018

File: SPD501.prn

County: SPEED NB

City: MILTON

Page: 4

Site Reference: 180060000535

Site ID: 00000000501

Location: RTE.138 NO. OF BRUSH HILL RD.

Direction: NORTH

Lane: 1

													N			
TIME	19	24	29	34	39	44	49	54	59	64	69	74	* 7 9	85	86+	Tota
					25											
01:00	3	9	65	58	19	1	0	0	2	0	0	0	0	0	0	157
02:00	2	3	39	42	12	0	1	0	0	0	0	0	0	0	0	99
03:00	1	1	13	23	17	2	0	0	×5 0	0	0	0	0	0	. 0	57
04:00	0	0	12	27	14	1	0	0	0	0	. 0	0	0	0	0	54
05:00	2	15	48	84	13	3	0	0	1	0	2	0	<i>*</i> 0	0	. 0	168
06:00	17	98	205	173	28	1	0	0	1	0	0	. 0	0	0	2	525
07:00	43	293	380	107	15	1	1	0	0	0	2	0	0	0	2	844
08:00	34	214	376	103	4	0	1	0	1	1	0	0	0	0	0	734
09:00	30	198	444	112	8	2	0	0	. 0	0	0	1	0	0	0	795
10:00	11	273	340	95	9	0	0	0	0	0	0	0	0	0	0	728
11:00	3	92	363	191	12	2	1	0	0	0	0	0	0	0	0	664
12:00	32	119	376	158	13	2	0	0	0	0	0	0	0	0	0	700
13:00	43	208	403	102	5	0	0	0	0	.0	0	0	0	0	0	761
14:00	21	272	377	69	8	0	0	0	0	0	0	0	0	0	0	747
15:00	37	383	267	56	4	0	0	0	0	0	0	0	0	0	0	747
16:00	8	291	380	60	3	0	0	0	0	0	0	0	0	0	0	742
17:00	139	213	302	54	1	0	0	0	0	0	0	0	0	0	. 0	709
18:00	77	264	406	58	2	0	0	0	. 0	0	0	0	· 0	0	0	807
19:00	14	217	409	137	11	1	0	0	0	0	0	0	0	0	0	789
20:00	14	190	295	76	11	1	0	0	0	0	0	0	0	0	0	587
21:00	2	141	315	92	6	0	0	. 0	0	0	0	0	0	0	0	556
22:00	3	174	286	86	8	1	0	0	0	0	0	0	0	0	0	558
23:00	6	85	278	91	4	0	0	0	0	0	0	0	0	. 0	0	464
24:00	0	32	175	109	16	0	0	0	0	0	0	0	0	0	0	332
DAY TOTAL	 542	3785	6554	2163	243	18	4	0	5	 1	 4	1	0	0	. <u></u>	13324
PERCENTS				16.3%	1.8%	0.1%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 20.9 mph

Median Speed 25.8 mph

10 MPH Pace Speed 19 mph to 29 mph 10339 vehicles in pace Representing 77.5% of the total vehicles 85th Percentile Speed 30.0 mph

Average Speed 25.5 mph

MassDOT Highway Division SPEED SUMMARY Sat 3/31/2018

File: SPD501.prn

County: SPEED NB

City: MILTON

Page: 5

Site Reference: 180060000535

Site ID: 00000000501

Location: RTE.138 NO. OF BRUSH HILL RD.

Direction: NORTH

Lane: 1

TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
01.00				105												
01:00	0	9	73	105	22	0	0	0	0	: 0	0	0	0	0	0	209
02:00 03:00	0	0	34	73 57	18 27	3 2	2	1	0	0	0	0	;0 0	0	0	131
	0	0 2	35	5 <i>7</i>	17	2	0	ō	0	0	0	Ü	0	0	0	123
04:00 05:00	1	6	23 31			2	0	0	0		0	U	0	- O	0	104 101
	Ü			43 96	19	2		_	_	0	0	U		0		
06:00 07:00	0	3 20	57 146	138	30 22	3	0	0	0	0	0	0	0	្រក	0	189 328
08:00	, U	27	224	160	17	2	0	0	0	0	0	0	0	0	0	433
09:00	23	59	274	217	27	9	0	Ô	0	ŏ	0	0	Ô	Ô	0	603
10:00	5	84	339	190	17	1	0	0	0	0	0	0	0	0	0	636
11:00		146	309	184	20	0	0	0	0	0	0	0	Ô	0	0	681
12:00	22 10	140	345	213	13	0	0	0	0	0	ă	0	0	ő	0	721
13:00	12	168	405	160	15	2	0	0	0	0	ŏ	0	0	0	ő	762
14:00	34	307	385	75	7	Õ	0	0	0	0	ő	0	Ô	0	ő	808
15:00	10	242	460	- 97	7	0	1	ň	0	ň	ŏ	0	0	0	ő	817
16:00	10	174	503	128	ģ	2	Ō	0	0	ň	ŏ	0	= 0	Ö	ő	B26
17:00	19	165	464	161	6	· 0	0	- 0	Ö	ñ	- ŏ	ň	0	Ô	ő	815
18:00	15	165	354	194	26	0	0	ŏ	0	ő	ŏ	ŏ	ő	. 0	ŏ	754
19:00	3	115	434	208	19	ŏ	0	Ö	Ö	Ô	8 ŏ	Ö	ő	ő	0	779
20:00	J 4	164	372	102	10	ŏ	Ö	ŏ	ŏ	Ö	ő	ő	ŏ	ő	ő	652
21:00	9	163	355	76	6	ŏ	Ŏ	ŏ	o,	ŏ	ŏ	ő	ŏ	8 0	ŏ	609
22:00	10	124	295	91	8	1	ŏ	ŏ	ŏ	ŏ	ŏ	ŏ	ŏ	ŏ	ŏ	529
23:00	1	43	307	119	10	2	ő	ŏ	ŏ	ň	ő	ñ	ő	ŏ	ŏ	482
24:00	î	51	193	120	24	ī	ŏ	Õ	ŏ	ŏ	ő	- 0	ő	ő	ŏ	390
27.00	_	J1		W		_		J	1		•				·	.020
DAY TOTAL	190	2377	6417	3066	396	30	4	2	0	0	0	0	0	0	0	12482
PERCENTS			51.5%		3.1%	0.2%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 22.5 mph

Median Speed 26.9 mph

10 MPH Pace Speed
24 mph to 34 mph
9483 vehicles in pace
Representing 75.9% of the total vehicles

85th Percentile Speed 31.7 mph

Average Speed 26.9 mph

File: SPD501.prn

County: SPEED NB

City: MILTON

MassDOT Highway Division SPEED SUMMARY Sun 4/1/2018

Site Reference: 180060000535

Site ID: 00000000501

Location: RTE.138 NO. OF BRUSH HILL RD.

Direction: NORTH

Lane: 1

	TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
==	01:00	2	8	137	104	17	0	0	0	0	0	. 0	0	0	0	0	268
	02:00	0	В	72	72	30	1	2	0	0.	. 0	0	Ó	0	0	0	185
	03:00	1	1	43	55	13	5	1	1	0	0	1	0	0	0	0	121
	04:00	0	2	32	42	13	1	0	1	0	0	0	0	0	0	0	91
	05:00	0	1	17	36	13	3	0	1	0	0	0	0	0	0	0	71.
	06:00	0	0	28	39	18	4	0	0	0	0	0	0	0	0	0	89
	07:00	.0	0	45	77	46	6	0	0	0	0	0	0	0	2	0	176
	08:00	0	7	76	127	39	6	1	0	0	0	. 0	0	0	0	0	256
	09:00	1	9	94	200	57	5	1	0	0	0	0	0	0	0	0	367
	10:00	0	16	286		36	5	0	0	0	0	. 0	0	0	0	0	567
	11:00	5	69	364	261	37	1	0	0	0	0	0	0	. 0	0	0	737
	12:00	4	69	368	258	38	2	0	0	0	0	0	0	0	0	0	739
	13:00	2	63	459	226	13	2	0	0	0	0	0	0	0	0	0	765
	14:00	20	184	494	130	10	1	0	0	0	0	0	0	0	0	0	839
	15:00	5	118	525		9	4	0	1	0	0	0	0	0	0	0	817
	16:00	15	154	408	158	14	0	0	0	0	0	0	0	0	0	0	749
	17:00	2	73	426	202	15	1	0	0	0	0	0	0	0	0	0	719
	18:00	7	112	479	188	14	0	0	0	0	0	0	0	0	0	0	800
	19:00	11	230	491	134	10	0	0	0	0	- O	0	0	0	0	0	876
	20:00	45	428	391	72	4	1	0	0	0	0	0	0	0	0	0	941
	21:00	40	300	383	60	5	1	0	0	0	0	0	0	0	0	0	789
	22:00	2	124	320		10	0	0	0	0	0	0	0	0	0	0	580
	23:00	0	41	201	130	18	1	0	0	0	0	0	0	0	0	0	391
	24:00	0	14	130	90	22	4	0	1	0	0	0	0	0	0	0	261
	Y TOTAL	162	2031	6269		501	54	5	5	0	0	1	0	0	2	-	12194
PE	RCENTS	1.4%	16.7%	51.5%	25.9%	4.1%	0.4%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 23.1 mph

Median Speed 27.1 mph

10 MPH Pace Speed
24 mph to 34 mph
9433 vehicles in pace
Representing 77.3% of the total vehicles

85th Percentile Speed 32.0 mph

Average Speed 27.3 mph

MassDOT Highway Division SPEED SUMMARY Mon 4/2/2018

File: SPD501.prn

City: MILTON County: SPEED NB

Page: 7

Site Reference: 180060000535

Site ID: 00000000501 Location: RTE.138 NO. OF BRUSH HILL RD.

Direction: NORTH

Lane: 1

										4							
	TIME	19	24	29	34	39	44	49	54	59	64	69	7.4	79	85	86+	Tota
ĦF			134														
	01:00	1	2	55	54	14	3	0	0	0	0	0	0	0	0	0	129
	02:00	0	0	10	31	17	1	1	0	0	0	0	0	0	0	0	60
	03:00	0	0	19	31	11	4	0	0	0	0	0	0	0	0	0	65
	04:00	0	3	13	41	11	3	0	0	0	0	0	0	0	0	0	71
	05:00	0	6	51	79	29	5	0	0	1	0	0	0	0	0	0	171
	06:00	15	114	380	167	19	4	0	0	0	0	0	0	0	0	0	699
	07:00	41	309	490	77	8	0	0	0	0	0	0	0	0.	0	0	925
	08:00	15	304	548	83	12	1	0	0	0	0	0	0	0	0	0	963
	09:00	4	222	512	93	6	0	0	0	0	0	0	0	0	0	0	837
	10:00	13	209	412	90	4	0	0	0	0	0	0	0	0	0	0	728
	11:00	31	132	369	105	7	0	0	0	0	0	0	.0	0	.0	0	644
	12:00	2	105	319	155	8	1	0	0	0	0	0	0	0	0	0	590
	13:00	7	108	343	126	· 11	0	0	0	0	0	0	0	0	0	0	595
	14:00	10	129	309	150	12	0	0	0	0	0	0	0	0	0	0	610
	15:00	2	98	375	159	20	0	0	0	0	0	0	0	0	0	0	654
	16:00	8	131	376		10	0	0	0	1	0	0	0	0	0	0	682
	17:00	8	131	400	123	5	0	0	0	0	0	0	0	0	0	0	667
	18:00	1	104	414	172	8	0	0	0	0	0	0	0	0	0	0	699
	19:00	16	95	309	148	14	1	0	0	0	0	0	0	0	0	0	583
	20:00	2	81	259	111	21	2	0	0	0	0	0	0	0	0	0	476
	21:00	0	54	283	111	10	0	- 0	0	0	0	0	0	0	0	0	458
	22:00	3	54	228	108	10	0	0	0	0	0	0	0	0	0	0	403
	23:00	0	30	131	112	18	1	<u>, , , , , , , , , , , , , , , , , , , </u>	. 0	0	0	0	0	0	0	0	293
	24:00	0	11	69	84	23	1	0	1	0	0	0	0	0	0	0	189
								- -				- -					10101
	AY TOTAL ERCENTS	179 1.5%	2432	6674 54.8%	2566 21.0%	308 2.5%	27 0.2%	2 0.0%	1 0.0%	2 0.0%	0 80.0	0.0%	0.08	0.0%	0.0%	0.0%	12191 100%

Statistical Information...

15th Percentile Speed 22.4 mph

Median Speed 26.6 mph

10 MPH Pace Speed 24 mph to 34 mph 9240 vehicles in pace Representing 75.7% of the total vehicles 85th Percentile Speed 31.1 mph

Average Speed 26.6 mph

MassDOT Highway Division SPEED SUMMARY Tue 4/3/2018

Page: 8

Site Reference: 180060000535

Site ID: 00000000501

Location: RTE.138 NO. OF BRUSH HILL RD.

Direction: NORTH

Lane: 1

File: SPD501.prn City: MILTON County: SPEED NB

									54	59	64	69	74	79	85	86+	Tota
										. 15	9						
	01:00	0	7	40	72	13	2	0	0	0	0	0	0	11 0	0	0	134
	02:00	0	. 2	12	33	14	0	0	0	0	0	0	0	0	0	0	61
	03:00	0	1	14	33	11	1	2	0	0	0	0	0	0	0	0	62
	04:00	0	0	11	31	12	1	1	0	0	0	0	. 0	0	0	0	56
	05:00	1	5	64	92	23	0	0	0	0	0	0	0	0	0	0	185
	06:00	24	111	360	164	16	4	:: 1	0	0	0	0	0	0	0	0	680
	07:00	17	241	547	116	16	1	-1	0	0	0	0	0	0	0	0	939
, N.	00:80	17	269	558	100	10	3	0	0	0	0	0	0 :	0	0	0	957
	09:00	9	360	454	113	6	2	0	0	0	0	0	0	0	0	0	944
	10:00	25	189	400	154	7	0	0	0	0	0	1	0	0	0	0	776
	TOTAL CENTS	 93 2.0%	1185 24.8%	2460 51.4%	908 18.9%	128 2.6%	14 0,2%	5 0.1%	0 0.0%	0.0%	0.0%	1 0.0%	0 0.0%	0.0%	0.0%	0.0%	4794

Statistical Information...

15th Percentile Speed 21.7 mph

Median Speed 26.3 mph

10 MPH Pace Speed 19 mph to 29 mph 3645 vehicles in pace Representing 76.0% of the total vehicles 85th Percentile Speed 30.9 mph

Average Speed 26.2 mph

MassDOT Highway Division SPEED SUMMARY Tue 3/27/2018

Page: 1

Site Reference: 180060000461 Site ID: 000000000502

Location: RTE.138 NO. OF BRUSH HILL RD.

Direction: SOUTH

Lane: 1

STA.5SB File: SPD502.prn City: MILTON

County: SPEED SB

TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
12:00	82	123	194	47	2	0	0	= 0	0	2	0	0	3	0	1	454
13:00	87	107	220	60	6	0	0	0	0	2	0	0	0	0	0	482
14:00	88	196	206	46	. 6	0	0	0	2	2	2	0	0	1	0	549
15:00	188	220	235	35	. 3	0	0	0	0	. 0	0	0	0	0	0	681
16:00	336	184	142	19	5	0	0	1	0	ି ୦	0	0	0	0	0	687
17:00	619	1	2	7	1	1	0	5	0	1	0	0	0	0	0	637
18:00	471	84	67	4	1	-0	0	0	0	0	0	0	. 0	0	0	627
19:00	181	231	239	29	1	0	0	0	0	0	0	. 0	0	0	Ō	681
20:00	37	189	264	53	4	0	1	0	0	0	0	0	0	0	2	550
21:00	58	169	217	42	2	0	0	Ō	0	0	0	0	0	0	0	488
22:00	40	139	180	54	1	1	0	0	0	0	Ō	0	0	0	0	415
23:00	12	59	158	61	9	0	0	0	Ö	Ō	Ö	Ō	Ō	Ō	1	300
24:00	6		116	56	9	25.51	0	0	0	Ö	0	0	Ö	0	0	217
DAY TOTAL	2205	1731	2240	513	50	3	1	 6	2	7	2			 1		6768
PERCENTS		25.6%		7.6%	0.8%	0.1%	0.1%	0.0%	0.0%	0.1%	0.0%	0.0%	0.0%	0.0%	0.0%	100%
121.021.10	52.00	20.00	00110		0.00	0.10		0.00	0.00	0.10	0.00	- 0.00	0.00	0.00		2000

Statistical Information...

15th Percentile Speed 8.8 mph

Median Speed 22.4 mph

10 MPH Pace Speed 19 mph to 29 mph 3971 vehicles in pace Representing 58.6% of the total vehicles 85th Percentile Speed 28.1 mph

Average Speed 20.3 mph

MassDOT Highway Division SPEED SUMMARY Wed 3/28/2018

SPEED SUMMARY Page: 2

File: SPD502.prn

County: SPEED SB

City: MILTON

Site Reference: 180060000461

Site ID: 00000000502

Location: RTE.138 NO. OF BRUSH HILL RD.

Direction: SOUTH

Lane: 1

TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
	·														14	
01:00	2	21	51	42	1	0	0	0	0	0	0	0	0	0	0	117
02:00	3	5	22	20	12	2	0	0	0	0	0	0	0	0	0	64
03:00	1	2	14	18	8	0	0	0	. 0	0	0	0	0	0	0	43
04:00	2	6	23	16	5	2	0	0	0	0	0	0	0	0	0	54
05:00	9	12	35	27	10	1	2	0	0	0	0	0	0	0	0	96
06:00	12	39	117	71	2	2	0	0	0	0	0	0	0	0	0	243
07:00	40	92	167	77	5	1	0	0	0	0	0	. 0	0	0	0	382
08:00	34	171	263	70	4	0	0	0	0	0	0	0	0	0	0	542
09:00	51	189	283	56	6	i 0	0	· 0	0	0	0	0	0	0	0	585
10:00	14	108	253	76	7	0	0	0	0	0	0	0	0	0	0	458
11:00	37	162	249	50	10	0	0	0	0	0	0	0	0	0	0	508
12:00	32	143	277	78	5	1	0	0	0	0	0	.0	0	0	0	536
13:00	59	183	297	60	10	0	0	. 0	0	0	0	0	0	0	0	609
14:00	48	194	287	58	4	0	0	0	0	0	· 0	0	0	0	0	591
15:00	227	187	180	50	3	0	0	0	0	0	0	0	-0	0	0	647
16:00	292	137	194	40	2	0	0	0	0	0	0	0	0	0	0	665
17:00	617	3	9	1	1	1	0	0	0	0	0	0	0	0	0	632
18:00	612	2	1	0	0	0	0	0	0	0	. 0	0	0	0	0	615
19:00	663	0	2	.0	0	0	0	2	0	0	0	0	0	0	0	667
20:00	108	268	237	34	0	3	0	0	0	0	0	0	0	0	0	650
21:00	22	229	285	38	.1	0	0	0	0	0	0	0	0	. 0	0	575
22:00	22	158	228	61	6	1	0	. 0	0	0	0	0	0	0	0	476
23:00	8	71	161	53	4	0	2	0	0	0	0	0	0	0	0	299
24:00	7	24	113	70	8	1	0	0	0	0	0	0	0	0	0	223
DAY TOTAL	2922		3748		114	15	4	2	0	0	0	0	0	0		
PERCENTS	28.5%	23.5%	36.5%	10.3%	1.1%	0.1%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 10.0 mph

Median Speed 23.6 mph

10 MPH Pace Speed 19 mph to 29 mph 6154 vehicles in pace Representing 59.8% of the total vehicles 85th Percentile Speed 28.6 mph

Average Speed 21.2 mph

MassDOT Highway Division SPEED SUMMARY Thu 3/29/2018

File: SPD502.prn

County: SPEED SB

City: MILTON

0.0%

Page: 3

Site Reference: 180060000461

Site ID: 000000000502

Location: RTE.138 NO. OF BRUSH HILL RD.

Direction: SOUTH

Lane: 1

TIME 7.4 86+ Tota 01:00 02:00 03:00 04:00 n 05:00 06:00 07:00 00:80 09:00 10:00 n n O O 11:00 12:00 13:00 14:00 15:00 n 16:00 17:00 18:00 O 19:00 20:00 a 21:00 O n 22:00 23:00 ń 24:00 3165 2507 3576 1037 2 10432 DAY TOTAL 80.0

Statistical Information...

PERCENTS

15th Percentile Speed 9.4 mph

Median Speed 23.1 mph

10 MPH Pace Speed 19 mph to 29 mph 6083 vehicles in pace Representing 58.3% of the total vehicles

1.1%

0.1%

0.0%

0.0%

30.4% 24.1% 34.3% 10.0%

85th Percentile Speed 28.5 mph

0.0% 0.0% 0.0%

Average Speed 20.8 mph

\$0.0 \$0.0

MassDOT Highway Division SPEED SUMMARY Fri 3/30/2018

File: SPD502.prn

County: SPEED SB

City: MILTON

Page: 4

Site Reference: 180060000461

Site ID: 00000000502

Location: RTE.138 NO. OF BRUSH HILL RD.

Direction: SOUTH

Lane: 1

TIME 86+ Tota 01:00 **B3** 02:00 03:00 04:00 05:00 0. 06:00 D O 07:00 08:00 09:00 10:00 n Λ n n n n n O O 11:00 12:00 n n n 13:00 14:00 15:00 16:00 17:00 18:00 Ð 19:00 20:00 O Ð 21:00 n O 22:00 23:00 24:00 3971 2459 0 10836 DAY TOTAL 0.0% 0.0% 0.0% 0.0% 0.0% 0.0% 0.0% 36.7% 22.7% 31.6% 8.0% 0.9% 0.1% 0.0% 0.0% PERCENTS

Statistical Information...

15th Percentile Speed 7.8 mph

Median Speed 22.0 mph

10 MPH Pace Speed
19 mph to 29 mph
5878 vehicles in pace
Representing 54.2% of the total vehicles

85th Percentile Speed 28.1 mph

Average Speed 19.7 mph

MassDOT Highway Division SPEED SUMMARY Sat 3/31/2018

File: SPD502.prn

County: SPEED SB

City: MILTON

Page: 5

Site Reference: 180060000461

Site ID: 000000000502

Location: RTE.138 NO. OF BRUSH HILL RD.

Direction: SOUTH Lane: 1

TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
															III,	
01:00	4	37	115	50	4	3	0	0	0	0	0	. 0	0	0	0	213
02:00	5	21	73	40	8	3	1	0	0	0	0	0	0	0	0	151
03:00	0	7	36	32	8	0	1	0	0	0	0	0	0	0	0	84
04:00	0	. 8	27	27	8	5	1	0	0	0	0	0	0	0	0	76
05:00	- 0	7	33	18	15	2	1	0	0	0	0	0	0	0	0	76
06:00	3	16	51	43	14	2	1	0	0	0	0	0	0	0	0	130
07:00	2	22	109	72	25	3	2	0	0	0	0	0	0	0	0	235
08:00	8	25	161	132	28	2	0	0	0	. 0	0	0	0	0	0	356
09:00	14	54	220	135	19	3	0	0	0	0	0	0	0	. 0	0	445
10:00	20	105	335	117	13	0	0	. 0	0	. 0	0	0	0	0	0	590
11:00	56	131	301	94	4	1	0	0	0	0	0	0	0	0	0	587
12:00	42	93	330	121	5	0	0	0	0	0	0	0	0	0	0	591
13:00	75	188	322	110	9	0	0	0	0	0	0	0	0	0	0	704
14:00	418	67	116	25	2	0	0	0	0	0	0	0	0	. 0	0	628
15:00	434	82	136	25	0	1	0	4	0	- 0	0 :	0	0	0	0	682
16:00	187	166	278	70	4	2	0	0	0	0	0	0	g 0	0	0	707
17:00	67	165	398	84	3	0	0	0	0	0	0	0	0	0	0	717
18:00	62	149	306	100	10	1	0	0	0	0	0	0	0	0	0	628
19:00	35	201	345	72	10	0	0	0	0	0	0	0	0	0	0	663
20:00	58	252	241	41	4	0	0	0	0	0	:: 0	0	0	0	0	596
21:00	42	232	202	25	5	· 1	0	0	0	0	0	0	0	0	0	507
22:00	14	116	206	51	5	0	1	0	0	0	0	0	0	0	0	393
23:00	14	109	208	40	2	0	○ 0	0	0	0	0	0	0	0	0	373
24:00	9	70	192	60	5	0	0	0	0	0	0	<u>:::</u> 0	0	<u> </u>	0	336
DAY TOTAL	1569	2323	4741	1584	210	29	В	4	0	0	0	0	0	0	0	10468
PERCENTS	15.0%	22.2%	45.3%	15.2%	2.1%	0.2%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	80.0	0.0%	100%

Statistical Information...

15th Percentile Speed 19.0 mph

Median Speed 25.4 mph

10 MPH Pace Speed 19 mph to 29 mph 7064 vehicles in pace Representing 67.4% of the total vehicles 85th Percentile Speed 29.8 mph

Average Speed 23.9 mph

MassDOT Highway Division SPEED SUMMARY Sun 4/1/2018

File: SPD502.prn City: MILTON

County: SPEED SB

- Page: 6

Site Reference: 180060000461

Site ID: 000000000502

Location: RTE.138 NO. OF BRUSH HILL RD.

Direction: SOUTH

Lane: 1

ngile. I																
TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
															4	
01:00	2	38	125	41	2	3	0	0	0	0	0	0	0	0	0	211
02:00	3	18	75	44	4	0	0	0	0	0	0	0	0	0	0	144
03:00	0	15	59	34	6	2	0	0	0	0	0	0	0	0	0	116
04:00	0	15	33	26	10	2	0	0	0	0	0	0	0	0	0	86
05:00	1.1	2	22	14	6	0	0	0	0	g 0	0	0	0	0	0	45
06:00	0	5	29	25	6	1	0	0	0	0	0	0	0	0	0	66
07:00	0	12	53	34	20	1	0	1	0	0	0	0	0	0	- 0	121
08:00	0	3	100	89	16	0	1	0	0	0	0	0	0	0	0	209
09:00	5	15	128	82	17	2	0	0	0	0	0	0	0	0	0	249
10:00	4	51	196	125	12	2	0	0	0	0	0	0	0	0	0	390
11:00	8	57	304	119	11	1	0	0	0	0	0	0	0	0	0	500
12:00	15	104	304	123	12	1	0	0	1	0	. 0	0	0	0	0	560
13:00	56	155	347	120	2	1	0	0	0	0	0	0	- 0	0	0	681
14:00	138	240	302	51	1	0	0	0	0	0	0	0	0	0	0	732
15:00	194	174	290	52	5	0	0	0	0	0	0	0	0-	0	0	715
16:00	90	245	322	79	6	0	0	0	0	0	0	0	0	0	0	742
17:00	76	189	321	74	5	0	0	0	- 0	0	0	0	0	0	0	665
18:00	24	152	345	113	8	. 0	0	0	0	0	0	. 0	0	0	0	642
19:00	65	168	344	74	3	0	0	0	0	0	0	0	0	0	0	654
20:00	47	255	300	43	5	0	0	0	0	0	0	0	0	0	0	650
21:00	26	213	281	53	1	0	0	0	0	Q	0	0	0	0	0	574
22:00	18	131	217	58	5	0	0	0	0	0	0	0	0	0	0	429
23:00	2	48	184	68	9	1	0	0	0	0	0	0	0	0	0	312
24:00	0	29	117	55	10	Ō	0	0	0	0	0	0	0	0	0	211
DAY TOTAL	774	2334	4798	 1596	-	17	 1	1	1	0	0	0	0	0	0	9704
PERCENTS				16.5%	1.8%	0.1%	0.0%	90.0	80.0	0.0%	0.0%	0.0%	90.0	0.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 20.5 mph

Median Speed 25.8 mph

10 MPH Pace Speed 19 mph to 29 mph 7132 vehicles in pace Representing 73.4% of the total vehicles 85th Percentile Speed 30.1 mph

Average Speed 25.0 mph

MassDOT Highway Division SPEED SUMMARY Mon 4/2/2018

File: SPD502.prn

County: SPEED SB

City: MILTON

Page: 7

Site Reference: 180060000461

Site ID: 00000000502

Location: RTE.138 NO. OF BRUSH HILL RD.

Direction: SOUTH

Lane: 1

TIME 86+ Tota 01:00 02:00 03:00 04:00 n n 05:00 06:00 07:00 08:00 09:00 10:00 11:00 n ብ 12:00 13:00 14:00 15:00 16:00 17:00 O 18:00 19:00 20:00 O Λ O 21:00 22:00 23:00 Λ 24:00 DAY TOTAL 4313 1401 0.0% 0.0% 0.0% 0.0% 0.0% 12.5% 24.2% 46.5% 15.0% 1.7% 0.1% 0.0% **PERCENTS**

Statistical Information...

15th Percentile Speed 19.5 mph

Median Speed 25.5 mph

10 MPH Pace Speed
19 mph to 29 mph
6552 vehicles in pace
Representing 70.5% of the total vehicles

85th Percentile Speed 29.7 mph

Average Speed 24.2 mph

MassDOT Highway Division SPEED SUMMARY Tue 4/3/2018

Page: 8

Site Reference: 180060000461

Site ID: 00000000502

Location: RTE.138 NO. OF BRUSH HILL RD.

Direction: SOUTH

Lane: 1

	TIME		19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
	01:00	0	 0	5	70	34	9		0	0	0	·	0		n	n	0	
	02:00		1	5	27	19	2	1	ñ	Ö	Ö	Ô	n	Ő	ő	ñ	ŏ	55
	03:00		ō	2	22	11	7	ō	ő	ő	Õ	ő	ŏ	ő	ŏ	Õ	Ö	42
	04:00		1	2	11	19	4	ŏ	ĭ	ŏ	ŏ	ŏ	ŏ	ŏ	ŏ	Ö	ŏ	38
	05:00		3	10	41	31	12	4	ō	Ō	0	Ö	Ō	0	0	Ō	0	101
	06:00		8	44	101	69	11	Ō	. 0	Ö	0	0	0	0	0	0	0	233
	07:00		11	45	198	91	11	0	0	0	0	0	0	0	0	0	0	356
	08:00		52	124	270	82	8	0	0	0	0	0	0	0	0	0	0	536
	09:00		150	128	238	58	5	0	0	0	0	0	0	0	0	0	0	579
	10:00		23	94	270	100	7	1	0	0	0	0	0	0	* 0	0	0	495
										54	ē i	30		111	53			6
DA	Y TOTAL		249	459	1248	514	76	6	1	0	0	0	0	0	0	0	0	2553
	ERCENTS			18.0%			2.9%	0.2%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 20.5 mph

Median Speed 26.3 mph

10 MPH Pace Speed 24 mph to 34 mph 1762 vehicles in pace

Representing 69.0% of the total vehicles

85th Percentile Speed 31.1 mph

Average Speed 25.3 mph

File: SPD502.prn City: MILTON County: SPEED SB

MassDOT Highway Division SPEED SUMMARY Tue 3/27/2018

Page: 1

574, 7NB

Site Reference: 180060000532

Site ID: 00000000701

Location: RTE. 138 NO. OF NEPONSET VALLEY PKWY

Direction: NORTH

Lane: 1

File: SPD8028_NB.prn City: MILTON County: SPEED NB

TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
										, in						X:
13:00	44	12	74	185	57	3	0	0	0	2	0	0	0	0	0	377
14:00	40	3	60	143	41	. 3	0	1	0	0	1	0	0	0	0	292
15:00	35	15	77	179	49	5	0	0	1	4	5	0	0	0	2	372
16:00	37	9	125	178	33	0	0	0	0	2	0	0	0	0	0	384
17:00	30	17	143	180	35	5	0	0	1	0	0	0	1	0	3	415
18:00	32	32	160	154	32	2	1	0	0	0	2	0	0	0	2	417
19:00	41	15	113	159	31	4	1	0	0	0	0	0	2	2	0	368
20:00	15	5	102	130	28	2	0	0	0	0	0	0	0	0	0	282
21:00	27	17	95	127	32	2	. 0	0	0	2	0	0	0	1	1	304
22:00	14	6	91	112	24	1	0	. 0	0	0	0	0	0	0	0	248
23:00	20	2	54	82	44	3	0	0	0	1	0	0	0	0	0	206
24:00	10	2	28	66	15	2	1	0	0	0	1	0	0	0	0	125
DAY TOTAL	345	135	1122	1695	421	32	3	1	2	11	. 9	0	3	3	8	3790
PERCENTS	େ 9.2%	3.6%	29.7%	44.8%	11.2%	0.9%	0.0%	0.0%	0.0%	0.2%	0.2%	90.0	0.0%	\$0.Q	0.2%	100%

Statistical Information...

15th Percentile Speed 24.4 mph

Median Speed 29.9 mph

10 MPH Pace Speed
24 mph to 34 mph
2817 vehicles in pace
Representing 74.3% of the total vehicles

85th Percentile Speed 33.8 mph

Average Speed 28.6 mph

Vehicles > 65 MPH 23 0.6%

MassDOT Highway Division SPEED SUMMARY Wed 3/28/2018

Page: 2

Site Reference: 180060000532 Site ID: 000000000701

Location: RTE. 138 NO. OF NEPONSET VALLEY PKWY

Direction: NORTH

Lane: 1

File: SPD8028_NB.prn City: MILTON County: SPEED NB

TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
								3								
01:00	9	4	16	36	12	1	0	1	0	0	0	0	0	0	0	79
02:00	2	0	5	14	7	1	0	0	1	0	0	0	0	0	0	30
03:00	2	1	11	8	5	1	2	0	0	0	0	0	0	0	0	= 30
04:00	2	0	6	9	7	2	1	_ 1	0	0	0	0	0	0	0	28
05:00	5	5	8	35	24	2	0	1	0	0	0	0	0	0	0	80
06:00	19	30	108	120	34	8	0	0	0	0	1	0	0	0	2	322
07:00	30	23	128	206	39	2	0	. 0	1	2	0	0	0	0	2	433
08:00	44	39	208	180	36	3	1	0	0	0	0	0	3	Ü	4	518
09:00	28	33	225	203	29	0	1	0	0	0	0	0	Ü	0	Ū	519
10:00	32	40	212	180	37	1	0	0	0	0	1	0	0	. 0	1	504
11:00	33	22	162	164	27	0	0	0	0	1	○ 0	1	0	0	U	410
12:00	35	5	78	153	43	1	0	1	0	. 0	5	2	0	0	0	323
13:00	35	В	107	147	50	1	1	0	0	0	0	0	Ü	0	2	351
14:00	38	В	82	196	55	2	0	0	0	. 0	3	2	ō	0	Ü	386
15:00	28	5	95	156	55	3	2	0	0	0	0	0	ō	0	U	344
16:00	46	20	128	183	44	0	0	0	0	3	0	0	Ţ	0	0	425
17:00	21	15	139	178	40	0	0	0	0	0	0	0	U	0	U	393
18:00	43	12	128	178	37	3	0	0	2	0	0	0	1	2	Ţ	407
19:00	24	30	161	160	17	1	0	0	0	0	0	0	U	0	0	393
20:00	21	15	143	154	23	2	0	0	0	0	0	0	0	0	0	358 333
21:00	11	25	114	138	38	5	0	0	0	0	2	0	0	0	0	308
22:00	17	12	93	149	30	5	0	0	0	0	2	0	_ I	. 0	0	
23:00	17	10	48	120	29	7	2	0	2	0	0	0	0	0	0	236 128
24:00	4	0	39	68	12	2	. 2	1	0	0	U	U	U	35	U	128
DAY TOTAL	546	362	2444	3135	730	53	12	 5	6	6	14	6	5	2	12	7338
PERCENTS	7.5%	5.0%			10.0%	0.8%	0.2%	0.1%	0.0%	0.0%	0.1%	0.0%	0.0%	0.0%	0.1%	100%
a	*- £													10		

Statistical Information...

15th Percentile Speed 24.4 mph

Median Speed 29.5 mph

10 MPH Pace Speed 24 mph to 34 mph 5579 vehicles in pace Representing 76.0% of the total vehicles 85th Percentile Speed 33.6 mph

Average Speed 28.5 mph

Vehicles > 65 MPH 39 0.5%

MassDOT Highway Division SPEED SUMMARY Thu 3/29/2018

Page: 3

File: SPD8028_NB.prn City: MILTON

County: SPEED NB

Site Reference: 180060000532

Site ID: 00000000701

Location: RTE. 138 NO. OF NEPONSET VALLEY PKWY

Direction: NORTH

Lane: 1

TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
01:00	9	0	18	30	22	4	1	0	0	0	0	0	0	0	0	84
02:00	3	Ō	11	13	6	1	0	1	Ó	Ó	0	0	0	0	Ó	35
03:00	1	0	9	9	12	11	1	0	0	0	0	0	0	0	0	33
04:00	1	1	5	13	9	1	0	0	0	0	0	0	0	0	0	30
05:00	3	2	6	27	17	5	3	0	0	0	0	0	0	0	. 0	63
06:00	16	23	97	123	40	4	. 0	0	0	0	0	0	0	0	0	303
07:00	48	28	159	187	38	8	0	0	3	0	0	0	0	0	0	471
08:00	43	46	189	166	33	2	0	0	1	3	0	0	0	2	1	486
09:00	.57	51	198	191	40	1	0	0	0	0	2	2	0	. 0	2	544
10:00	51	32	157	207	31	0	3	0	1	2	. 2	0	0	0	0	486
11:00	38	30	145	194	36	4	1	1	0	0	0	0	0	0	0	449
12:00	27	14	124	149	52	. 3	1	1	0	0	0	0	0	0	0	371
13:00	32	7	105	166	36	0	0	0	0	0	0	0	0	0	0	346
14:00	15	13	114	194	33	3	0	1	0	0	0	0	0	0	0	373
15:00	25	17	114	179	50	- 3	2	. 0	0	1	0	0	0	0	0	391
16:00	28	28	180	212	29	1	0	- O	0	0	0	0	1	0	1	480
17:00	21	26	162	208	37	. 0	0	3	0	0	0	0	0	0	1	458
18:00	26	33	150	221	38	2	0	0	0	0	0	0	0	* 0	° 0	470
19:00	a 31	38	206	151	17	1	0	0	1	0	. 0	0	0	0	2	447
20:00	22	13	158	120	20	4	0	0	0	0	0	0	0	0	0	337
21:00	11	17	123	147	29	2	1	0	0	0	0	0	0	0	0	330
22:00	13	18	113	121	28	0	. 0	0	0	0	0	0	0	0	0	293
23:00	5	6	76	108	29	3	0	. 0	0	0	0	0	0	0	0	227
24:00	7	8	38	82	32	1	0	0	0	0	0	0	0	0	0	168
DAY TOTAL	533	451	2657	3218	 	 54	13	 7	6	. 6	4	2	1	2	 7	7675
PERCENTS	7.0%		34.7%		9.4%	0.8%	0.2%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 24.3 mph

Median Speed 29.3 mph

10 MPH Pace Speed 24 mph to 34 mph 5875 vehicles in pace Representing 76.5% of the total vehicles 85th Percentile Speed 33.5 mph

Average Speed 28.3 mph

Vehicles > 65 MPH 16 0.2%

MassDOT Highway Division SPEED SUMMARY Fri 3/30/2018

Page: 4

File: SPD8028 NB.prn

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City: MILTON

County: SPEED NB

Site Reference: 180060000532

Site ID: 000000000701

Location: RTE. 138 NO. OF NEPONSET VALLEY PKWY

Direction: NORTH

Lane: 1

16:00

17:00

18:00

19:00 20:00

21:00

22:00

23:00

24:00

TIME 64 69 86+ Tota 01:00 n 02:00 03:00 0 -04:00 : 3 05:00 06:00 07:00 - 2 08:00 09:00 Ω 10:00 11:00 Λ O 12:00 13:00 14:00 15:00 O

511 537 2883 3258 5 7992 DAY TOTAL 6.8% 36.1% 40.8% 9.1% 0.6% 0.1% 0.1% 0.0% 0.0% 0.0% 0.0% 0.0% 0.0% 0.0% 100% PERCENTS 6.4%

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Statistical Information...

15th Percentile Speed 24.3 mph

Median Speed 29.1 mph

10 MPH Pace Speed
24 mph to 34 mph
6141 vehicles in pace
Representing 76.8% of the total vehicles

85th Percentile Speed 33.4 mph

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Average Speed 28.3 mph

Vehicles > 65 MPH 0.2%

File: SPD8028 NB.prn

City: MILTON

County: SPEED NB

MassDOT Highway Division SPEED SUMMARY Sat 3/31/2018

Site Reference: 180060000532

Site ID: 000000000701

Location: RTE. 138 NO. OF NEPONSET VALLEY PKWY

Direction: NORTH

Lane: 1

TIME 86+ Tota 01:00 02:00 03:00 O n 04:00 05:00 11. 06:00 07:00 08:00 09:00 10:00 11:00 n O O Ð n 12:00 13:00 14:00 . 2 15:00 16:00 В n Ω O 17:00 18:00 19:00 20:00 n n n n n O 21:00 22:00 23:00 24:00

. 0

0.0% 0.0% 0.0% 0.0%

1.6% 0.1% 0.0% 0.0%

Statistical Information...

DAY TOTAL

PERCENTS

15th Percentile Speed 25.3 mph

6.9%

Median Speed 30.8 mph

10 MPH Pace Speed
24 mph to 34 mph
5179 vehicles in pace
Representing 72.4% of the total vehicles

163 1673 3506 1182

2.3% 23.4% 49.1% 16.6%

85th Percentile Speed 35.0 mph

100%

0.0%

Average Speed 29.7 mph

File: SPD8028 NB.prn

City: MILTON

County: SPEED NB

MassDOT Highway Division SPEED SUMMARY Sun 4/1/2018

Site Reference: 180060000532

Site ID: 00000000701

Location: RTE. 138 NO. OF NEPONSET VALLEY PKWY

Direction: NORTH

Lane: 1

TIME 86+ Tota 01:00 02:00 03:00 04:00 05:00 06:00 07:00 08:00 n 09:00 10:00 11:00 12:00 13:00 14:00 15:00 16:00 17:00 18:00 19:00 O Λ 20:00 21:00 ß n ß n 22:00 23:00 24:00 O Ð 465 231 1851 3305 1116 107 22 0 3 1 0 0 0 3 7104 DAY TOTAL PERCENTS

Statistical Information...

15th Percentile Speed 25.0 mph

Median Speed 30.5 mph

10 MPH Pace Speed
24 mph to 34 mph
5156 vehicles in pace
Representing 72.5% of the total vehicles

85th Percentile Speed 34.8 mph

Average Speed 29.4 mph

MassDOT Highway Division SPEED SUMMARY Mon 4/2/2018

Page: 7

File: SPD8028_NB.prn

City: MILTON

County: SPEED NB

Site Reference: 180060000532

Site ID: 00000000701

Location: RTE. 138 NO. OF NEPONSET VALLEY PKWY

Direction: NORTH

Lane: 1

TIME 86+ Tota 01:00 02:00 03:00 В n 04:00 05:00 06:00 07:00 08:00 n 09:00 . 0 10:00 11:00 Λ 12:00 13:00 14:00 15:00 16:00 40B 17:00 24. 40B 18:00 19:00 20:00 21:00 22:00 23:00 24:00

Statistical Information...

DAY TOTAL

PERCENTS

15th Percentile Speed 24.9 mph

Median Speed 30.1 mph

10 MPH Pace Speed
24 mph to 34 mph
5413 vehicles in pace
Representing 76.5% of the total vehicles

251 2117 3296

85th Percentile Speed 33.9 mph

0 0 7075

Average Speed 29.1 mph

- 4 0

MassDOT Highway Division SPEED SUMMARY Tue 4/3/2018

Page: 8

Site Reference: 180060000532

Site ID: 000000000701

Location: RTE. 138 NO. OF NEPONSET VALLEY PKWY

Direction: NORTH

Lane: 1

rane. I							9									
TIME	19	24	29	3,4	39	44	49	54	59	64	69	74	79	85	86+	Tota
01:00	4	0	19	50	12	1	0	0	0	0	0	0	0	0	0	86
02:00	ਂ 0	0	8	20	9	0	0	0	0	0	0	0	0	0	0	37
03:00	0	0	9	21	7	2	0	1	0	0	0	0	0	0	0	40
04:00	1	0	5	14	16	3	1	0	0	0	0	0	0	0	0	40
05:00	1	4	9	36	22	2	# 1	0	0	0	· 0	0	0	0	0	75
06:00	20	4	74	153	57	6	0	3	0	0	2	0	0	0	0	319
07:00	26	19	163	238	52	7	0	Ō	0	Ó	2	2	0	0	0	509
08:00	37	40	186	197	44	2	Ö	Ō	1	0	0	2	0	2	1	512
09:00	55	56	204	193	51	6	Ō	Ō	ō	2	Ö	Ö	Ö	0	0	567
10:00	36	36	140	225	61	3	Ö	Ŏ	Ö	ō	Ō	Ō	1	Ō	0	502
DAY TOTAL	180	159	817	1147	331	32	2	4	1	2	4	4	1	2	1	2687
PERCENTS	6.7%	6.0%	30.5%	42.7%	12.4%	1.2%	0.1%	0.2%	0.0%	0.0%	0.1%	0.1%	0.0%	90.0	0.0%	100%

Statistical Information...

15th Percentile Speed 24.4 mph

Median Speed 29.8 mph

10 MPH Pace Speed
24 mph to 34 mph
1964 vehicles in pace
Representing 73.0% of the total vehicles

85th Percentile Speed 33.9 mph

Average Speed 28.9 mph

File: SPD8028_NB.prn

City: MILTON County: SPEED NB

Vehicles > 65 MPH 12 0.4%

MassDOT Highway Division SPEED SUMMARY Tue 3/27/2018

STA.75B

Site Reference: 180060000822 Site ID: 000000000702

Location: RTE. 138 NO. OF NEPONSET VALLEY PKWY

Direction: SOUTH

Lane: 1

File: SPD8028.prn City: MILTON County: SPEED SB

	TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	.85	86+	Tota
	13:00	0	0	10	43	138	124	50	18	0	0	0	0	0	0	0	383
	14:00	0	0	26	71	131	146	62	6	0	0	0	. 0	0	0	0	442
	15:00	0	5	8	79	218	217	58	8	2	0	0	0	0	. 0	0	595
4	16:00	92	24	36	89	150	122	43	4	1	0	0	0	0	0	2	563
	17:00	17	21	66	116	138	86	15	3	0	0	0	0	0	0	0	462
	18:00	37	35	48	63	139	118	34	9	0	0	0	0	0	0	0	483
	19:00	0	1	20	93	214	145	31	4	. 0	0	0	0	0	0	0	508
	20:00	0	0	5	34	217	152	37	3	0	1	0	0	0	0	0	449
	21:00	0	0	3	26	205	145	37	9.	0	0	0	0	0	0	0	425
2	22:00	0	0	2	60	138	115	30	8	1	0	0	0	0	0	0	354
	23:00	1	1	0	10	65	115	45	8	4	0	0	0	0	0	0	249
	24:00	Ō	Ō	0	2	43	73	31	11	1	0	0	0	0	0	0	161
		 				1706		473									
	Y TOTAL RCENTS	147 2.9%	87 1.8%	224 4.5%	686 13.6%	1796 35.4%		473 9.3%	91 1.7%	9 0.1%	0.0%	0 0.0%	0.0%	0.0%	0.0%	2 0.0%	5074 100%

Statistical Information...

15th Percentile Speed 31.2 mph

Median Speed 37.9 mph

10 MPH Pace Speed 34 mph to 44 mph 3354 vehicles in pace Representing 66.1% of the total vehicles 85th Percentile Speed 43.4 mph

Average Speed 37.1 mph

Vehicles > 65 MPH 0.0%

Page: 1

MassDOT Highway Division SPEED SUMMARY Wed 3/28/2018

File: SPD8028.prn

County: SPEED SB

City: MILTON

Site Reference: 180060000822

Site ID: 000000000702

Location: RTE. 138 NO. OF NEPONSET VALLEY PKWY

Direction: SOUTH

Lane: 1

							1.74			65						
TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
			ġ.													
01:00	0 -	0	0	2	12	33	22	2	2	1	0	0	0	0	0	74
02:00	0	0	0	1	7	9	12	8	3	1	0	0	0	0	0	41
03:00	. 0	0	0	1	0	6	4	6	4	1	0	0	0	0	0	22
04:00	0	0	0	1	6	3	16	7	1	1	0	0	0	0	0	35
05:00	0	0	0	2	10	22	18	8	5	1	0	0	0	0	0	66
06:00	0	0	1	4	32	82	35	14	3	0	0	0	0	0	0	171
07:00	0	1	6	19	99	125	66	22	1	0	0	0	0	0	0	339
08:00	_ 0	0	6	55	182	177	68	12	0	. 0	0	0	0	0	0	500
09:00	0	0	4	72	215	170	53	5	1	0	. 0	0	0	0	0	520
= 10:00	0	. 0	1	43	128	128	57	6	1	0	0	0	63 O	0	0	364
11:00	0	5	6	61	135	127	42	10	0	0	0	0	0	0	0	386
12:00	0	0	5	50	136	140	49	11	0	1	0	0	0	0	0	392
13:00	0	1	7	54	181	198	62	17	0	. 0	0	0	0	0	0	520
14:00	1	0	4	68	142	164	58	9	0	0	0	0	0	0	0	446
15:00	1	1	14	78	208	160	46	12	0	0	0	0	0	0	0	520
16:00	29	25	35	91	149	117	51	7	2	0	0	0	0	0	0	506
17:00	15	36	88	141	131	34	10	1	0	0	0	0	0	0	0	456
18:00	119	28	61	88	91	47	. 9	3	2	0	2	0	0	0	2	452
19:00	236	79	85	63	39	4	4	1	0	0	1	0	0	0	0	512
20:00	· 0	1	8	63	226	167	43	5	1	2	0	0	0	0	0	516
21:00	0	. 0	4	62	219	152	25	4	2	0	0	0	0	0	0	468
22:00	0	0	0	41	176	146	38	3	2	0	0	1	0	0	0	407
23:00	0	0	0	11	60	116	44	10	0	0	0	0	0	0	0	241
24:00	0	0	0	0	21	70	61	13	5	0	1	1	0	0	0	172
DAY TOTAL	401	177	335	1071	2605	2397	 893	196	 35	8	4	2	0	0	2	8126
PERCENTS	5.0%	2.2%	4.2%	13.2%	32.1%	29.5%	11.0%	2.4%	0.4%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 30.4 mph

Median Speed 38.0 mph

10 MPH Pace Speed
34 mph to 44 mph
5002 vehicles in pace
Representing 61.5% of the total vehicles

85th Percentile Speed 43.8 mph

Average Speed 36.9 mph

MassDOT Highway Division SPEED SUMMARY Thu 3/29/2018

File: SPD8028.prn

County: SPEED SB

City: MILTON

Page: 3

Site Reference: 180060000822

Site ID: 000000000702

Location: RTE. 138 NO. OF NEPONSET VALLEY PKWY

Direction: SOUTH

Lane: 1

nunc. 1																
TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
7.7.														!	35	
01:00	0	1	0	1	21	40	34	9	2	0	0	0	0	0	0	108
02:00	0	.0	0	1	5	19	13	5	2	1	0	0	0	0	0	46
03:00	0	0	0	1	6	7	11	7	1	0	0	0	1	0	0	34
04:00	0	0	0	1	4	4	9	6	2	1	1	0	0	0	0	28
05:00	0	0	0	1	3	20	18	15	5	1	0	0	0	0	0	63
06:00	0	0	0	1	22	56	49	22	6	1	1	0	0	0	0	158
07:00	1	0	0	17	68	147	91	9	2	0	0	0	0	0	0	335
08:00	1	0	17	51	152	177	62	10	0	0	0	Ω	0	0	0	470
09:00	0	0	8	52	112	202	81	20	1	0	0	0	0	0	0	476
10:00	0	0	13	50	143	138	56	7	0	0	1	0	0	0	0	408
11:00	0	1	8	60	101	127	49	8	1	0	0	0	0	. 0	0	355
12:00	0	4	16	56	165	150	42	7	2	1	0	0	0	. 0	0	443
13:00	3	2	11	63	231	161	41	4	1	0	0	0	0	0	0	517
14:00	9	3	14	71	157	152	40	6	1	0	0	0	0	0	0	453
15:00	1	2	22	95	220	181	33	5	3	1	0	0	0	0	0	563
16:00	4	1	34	89	225	144	32	7	. 0	0	0	0	0	0	0	536
17:00	33	24	29	81	158	95	29	1	0	0	0	0	0	0	0	450
18:00	34	17	58	93	172	79	27	2	0	0	0	0	0	0	0	482
19:00	465	13	5	10	0	. 0	0	2	0	0	2	0	0	0	0	497
20:00	18	0	12	101	239	126	24	2	0	0	0	0	- O	0	0	522
21:00	0	0	1	81	216	114	46	3	2	0	0	0	0	0	0	463
22:00	0	0	7	34	173	120	43	4	0	0	0	0	0	0	0	381
23:00	. 0	0	0	30	108	99	31	5	2	0	0	0	0	0	0	275
24:00	0	0	0	6	63	96	38	9	1	0	0	0	0	0	0	213
DAY TOTAL	569	68	255	1046	2764	2454	899	175	34	6	5	0	1	0	0	8276
PERCENTS	6.9%	0.9%	3.1%	12.7%	33.4%	29.7%	10.8%	2.1%	0.4%	0.0%	0.0%	0.0%	0.0%	9.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 30.7 mph

Median Speed 38.0 mph

10 MPH Pace Speed
34 mph to 44 mph
5218 vehicles in pace
Representing 63.0% of the total vehicles

85th Percentile Speed 43.8 mph

Average Speed 36.6 mph

MassDOT Highway Division SPEED SUMMARY Fri 3/30/2018

SPEED SUMMARY Page: 4

File: SPD8028.prn City: MILTON County: SPEED SB

Site Reference: 180060000822

Site ID: 00000000702

Location: RTE. 138 NO. OF NEPONSET VALLEY PKWY

Direction: SOUTH

Lane: 1

TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
01:00	0 mm	0	0	4	32	58	21	11	3	0	0	0	0	0	0	129
02:00	ň	ŏ	ŏ	3	15	17	11	2	3.	ō	ō	ŏ	ŏ	Õ	ō	51
03:00	- ŏ	ŏ	ō	9	- 9	12	19	9	3	ō	i ō	Ŏ	ŏ	ŏ	ŏ	61
04:00	ō	ō	ō	ī	6	7	17	.8	ĭ	Ŏ	ő	Õ	ŏ	ō	ŏ	40
05:00	ō	Ö	ŏ	3	ž	16	21	17	6	۱	ŏ	ō	ō	Ŏ	Ō	71
06:00	ŏ	Ŏ	2	8	19	44	45	25	5-	Ō	ō	Ŏ	Ŏ	Ŏ	. 0	148
07:00	ō	ō	ō	4	73	150	50	18	3	1	0	0	0	0	0	299
08:00	3	. 3	18	ii 53	148	119	57	16	0	0	. 0	0	0	0	0	417
09:00	Ō	0	22	68	153	159	59	6	0	0	0	0	0	0	0	467
10:00	Ö	11	12	60	135	140	40	4	1	0	0	0	0	0	0	403
11:00	0	0	22	59	167	145	49	13	1	0	0	0	0	0	0	456
12:00	0	1	4	49	189	172	56	4	2	1	0	0	0	0	0	478
13:00	. 0	3	17	97	253	167	34	4	0	0	0	0	0	0	0	575
14:00	17	9	44	116	241	136	37	4	0	0	0	0	0	0	0	604
15:00	442	27	18	13	В	§ 3	2	3	0	1	0	1	0	0	0	518
16:00	471	2	2	4	0	1	4	0	0	0	0	0	0	0	0	484
17:00	407	2	2	12	4	6	0	3	0	0	0	0	0	0	0	436
18:00	292	17	35	27	40	9	0	2	0	0	0	2	2	1	1	428
19:00	0	6	24	66	194	156	30	5	1	0	0	0	0	0	0	482
20:00	0	0	4	86	222	135	26	2	0	0	0	0	0	0	0	475
21:00	0	0	4	66	185	114	28	1	1	0	0	0	0	0	0	399
22:00	0	0	12	52	167	144	28	9	1	0	0	0	0	0	0	413
23:00	0	0	1	45	132	124	49	6	1	0	0	0	0	. 0	0	358
24:00	0	0	0	17	88	95	42	14	1	0	0	0	0	. 0	0	257
DAY TOTAL	1632	81	243	922	2487	2129	725	186	33	4	0	3	2	1	1	8449
PERCENTS	19.4%	1.0%	2.9%	11.0%	29.5%	25.2%	8.5%	2.2%	0.3%	0.0%	0.0%	80.0	0.0%	0.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 14.8 mph

Median Speed 36.7 mph

10 MPH Pace Speed
34 mph to 44 mph
4616 vehicles in pace
Representing 54.6% of the total vehicles

85th Percentile Speed 43.3 mph

Average Speed 32.9 mph

MassDOT Highway Division SPEED SUMMARY Sat 3/31/2018

File: SPD8028.prn City: MILTON

County: SPEED SB

Page: 5

Site Reference: 180060000822

Site ID: 00000000702

Location: RTE. 138 NO. OF NEPONSET VALLEY PKWY

Direction: SOUTH

Lane: 1

	71															
TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
	9															ii
01:00	0	1	0	15	40	64	34	7	0	0	0	0	0	0	. 0	161
02:00	0	0	0.0	5	29	48	27	11	. 2	2	0	0	0	0	0	124
03:00	0	0	0	0,	14	19	16	8	1	2	0	0	0	0	0	60
04:00	0	0	0	1	6	17	19	8	6	1	1	.0	0	0	0	59
05:00	0	0	0	0	6	15	13	11	5	0	1	0	0	0	0	51
06:00	0	0	0	1	8	27	28	14	3	1	0	0 :	0	0	0	82
07:00	0	0 ;	0	3	34	53	40	28	4	1	1	0	0	0	0	164
08:00	1	0	0	5	43	119	67	29	- 8	3	0	0	0	0	0	275
09:00	0	-0	2	17	83	121	95	25	3	0	0	0	0	0	0	346
10:00	0	0	0	17	141	174	81	13	3	0	0	0	0	0	0	429
11:00	0 :	0	0	13	124	198	94	19	0	0	0	0	0	0	0	448
12:00	3	0	6	19	139	181	75	10	2	2	0	0	0	0	0	437
13:00	0	1	0	33	181	226	91	15	2	1	0	0	0	0	0 :	550
14:00	3	6	15	81	209	182	44	4	0	2	0	0	0	0	0	546
15:00	1	2	21	95	185	148	59	10	1	1	0	0	0	0	0	523
16:00	0	0	1	68	185	229	71	7	2	0	0	0	0	0	0	563
17:00	0	1	3	38	188	233	71	14	2	0	0	0	0	0	0	550
18:00	0	0	0	58	185	171	67	9	1	. 0	0	0	0	0	0	491
19:00	0	1	5	70	221	161	42	7	1	0	0	0	0	0	0	508
20:00	Ō	0	3	55	241	135	41	8	0	0	0	0	. 0	0	0	483
21:00	0	0	4	74	185	113	25	7	0	1	0	0	0	0	· 0	409
22:00	Ó	0	Ō	22	106	140	38	5	0	0	0	0	0	0	0	311
23:00	ō	0	2	32	131	116	39	6	Ō	Ō	Ó	0	0	0	0	326
24:00	Ō	0	3	16	84	113	50	7	0	0	0	0	0	1	0	274
9		, m														
DAY TOTAL	8	12	65	738		3003	1227	282	46	17	3	0	0	1	0	8170
PERCENTS	0.1%	0.2%	0.8%	9.1%	33.9%	36.8%	15.0%	3.4%	0.5%	0.2%	0.0%	90.0	0.0%	0.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 34.7 mph

Median Speed 39.8 mph

10 MPH Pace Speed
34 mph to 44 mph
5771 vehicles in pace
Representing 70.6% of the total vehicles

85th Percentile Speed 45.4 mph

Average Speed 40.0 mph

MassDOT Highway Division SPEED SUMMARY Sun 4/1/2018

File: SPD8028.prn

County: SPEED SB

City: MILTON

Page: 6

Site Reference: 180060000822

Site ID: 000000000702

Location: RTE. 138 NO. OF NEPONSET VALLEY PKWY

Direction: SOUTH

Lane: 1

TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
											190					
01:00	0	0	0	9	38	86	37	3	1	0	0	. 0	0	0	0	174
02:00	0	0	0	3	25	42	33	7	0	0	0	0	0	0	0	110
03:00	0	0	2	0	15	43	21	6	3	1	0	0	0	0	0	91
04:00	. 0	0	0	2	16	16	21	4	3	1	1	0	0	0	0	64
05:00	0	0	0	1	2	15	10	10	4	0	0	0	0	0	0	42
06:00	0	0	1	1	В	11	17	9	1	0	0	0	0	0	0	48
07:00	0	0	0	1	13	24	22	16	7	0	0	0	0	0	0	83
08:00	0	0	0	0	27	61	45	16	5	0	2	.0	0	0	0	156
09:00	0	0	0	3	40	80	48	25	2	0	0	. 0	0	0	0	198
10:00	0	0	2	7	60	151	64	18	0	0	. 0	0	0	0	0	302
11:00	. 0	0	0	14	115	151	77	13	1	0	0	0	0	0	0	371
12:00	· 0	0	0	17	131	167	72	35	1	1	0	0	0	0	0	424
13:00	0	0	1	52	194	200	61	15	2	0	0	0	0	0	0	525
14:00	0	0	10	50	276	188	53	6	3	0	0	1	0	0	. 0	587
15:00	0	0	3	70	274	202	48	4	0	0	0	0	0	0	0	601
16:00	0	0	17	83	255	168	41	7	0	0	0	0	0	0	0	571
17:00	0	0	0	40	219	182	72	13	0	0	0	0	0	0	0	526
18:00	0	0	3	33	152	210	70	13	1	0	0	0	0	0	0	482
19:00	0	0	9	52	186	172	63	7	2	0	0	0	0	0	0	491
20:00	0	0	2	78	232	179	37	3	0	0	0	0	0	0	0	531
21:00	0	0	4	82	246	134	26	6	0	0	0	0	0	0	0	498
22:00	0	0	14	32	140	128	37	7	0	0	0	0	0	0	0	358
23:00	0	0	1	9	97	106	46	6	1	0	0	. 0	0	0	0	266
24:00	0	- 0	0	4	51	85	28	10	5	0	0	0	0	0	0	183
370													40		(6)	٤,
DAY TOTAL	0	0	69	643	2812			259	42	3	3	1	0	0	0	7682
PERCENTS	0.0%	0.0%	0.9%	8.4%	36.7%	36.5%	13.7%	3.3%	0.5%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 34.8 mph

Median Speed 39.6 mph

10 MPH Pace Speed
34 mph to 44 mph
5613 vehicles in pace
Representing 73.0% of the total vehicles

85th Percentile Speed 45.0 mph

Average Speed 39.8 mph

MassDOT Highway Division SPEED SUMMARY Mon 4/2/2018

Page: 7

File: SPD8028.prn City: MILTON

County: SPEED SB

Site Reference: 180060000822 Site ID: 000000000702

Location: RTE. 138 NO. OF NEPONSET VALLEY PKWY

Direction: SOUTH

Lane: 1

TIME	19	24	29	34	:39	44	49	54	59	64	69	74	79	85	86+	Tota
01:00					21	- 43	30	7		1	1	0	0	0	0	106
02:00	0	0	0	1	6	14	16	5	2	1	1	0	ő	Ö	Ö	45
03:00	0	Ö	Ö	ő	5	9	6	6	2	1		0	1	ŏ	Ô	30
04:00	0	ő	20 0	0	3	9	15	10	1	Ď	ŏ	ő	ñ	ō	Ö	38
05:00	ŏ	0	0	3	5	8	8	14	1	ő	ŏ	ŏ	Õ	Õ	Õ	42
06:00	Ŏ	0	2	2	21	61	52	17	2	1	ő	1	ñ	Ô	Ö	159
07:00	=1	ŏ	ō	4	50	131	76	28	- 5	ō	ŏ	ō	Ö	Ô	ŏ	295
08:00	ñ	Ö	5	42	186	170	66	7	ñ	n	Õ	ñ	ň	ň	ő	476
09:00	ŏ	Ö	16	64	194	142	61	5	ñ	ñ	Õ	ő	ŏ	ñ	ŏ	482
10:00	Õ	ŏ	- 4	18	110	160	55	19	ŏ	- 1	Ö	Õ	ă	Õ	ő	367
11:00	Õ	2	14	51	103	107	51	9	1	0	ŏ	Ď	ă	Ŏ	ŏ	338
12:00	ŏ	. 0	3	28	108	151	72	10	2	y 1	Ŏ	- ŭ	o .	. Ŏ:	Ō	375
13:00	Õ	ŏ	S 1	25	115	170	59	10	3	0	Ŏ	Ō	Ŏ	0	Ŏ	383
14:00	1	ŏ	18	48	190	120	65	11	ō	í	ŏ	Ŏ	Ŏ	ō	ō	454
15:00	ō	3	24	95	218	177	38	9	2	ō	ō	Ō	70 O	Ö	ō	566
16:00	ŏ	5	15	76	240	166	43	7	1	ō	. 0	Ö	Ö	0	Ö	553
17:00	ŏ	ī	7	45	170	191	46	11	ō	Ŏ	Ö	Ŏ	Ŏ	Ō	Ō	471
18:00	ŏ	· 6	13	63	173	180	51	10	ĺ	Ŏ	Ō	Ō	Ō	Ō	Ō	497
19:00	ō	ō	3	63	143	182	61	4	0	Ō	0	Ō	0	0	0	456
20:00	Ō	Ó	Ō	47	174	129	33	4	2	0	:0	0	0	0	0	389
21:00	0	0	0	19	138	159	31	8	1	0	0	0	0	0	0	356
22:00	- 0	0	0	23	129	112	28	5	1	0	0	0	0	0	0	298
23:00	0	0	4	14	71	93	33	10	0	0	0	0	= O	0	0	225
24:00	0	0	1	11	30	83	29	13	1	0	0	0	0	0	0	168
					2603	0767	1005								0	7560
DAY TOTAL PERCENTS	2 0.1%	17 0.3%	130 1.8%	742 9.9%	2603 34.4%	2767 36.5%		239 3.1%	32 0.4%	o.०%	4 0.0%	1 0.0%	0.0%	0.0%	0.0%	7569 100%

Statistical Information...

15th Percentile Speed 34.5 mph

Median Speed 39.5 mph

10 MPH Pace Speed 34 mph to 44 mph 5370 vehicles in pace Representing 70.9% of the total vehicles 85th Percentile Speed 44.9 mph

Average Speed 39.6 mph

MassDOT Highway Division SPEED SUMMARY Tue 4/3/2018

Page: 8

Site Reference: 180060000822

Site ID: 000000000702

Location: RTE. 138 NO. OF NEPONSET VALLEY PKWY

Direction: SOUTH

Lane: 1

	TIME		19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
	01:00		0	0	0	3	21	38	25	7	2	0	0	0	0	0	0	96
	02:00		0	0	0	2	4	13	13	3	2	1	0	0	0	0	0	38
	03:00		0	0	0	0	4	15	6	4	3	.0	1	0	0	0	0	33
	04:00	554	0	1	0	. 0	3	4	8	5	1	2	0	0	- 0	. 0	0	24
	05:00		0	0	0	4	11	17	20	9	6	0	1	0	0	0	0	68
	06:00		1	1	0	2	26	64	47	19	3	1	0	0	0	0	0	164
	07:00		3	0	0	6	96	135	64	14	2	0	0	0	0	0	0	320
	08:00		0	1	10	67	195	185	69	14	1	0	0	0	0	0	0	542
	09:00		0	0	12	68	175	176	68	5	1	- 0	0	- 0	0	0	0	505
	10:00		0	0	6	31	134	151	49	13	0	1	0	0	0	0	0	385
DA:	Y TOTAL			a	28	183	669	 798	 369	93	 21	5	2			0		2175
	RCENTS		0.2%	0.2%	1.3%		30.8%			4.2%	0.9%	0.2%	0.0%	0.0%	0.0%	0.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 34.8 mph

Median Speed 40.3 mph

10 MPH Pace Speed
34 mph to 44 mph
1467 vehicles in pace
Representing 67.4% of the total vehicles

85th Percentile Speed 46.2 mph

Average Speed 40.3 mph

File: SPD8028.prn

County: SPEED SB

City: MILTON

MassDOT Highway Division SPEED SUMMARY Tue 3/27/2018

Site Reference: 180060000787

Site ID: 00000001301

Location: RTE 138 NORTH OF BRADLEE RD. Direction: NORTH

Lane: 1

STA.13 NB File: SPD1301.prn City: MILTON County: SPEED NB

TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
			¥75													
4		14														
14:00	53	4	21	95	109	= 17	⁷ 5	0	0	0	0	0	0	0	0	304
15:00	55	10	28	117	125	29	5	0	0	0	0	0	0	0	0	369
16:00	62	3	28	139	131	23	4	0	0	0	0	0	0	0	0	390
17:00	33	7	28	164	123	45	0	0	0	0	0	0	0	0	0	400
18:00	34	19	41	169	150	33	1	1	0	0	0	0	0	0	0	448
19:00	16	1	33	138	159	42	3	2	0	0	0	0	0	0	0	394
20:00	9	1	22	125	81	19	2	. 0	0	0	0	0	0	0	0	259
21:00	10	1	31	129	99	ੁ 11	0	1	0	0	0	0	0	0	0	282
22:00	2	7	28	108	69	11	1	. 0	0	0	0	0	0	0	- 0	226
23:00	2	5	19	79	64	15	0	0	0	0	0	0	0	0	0	184
24:00	2	1	12	37	45	13	2	2	0	0	0	0	0	0	0	114
DAY TOTAL	278	 59	291	1300	1155	258	23	 6	0	0			0	0		3370
PERCENTS	8.3%	1.8%		38.6%		7.6%	0.6%	0.1%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 26.9 mph

Median Speed 33.1 mph

10 MPH Pace Speed 29 mph to 39 mph 2455 vehicles in pace Representing 72.8% of the total vehicles 85th Percentile Speed 38.1 mph

Average Speed 31.7 mph

Vehicles > 65 MPH 0 0.0%

Page: 1

MassDOT Highway Division SPEED SUMMARY Wed 3/28/2018

File: SPD1301.prn

County: SPEED NB

City: MILTON

Page: 2

Site Reference: 180060000787

Site ID: 00000001301

Location: RTE 138 NORTH OF BRADLEE RD.

Direction: NORTH

Lane: 1

TIME 86+ Tota 01:00 02:00 03:00 Θ 04:00 O n 05:00 06:00 07:00 n n 00:00 **4B8** 09:00 10:00 O O O n O O 11:00 12:00 13:00 n В 14:00 15:00 16:00 17:00 18:00 19:00 20:00 б O n 21:00 O O 22:00 23:00 24:00 DAY TOTAL 0.0% 6.6% 37.8% 35.0% 0.2% 0.0% 80.0 0.0% 0.0% **PERCENTS** 7.9% 1.1% 0.0% 10.2% 1.2%

Statistical Information...

15th Percentile Speed 26.8 mph

Median Speed 33.3 mph

10 MPH Pace Speed
29 mph to 39 mph
4918 vehicles in pace
Representing 72.7% of the total vehicles

85th Percentile Speed 38.2 mph

Average Speed 31.6 mph

MassDOT Highway Division SPEED SUMMARY Thu 3/29/2018

File: SPD1301.prn City: MILTON

County: SPEED NB

Page: 3

Site Reference: 180060000787 Site ID: 000000001301

Location: RTE 138 NORTH OF BRADLEE RD.

Direction: NORTH

Lane: 1

TIME	19	24	29	-34	39	44	49	54	59	64	69	74	79	85	86+	Tota
01:00	4	1	2	25	- 33	15	1	1	0	0	0	0	0	0	0	82
02:00	0	# O	3	12	17	1	2	3	0	0	· O	0	0	0	0	38
03:00	0	0	1	9	11	3	3	0	1	0	0	0	0 :	. 0	0	28
04:00	2	0	0	7	17	6	1	0	0	0	0	0	0	0	0	33
05:00	2	0	2	11	18	10	3	4	0	0	0	0	0	0	0	50
06:00	13	1	11	116	133	26	8	1	0	0	0	0	0	0	0	309
07:00	29	1	30	186	149	29	6	0	0	0	0	0	0	0	0	430
08:00	47	7	41	182	157	42	4	0	0	0	0	0	0	0	0	480
09:00	40	2	46	201	137	19	6	0	0	0	0	0	0	0	. 0	451
10:00	35	16	37	143	142	10	- 0	1	0	. 0	. 0	0	0	0	0	392
11:00	49	5	14	145	155	15	4	0	- 0	0	0	- 0	0	0	0	387
12:00	68	5	41	113	92	20	2	0	0	0	0	0	0	0	0	341
13:00	50	6	15	125	99	11	0	2	0	0	0	0	0	0	0	308
14:00	44	4	15	119	110	18	0	0	ା 0	0	0	0	0	. 0	0	310
15:00	57	9	10	95	119	29	. 9	0	0	0	0	0	0	. 0	0	328
16:00	71	3	52	192	115	16	1	0	0	0	0	0	0	0	· 0	450
17:00	40	. 2	39	174	123	29	2	0	1	0	0	0	0	0	0	410
18:00	59	10	28	154	149	43	0	1	0	0	0	0	0	0	0	444
19:00	41	5	35	198	120	. 18	0	0	0	0	0	0	0	0	0	417
20:00	6	2	50	144	104	10	1	1	0	0	0	0	0	0	0	318
21:00	7	14	57	122	76	17	3	1	0	0	0	0	0	0	0	297
22:00	11	1	18	134	78	11	2	2	0	0	0	0	0	0	0	257
23:00	9	10	19	98	68	14	2	0	0	0	0	0	0	0	0	220
24:00	6	1	12	62	61	16	1	0	0	0	0	0	0	0	0	159
DAY TOTAL	690	105	 578	2767	 2283	436	 61	17	2	0	 0	0	0		 0	6939
PERCENTS	- 10.0%	1.6%			32.9%	6.2%	0.8%	0.2%	0.0%	90.0	90.0	0.0%	0.0%	0.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 26.1 mph

Median Speed 32.8 mph

10 MPH Pace Speed 29 mph to 39 mph 5050 vehicles in pace Representing 72.7% of the total vehicles 85th Percentile Speed 37.9 mph

Average Speed 31.2 mph

MassDOT Highway Division SPEED SUMMARY Fri 3/30/2018

File: SPD1301.prn

County: SPEED NB

City: MILTON

Page: 4

Site Reference: 180060000787

Site ID: 00000001301

Location: RTE 138 NORTH OF BRADLEE RD.

Direction: NORTH

Lane: 1

				1.0							1.0					
ŢIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
01:00	1	2	13	33	21	13	2	0	0	0	0	0	0	0	0	. 85
02:00	1	1	10	14	22	20	6	0	0	0	0	0	0	0	0	74
03:00	2	2	3	11	9	6	0	2	0	0	0	0	0	0	0	35
04:00	0	1	1	5	11	10	3	1	0	0	0	0	0	0	0	32
05:00	5	1	3	11	42	9	3	2	. 0	0	0	0	0	0	0	76
06:00	29	2	9	50	116	34	13	1	0	0	0	0	0	0	0	254
07:00	48	1	14	138	155	36	7	0	0	0	0	0	0	0	0	399
08:00	49	1	21	178	153	20	1	0	0	0	0	0	0	0	0	423
09:00	41	9	44	187	117	19	2	0	0	0	0	0	0	0	0	419
10:00	29	9	39	123	114	24	4	1	0	0	0	0	0	0	0	343
11:00	52	4	23	85	107	37	2	0 .	2	1	0	0	0	0	0	313
12:00	67	7	22	104	132	25	3	0	. 0	0	0	0	0	, O	0	360
13:00	68	5	47	124	90	31	6	0	0	0	0	0	0	0	0	371
14:00	53	0	28	152	126	30	0	0	0	0	0	0	0	0	0	389
15:00	48	3	38	147	116	18	2	0	0	0	0	0	0	0	0	372
16:00	53	3	40	146	132	15	0	0	0	0	0	0	0	0	0	389
17:00	47	3	32	173	151	26	0	0	0	0	0	0	0	0	0	432
18:00	49	6	50	179	130	25	0	0	0	0	0	0	0	0	0	439
19:00	42	g: 1	35	169	160	32	9	0	0	0	0	0	0	0	0	448
20:00	26	1	33	131	93	13	2	0	0	0	0	0	0	0	0	299
21:00	12	5	44	131	88	10	1	0	0	0	0	0	0	0	0	291
22:00	8	12	41	138	74	13	2	0	0	0	0	0	0	0	0	288
23:00	11	2	40	127	76	18	2	0	0	0	0	0	0	0	0	276
24:00	6	1	16	∞ 69	70	15	1	0	0	0	0	0	0	0	0	178
DAY TOTAL	747	82	646	2625	2305	499	71	7	2	1	0	0	0	0	0	6985
PERCENTS	10.7%	1.2%	9.3%	37.6%	33.0%	7.1%	1.0%	0.1%	90.0	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 25.7 mph

Median Speed 32.9 mph

10 MPH Pace Speed
29 mph to 39 mph
4930 vehicles in pace
Representing 70.5% of the total vehicles

85th Percentile Speed 38.0 mph

Average Speed 31.1 mph

Vehicles > 65 MPH 0 0.0%

MassDOT Highway Division SPEED SUMMARY

File: SPD1301.prn

County: SPEED NB

City: MILTON

Sat 3/31/2018

Site Reference: 180060000787 Site ID: 00000001301

Location: RTE 138 NORTH OF BRADLEE RD.

Direction: NORTH

Lane: 1

TIME 86+ Tota 01:00 02:00 03:00 n O 04:00 05:00 n n n 06:00 07:00 08:00 n 09:00 10:00 11:00 .353 12:00 13:00 .18 14:00 n O Ð 15:00 16:00 17:00 В 18:00 19:00 Λ n n n 20:00 21:00 22:00 23:00 24:00 361 2002 2221 DAY TOTAL 5.7% 31.3% 34.7% 0.0% 0.0% 0.0% 0.0% 0.0% 0.0% 0.0% 100% PERCENTS 15.1% 1.6% 9.8% 1.7% 0.1%

Statistical Information...

15th Percentile Speed 18.9 mph

Median Speed 33.4 mph

10 MPH Pace Speed
29 mph to 39 mph
4223 vehicles in pace
Representing 65.9% of the total vehicles

85th Percentile Speed 38.5 mph

Average Speed 30.8 mph

Vehicles > 65 MPH

1
0.0%

Page: 5

MassDOT Highway Division SPEED SUMMARY Sun 4/1/2018

File: SPD1301.prn

County: SPEED NB

City: MILTON

Page: 6

Site Reference: 180060000787

Site ID: 00000001301

Location: RTE 138 NORTH OF BRADLEE RD.

Direction: NORTH

Lane: 1

Ъ	me: 1				- 0												
	TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
													St.		7,		
	01:00	4	0	4	49	65	16	3	0	1	0	0	0	0	0	0	142
	02:00	3	0	5	40	43	13	2	0	2	0	0	0	0	0	0	108
	03:00	4	1	7	26	35	14	. 2	4	0	0	0	0	0	0	0	93
	04:00	1	0	5	16	29	10	0	0	0	1	0	+ 0	0	1	0	63
	05:00	1	0	0	7	10	6	7	0	2	0	0	0	0	0	0	33
	06:00	3	0	1	8	22	9	8	0	0	0	0	0	0	0	0	51
	07:00	5	2	1	14	36	22	12	5	0	0	0	0	0	0	0	97
	08:00	9	2	4	21	59	33	4	3	0	1	0	0	0	0	0	136
	09:00	20	3	7	37	110	. 26	4	0	1	0	. 0	0	0	0	0	208
	10:00	28	4	15	72	152	49	7	0	0	0	0	0	0	0	0	327
	11:00	72	5	4	115	163	32	6	0	0	0	0	0	0	0	- 0	397
	12:00	96	10	9	91	108	33	8	2	0	0	0	. 0	0	0	0	357
	13:00	82	7	10	85	133	27	0	· 4	0	0	0	1	0	0	0	349
	14:00	82	5	25	131	86	38	7	4	0	0	0	0	0	0	0	378
	15:00	79	1	26	106	122	45	8	0	0	0	0	0	0	0	0	387
	16:00	51	5	14	124	147	31	9	0	0	0	0	1	0	0	0	382
	17:00	63	4	28	123	117	35	3	0	0	0	0	0	0	. r 0	0	373
	18:00	47	3	14	112	133	49	5	2	0	6	0	0	0	0	0	371
	19:00	36	3	23	181	161	28	6	0	0	0	0	0	0	0	0	438
	20:00	30	10	65	212	125	15	3	0	0	0	0	0	0	0	0	460
	21:00	8	16	45	209	86	26	2	0	0	0	0	0	0	0	0	392
	22:00	7	0	28	160	113	11	2	0	0	0	0	0	0	0	0	321
	23:00	2	= 3	. 15	78	85	14	5	1	0	0	0	0	0	0	0	203
	24:00	5	1	8	34	71	21	8	4	0	0	0	0	0	0	0	152
Di	AY TOTAL	738	85	363	2051	2211	603	121	29	6	8	0	2	0	1	0	6218
P	ERCENTS	11.9%	1.4%	5.9%	33.0%	35.6%	9.7%	2.0%	0.4%	0.0%	0.1%	0.0%	0.0%	0.0%	0.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 25.5 mph

Median Speed 33.7 mph

10 MPH Pace Speed
29 mph to 39 mph
4262 vehicles in pace
Representing 68.5% of the total vehicles

85th Percentile Speed 38.6 mph

Average Speed 31.7 mph

Vehicles > 65 MPH 3 0.0%

MassDOT Highway Division SPEED SUMMARY Mon 4/2/2018

File: SPD1301.prn

County: SPEED NB

City: MILTON

Page: 7

Site Reference: 180060000787

Site ID: 00000001301

Location: RTE 138 NORTH OF BRADLEE RD.

Direction: NORTH

Lane: 1

TIME	19	24	4	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
							2		ò			81					
01:00	1		1	. 5	25	31	9	2	1	0	0	0	0	0	0	0	75
02:00	0		1	2	6	16	10	1	2	0	0	0	0	0	0	0	38
03:00	0		0	0	5	11	7	4	1	0	0	0	0	0	0	0	28
04:00	1	24	1	0	10	15	8	, 0	2	0	° 0	0	0	₌ 0	0	0	37
05:00	3		0	0	6	29	18	6	0	2	1:	0	0	. 0	0	0	65
06:00	16		4	12	73	134	53	7	0	0	0	0	. 0	0	0	0	29 9
07:00	50		3	12	147	150	48	0	0	2	0	0	. 0	0	0	0	412
08:00	50		6	41	185	184	32	3	0	0	0	0	0	. 0	0	0	501
09:00	30		5	37	145	170	24	0	0_	0	> 0	0	0	0	0	0	411
10:00	24	Ji.	11	58	156	114	13	0	0	.0	° 0	0	0	. 0	0	0	376
11:00	10		2	44	145	104	15	1	1	0	0	0	0	0	0	0	322
12:00	22		11	31	107	99	15	5	_ 1	0	0	0	0	0 :	. 0	0	291
13:00	24		2	13	110	106	23	2	0	1	1	0	0	0	0	0	282
14:00	27		3	14	83	99	31	10	2	0	0	0	0	. 0	0	0	269
15:00	40		В	26	87	102	36	3	0	0	0	0	0	0	0	0	302
16:00	26		2	35	149	121	20	6	1	0	0	0	0	0	0	0	360
17:00	34		8	30	160	122	22	2	1	0	0	0	0	0	0	.0	379
18:00	20		4	25	135	170	42	2	1	0	0	0	. 0	0	0	0	399
19:00	32		3	7	111	133	38	· 6	0	0	0	0	0	0	0	0	330
20:00	4		3	19	118	88	31	6	0	0	0	0 0	0	0	0	0	269
21:00	5		0	34	103	76	18	2	0	0	0	0	0	0	0	0	238
22:00	3		1	21	121	69	10	0	0	2	0	0	0	0	0	0	227
23:00	5	1	10	34	59	41	13	2	0	0	0	0	0	0	0	0	164
24:00	3		0	1	37	42	16	1	1	2	0	0	0	0	0	0	103
DAY TOTAL	430		 89	501	22B3	2226	 552	71	14	 9	2	0	0	0	0	0	6177
PERCENTS	7.0%	1.			37.0%		8.9%	1.1%	0.2%	0.1%	0.0	0.0%	0.0%	0.0%	90.0	0.0%	

Statistical Information...

15th Percentile Speed 28.1 mph

Median Speed 33.5 mph

10 MPH Pace Speed
29 mph to 39 mph
4509 vehicles in pace
Representing 72.9% of the total vehicles

85th Percentile Speed 38.4 mph

Average Speed 32.4 mph

Vehicles > 65 MPH 0 0.0%

MassDOT Highway Division SPEED SUMMARY Tue 4/3/2018

Page: 8

Site Reference: 180060000787 Site ID: 000000001301

Location: RTE 138 NORTH OF BRADLEE RD.

Direction: NORTH

Lane: 1

				8	ŧ			55							13		
	TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
									 o'								
	01:00	0	0	6	30	32	13	2	0	0	0	0	0	0	0	0	83
	02:00	0	1	2	11	12	4	2	2	0	0	0	0	0	0	0	34
	03:00	2	1	1	6	14	4	4	1	1	0	0	0	0	0	. 0	34
	04:00	1	0	0	3	14	10	4	0	0	0	0	0	0	0	0	32
	05:00	0	1	5	10	34	16	. 3	0	1	0	0	0	0	0	0	70
	06:00	15	1	. 7	93	147	42	3	2	0	0	0	0	0	0	0	310
	07:00	36	1	18	170	188	64	4	2	. 0	0	0	0	0	0	0	483
	08:00	44	1	42	199	172	22	5	0	0	0	0	0	0	0	0	485
	09:00	65	4	40	155	156	21	5	2	0	0	0	0	0	0	0	448
	10:00	64	2	22	137	151	26	1	2	0	0	0	0	0	0	0	405
	11:00	67	8	13	117	99	23	0	0	1	. 0	0	0	0	0	0	328
DA	Y TOTAL	 294	20	156	931	1019	245	 33	11	3	0	0	 0	0	0	0	2712
PE	RCENTS	10.9%	0.8%	5.8%	34.3%	37.5%	9.0%	1.2%	0.4%	0.1%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 27.0 mph

Median Speed 33.8 mph

10 MPH Pace Speed 29 mph to 39 mph 1950 vehicles in pace Representing 71.9% of the total vehicles 85th Percentile Speed 38.4 mph

Average Speed 31.8 mph

File: SPD1301.prn City: MILTON

County: SPEED NB

Vehicles > 65 MPH 0 0.0%

MassDOT Highway Division SPEED SUMMARY Tue 3/27/2018

STA.13 SB

Site Reference: 180060000554

Site ID: 00000001302

Location: RTE 138 NORTH OF BRADLEE RD.

Direction: SOUTH

Lane: 1

TIME	19	24	29	34	39	44	49	54	59	64	, 69	74	79	85	86+	Tota
-77																
14:00	0	1	1	1	22	110	130	63	17	1	0	0	0	0	0	346
15:00	4	0	0	8	39	147	155	94	19	3	1	0	1	-0	2	473
16:00	2	0	1	8	41	130	175	77	16.	1	0	0	0	0	2	453
17:00	0	0:	1	12	19	105	149	56	14	2	0	0	0	0	0	358
18:00	1	1	2	3	18	105	179	68	20	0	0	1	0	0	0	398
19:00	1	1	1	12	27	114	149		6	1	Ō	0	Ō	Ō	Ō	372
20:00	ō	0	Ō	5	29	125	130		10	2	Ö	Ō	Ŏ	Ŏ	i	351
21:00	Ō	Ō	Ō	5	46	125	97	32	2	1	0	Ō	Ō	- 0	0	308
22:00	i	Õ	Ö	18	56	100	75		3	5	Ŏ	í	Ŏ	Ö	Ö	289
23:00	ō	ō	ō	0	9	88	77	26	10	2	ō	- 1	ō	Ō	ŏ	213
24:00	Ö	ō	ō	ī	6	41	62	16	5	ī	ĭ	ī	- 0	Ö	Ö	134
45	ħ	•	·	_	·					_	_	_			·	
DAY TOTAL	9	3	6	73	312	1190	1378	571	122	19	2	4	1	0	5	3695
PERCENTS	0.3%	0.1%	0.2%	2.0%	8.5%	32.3%	.37.2%	15.4%	3.3%	0.5%	0.0%	0.1%	0.0%	90.0	0.1%	100%

Statistical Information...

15th Percentile Speed 39.6 mph

Median Speed 44.9 mph

10 MPH Pace Speed
39 mph to 49 mph
2568 vehicles in pace
Representing 69.4% of the total vehicles

85th Percentile Speed 50.5 mph

Average Speed 44.8 mph

File: SPD1302.prn

County: SPEED SB

City: MILTON

Vehicles > 65 MPH 12 0.3% Page: 1

File: SPD1302.prn

County: SPEED SB

City: MILTON

MassDOT Highway Division SPEED SUMMARY Wed 3/28/2018

Site Reference: 180060000554

Site ID: 00000001302

Location: RTE 138 NORTH OF BRADLEE RD.

Direction: SOUTH

Lane: 1

TIME 86+ Tota 01:00 02:00 O n 03:00 04:00 05:00 06:00 07:00 08:00 n 09:00 10:00 O Ω 11:00 O 12:00 13:00 14:00 В 15:00 16:00 Û 17:00 18:00 n O n 19:00 20:00 21:00 22:00 Λ n n 23:00 24:00 DAY TOTAL 430 2137 2567 1013 0.1% 0.3% 1.9% 6.5% 32.3% 38.8% 15.3% 3.5% 0.7% 0.3% 0.0% 0.0% 0.0% 0.0% PERCENTS 0.3% 100%

Statistical Information...

15th Percentile Speed 39.9 mph

Median Speed 45.1 mph

10 MPH Pace Speed
39 mph to 49 mph
4704 vehicles in pace
Representing 70.9% of the total vehicles

85th Percentile Speed 50.7 mph

Average Speed 45.1 mph

Vehicles > 65 MPH 0.5%

MassDOT Highway Division SPEED SUMMARY Thu 3/29/2018

File: SPD1302.prn

County: SPEED SB

City: MILTON

Page: 3

Site Reference: 180060000554

Site ID: 00000001302

Location: RTE 138 NORTH OF BRADLEE RD.

Direction: SOUTH

Lane: 1

TIME 86+ Tota 01:00 - 2 O n O 02:00 03:00 04:00 05:00 06:00 Θ 07:00 08:00 09:00 10:00 n 11:00 12:00 13:00 14:00 15:00 16:00 17:00 18:00 19:00 20:00 O n O 21:00 22:00 23:00 24:00 586 2128 2462 1036 DAY TOTAL 8.7% 31.5% 36.4% 15.3% 3.9% 0.7% 0.1% 0.1% 0.0% 0.0% 0.0% PERCENTS 0.7% 0.1% 0.3% 2.2%

Statistical Information ...

15th Percentile Speed 39.5 mph

Median Speed 44.9 mph

10 MPH Pace Speed
39 mph to 49 mph
4590 vehicles in pace
Representing 67.7% of the total vehicles

85th Percentile Speed 50.8 mph

Average Speed 44.8 mph

Vehicles > 65 MPH 0.4%

MassDOT Highway Division SPEED SUMMARY Fri 3/30/2018

File: SPD1302.prn

County: SPEED SB

City: MILTON

Page: 4

Site Reference: 180060000554

Site ID: 00000001302

Location: RTE 138 NORTH OF BRADLEE RD.

Direction: SOUTH

Lane: 1

TIME 86+ Tota 01:00 Ð 02:00 03:00 04:00 05:00 06:00 07:00 08:00 09:00 10:00 11:00 12:00 13:00 14:00 15:00 16:00 17:00 n 18:00 19:00 20:00 a n 21:00 22:00 23:00 24:00 DAY TOTAL 1.9% 9.3% 32.2% 37.9% 13.9% 3.0% 0.6% 0.1% 0.1% 0.0% 0.0% PERCENTS 0.5%

Statistical Information...

15th Percentile Speed 39.5 mph

Median Speed 44.8 mph

10 MPH Pace Speed
39 mph to 49 mph
4928 vehicles in pace
Representing 70.1% of the total vehicles

85th Percentile Speed 50.1 mph

Average Speed 44.7 mph

Vehicles > 65 MPH 0.3%

MassDOT Highway Division SPEED SUMMARY Sat 3/31/2018

File: SPD1302.prn

City: MILTON County: SPEED SB Page: 5

Site Reference: 180060000554

Site ID: 00000001302

Location: RTE 138 NORTH OF BRADLEE RD.

Direction: SOUTH

Lane: 1

TIME 86+ Tota 01:00 n 02:00 03:00 04:00 05:00 06:00 07:00 08:00 09:00 10:00 11:00 12:00 13:00 14:00 15:00 16:00 17:00 Ð 18:00 19:00 20:00 Ð 21:00 22:00 23:00 O 24:00

Statistical Information...

DAY TOTAL

PERCENTS

15th Percentile Speed 40.3 mph

0.2%

0.2%

Median Speed 45.8 mph

10 MPH Pace Speed
39 mph to 49 mph
4665 vehicles in pace
Representing 67.2% of the total vehicles

0.3%

1.2%

6.2% 28.6% 38.7% 18.7%

4.5%

1.1%

0.2%

85th Percentile Speed 51.7 mph

0.0%

Average Speed 45.8 mph

Vehicles > 65 MPH 0.6%

MassDOT Highway Division SPEED SUMMARY Sun 4/1/2018

File: SPD1302.prn

City: MILTON County: SPEED SB Page: 6

Site Reference: 180060000554

Site ID: 00000001302

Location: RTE 138 NORTH OF BRADLEE RD.

Direction: SOUTH

Lane: 1

TIME 86+ Tota 01:00 O n 02:00 03:00 04:00 05:00 06:00 07:00 08:00 09:00 10:00 11:00 Q 0 . 12:00 ٥. 13:00 14:00 15:00 n 16:00 17:00 18:00 19:00 20:00 21:00 22:00 23:00 24:00 2026 2489 1077 DAY TOTAL 7.8% 31.0% 38.1% 16.4% PERCENTS 0.2% 0.3% 1.1% 3.9% 0.8% 0.2% 80.0 0.0% 0.0%

Statistical Information...

15th Percentile Speed 39.9 mph

Median Speed 45.3 mph

10 MPH Pace Speed
39 mph to 49 mph
4515 vehicles in pace
Representing 69.0% of the total vehicles

85th Percentile Speed 51.0 mph

Average Speed 45.3 mph

Vehicles > 65 MPH 0.4%

MassDOT Highway Division SPEED SUMMARY Mon 4/2/2018

Page: 7

Site Reference: 180060000554

Site ID: 00000001302

Location: RTE 138 NORTH OF BRADLEE RD.

Direction: SOUTH

Lane: 1

File: SPD1302.prn City: MILTON County: SPEED SB

TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
01:00	0	0	0	0	3	25	22	20	4	2	' 3	0	0	0	0	79
02:00	0	0	0	1	1	4	12	10	2	2	0	0	1	0	0	33
03:00	0	0	- 0	0	1	4	7	8	4	2	0	0	2	0	0	28
04:00	0	0	0	0	3	7	6	6	4	2	1	0	0	0	0	29
05:00	0	0	0	1	0	0	12	17	10	3	2	0	0	0	0	45
06:00	0	0	1	1	3	° 21	53	37	23	5	1	1	1	0	0	147
07:00	0	0	1	1	3	63	105	59	21	5	0	0	0	0	0	258
08:00	2	1	4	6	44	135	170	44	10	4	1	1	0	0	0	422
09:00	0	0	2	10	38	155	172	53	9	0	1	0	0	0	0	440
10:00	- 0	0	0	5	30	94	138	49	11	2	1	0	0	0	0	330
11:00	0	0	0	9	48	81	92	45	7	1	0	0	0	0	0	283
12:00	0	0	0	1	27	73	135	49	12	0	1	0	0	0	0	298
13:00	0	1	0	1	17	65	131	64	10	3	2	0	0	0	0	294
14:00	0	1	9	9	27	99	114	53	12	2	0	0	1	0	0	327
15:00	ō	0	46 1	Ō	37	147	171	56	10	1	Ó	Ô	Ō	Ō	Ō	423
16:00	2	4	2	6	28	112	174	86	6	Ō	Ó	1	0	0	0	421
17:00	ō	. 0	Ō	2	10	80	168	68	15	3	1	0	0	Ō	Ō	347
18:00	i	Ö	4	9	41	134	149	69	5	2	ō	Ŏ	Ŏ	Õ	ō	414
19:00	2	ō	ō	ō	12	144	125	47	8	2	ŏ	Ŏ	Ŏ	Ō	Ŏ	340
20:00	ō	ō	Ŏ	ō	24	120	115	36	7	2	ō	ō	Ō	Õ	ō	304
21:00	ŏ	□ 1	ŏ	5	14	102	125	26	3	ī	1	ō	ō	ō	ō	278
22:00	ŏ	ō	ō	ī	18	93	78	27	3	- 0	ō	ŏ	Õ	Ŏ	ō	220
23:00	27	8	15	10	14	43	36	26	4	5	ŏ	ŏ	ŏ	Ŏ	ŏ	188
24:00	0	Ö	0	1	-6			45	10	6	2	1	ō	Ō	Ō	143
DAY TOTAL	34	16	 39	 79	449	1831	2352	1000	210	 55	17	4	 5	0		6091
PERCENTS	34 0.6%	0.3%	0.7%	1.3%			38.7%		3.4%	0.9%	0.2%	0.0%	0.0%	0.0%	0.0%	100%
tevcen19	0.08	0.56	0.16	1.30	7.770	30.10	20.18	10.42	3.40	0.96	0.20	0.00	0.08	0.08	0.08	1009

Statistical Information...

15th Percentile Speed 39.8 mph

Median Speed 45.3 mph

10 MPH Pace Speed
39 mph to 49 mph
4183 vehicles in pace
Representing 68.6% of the total vehicles

85th Percentile Speed 50.9 mph

Average Speed 45.1 mph

Vehicles > 65 MPH 26 0.4%

MassDOT Highway Division SPEED SUMMARY Tue 4/3/2018

Page: 8

Site Reference: 180060000554 Site ID: 000000001302

Location: RTE 138 NORTH OF BRADLEE RD.

Direction: SOUTH

Lane: 1

TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
01:00	0	0	0	0	4	18	25	15	12	2	2	0	0	0	0	78
02:00	0	0	0	0	2	8	8	9	3	1	1	2	0	0	0	34
03:00	0	0	0	0	1	4	10	9	2	1	3	1	0	0	0	31
04:00	0	0	0	0	1	4	3	9	4	2	2	n 1	0	0	0	26
05:00	0	. 0	0	1	2	6	. 18	19	9	6	1	0	0	0	0	62
06:00	0	0	0	0	_ 5	27	49	46	21	5	1	0	0	0	0	154
07:00	0	0	0	0	5	68	109	84	22	4	2	1	0	0	0	295
08:00	3	0	8	5	48	153	191	57	12	0	2	0	0	0	0	479
09:00	3	0	0	3	27	147	192	82	9	3	0	0	0	0	2	468
10:00	1	0	0	1	9	96	134	63	17	1	0	0	0	0	0	322
11:00	12	2	8	1	5	46	100	50	13	7	0	0	1	0	0	245
DAY TOTAL	 19	2	16	11	109	577	839	443	124	32	14	5	1	0	2	2194
PERCENTS	0.9%	0.1%	0.8%	0.6%	5.0%	26.3%	38.3%	20.2%	5.6%	1.4%	0.6%	0.2%	0.0%	80.0	0.0%	100%

Statistical Information...

15th Percentile Speed 40.5 mph

Median Speed 46.2 mph

10 MPH Pace Speed 39 mph to 49 mph 1416 vehicles in pace Representing 64.5% of the total vehicles 85th Percentile Speed 52.3 mph

Average Speed 46.1 mph

File: SPD1302.prn City: MILTON

County: SPEED SB

Vehicles > 65 MPH 22 1.0%

Appendix C: Traffic Signal Data

- 1. Existing Signal Timing Plans
- 2. Signal Warrant Analysis

Part 1: Existing Signal Timing Plans

COMMONWEALTH OF MASSACHUSETTS

DEPARTMENT OF PUBLIC WORKS

MILTON

5R83-2

TRAFFIC SIGNAL LAYOUT

BLUE HILL AVE. (RTE. 138) & BRUSH HILL RD:

Scale: |" = 40

Signal No. 8 - 508

Date: Aug. 18,1978

SEQUENCE AND TIMING

		Housings	1	2	3	_4	5	6	7	8	9	10	11	12	o.	
INITIAL INTERS	LL .		7	12.1	111	*10		1		= =					OPE	S
VEHICLE INTE		10				5									x	SE
MINIMUM	A NAME OF THE PARTY OF THE PART		45												AS	ST.
MUMIXA						30						-			교	EM
	THER INTERVALS		7 11 9	5	1	50	4								_	L
Blue Hill Ave	(N.B.)	A-B-C	4	Y	R	R	R	R							1	
Blue Hill Ave.	(S.B)	D-E	1-0	Y	R	R	R	R		$E^{(n)}$					Y	
Brush Hill Rd	(W.B.)	F-G	R	R	R	G	Y	R		*					F	
Brush Hill Rd.	(E.B.)	H-J	R	R	R	G.	Y	R		1,3					F	1
				1		3		1.								
			PH	iase	A	PI	ose	8			1				1	1.5

MAJOR ITEMS REQUIRED

NOTES

NUMBER	ITEMS
2	Star pard Signal Posts (8')
S	Most Arms (1-25', 1-30')
3	2 V: 1-4,5-3 Lens HousingAll 12"
2	I Wo, 3 Lens Housings All 12"
	I Wor 4 Lens Housings All 12"
	Controller Type 2
2	Magnetic Detectors
	Mogretic Detector Relay
2	12 × 12 Pull Boxes
2	12" > 12" Pull Boxes 8" X 23" Pull Boxes
1	Service Connection
	Necessary Duct, Cable, Misc. Material, Lazor & Equipment To Complete
	The installation.

OFFICE	RECORD AND REV	ISIONS
Power Pole No	- 1/128	
Meter No	* - V I Y	
A No.		
Completed: 10	/2/73	
Checked:	V	
Revised: Oct.	29,1979	20
- '4"	4 5 4 12 14	
Supersedes i	_ayout Dated 3/2	24/70
Interval I, Mini	mum, changed from	n 40 sec. to
45 sec. 10/1	0/79	
Interval 2,01	her Intervals, char	ged from
	ec. 10/10/79	La residente
Initial Int.4 Cho	ntoiO From 7, 9/1	4/60
	chan, from 25 to 3	
		16

Ø.
1
3
1
NC

-	SIGNAL OR F	LASHER HOUSING	
	R = RED	W = WALK	
	Y = YELLOW	FDW = FLASH DON'T WALK	
13	G = GREEN	.DW = DON'T WALK	D
	VA = VERT	CAT ARROW.	

PROPOSED CONDUIT - EXISTING CONDUIT OVERHEAD CABLE

23

LOOP DETECTOR
WALK - DON'T WALK
PEDESTRIAN HOUSING
短音LASHER & METER PEDESTAL - LA = LEFT ARROW RA = RIGHT ARROW

Vision and the		Date	Ву
Field Survey	Made		
I leid Survey	Checked		
Denvise	Made	8/18/72	SEL
Drawing	Checked	10/30/79	V MAN

SIGNAL POST

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i samuali ilin			
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LOC#3

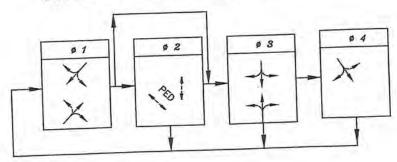
APPROX. NORTH



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SEQUENCE AND TIMING F	OR PRETIM	ED/F03H	501			7111			~	0	9	10	#1	12	18	14	15	16	17	18	19	20	21	22	23	24	FLASH	
STREET	DIRECTION	HOUSINGS	1	2	3	1	5	6	R R	R	R	R	R	R					-	1	4.0	-					FY	
LUE HILL AVE.	NB	A,B	G	Y	R	R	R	R	R	R	R	R	R	R	+	1	-	1	1, 1			P					FR	
LUE HILL AVE.	SB	C,D	G	Y	R	R	R	R	G	Y	R	R	R	R					100		111			-	-		FR	
THERTON ST.	WB	E,F,G	R	R	R	R	R	R	R	R	R	G	Y	R						-	111	-	-		-	-	FR	
THERTON ST.	EB	H,J	R	R	R	R	R	R	G	Y	R	R	R	R			-						-		-		FR	
RADLEE RD.	EB	K,L	R	R	R	R	R	R	G	-				1. +					-	-	15.5		-	-	-	-		
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PASSAGE TIME (VECHICLE)			18		-	-	-	-	5	+	-	8								1	12	-	-	-	+	+-	4 -	
MAXIMUM 1			40			1	-	-	120	-	-	-											1	+	+	+	- 3.	
MAXIMUM 2						-	-	-	+	4	-	+	4	1									-	+	+	-	- 33	
YELLOW CLEARANCE				5	1	-	-	-	-	+	1	+	1	1									1	-	+	+	1 83	
RED CLEARANCE				1	1	-	-	2	+	-	+	+	1			1			III.				-	-	-	-	EMERGENCY	
WALK (W)						8	1.	-	+	+	-	+	-								1			+	+	-		
PEDESTRIAN CLEARANCE							10	-	-	-	+	+	-	1	1			1					-1				4	
Phobolium, omining			-	1	100		OF		-	OF	-	-	OF	F													-	

PREFERENTIAL PHASING SEQUENCE



DETECT(NUMBE

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3

COMMONWEALTH OF MASSACHUSETTS

DEPARTMENT OF PUBLIC WORKS

TRAFFIC SIGNAL LAYOUT

Blue Hill Ave. (Route 138) and Robbins Street.
Scale: 1 = 20

SEQUENCE AND TIMING

	Housings	1	2	3	4	5	6	7	8	9	10	11	12	oi.	>
INITIAL INTERVAL							7							OPE	1
VEHICLE INTERVAL	1	- 31	Y				3							F	4
MINIMUM		45												AS	1
MAXIMUM							20							T.	1
OTHER INTERVALS			5	1	7	11		4							
Blue Hill Ave.	ABCD	G	Y	R	R	R	R	R	R						_
Robbins St	EFGH	R	R	R	R	R	G	Y	R						1
Robbins St. Pedestrian			DW	DW	W	FDW	DW	DW	DW		_			0	u
	-									-	-			-	_
		-		-		-		-						1	-

MAJOR ITEMS REQUIRED

NOTES

		MAJOR ITEMS RESIDENCES		
-	NUMBER	ITEMS		OFFICE RECO
1	2	Signal Mast Arm 20Ft. Aluminum		Power Pole No 28
		Controller Type 3W	, 1	Meter No S-664
/		Control, Cabinet Type CA or CB		Cost Account No.
		Loop Detector Relay		Completed: 10/31/7
	2	Loop Detectors 6'X4'		Checked:
	2	2-Way 3-3 lens housings 3-8" 3-12"	Mast	ražinia ir dienomialis ir ir ir
	2	2-Way 3-3 lens housings 3-8" 3-12"	Post	Timing Changes: I
	2	Pedestrian Housings		2 sec. to I sec.
	2	Pedestrian Buttons	1	7 sec Int. 5, f
	2	Pull Boxes 12" X12"		int. 6, Initial f
	1	Service Connection		int. 8 added,
		Conduit, cable, etc.		Sept. 4, 1980
]	
		1 2		100

ORD AND REVISIONS 756 006-824-121 Interval 3, changed from

.- Int.4, from 8 sec. to from 7 sec. to II sec.from 4 sec. to 7 sec.-Isec., R,R & DW,

O SIGNAL POST PULL BOX (Size Specified)

CONTROL **贝 METER** O WOODEN POLE O-O MAST ARM -O SONIC DETECTOR PRESSURE DETECTOR
MAGNETIC DETECTOR PRESSURE DETECTOR **⊗ PEDESTRIAN PUSH BUTTON**

SIGNAL OR FLASHER HOUSING R = RED W = WALK Y = YELLOW FOW = FLASH DON'T WALK G = GREEN DW = DON'T WALK VA = VERTICAL ARROW - LA = LEFT ARROW -- RA = RIGHT ARROW --- PROPOSED CONDUIT --- EXISTING CONDUIT

- OVERHEAD CACLE

LOOP DETECTOR. THE WALK - DON'T WALK PEDESTRIAN HOUSING ENTER & METER PEDESTAL

		Date	Ву
Field Survey	Made	5-1-70	RAP
. Italia darray	Checked		
Drawing		10-7-70	
Drawing	Checked	10-7-70	BHR

SIGNAL OPERATION CHART

N		
1	A 10	7
- 1	1	

	PHA	SE			Ø1		ž.=	Ø2		-	Ø3			Ø4	,	FLASHING
STREET	DIRECTION	MOVEMENT	HOUSING	1	2	3	4	5	6	7	8	9	10	W+		
ROUTE 138	NB	ALL	A,B	R	R		G	2.0	-3-	-			10	111	12	OPERATION
ROUTE 138	SB	ALL	C,D	R	R			, Y.	R	R	R	R	R	R	R	Y
BLUE HILL TERR.	WB	ALL	G,H	R	R		. G	Υ.	R	R	R	R	R	R	R	Y
CHEEVER ST.	EB	ALL	E,F	1.55	-	-	R	R	R	G	Y	R	R	R	R	R
PEDESTRIAN	EB/WB			R	R		Ŗ	R	R	R	R	R	G	Y	R	R
. 200227 1100	20/110	ALL	P1,P2	W	FDW		DW	DW	DW	DW	DW	DW	DW	DW	DW	

TIMING IN SECONDS

	OFF			ON			OFF			OFF	
	LOCK		NO	N-LC	CK	NO	N-LC	CK	NO	N-LC	CK
6	10										
					2			2		1	2
		1		5			5	1=1		5	
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		-									-
			-		-	-			0		-
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4			2			2			2		
			6			4			4		
	6	LOCK	LOCK	2 28 23 23 6 10 LOCK NO	2 28 23 5 5 6 10 LOCK NON-LC	2 28 23 23 5 2 6 10 LOCK NON-LOCK	2 2 2 2 2 2 2 2 3 6 6 4 5 5 5 5 6 10 LOCK NON-LOCK NO	2 2 2 2 2 2 2 3 6 6 2 3 6 6 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	2 2 2 2 2 2 3 6 5 5 5 5 5 5 5 5 6 10 LOCK NON-LOCK NON-LOCK	2 2 2 2 2 2 2 2 3 3 6 6 6 6 6 6 6 6 6 6	2 2 2 2 2 2 2 2 2 2 2 3 3 6 6 6 6 6 6 6

NOTES: 1. IF THE ASSIGNED RIGHT OF WAY FOR ANY TRAFFIC MOVEMENT IS TO REMAIN IN EFFECT DURING THE NEXT CALLED PHASE, THEN THE SIGNAL INDICATIONS FOR THAT TRAFFIC MOVEMENT WILL NOT CHANGE DURING THE CLEARANCE INTERVALS.

2. ANY PHASE NOT CALLED WILL BE SKIPPED.

3. THE RIGHT OF WAY MAY BE ASSIGNED TO ANY SIGNAL PHASE, OR ANY COMBINATION OF NON-CONFLICTING PHASES. 4. IF CALLS EXIST ON ALL PHASES, THE ASSIGNMENT OF RIGHT OF WAY SHALL BE IN ACCORDANCE WITH THE PREFERENTIAL

MILTON

TRAFFIC SIGNAL LAYOUT

BLUE HILL AVE. (ROUTE 138) AT BROOK ROAD.

Scale: |" = 40'

Signal No. 8-277

Date: March 21,1967

SE	QUE	NCE	AND	TIM	IING					
Mousings-	1	2	3	4	5	6	7	8	12	ASA PEN
							*************	O NO SECULIAR SECULIA	1	maritainers of
		1							1	
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ABCDE	G	Y	R	R	R	R			1	TY
G-F	R	R	R	R	G	Y		1	1	R
	DW	DW	W	FDW	DW	DW				out
									1	
	Mousings A BCDE	ABCDE G G-F R	Mausings / 2 30 ABCDE G Y G-F R R	30 30 3 7 R R R R R R R R R	Nousings	Mausings / 2 3 4 5 7 30 25 ABCDE G Y R R R G-F R R R G	Mausings	Mausings	Mausings	Mausings

MAJOR ITEMS REQUIRED

MANDER	LITEMS
5 .	Standard Signal Posts (4-10:1-8)
2.	2 Way 3 Lens Housings (12"R)
2.	I Way 3 Lens Housings (I2"R)
8	Pedestrian Indications
1	Magnetic Detectors
4	Ped. Push Buttons
1	Controller TCT-IP
1	I Way 3 Lens Housing
	Necessary Duct, Cable etc.

NOTES

OFFICE RECORD AND REVISIONS	CAN NUMBER
Meter No \$478937	JUL NOW
Power Pole No12	
Reconstructed: 2/1/68	
Da	
MECEIVED	
MAR 1 1 200 1	
100	
DIST 8 M D D W]

OSIGNAL POST - SIG	WAL OR FLASHER HOUSING		
EFULL BOX	R = RED	Field Survey	Mode
MA CONTROL MA METER	Y = YELLOW G = GREEN	e man saway	Checked
O WOODEN POLE	VA = VERTICAL ARROW	Descripe	isiasia
\$-MAST ON WOODEN POLE	LA= LEFT ARROW		Cascand
ACTUATING UNIT	RA= RIGHT ARROW	Timing and	Listed
PRESSURE DETECTOR	PROPOSED CONDUIT	Sampliance	Granked
MAGNETIC DETECTOR	EXISTING CONDUIT	Equipment	1.lalad
@ PEDESTRIAN PUSH BUTTON	OVERNEAD CABLE	I an along heart 2012	Checked

Part 2: Signal Warrant Analysis

Table D-1
Summary of Hourly Volumes and Warrant Analyses
Route 138 at Brush Hill Road, Milton

	ľ	Major Stre	et		Minor	Street			War	rants Satis	sfied	
Hourly		Route 138	8	Brush H	ill Road	Brush H	lill Road	Morront	Morront	Morront	Morront	Morront
period starting	NB	SB	Crossing	Е	В	W	'B	Warrant	Warrant 2	Warrant 3	Warrant 4	Warrant 7
Julia	Volume	Volume	Peds	Volume	Delay	Volume	Delay	'	2	,	7	•
6:00	1,229	797	0	152	1.3	173	9.2	✓	✓	✓		
7:00	1,320	1,253	0	290	Error	286	284.1	✓	✓	✓		
8:00	1,227	1,428	0	305	Error	280	167.0	✓	✓	✓		
9:00	1,061	993	-	258	-	219	-	✓	✓	✓		
10:00	867	992	-	229	-	223	-	✓	✓	✓		
11:00	703	999	-	247	-	197	-	√	✓	✓		0.4
12:00	735	1,106	-	256	-	199	-	√	✓	✓		correct-
13:00	755	1,169	-	284	-	202	-	✓	✓	✓		able
14:00	767	1,418	-	327	-	247	-	√	✓	✓		crash(es) per year
15:00	866	1,708	0	386	67.5	278	Error	√	✓	✓		2011–15
16:00	813	1,732	0	418	16.4	314	435.5	√	✓	✓		
17:00	847	1,771	0	428	26.0	322	Error	√	✓	✓		
18:00	767	1,650	-	378	-	294	-	√	✓	✓		
19:00	630	1,202	-	251	ı	135	•	✓	✓	√		
								MET	MET	MET	NOT MET	NOT MET

- Automatic Traffic Recorder (ATR) counts were conducted over a three-day period between 11 AM Tuesday 3/27/18 and 12 AM Friday 3/30/18.
- Turning Movement Counts (TMC) were conducted on Thursday 3/29/18 and are used for pedestrian volumes, vehicle delay, and peak hour volumes where ATR data isn't available.
- Dashes indicate where hourly data is not available.
- Vehicle delay is in vehicle-hours and is calculated using Highway Capacity Manual 2010 Two-Way Stop Controlled methodology.

Table D–2
Summary of Hourly Volumes and Warrant Analyses
Route 138 at Neponset Valley Parkway, Milton

	N	/lajor Stre	et	Minor	Street		War	rants Satis	sfied	
Hourly period		Route 13	8	Neponset Va	Illey Parkway	Warrant	Warrant	Warrant	Warrant	Warrant
starting	NB	SB	Crossing	Ш	В	Warrani 1	warrani 2	Warrani 3	warrani 4	warrani 7
otarting	Volume	Volume	Peds	Volume	Delay	'		,	7	•
6:00	462	337	0	136	1.0					
7:00	524	485	0	169	2.6	✓	✓			
8:00	548	498	0	162	1.5	✓	✓			
9:00	503	386	-	155	-	✓	✓			
10:00	443	371	-	176	-	✓	✓			
11:00	362	418	-	179	-	✓	✓			1.8
12:00	369	473	-	187	-	✓	✓			correct-
13:00	368	447	-	211	-	✓	✓			able
14:00	390	559	-	232	-	✓	✓	✓		crash(es) per year
15:00	447	535	0	260	59.0	✓	✓	✓		2011–15
16:00	433	456	0	295	37.4	✓	✓	✓		
17:00	445	472	0	283	38.6	✓	✓	✓		
18:00	412	506	-	232	-	✓	✓			
19:00	335	496	-	189	-	✓	✓			
						MET	MET	MET	NOT MET	NOT MET

- Automatic Traffic Recorder (ATR) counts were conducted over a three-day period between 11 AM Tuesday 3/27/18 and 12 AM Friday 3/30/18.
- Turning Movement Counts (TMC) were conducted on Thursday 3/29/18 and are used for pedestrian volumes, vehicle delay, and peak hour volumes where ATR data isn't available.
- Dashes indicate where hourly data is not available.
- Vehicle delay is in vehicle-hours and is calculated using Highway Capacity Manual 2010 Two-Way Stop Controlled methodology.

Table D–3
Summary of Hourly Volumes and Warrant Analyses
Route 138 at Dollar Lane and Milton Street, Milton

	ı	Major Stre	et		Minor	Street			War	rants Satis	sfied	
Hourly period		Route 138	3	Milton	Street	Dollar	Lane	Warrant	Warrant	Warrant	Warrant	Warrant
starting	NB	SB	Crossing	Е	В	W	В	warrant 1	warrant 2	warrant 3	warrant 4	vvarrant 7
otal tillig	Volume	Volume	Peds	Volume	Delay	Volume	Delay	'	2	,	7	•
6:00	462	392	0	77	1.2	174	3.0	✓	✓			
7:00	524	585	0	176	Error	280	21.1	✓	✓	√		
8:00	548	545	0	137	12.3	230	14.8	✓	\	\		
9:00	503	399	-	118	-	135	-		✓			
10:00	443	375	-	84	-	105	-					
11:00	362	455	-	99	-	103	-					1.4
12:00	369	491	-	104	-	115	-					correct-
13:00	368	474	-	112	-	109	-					able crash(es)
14:00	390	602	-	140	-	126	-		✓			per year
15:00	447	605	1	181	16.8	167	8.6	✓	✓	√		2011–15
16:00	433	478	0	205	19.3	168	11.0	✓	✓	✓		
17:00	445	490	0	231	22.0	198	9.4	✓	✓	✓		
18:00	412	665	-	182	-	152	-	✓	✓			
19:00	335	583	-	95	-	94	-					
								NOT MET	MET	MET	NOT MET	NOT MET

- Automatic Traffic Recorder (ATR) counts were conducted over a three-day period between 11 AM Tuesday 3/27/18 and 12 AM Friday 3/30/18.
- Turning Movement Counts (TMC) were conducted on Thursday 3/29/18 and are used for pedestrian volumes, vehicle delay, and peak hour volumes where ATR data isn't available.
- Dashes indicate where hourly data is not available.
- Vehicle delay is in vehicle-hours and is calculated using Highway Capacity Manual 2010 Two-Way Stop Controlled methodology.

Table D-4
Summary of Hourly Volumes and Warrant Analyses
Route 138 at Bradlee Road and Atherton Street, Milton

	ľ	Major Stre	et		Minor	Street			War	rants Satis	sfied	
Hourly period		Route 138	3	Bradle	e Road	Atherto	n Street	Morront	Morront	Morront	Morront	Morront
starting	NB	SB	Crossing	Е	В	W	В	Warrant 1	Warrant 2	Warrant 3	Warrant 4	Warrant 7
Starting	Volume	Volume	Peds	Volume	Delay	Volume	Delay	'	2	,	7	'
6:00	527	294	0	40	0.2	57	0.3					
7:00	628	453	0	63	0.5	108	0.7		✓			
8:00	630	465	0	49	0.4	122	8.0		✓			
9:00	570	327	-	45	-	87	-					
10:00	496	294	-	39	-	51	-					
11:00	404	338	-	43	-	66	-					1.0
12:00	399	367	-	53	-	67	-					correct-
13:00	394	345	-	39	-	53	-					able crash(es)
14:00	409	455	-	55	-	66	-					per year
15:00	502	433	0	57	0.5	84	0.6					2011–15
16:00	480	366	0	74	0.6	87	0.6					
17:00	517	385	1	78	0.6	87	0.6					
18:00	494	406	-	63	-	61	-					
19:00	400	389	-	46	-	58	-					
								NOT	NOT	NOT	NOT	NOT
								MET	MET	MET	MET	MET

- Automatic Traffic Recorder (ATR) counts were conducted over a three-day period between 11 AM Tuesday 3/27/18 and 12 AM Friday 3/30/18.
- Turning Movement Counts (TMC) were conducted on Thursday 3/29/18 and are used for pedestrian volumes, vehicle delay, and peak hour volumes where ATR data isn't available.
- Dashes indicate where hourly data is not available.
- Vehicle delay is in vehicle-hours and is calculated using Highway Capacity Manual 2010 Two-Way Stop Controlled methodology.

Table D-5
Summary of Hourly Volumes and Warrant Analyses
Route 138 at Robbins Street, Milton

		Major Stre	et		Minor	Street			War	rants Satis	sfied	
Hourly		Route 138	3	Robbins	s Street	Robbins	s Street	Morront	Morront	Morront	Morront	Morront
period starting	NB	SB	Crossing	Е	В	W	В	Warrant 1	Warrant 2	Warrant 3	Warrant 4	Warrant 7
Starting	Volume	Volume	Peds	Volume	Delay	Volume	Delay	·	2	7	4	'
6:00	441	462	0	12	0.1	10	0.0					
7:00	484	548	0	30	0.2	61	0.5					
8:00	459	508	1	39	0.2	58	0.5					
9:00	410	428	-	23	-	-	-					
10:00	364	385	-	17	-	-	-					
11:00	323	322	-	26	-	-	-					0.0
12:00	317	307	-	18	-	-	-					correct-
13:00	309	322	-	26	-	-	-					able crash(es)
14:00	342	335	-	28	-	-	-					per year
15:00	407	413	0	32	0.2	48	0.3					2011–15
16:00	392	397	1	34	0.1	58	0.4					
17:00	430	444	0	40	0.1	44	0.3					
18:00	397	383	-	38	-	-	-					
19:00	307	286	-	28	-	-	-					
								NOT	NOT	NOT	NOT	NOT
								MET	MET	MET	MET	MET

- Automatic Traffic Recorder (ATR) counts were conducted over a three-day period between 11 AM Tuesday 3/27/18 and 12 AM Friday 3/30/18.
- Turning Movement Counts (TMC) were conducted on Thursday 3/29/18 and are used for pedestrian volumes, vehicle delay, and peak hour volumes where ATR data isn't available.
- Dashes indicate where hourly data is not available.
- Vehicle delay is in vehicle-hours and is calculated using Highway Capacity Manual 2010 Two-Way Stop Controlled methodology.

Table D-6
Summary of Hourly Volumes and Warrant Analyses
Route 138 at Blue Hill Terrace Street and Cheever Street, Milton

	ľ	Major Stre	et		Minor	Street			War	rants Satis	sfied	
Hourly		Route 138	3	Cheeve	r Street	Blue Hill	Terrace	Morront	Morront	Morrout	Morrout	Morront
period starting	NB	SB	Crossing	Е	В	W	В	Warrant 1	Warrant 2	Warrant 3	Warrant 4	Warrant 7
Starting	Volume	Volume	Peds	Volume	Delay	Volume	Delay] '	2	,	4	,
6:00	229	462	0	52	0.0	47	0.3					
7:00	347	548	2	156	0.2	153	13.1	✓	✓	✓		
8:00	436	508	0	119	0.1	108	1.5		✓			
9:00	265	428	-	57	-	-	-					
10:00	203	385	-	46	-	-	-					
11:00	224	322	-	52	-	-	-					0.4
12:00	229	307	-	47	-	-	-					correct-
13:00	163	322	-	48	-	-	-					able crash(es)
14:00	179	335	-	66	-	-	-					per year
15:00	226	413	7	77	0.1	59	0.5					2011–15
16:00	192	397	3	78	0.1	65	0.5					
17:00	177	444	1	74	0.1	77	0.8					
18:00	222	383	-	61	-	-	-					
19:00	186	286	-	48	-	-	-					
								NOT MET	NOT MET	MET	NOT MET	NOT MET

- Automatic Traffic Recorder (ATR) counts were conducted over a three-day period between 11 AM Tuesday 3/27/18 and 12 AM Friday 3/30/18.
- Turning Movement Counts (TMC) were conducted on Thursday 3/29/18 and are used for pedestrian volumes, vehicle delay, and peak hour volumes where ATR data isn't available.
- Dashes indicate where hourly data is not available.
- Vehicle delay is in vehicle-hours and is calculated using Highway Capacity Manual 2010 Two-Way Stop Controlled methodology.

Table D-7
Summary of Hourly Volumes and Warrant Analyses
Route 138 at Oak Street, Milton

	ľ	Major Stre	et		Minor	Street		Warrants Satisfied				
Hourly		Route 13	8	Oak S	Street	Oak S	Street	Morront	Morront	Morront	Morront	Morront
period starting	NB	SB	Crossing	Е	В	W	В	Warrant	Warrant 2	Warrant 3	Warrant 4	Warrant 7
Starting	Volume	Volume	Peds	Volume	Delay	Volume	Delay] '	2	,	4	,
6:00	229	462	1	3	0.0	11	0.0					
7:00	347	548	5	15	0.1	25	0.2					
8:00	436	508	19	14	0.1	7	0.1					
9:00	265	428	-	5	-	-	-					
10:00	203	385	-	5	-	-	-					
11:00	224	322	-	6	-	-	-					0.2
12:00	229	307	-	4	-	-	-					correct-
13:00	163	322	-	8	-	-	-					able crash(es)
14:00	179	335	-	8	-	-	-					per year
15:00	226	413	66	13	0.1	9	0.1					2011–15
16:00	192	397	8	7	0.0	19	0.1					
17:00	177	444	3	12	0.0	54	0.3					
18:00	222	383	-	12	-	-	-					
19:00	186	286	-	9	-	-	-					
								NOT	NOT	NOT	NOT	NOT
								MET	MET	MET	MET	MET

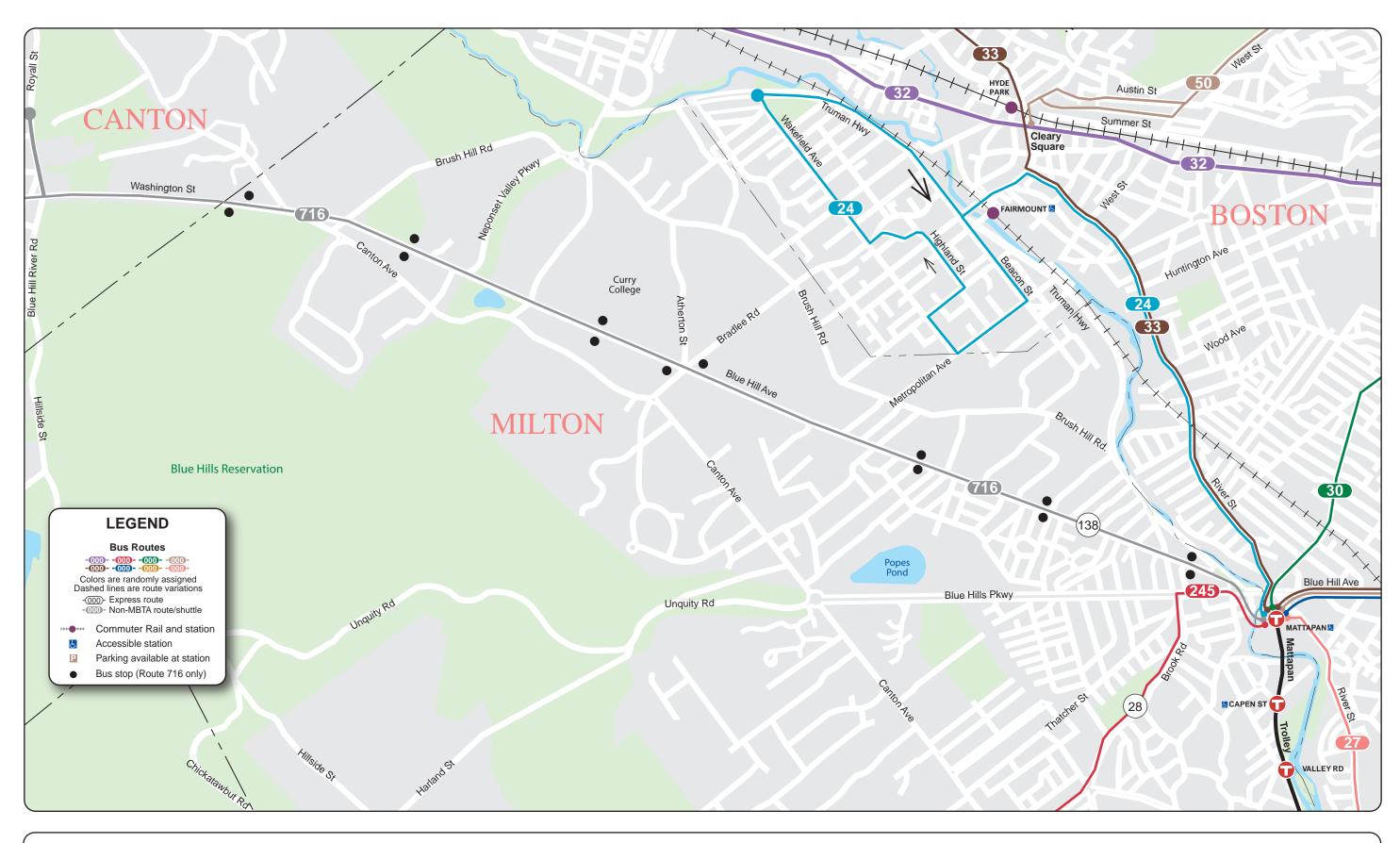
- Automatic Traffic Recorder (ATR) counts were conducted over a three-day period between 11 AM Tuesday 3/27/18 and 12 AM Friday 3/30/18.
- Turning Movement Counts (TMC) were conducted on Thursday 3/29/18 and are used for pedestrian volumes, vehicle delay, and peak hour volumes where ATR data isn't available.
- Dashes indicate where hourly data is not available.
- Vehicle delay is in vehicle-hours and is calculated using Highway Capacity Manual 2010 Two-Way Stop Controlled methodology.
- This intersection is near a school (Oak Street School) which may make it eligible for Warrant 5 School Crossing. However, this warrant requires a separate engineering study to assess property.

Table D–8
Summary of Hourly Volumes and Warrant Analyses
Route 138 at Brook Road, Milton

	ı	Major Stre	et	Minor	Street		Warrants Satisfied						
Hourly		Route 138	8	Brook	Road	Morront	Morront	Morront	Morront	Morront			
period starting	NB	SB	Crossing	E	В	Warrant 1	Warrant 2	Warrant 3	Warrant 4	Warrant 7			
otarting	Volume	Volume	Peds	Volume	Delay	'	2	3	7	•			
6:00	465	255	2	75	0.2								
7:00	530	434	3	180	2.3	✓	✓						
8:00	508	383	3	163	1.5	✓	✓						
9:00	-	-	-	-	-								
10:00	-	-	-	-	-								
11:00	-	-	-	-	-					1.0			
12:00	-	-	-	-	-					correct-			
13:00	-	-	-	-	-					able crash(es)			
14:00	-	-	-	-	-					per year			
15:00	454	480	2	171	2.2	1	✓			2011–15			
16:00	401	435	6	207	2.7	✓	✓						
17:00	437	482	6	186	1.4	✓	✓						
18:00	-	-	-	-	-								
19:00	-	-	-	-	-								
						NOT MET	MET	NOT MET	NOT MET	NOT MET			

- Automatic Traffic Recorder (ATR) counts were conducted over a three-day period between 11 AM Tuesday 3/27/18 and 12 AM Friday 3/30/18.
- Turning Movement Counts (TMC) were conducted on Thursday 3/29/18 and are used for pedestrian volumes, vehicle delay, and peak hour volumes where ATR data isn't available.
- Dashes indicate where hourly data is not available.
- Vehicle delay is in vehicle-hours and is calculated using Highway Capacity Manual 2010 Two-Way Stop Controlled methodology.

Appendix D: Bus Schedules



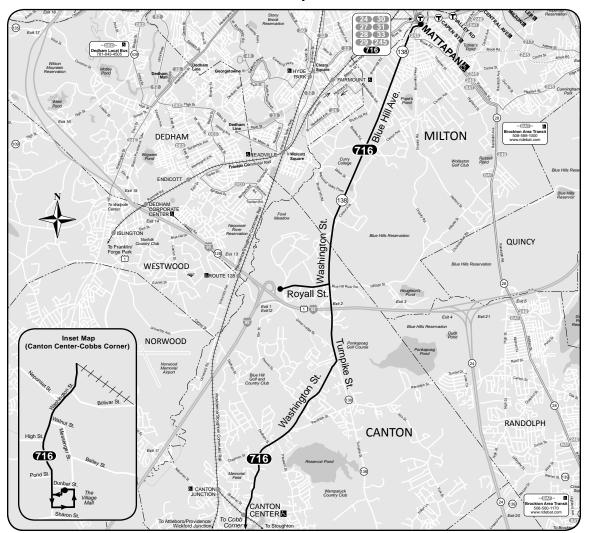
BOSTON REGION MPO

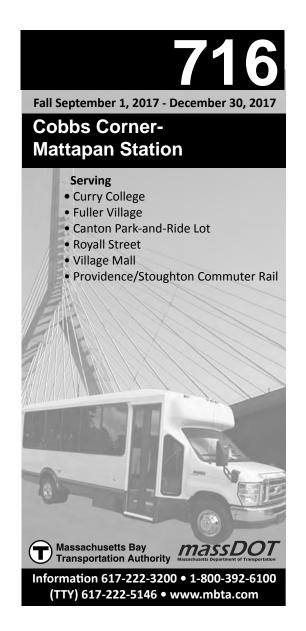


Figure 19
Regional Transit Service in the Study Area

Addressing Priority Corridors from the LRTP Needs Assessment: Route 138 in Milton

Route 716 Cobbs Corner - Mattapan Station





716		Inbound		Wee	kday		Outbound			
Cobbs Corner	Canton Center	Royall St.	Curry College	Mattapan Station	Mattapan Station	Curry College	Royall St. Trailside Museum Park-nRide	Canton Center	Cobbs Corner	
6:20A	6:25A	6:35A	6:45A	6:55A	5:50A	6:00A	6:05A	6:10A	6:15A	
7:40	7:45	7:55	8:10	8:25	7:00	7:10	7:20	7:30	7:35	
9:20	9:25	9:35	9:45	9:55	8:30	8:45	9:00	9:10	9:15	
10:40	10:45	10:55	11:05	11:15	10:00	10:10	10:20	10:30	10:35	
12:00N	12:05P	12:15P	12:25P	12:35P	11:20	11:30	11:40	11:50	11:55	
2:45P	2:50	3:00	3:10	3:20	2:05P	2:15P	2:25P	2:35P	2:40P	
4:15	4:25	4:40	4:55	5:10	3:25	3:40	3:55	4:05	4:10	
6:05	6:10	6:20	6:30	6:40	5:15	5:30	5:45	5:55	6:00	
					6:45	6:55	7:05	7:15	7:20	

Weekday

716		Inbound		Satu	ırday		Outbound		
Cobbs Corner	Canton Center	Royall St.	Curry College	Mattapan Station	Mattapan Station	Curry College	Royall St. Trailside Museum Park-nRide	Canton Center	Cobbs Corner
8:30A	8:34A	8:42A	8:50A	8:55A	8:00A	8:04A	8:12A	8:20A	8:25A
9:30	9:34	9:42	9:50	9:55	9:00	9:04	9:12	9:20	9:25
10:30	10:34	10:42	10:50	10:55	10:00	10:04	10:12	10:20	10:25
11:30	11:34	11:42	11:50	11:55	11:00	11:04	11:12	11:20	11:25
1:30P	1:34P	1:42P	1:50P	1:55P	1:00P	1:04P	1:12P	1:20P	1:25P
2:30	2:34	2:42	2:50	2:55	2:00	2:04	2:12	2:20	2:25
3:30	3:34	3:42	3:50	3:55	3:00	3:04	3:12	3:20	3:25
4:30	4:34	4:42	4:50	4:55	4:00	4:04	4:12	4:20	4:25
5:30	5:34	5:42	5:50	5:55	5:00	5:04	5:12	5:20	5:25

All buses are accessible to persons with disabilities

Fall 2017 Holidays

October 9: see Weekday November 11: see Saturday September 4, November 23 & December 25: see Sunday

No Service: Sundays

This service is operated by A&A Metro, 800-437-3844, under contract to the MBTA.

Bus will stop at any safe location along the route, except Royall Street which has designated stops. Please signal to the driver if you wish to board.

Rt.138 is dark and the traffic moves quickly. If you want to board the bus at a location which you do not use daily, please call A&A Metro at 800-437-3844. The dispatcher will tell the driver to look for you.

> Service subject to change. In the event of weather emergency please call the carrier.

Exact fare only.

FARE: \$1.70 STUDENT: \$0.85* **SENIOR/TAP:** \$0.85**

No transfers are given or accepted on this Route.

FREE FARES: Children 11 and under ride free when accompanied by an adult.

Blind Access CharlieCard holders ride free; if using a guide, the guide rides free.

- *Available to students through participating middle schools and high schools.
- **Available to Medicare cardholders, seniors 65+, and persons with disabilities.

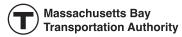
The following MBTA passes are accepted: Monthly Commuter Rail Zone 1A pass (or higher) recommended for frequent subway/bus customers.

Monthly Local Bus Pass on CharlieTicket only.

Monthly Senior/TAP pass on CharlieTicket available ONLY at the CharlieCard Store. Call 617-222-3200 or 617-222-5854 (TTY) for more information.

1 Day and 7 Day Link Passes.

PROVIDENCE/STOUGHTON LINE Effective May 22, 2017





Monday to Friday

Inbound to Boston			AM														PM																				
ONE STATION TRA	un#	800	802	900	804	902	806	842	808	904	810	812	906	814	908	816	910	818	820	912	822	824	914	826	916	828	918	830	920	922	832	924	834	836	926	838	92
Bikes Allowed																56	₫6	<i>6</i> %	₫6	₫6	50	66	640	₫6	949	₫6	50	<i>6</i> %	66	60	66	50	₫6	<i>6</i> %	66	<i>6</i> %	€
10 Wickford Junction	18	-	4:45	-	-	-	5:45	-	6:35	-	-	-	-	7:45	-	9:20	-	-	-	-	1:25	-	-	-	-	-	-	5:30	-	-	6:55	-	7:45	8:53	-	-	-
9 TF Green Airport	b	-	5:00	-	-	-	6:00	-	6:50	-	-	-	-	8:00	-	9:34	-	-	-	-	1:39	-	-	-	-	-	-	5:44	-	-	7:09	-	7:59	9:07	-	-	-
8 Providence	b	5:00	5:25	-	6:00	-	6:25	-	7:15	-	7:30	7:50	-	8:25	-	9:50	-	11:10	1:05	-	1:55	3:02	-	4:08	-	5:20	-	6:00	-	-	7:25	- 1	8:15	9:23	-	10:30	-
7 South Attleboro	8	5:09	5:34	-	6:09	-	6:34	-	7:25	-	7:39	7:59	-	8:34	-	9:59	-	11:19	1:16	-	2:06	3:13	-	4:20	-	5:29	-	6:09	-	-	7:34	-	8:24	9:34	-	10:39	-
7 Attleboro	b	5:19	5:44	-	6:19	-	6:44	7:12	7:35	-	7:49	8:09	-	8:44	-	10:09	-	11:29	1:26	-	2:16	3:23	-	4:30	-	5:39	-	6:19	-	-	7:44	- 1	8:34	9:44	-	10:49	-
6 Mansfield	8	5:29	5:54	-	6:29	-	6:54	7:22	7:45	-	7:59	8:19	-	8:54	-	10:19	-	11:39	1:36	-	2:26	3:33	-	4:40	-	5:48	-	6:28	-	-	7:54	-	8:44	9:54	-	10:59	-
4 Sharon	b	5:38	6:03	-	6:39	-	7:04	7:33		-	8:08	8:28	-	9:03	-	10:28	-	11:48	1:45	-	2:35	3:42	-	4:49	-	5:55	-	6:37	-	-	8:03	- 1	8:53	10:03	-	11:08	-
4 Stoughton	8	-	-	6:20	-	6:45	-	-	-	7:50	-	-	8:30	-	9:15	-	10:40	-	-	2:15	-	-	4:30	-	5:15	-	6:14	-	7:20	7:40	-	8:35	-	-	10:35	-	11:
3 Canton Center	b	-	-	6:28	-	6:53	-	-	-	7:58	-	-	8:38	-	9:23	-	10:48	-	-	2:23	-	-	4:38	-	5:23	-	6:22	-	7:28	-	-	8:43	-	-	10:43	-	11:4
3 Canton Junction	b	5:45	6:10	6:31	-	6:56	-	7:40	-	8:01	-	-	8:41	9:10	9:26	-	10:51	11:55	1:53	2:26	-	3:50	-	4:56	5:26		6:24	-	7:32	-	8:10	8:46	9:00	10:10	10:46	11:15	11:4
2 Route 128	b	5:50	6:15	6:37	6:47	7:02	7:12	7:45	-	8:06	8:16	8:36	8:46	9:15	9:31	10:37	10:56	12:00	1:58	2:31	2:43	3:55	4:47	5:01	5:31	6:03	6:29	6:45	7:37	-	8:15	8:51	9:05	10:15	10:51	11:20	11:
1 Hyde Park	8	5:55	6:20	6:43		7:08	-	7:51	-	-	8:21	-	8:51	9:20	9:36	-	11:01	12:05	2:03	-	2:48	4:00	-	5:06	5:36	-	6:34	-	7:42	-	8:20	-	9:10	10:20	10:56	11:25	11:5
1A Ruggles	b	L 6:04	L 6:30	-	-	-	L 7:26	-	-	L 8:20	-	-	-	L 9:29	-	L 10:50	-	L 12:14	-	-	-	-	-	-	-	-	-	-	-	-	-	- 1	-	-	-	-	-
IA Back Bay	b	L 6:08	L 6:34	L 6:52	L 6:59	L 7:17	L 7:30	L 8:00	L 8:10	L 8:24	L 8:30	L 8:49	L 9:00	L 9:33	L 9:45	L 10:54	L 11:12	L 12:18	L 2:12	L 2:44	L 2:57	L 4:10	L 5:02	L 5:17	L 5:45	L 6:17	L 6:43	L 6:55	L 7:51	L 8:10	L 8:29	L 9:03	L 9:19	L 10:29	L 11:05	L 11:34	L 12:
IA SOUTH STATION	8	6:14	6:40	6:58	7:05	7:23	7:36	8:06	8:16	8:30	8:36	8:55	9:06	9:39	9:51	10:59	11:17	12:23	2:17	2:49	3:02	4:15	5:07	5:22	5:50	6:22	6:48	7:00	7:56	8:15	8:34	9:08	9:24	10:34	11:10	11:39	12:
		Traine	in nurr	le boy i	ndicato	noak no	riod train	c																													

Manday to Eriday

Monday to Friday	/																																					
Outbound from Bos	ston	AM														PM																						
ZONE STATION	TRAIN#	8801	8803	901	801	8805	843	803	903	805	905	907	807	909	809 81	1 911	813	815	817	913	819	915	821	823	917	825	919	827	921	829	923	831	833	925	835	927	837 8	139
Bikes Allowed		6%	రాం	రశు	<i>6</i> %	<i>ర</i> శ్	8	<i>ర</i> శ్	రాం	<i>6</i> %	<i>రే</i> శ్రీ	<i>6</i> %	<i>రే</i> శ్రీ	<i>6</i> %	<i>6</i> % <i>6</i> 4	<i>6</i> %	6%	6%	<i>6</i> %												<i>రే</i> శ్రీ	<i>6</i> %	65	<i>6</i> %	66	66	<i>6</i> % 0	<i>6</i> %
1A SOUTH STATIC	ON &	-	-	5:20	5:30	-	6:18	6:31	6:59	7:25	7:35	8:23	9:35	9:45	10:25 11:	25 1:20	1:43	2:30	3:25	3:35	3:55	4:20	4:30	4:55	5:12	5:40	5:50	6:10	6:30	6:50	7:20	7:30	8:30	9:40	10:00	10:40	11:00 11	1:59
1A Back Bay	8	-	-	5:25	5:35	-	6:23	6:36	7:04	7:30	7:40	8:28	9:40	9:50	10:30 11:	30 1:25	1:48	2:35	3:30	3:40	4:00	4:25	4:35	5:00	5:17	5:45	5:55	6:15	6:35	6:55	7:25	7:35	8:35	9:45	10:05	10:45	11:05 17	2:04
1A Ruggles	8	-	-	-	-	-	-	6:39	-	7:33	-	-	9:44	9:53	10:33 11:	33 1:28	1:52	2:38	3:34	3:43	4:04	4:29	4:39	5:04	5:21	5:49	5:59	6:19	6:39	6:58	7:28	7:38	8:38	9:48	10:08	10:48	11:08 17	2:07
1 Hyde Park	8	-	-	-	-	-	-	-	-	-	-	-	-	10:01	10:43 -	1:38	-	-	-	3:53	-	-	-	-	5:32	-	6:10	-	6:49	7:08	7:38	-	8:46	-	10:18	10:58	11:18 12	2:17
2 Route 128	8	-	-	5:37	5:49	-	6:37	6:51	7:19	7:45	7:55	8:43	9:56	10:08	10:48 11:	15 1:43	2:04	2:51	-	3:58	-	4:42	4:53	5:18	5:43	-	6:16	-	6:54	7:13	7:43	7:52	8:51	10:02	10:23	11:03	11:23 12	2:22
3 Canton Junction	on &	-	-	-	-	-	-	6:56	7:25	7:51	8:01	8:49	10:02	10:14	10:54 -	1:49	-	2:57	-	4:04	-	4:48	-	-	5:49	-	6:23	-	7:00	7:19	7:49	7:58	8:57	10:08	10:29	11:09	11:29 12	2:28
3 Canton Center	r 8	-	-	5:45	-	-	-	-	7:28	-	8:04	8:52	-	10:17	- -	1:52	-	-	-	4:07	-	4:51	-	-	5:52	-	6:28	-	7:03	-	7:52	-	-	10:11	-	11:12	-	-
4 Stoughton	8	-	-	5:53	-	-	-	-	7:36	-	8:12	9:00	-	10:25		2:00) -	-	-	4:15	-	4:59	-	-	6:00	-	6:36	-	7:11	-	8:00	-	-	10:19	-	11:20	-	-
4 Sharon	8	-	-	-	5:57	-	-	7:02	-	7:57	-	-	10:08	-	11:00 11:	- 3	2:12	3:03	3:51	-	4:22	-	5:01	5:26	-	6:07	-	6:37	-	7:25	-	8:04	9:03	-	10:35	-	11:35 12	2:34
6 Mansfield	8	-	-	-	6:04	-	-	7:09	-	8:05	-	-	10:16	-	11:08 12:	00 -	2:20	3:11	3:59	-	4:30	-	5:09	5:34	-	6:15	-	6:45	-	7:33	-	8:12	9:11	-	10:43	-	11:43 12	2:42
7 Attleboro	8	-	-	-	6:12	-	6:55	7:17	-	8:13	-	-	10:24	-	11:16 12:)8 -	2:28	3:19	4:07	-	4:38	-	5:17	5:44	-	6:23	-	6:53	-	7:41	-	8:20	9:19	-	10:51	-	11:51 12	2:50
7 South Attlebor	ro &	-	-	-	6:21	-	-	7:26	-	8:20	-	-	10:34	-	11:26 12:	17 -	2:35	3:29	4:17	-	4:48	-	5:27	5:57	-	6:33	-	7:03	-	7:51	-	8:30	9:29	-	11:01	-	12:01 1:	:00
8 Providence	8	4:58	5:49	-	6:30	6:55	-	7:35	-	8:29	-	-	10:44	-	11:36 12:	26 -	2:45	3:39	4:27	-	4:58	-	5:37	6:07	-	6:43	-	7:13	-	8:01	-	8:40	9:39	-	11:11	-	12:11 1:	:10
9 TF Green Airpo	ort &	5:14	6:07	-	-	7:11	-	-	-	8:45	-	-	-	-	- 12:	12 -	-	-	4:43	-	-	-	5:53	-	-	6:59	-	-	-	8:17	-	-	-	-	11:27	-	-	-
10 Wickford Junct	tion 🕹	5:30	6:21	-	-	7:27	-	-	-	9:01	-	-	-	-	- 12:	- 88	-	-	4:59	-	-	-	6:09	-	-	7:15	-	-	-	8:33	-	-	-	-	11:43	-	-	-

Trains in purple box indicate peak period trains.

Sat	urday & Sun	day											S	aturday & Sunday	,									
	nbound to Bo	ston			AM		PM							Outbound from Bos		AM		PM						
SATURDAY TRAIN #			4#	1802	1804	1806	1808	1810	1812	1814	1816	1818	SATURDAY TRAIN		RAIN#	1801	1803	1805	1807	1809	1811	1813	1815	1817
ZOI	ONE STATION SUNDAY TRAIN #		4#	SAT ONLY	SAT ONLY	2806	2808	2810	2812	2814	2816	2818	ZONE STATION SUNDAY TRAIN #		SAT ONLY	SAT ONLY	2805	2807	2809	2811	2813	2815	2817	
	Bikes Allowed			₫6	₫6	ఈ	ఈ	₫6	₫6	645	<i>క</i> శ్	ఈ		Bikes Allowed		640	66	<i>6</i> %	<i>6</i> %	<i>6</i> %	<i>6</i> %	<i>రే</i> శ్రీ	<i>6</i> %	640
8	B Providence		8	6:35	8:35	11:20	12:55	2:56	4:56	7:05	8:52	10:00	1	A SOUTH STATIO	N 6	6:45	10:05	11:05	1:05	2:25	4:35	6:45	8:45	11:10
7	7 South Attle	boro	b	6:45	8:45	11:30	1:05	3:06	5:06	7:15	9:02	10:10	1	A Back Bay	8	6:50	10:10	11:10	1:10	2:30	4:40	6:50	8:50	11:15
7	7 Attleboro		8	6:53	8:53	11:38	1:13	3:14	5:14	7:23	9:10	10:18	1	A Ruggles	8	6:53	10:13	11:13	1:13	2:33	4:43	6:53	8:53	11:18
e	6 Mansfield		8	7:00	9:00	11:45	1:20	3:24	5:24	7:30	9:20	10:25		1 Hyde Park	8	7:01	10:21	11:21	1:21	2:41	4:51	7:01	9:01	11:26
4	Sharon		8	7:08	9:08	11:53	1:28	3:32	5:32	7:38	9:28	10:33		2 Route 128	8	7:06	10:26	11:31	1:31	2:46	4:56	7:06	9:06	11:31
3	Canton Jun	ction	8	7:15	9:15	12:00	1:35	3:39	5:39	7:45	9:35	10:40		3 Canton Junction	1 6	7:11	10:31	11:36	1:36	2:51	5:01	7:11	9:11	11:36
2	Route 128		8	7:20	9:20	12:05	1:40	3:44	5:44	7:50	9:40	10:45		4 Sharon	4	7:16	10:36	11:41	1:41	2:56	5:06	7:16	9:16	11:41
1	Hyde Park		8	7:25	9:25	12:10	1:45	3:49	5:49	7:55	9:45	10:50		6 Mansfield	8	7:24	10:44	11:49	1:49	3:04	5:14	7:24	9:24	11:49
1/	A Ruggles		8	L 7:35	L 9:35	L 12:22	L 1:55	L 3:59	L 5:59	L 8:05	L 9:55	L 11:00		7 Attleboro	8	7:31	10:51	11:56	1:56	3:11	5:21	7:31	9:31	11:56
1/	A Back Bay		8	L 7:38	L 9:38	L 12:25	L 1:58	L 4:02	L 6:02	L 8:08	L 9:58	L 11:03		7 South Attleboro	8	7:40	11:00	12:05	2:05	3:20	5:30	7:40	9:40	12:05
1/	A SOUTH STA	ATION	8	7:43	9:43	12:30	2:03	4:07	6:07	8:13	10:03	11:08		8 Providence	8	7:50	11:10	12:15	2:15	3:30	5:40	7:50	9:50	12:15

Trains 1802 and 1804 are Saturday only trains and will not operate on Sunday.

Trains 1801 and 1803 are Saturday only trains and will not operate on Sunday.

Keep in Mind:

This schedule will be effective from May 22, 2017 and will replace the schedule of November 21, 2016.

Presidents' Day and 4th of July operate on a Saturday service schedule.

New Year's Day, Memorial Day, Labor Day, Thanksgiving Day, and Christmas Day operate on a Sunday service schedule.

For all other holiday schedules, please check MBTA.com or call 617-222-3200.

For additional service to Ruggles Station, refer to the Needham and Franklin Line schedules for particular trains.

For additional service to Hyde Park Station, refer to the Franklin Line schedule for particular trains.

Times in purple with "f" indicate a flag stop: Passengers must tell the conductor that they wish to leave. Passengers waiting to board must be visible on the platform for the train to stop.



Bikes: Bicycles are allowed on trains with the bicycle symbol shown below the train number.









Appendix E: Traffic Safety Data

- 1. Collision Diagrams
- 2. Expected Crashes Analysis

Part 1: Collision Diagrams

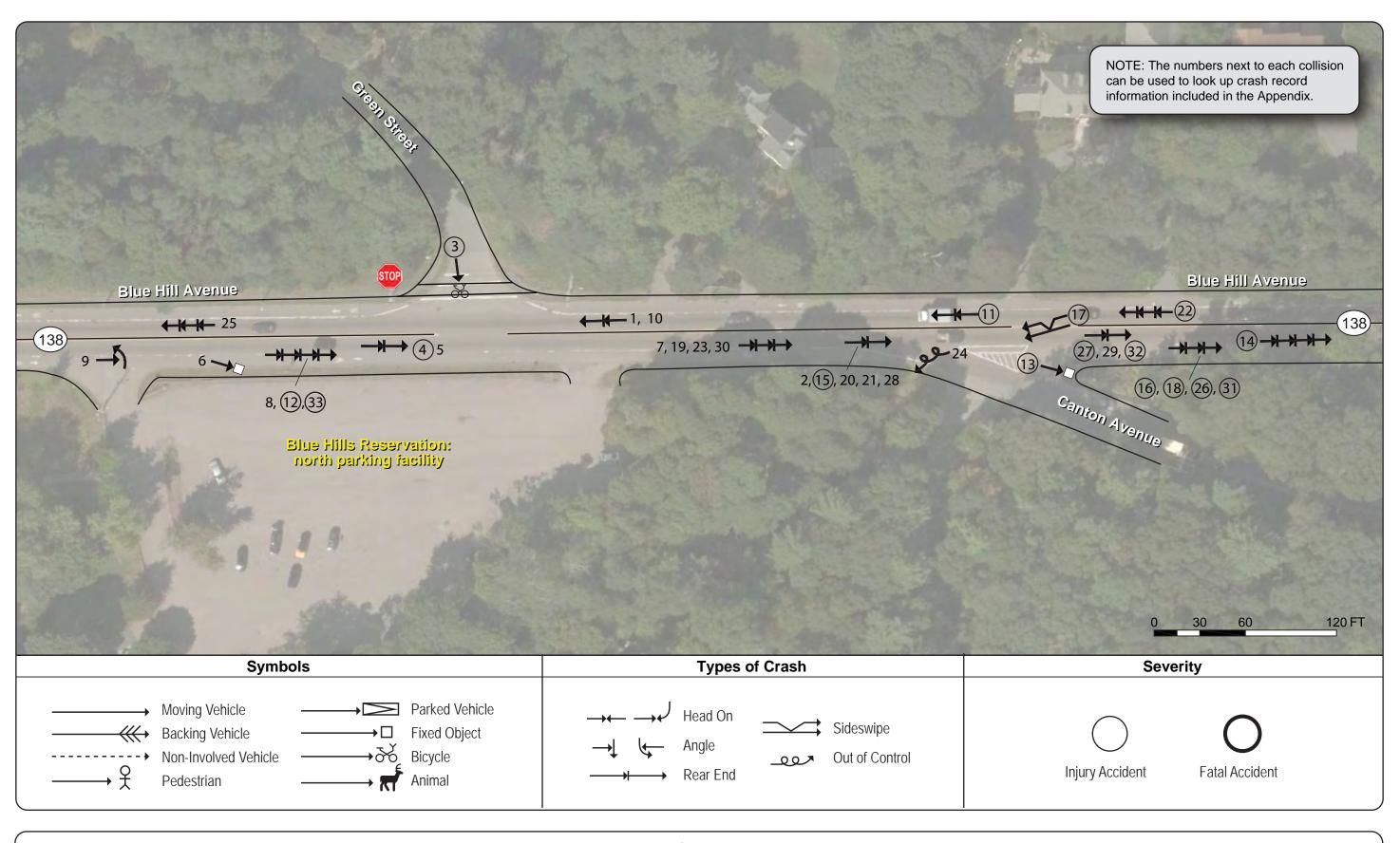




Table E–1
Route 138 at Green Street and Canton Avenue
Route 138 in Milton

Index	Crash Number	Crash Time	Crash Date	Street Address	Crash Severity	# of Vehicles	Manner of Collision		Ambient Light Condition	Weather Condition	Bike/ Ped	Vehicle Traveled Direction	Vehicle Action	Driver Contributing Code
1	2792473	5:11 PM	2011-10-07	Blue Hill Avenue Rte 138 S / Green Street Rte 138 S	Property damage only (n	o 2	Rear-end	Dry	Daylight	Clear/Clear		V1:Southbound / V2:Southbound	V1: Slowing or stopped in traffic / V2:Travelling straight	ah Can't see
2	3452138	9:17 AM	2013-05-29	Blue Hill Avenue Rte 138 N / Green Street	Property damage only (n	o 2	Rear-end	Wet	Daylight	Rain/Rain		V1:Northbound / V2:Northbound	V1: Travelling straight ahead / V2:Slowing or stopped in	tra Distracted
3	3714615	8:54 AM	2013-11-13	Blue Hill Avenue Rte 138 / Green Street / Rte 138	Non-fatal injury	1	Angle	Dry	Daylight	Clear	сус	V1:Eastbound	V1: Slowing or stopped in traffic	Wrong way
4	3739832	2:56 PM	2013-12-19	Green Street / Canton Avenue Rte 138 N	Not Reported	2	Rear-end	Dry	Daylight	Clear		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Turning left	
5	3801600	8:44 AM	2014-04-17	Canton Avenue / Green Street	Property damage only (n	o 2	Rear-end	Dry	Daylight	Clear/Clear		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straight	ah
6	3867884	7:55 AM	2014-06-20	Green Street / Blue Hill Avenue Rte 138 N	Property damage only (n	o 1	Single vehicle crash	Dry	Daylight	Clear		V1:Northbound	V1: Leaving traffic lane	Object in road
7	3881075	8:12 AM	2014-07-09	Blue Hill Avenue Rte Sr138 N / Green Street	Property damage only (n	o 3	Rear-end	Dry	Daylight	Clear		V1:Northbound / V2:Northbound / V3:	Nc V1: Travelling straight ahead / V2:Slowing or stopped in	tra Inattention
8	3950292	6:53 AM	2014-09-22	Blue Hill Avenue Rte Sr138 N / Green Street	Non-fatal injury	5	Angle	Dry	Daylight	Clear		V1:Northbound / V2:Northbound / V3:	Sc V1: Travelling straight ahead / V2:Slowing or stopped in	tra Tailgating
9	4030462	9:03 PM	2015-02-27	1900 Blue Hill Avenue	Property damage only (n	o 2	Angle	Dry	Dark - lighted	r Clear/Clear		V1:Southbound / V2:Northbound	V1: Turning left / V2:Travelling straight ahead	Disregarding signs
10	4100400	3:27 PM	2015-06-19	Blue Hill Avenue Rte Sr138 N / Green Street	Property damage only (n	o 2	Rear-end	Dry	Daylight	Clear		V1:Southbound / V2:Southbound	V1: Slowing or stopped in traffic / V2:Travelling straight	ah Inattention
11	2712795	3:18 PM	2011-02-21	Blue Hill Avenue Rte 138 S / Canton Avenue	Non-fatal injury	2	Rear-end	Dry	Daylight	Clear/Clear		V1:Southbound / V2:Southbound	V1: Turning left / V2:Travelling straight ahead	
12	2718234	3:51 PM	2011-04-15	Blue Hill Avenue / Canton Avenue	Property damage only (n	o 4	Rear-end	Dry	Daylight	Clear/Clear		V1:Northbound / V2:Northbound / V3:	Nc V1: Slowing or stopped in traffic / V2:Travelling straight	ah
13	2760086	6:31 PM	2011-08-22	Rte 138 / Blue Hill Avenue / Canton Avenue	Non-fatal injury	1	Single vehicle crash	Dry	Daylight	Clear/Clear		V1:Northbound	V1: Changing lanes	Reckless
14	3108532	10:24 AM	2012-05-12	Blue Hill Avenue / Canton Avenue	Non-fatal injury	5	Rear-end	Dry	Daylight	Clear		V1:Northbound / V2:Northbound / V3:	Nc V1: Slowing or stopped in traffic / V2:Slowing or stopped	d ir
15	3233106	6:41 AM	2012-07-31	Blue Hill Avenue / Canton Avenue	Non-fatal injury	2	Rear-end	Dry	Daylight	Clear/Clear		V1:Northbound / V2:Northbound	V1: Travelling straight ahead / V2:Slowing or stopped in	tri
16	3372335	5:07 PM	2013-02-05	Blue Hill Avenue Rte 138 / Canton Avenue Rte 138	Property damage only (n	о 3	Rear-end	Dry	Dusk	Clear		V1:Northbound / V2:Northbound / V3:	Nc V1: Slowing or stopped in traffic / V2:Slowing or stopped	d ir
17	4003513	1:05 PM	2014-12-21	Blue Hill Avenue / Canton Avenue	Property damage only (n	o 2	Sideswipe, same direct	ic Wet	Daylight	Clear		V1:Eastbound / V2:Eastbound	V1: Travelling straight ahead / V2:Turning left	Fail to yield
18	4113690	4:44 PM	2015-06-07	Canton Avenue / Blue Hill Avenue	Non-fatal injury	4	Rear-end	Dry	Daylight	Clear/Clear		V1:Not reported / V2:Northbound / V3	:N V1: Not reported / V2:Travelling straight ahead / V3:Trav	ve
19	4117529	12:34 PM	2015-09-26	Blue Hill Avenue / Canton Avenue	Property damage only (n	о 3	Rear-end	Dry	Daylight	Clear/Clear		V1:Northbound / V2:Northbound / V3:	Nc V1: Travelling straight ahead / V2:Travelling straight ahe	eac
20	4130814	8:02 AM	2015-12-15	Blue Hill Avenue Rte 138 N / Canton Avenue	Property damage only (n	o 2	Rear-end	Wet	Daylight	Other/Rain		V1:Northbound / V2:Northbound	V1: Travelling straight ahead / V2:Travelling straight ahe	eac
21	4130808	8:17 AM	2015-12-15	Blue Hill Avenue Rte 138 N / Canton Avenue	Property damage only (n	o 2	Rear-end	Wet	Daylight	Cloudy/Cloudy		V1:Northbound / V2:Northbound	V1: Travelling straight ahead / V2:Slowing or stopped in	tra
22	4162330	8:19 AM	2015-12-15	Blue Hill Avenue / Canton Avenue	Property damage only (n	о 3	Rear-end	Wet	Daylight	Clear/Clear		V1:Southbound / V2:Southbound / V3	:S ₁ V1: Travelling straight ahead / V2:Slowing or stopped in	tr: Inattention
23	2955664	6:53 AM	2011-12-09	Blue Hill Avenue / Canton Avenue	Property damage only (n	о 3	Rear-end	Dry	Daylight	Cloudy		V1:Northbound / V2:Northbound / V3:	Nc V1: Slowing or stopped in traffic / V2:Slowing or stopped	d ir
24	2995291	5:10 PM	2012-01-19	Canton Avenue / Blue Hill Avenue	Property damage only (n	o 1	Single vehicle crash	Dry	Dark - lighted	r Clear/Clear		V1:Westbound	V1: Travelling straight ahead	
25	4048687	3:35 PM	2015-04-17	Near Canton Line	Property damage only (n	o 3	Rear-end	Dry	Daylight	Cloudy/Cloudy		V1:Southbound / V2:Southbound / V3	:S(V1: Slowing or stopped in traffic / V2:Travelling straight	ah
26	3728108	8:02 AM	2013-11-18	1433 Blue Hill Avenue	Non-fatal injury	3	Rear-end	Wet	Daylight	Rain		V1:Northbound / V2:Northbound / V3:	Nc V1: Slowing or stopped in traffic / V2:Slowing or stopped	d ir
27	2792465	11:49 AM	2011-10-13	1433 Blue Hill Avenue	Non-fatal injury	2	Rear-end	Wet	Daylight	Rain		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straight	ah Tailgating
28	3974992	9:14 AM	2014-11-14	100 Feet S From Intersection Blue Hill Avenue Rte 138 N / Canton Ave	er Property damage only (n	o 2	Rear-end	Wet	Daylight	Clear		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straight	ah
29	2789099	2:08 PM	2011-09-23	1434 Blue Hill Avenue	Property damage only (n	o 2	Rear-end	Dry	Daylight	Clear		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straight	ah Tailgating
30	3057933	2:53 PM	2012-04-25	150 Feet S From Intersection Blue Hill Avenue / Canton Avenue	Property damage only (n	o 3	Rear-end	Unknown	Daylight	Clear		V1:Northbound / V2:Northbound / V3:	Nc V1: Slowing or stopped in traffic / V2:Slowing or stopped	d ir
31	3427633	1:27 PM	2013-04-27	100 Feet N From Intersection 1433 Blue Hill Avenue Rte 138 N / Canto	or Non-fatal injury	3	Rear-end	Dry	Daylight	Clear/Clear		V1:Northbound / V2:Northbound / V3:	Nc V1: Slowing or stopped in traffic / V2:Travelling straight	ah Tailgating
32	3049855	5:30 PM	2012-02-05	Rte 138 N Milemarker 40.4	Non-fatal injury	2	Rear-end	Dry	Dark - roadw	a ₎ Clear		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straight	
33	3292260	8:35 AM	2012-10-10	400 Feet S From Intersection 1425 Blue Hill Avenue Rte 138 N / Greet	n Non-fatal injury	5	Rear-end	Wet	Daylight	Rain		V1:Northbound / V2:Northbound / V3:	Nc V1: Travelling straight ahead / V2:Slowing or stopped in	tra

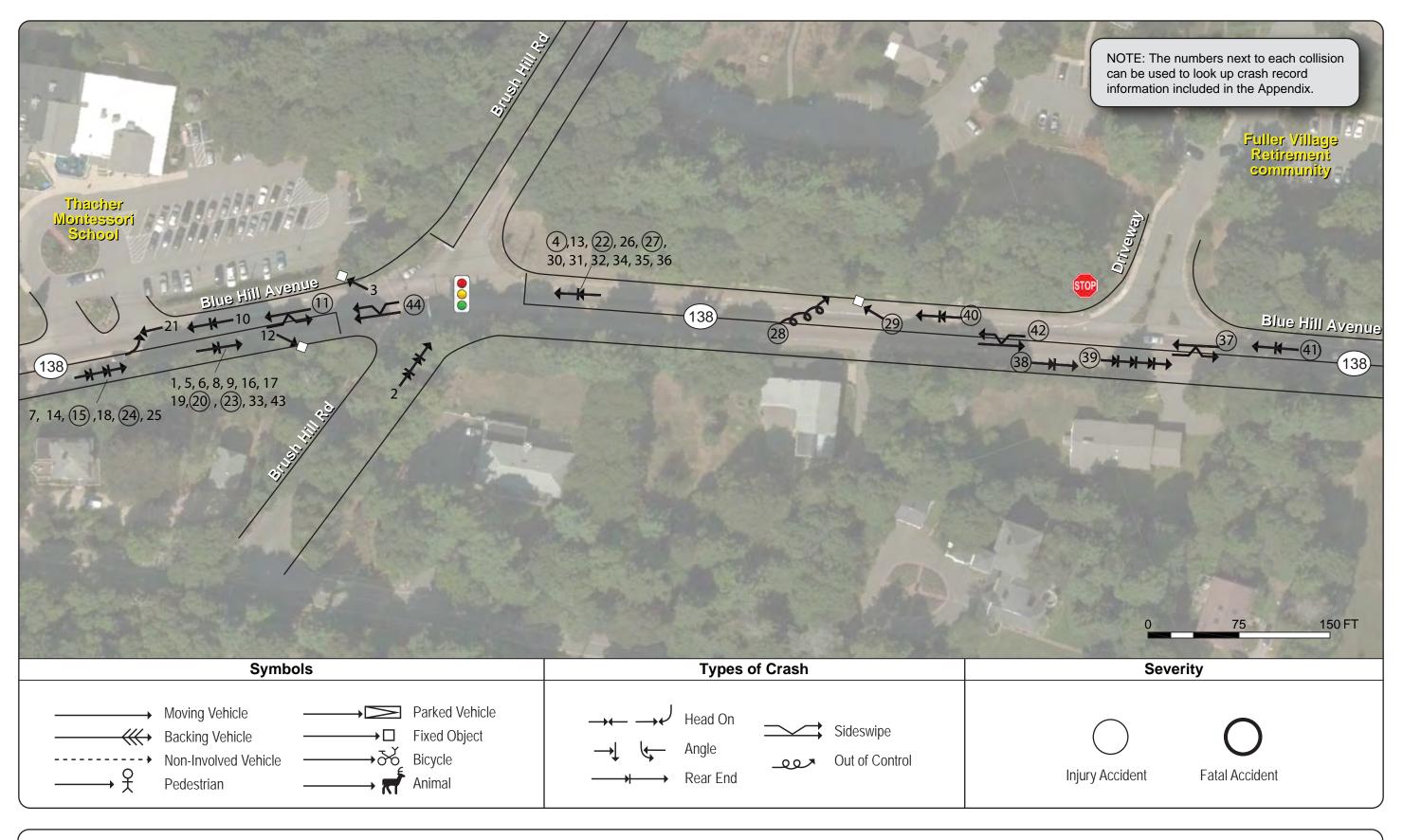




Figure E-2
Collision Diagram 2011-15
Route 138 at Brush Hill Road

Table E–2
Route 138 at Brush Hill Road
Route 138 in Milton

Index	Crash Number	Crash Time		Street Address	Crash Severity	# of Vehicles	Manner of Collision	Condition	Ambient Light Condition	Weather Condition	Bike/ Ped	Vehicle Traveled Direction	Vehicle Action	Driver Contributing Code
1	3728098	1:34 PM	2013-11-22	Blue Hill Avenue / Canton Avenue Rte 138 N	Property damage only (n		Rear-end	Wet	Daylight	Rain		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straigh	
2	4066116	3:16 PM	2015-07-09	Blue Hill Avenue / Canton Avenue / Brush Hill Road	Property damage only (n	o 3	Rear-end	Dry	Daylight	Clear/Clear			e V1: Slowing or stopped in traffic / V2:Slowing or stoppe	ed ir
3	3936933	3:07 PM	2014-08-09	Canton Avenue Rte 138 / Blue Hill Avenue	Property damage only (n	o 1	Single vehicle crash	Dry	Daylight	Clear/Clear		V1:Westbound	V1: Not reported	
4	2712788	5:11 PM	2011-03-09	1396 Blue Hill Avenue	Non-fatal injury	3	Rear-end	Dry	Daylight	Clear/Clear			Si V1: Slowing or stopped in traffic / V2:Slowing or stopped	
5	3527031	10:22 AM	2013-07-01	300 Feet S From Intersection 1434 Blue Hill Avenue / Brush Hill Road F	<u> </u>		Rear-end	Dry	Daylight	Cloudy		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straigh	
6	4009891	2:43 PM	2015-01-24	250 Feet S From Intersection 1425 Blue Hill Avenue / Brush Hill Road			Rear-end	Wet	Daylight	Cloudy/Snow		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straigh	0 0
7	4149577	3:07 PM	2015-11-26	200 Feet S From Intersection Blue Hill Avenue Rte 138 S / Brush Hill Re	Property damage only (n	o 3	Rear-end	Dry	Daylight	Clear			c V1: Slowing or stopped in traffic / V2:Slowing or stoppe	
8	4132326	4:31 PM	2015-10-24	1425 Blue Hill Avenue Rte 138 S / Brush Hill Road	Property damage only (n		Rear-end	Dry	Daylight	Clear/Clear		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straigh	t ah
9	3058836	12:47 PM	2012-03-28	100 Feet S From Intersection 1425 Blue Hill Avenue Rte 138 / Brush Hi	Property damage only (n	o 2	Rear-end	Wet	Daylight	Rain/Cloudy		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straigh	
10	3235294	7:15 PM	2012-07-24	1399 Blue Hill Avenue / Brush Hill Road	Property damage only (n	o 2	Rear-end	Wet	Daylight	Rain		V1:Southbound / V2:Southbound	V1: Slowing or stopped in traffic / V2:Slowing or stopped	ed ir
11	3282193	3:35 PM	2012-08-11	1425 Blue Hill Avenue / Brush Hill Road	Non-fatal injury	2	Sideswipe, opposite dire	e Dry	Dark - lighted	r Clear/Clear		V1:Southbound / V2:Northbound	V1: Other / V2:Travelling straight ahead	Reckless
12	3401946	12:00 AM	2013-04-26	Brush Hill Road / Blue Hill Avenue Rte 138 S	Property damage only (n	o 1	Single vehicle crash	Dry	Dark - lighted	r Clear		V1:Northbound	V1: Travelling straight ahead	
13	3927834	2:21 PM	2014-09-02	1399 Blue Hill Avenue Rte 138 S / Brush Hill Road	Property damage only (n	o 2	Rear-end	Dry	Daylight	Cloudy		V1:Southbound / V2:Southbound	V1: Slowing or stopped in traffic / V2:Travelling straigh	t ah
14	3983025	6:45 PM	2014-10-31	1425 Blue Hill Avenue / Brush Hill Road	Property damage only (n	o 3	Rear-end	Dry	Dark - lighted	r Clear/Clear		V1:Northbound / V2:Northbound / V3:N	c V1: Travelling straight ahead / V2:Travelling straight ah	neac
15	3983031	10:43 AM	2014-11-08	1431 Blue Hill Avenue Rte 138 N / Brush Hill Road	Non-fatal injury	3	Rear-end	Dry	Daylight	Clear		V1:Northbound / V2:Northbound / V3:N	c V1: Slowing or stopped in traffic / V2:Slowing or stopped	ed ir
16	4033161	7:16 AM	2015-04-08	1433 Blue Hill Avenue / Rte 138	Property damage only (n	o 2	Rear-end	Dry	Daylight	Clear		V1:Northbound / V2:Northbound	V1: Travelling straight ahead / V2:Travelling straight ah	near Inattention
17	2829506	9:09 AM	2011-11-04	Blue Hill Avenue Rte 138 S / Brush Hill Road	Property damage only (n	o 2	Rear-end	Dry	Daylight	Cloudy		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straigh	t ah Inattention
18	2976531	8:48 AM	2012-03-09	1425 Blue Hill Avenue / Brush Hill Road	Property damage only (n	о 3	Rear-end	Dry	Daylight	Clear		V1:Northbound / V2:Northbound / V3:N	c V1: Slowing or stopped in traffic / V2:Slowing or stoppe	ed ir
19	2742092	10:27 PM	2011-07-08	1425 Blue Hill Avenue	Property damage only (n	o 2	Rear-end	Wet	Dark - lighted	r Rain		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Slowing or stopped	ed ir
20	3336270	9:02 PM	2012-09-02	1425 Blue Hill Avenue	Non-fatal injury	2	Rear-end	Dry	Dark - lighted	r Clear		V1:Northbound / V2:Northbound	V1: Travelling straight ahead / V2:Travelling straight ah	neac
21	3297115	3:43 PM	2012-10-18	1425 Blue Hill Avenue	Property damage only (n	o 2	Angle	Dry	Daylight	Clear		V1:Westbound / V2:Southbound	V1: Turning left / V2:Travelling straight ahead	
22	3389819	6:07 PM	2013-03-03	1425 Blue Hill Avenue	Non-fatal injury	2	Rear-end	Wet	Dark - lighted	r Cloudy/Rain		V1:Southbound / V2:Southbound	V1: Slowing or stopped in traffic / V2:Slowing or stopped	ed ir
23	3602794	2:46 PM	2013-09-12	Blue Hill Brush	Non-fatal injury	2	Rear-end	Dry	Daylight	Clear		V1:Northbound / V2:Northbound	V1: Travelling straight ahead / V2:Travelling straight ah	neac
24	4033165	7:32 AM	2015-04-08	1425 Blue Hill Avenue	Non-fatal injury	3	Rear-end	Dry	Daylight	Clear		V1:Northbound / V2:Northbound / V3:N	c V1: Slowing or stopped in traffic / V2:Slowing or stoppe	ed ir
25	4125339	3:30 PM	2015-10-16	1425 Blue Hill Avenue	Property damage only (n	о 3	Rear-end	Dry	Daylight	Clear		V1:Northbound / V2:Northbound / V3:N	c V1: Travelling straight ahead / V2:Slowing or stopped i	n tra
26	3342530	2:05 PM	2012-11-22	200 Feet N From Intersection 1399 Blue Hill Avenue Rte 138 S / Brush	Property damage only (n	o 2	Rear-end	Dry	Daylight	Clear		V1:Southbound / V2:Southbound	V1: Slowing or stopped in traffic / V2:Travelling straigh	t ah
27	2760735	9:59 AM	2011-08-07	250 Feet N From Intersection 1396 Blue Hill Avenue Rte 138 / Brush Hi	Non-fatal injury	3	Rear-end	Wet	Daylight	Rain		V1:Southbound / V2:Southbound / V3:N	V1: Travelling straight ahead / V2:Travelling straight al	near Illness
28	3125993	12:00 AM	2011-03-06	1396 Blue Hill Avenue	Non-fatal injury	2	Single vehicle crash	Dry	Dark - lighted	r Clear		V1:Northbound / V2:Southbound	V1: Turning right / V2:Travelling straight ahead	
29	3392809	11:37 AM	2013-02-13	1399 Blue Hill Avenue	Not Reported	1	Single vehicle crash	Dry	Daylight	Clear/Clear		V1:Southbound	V1: Travelling straight ahead	
30	3451832	2:27 PM	2013-06-01	400 Feet N From Intersection 1399 Blue Hill Avenue Rte 138 S / Bush H	Property damage only (n	o 2	Rear-end	Dry	Daylight	Clear		V1:Southbound / V2:Southbound	V1: Slowing or stopped in traffic / V2:Travelling straigh	t ah Tailgating
31	3801605	4:49 PM	2014-04-12	1399 Blue Hill Avenue	Property damage only (n	o 2	Rear-end	Dry	Daylight	Clear		V1:Southbound / V2:Southbound	V1: Travelling straight ahead / V2:Slowing or stopped i	
32	3805539	3:05 PM	2014-05-02	1399 Blue Hill Avenue	Property damage only (n	o 2	Rear-end	Dry	Daylight	Clear		V1:Southbound / V2:Southbound	V1: Slowing or stopped in traffic / V2:Travelling straigh	
33	4033163	1:32 PM	2015-04-14	1399 Blue Hill Avenue	Property damage only (n		Rear-end	Wet	Daylight	Clear		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Slowing or stopped	
34	4170427	12:42 PM	2015-12-05	1399 Blue Hill Avenue	Property damage only (n	o 2	Rear-end	Dry	Daylight	Clear/Clear		V1:Southbound / V2:Southbound	V1: Slowing or stopped in traffic / V2:Travelling straigh	
35	3950620	2:41 PM	2014-09-10	Blue Hill Bushes	Property damage only (n		Rear-end	Dry	Daylight	Clear		V1:Southbound / V2:Southbound	V1: Travelling straight ahead / V2:Travelling straight ah	
36	3588718	1:56 PM	2013-09-07	500 Feet N From Intersection 1399 Blue Hill Avenue Rte 138 S / Brush			Rear-end	Dry	Daylight	Clear		V1:Southbound / V2:Southbound	V1: Travelling straight ahead / V2:Slowing or stopped i	
37	3998257	3:16 PM	2015-01-15	1338 Blue Hill Avenue	Non-fatal injury	2	Sideswipe, opposite dire		Daylight	Clear		V1:Southbound / V2:Northbound	V1: Travelling straight ahead / V2:Unknown	<u> </u>
38	2789104	10:45 AM	2011-09-28	Blue Hill Avenue Rte 138 N / Caroline Drive	Non-fatal injury	2	Rear-end	Dry	Daylight	Clear		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straigh	t ah
39	3282198	1:29 PM	2012-08-08	Blue Hill Avenue / Caroline Drive	Not Reported	4	Rear-end		Daylight	Clear			V1: Slowing or stopped in traffic / V2:Slowing or stopped	
40	3288057	5:52 PM	2012-09-20	Blue Hill Avenue / Caroline Drive	Not Reported	2	Rear-end	Dry	Daylight	Clear/Clear		V1:Southbound / V2:Southbound	V1: Travelling straight ahead / V2:Slowing or stopped i	
41	3981148	1:43 PM	2014-11-25	Blue Hill Avenue Rte 138 / Caroline Drive Rte 138	Property damage only (n		Rear-end	Dry	Daylight	Cloudy		V1:Southbound / V2:Southbound	V1: Slowing or stopped in traffic / V2:Travelling straigh	
42	4035198	4:12 AM	2015-02-07	Blue Hill Avenue Rte Sr138 N / Caroline Drive	Non-fatal injury	2	Sideswipe, opposite dire		Dark - lighted			V1:Northbound / V2:Southbound	V1: Travelling straight ahead / V2:Travelling straight ah	
43	3950294	1:21 PM	2014-08-25	1300 Blue Hill Avenue	Property damage only (n		Rear-end	Dry	Daylight	Clear/Clear		V1:Northbound / V2:Northbound	V1: Not reported / V2:Not reported	Tailgating
44	3393018	12:08 PM	2013-02-11	78 Blue Hill Avenue	Non-fatal injury	2	Sideswipe, same directi		Daylight	Sleet, hail (free		V1:Southbound / V2:Southbound	V1: Travelling straight ahead / V2:Travelling straight ah	
	0000010	12.001111	_010 02 11	. 5 5.55 mil / (10100	iatai injury		5.355mps, buille ullout		Jajiigiit	3.00t, ridii (i10t				

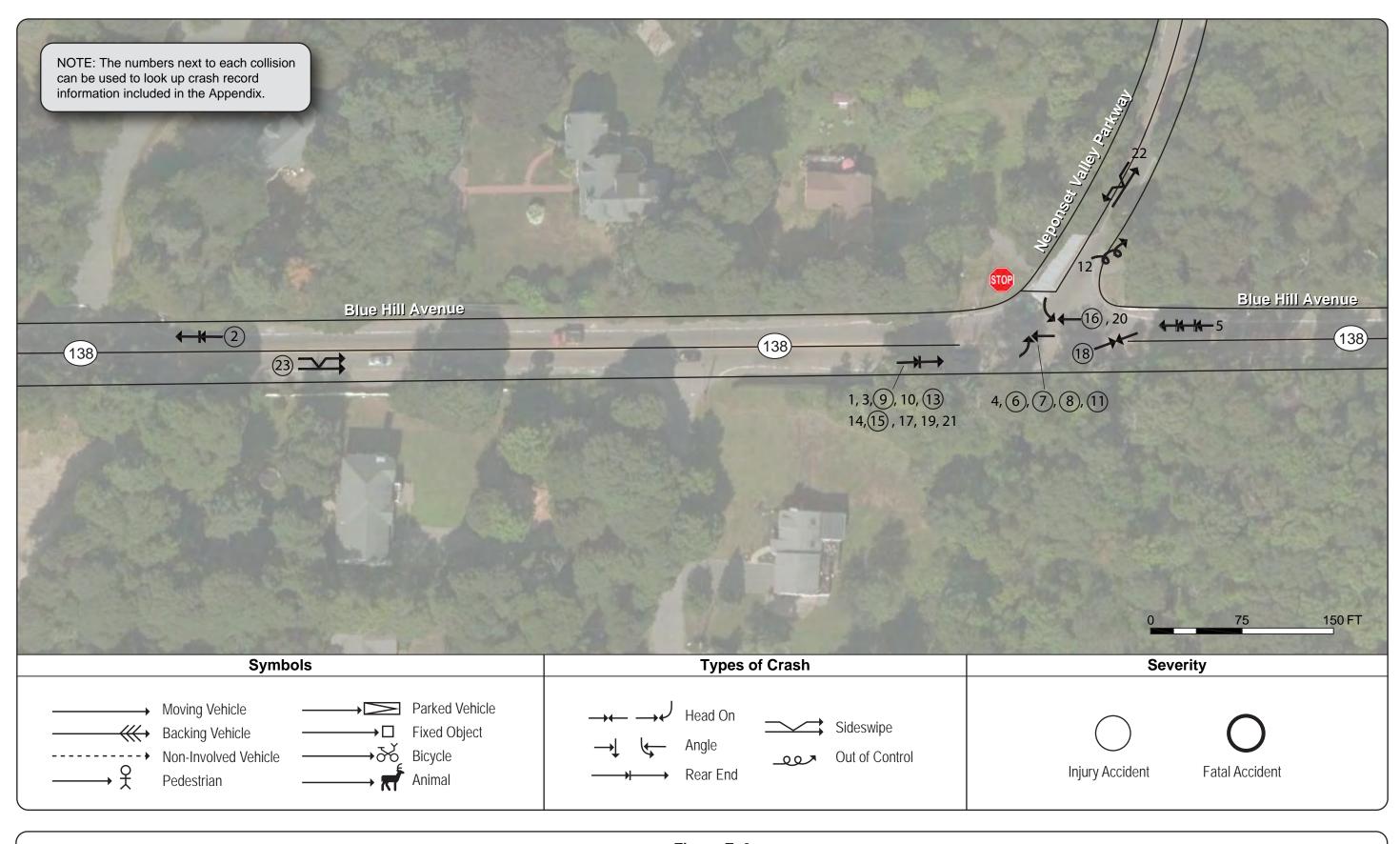




Figure E-3
Collision Diagram 2011-15
Route 138 at Neponset Valley Parkway

Table E–3
Route 138 at Neponset Valley Parkway
Route 138 in Milton

Index	Crash Number	Crash Time	Crash Date	Street Address	Crash Severity	# of Vehicles	Manner of Collision		Ambient Light Condition		Bike/ Ped	Vehicle Traveled Direction	Vehicle Action	Driver Contributing Code
1	3372344	8:53 PM	2013-02-04	Blue Hill Avenue Rte 138 / Surrey Lane	Property damage only (no	2	Rear-end	Dry	Dark - lighted	r Clear		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Slowing or stopped	ir
2	3148834	4:10 PM	2012-06-14	Blue Hill Avenue / Surrey Lane	Non-fatal injury	2	Rear-end	Dry	Daylight	Clear/Clear		V1:Southbound / V2:Southbound	V1: Slowing or stopped in traffic / V2:Slowing or stopped	ir
3	4128264	9:42 AM	2015-10-05	Blue Hill Avenue Rte 138 N / Surrey Lane	Property damage only (no	2	Rear-end	Dry	Daylight	Cloudy		V1:Northbound / V2:Northbound	V1: Travelling straight ahead / V2:Slowing or stopped in	tra
4	2732045	4:48 PM	2011-06-08	Blue Hill Avenue / Neponset Valley Parkway	Property damage only (no	2	Sideswipe, opposite dire	e Dry	Daylight	Clear		V1:Westbound / V2:Southbound	V1: Turning left / V2:Travelling straight ahead	
5	2703346	9:42 AM	2011-02-08	Blue Hill Avenue Rte 138 S / Neponset Valley Parkway	Property damage only (no	3	Rear-end	Wet	Daylight	Sleet, hail (freezi		V1:Southbound / V2:Southbound / V3:	So V1: Slowing or stopped in traffic / V2:Slowing or stopped	ir Tailgating
6	2792452	4:40 PM	2011-10-14	Blue Hill Avenue Rte 138 S / Blue Hill Avenue Rte 138 / Neponset Valle	y Non-fatal injury	2	Angle	Wet	Daylight	Rain/Cloudy		V1:Southbound / V2:Northbound	V1: Travelling straight ahead / V2:Turning left	
7	2792462	10:54 PM	2011-10-14	Neponset Valley Parkway Rte 138 / Blue Hill Avenue Rte 138 / Nepons	e Non-fatal injury	2	Angle	Wet	Dark - lighted	r Rain		V1:Southbound / V2:Northbound	V1: Travelling straight ahead / V2:Turning left	Fail to yield
8	2869265	10:59 PM	2011-12-29	Blue Hill Avenue Rte 138 / Neponset Valley Parkway	Non-fatal injury	2	Angle	Dry	Daylight	Clear		V1:Southbound / V2:Westbound	V1: Travelling straight ahead / V2:Turning left	Fail to yield
9	3069349	4:47 PM	2012-04-20	Blue Hill Avenue / Neponset Valley Parkway	Unknown	2	Rear-end	Dry	Daylight	Clear		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straight a	h
10	3195932	10:20 PM	2012-05-22	Blue Hill Avenue / Neponset Valley Parkway	Property damage only (no	2	Rear-end	Wet	Dark - lighted	r Rain		V1:Northbound / V2:Northbound	V1: Turning left / V2:Travelling straight ahead	
11	3728237	6:08 PM	2013-11-22	Blue Hill Avenue / Neponset Valley Parkway	Non-fatal injury	2	Head-on	Wet	Dark - lighted	r Rain/Rain		V1:Northbound / V2:Southbound	V1: Turning left / V2:Travelling straight ahead	
12	3743362	2:39 PM	2014-02-18	Neponset Valley Parkway Rte Unknow / Blue Hill Avenue	Property damage only (no	2	Rear-End	Snow/Ice	Daylight	Snow		V1:Westbound / V2:Westbound	V1: Travelling straight ahead / V2:Travelling straight ahe	ac
13	3801603	2:23 PM	2014-04-14	Blue Hill Avenue / Neponset Valley Parkway / Blue Hill Avenue	Non-fatal injury	1	Rear-end	Dry	Daylight	Clear		V1:Northbound	V1: Turning left	
14	3827609	10:06 PM	2014-05-09	Blue Hill Avenue Rte 138 N / Neponset Valley Parkway	Property damage only (no	2	Rear-end	Dry	Dark - lighted	r Clear		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straight a	h Inattention
15	3867860	9:41 PM	2014-06-10	Neponset Valley Parkway / Blue Hill Avenue	Non-fatal injury	2	Rear-end	Dry	Dark - lighted	r Clear		V1:Northbound / V2:Northbound	V1: Turning left / V2:Travelling straight ahead	
16	3950628	11:27 PM	2014-09-07	Neponset Valley Parkway / Blue Hill Avenue Rte Sr138 N	Non-fatal injury	2	Angle	Dry	Dark - lighted	r Clear/Clear		V1:Eastbound / V2:Southbound	V1: Turning left / V2:Travelling straight ahead	Fail to yield
17	4067233	3:04 PM	2015-05-10	Blue Hill Avenue Rte 138 N / Neponset Valley Parkway	Property damage only (no	2	Rear-end	Dry	Daylight	Clear		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straight a	h Distracted
18	4117517	7:24 PM	2015-09-29	Blue Hill Avenue Rte 138 / Neponset Valley Parkway	Non-fatal injury	4	Head-on	Wet	Dark - lighted	r Rain/Rain		V1:Southbound / V2:Northbound / V3:S	Sc V1: Travelling straight ahead / V2:Travelling straight ahe	acLeaving lane
19	4159133	2:44 PM	2015-12-29	Blue Hill Avenue / Neponset Valley Parkway	Property damage only (no	2	Rear-end	Wet	Daylight	Clear		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straight a	h Inattention
20	2937814	5:58 PM	2011-11-15	Neponset Valley Parkway / Blue Hill Avenue	Property damage only (no	2	Angle	Dry	Dark - lighted	r Clear		V1:Southbound / V2:Eastbound	V1: Travelling straight ahead / V2:Turning left	_
21	2937130	5:52 PM	2011-11-17	Neponset Valley Parkway / Blue Hill Avenue	Property damage only (no	2	Rear-end	Wet	Dark - lighted	r Rain		V1:Northbound / V2:Northbound	V1: Travelling straight ahead / V2:Turning left	
22	4069892	3:45 PM	2015-08-03	Neponset Valley Parkway Rte Unknow / Blue Hill Avenue	Property damage only (no	2	Sideswipe, opposite dire	e Dry	Daylight	Clear		V1:Westbound / V2:Eastbound	V1: Travelling straight ahead / V2:Entering traffic lane	Wrong way
23	3292235	11:39 AM	2012-10-12	20 Feet S From Intersection 1034 Blue Hill Avenue	Non-fatal injury	2	Sideswipe, same directi	ic Wet	Daylight	Rain/Cloudy		V1:Northbound / V2:Northbound	V1: Overtaking/passing / V2:Travelling straight ahead	Inattention

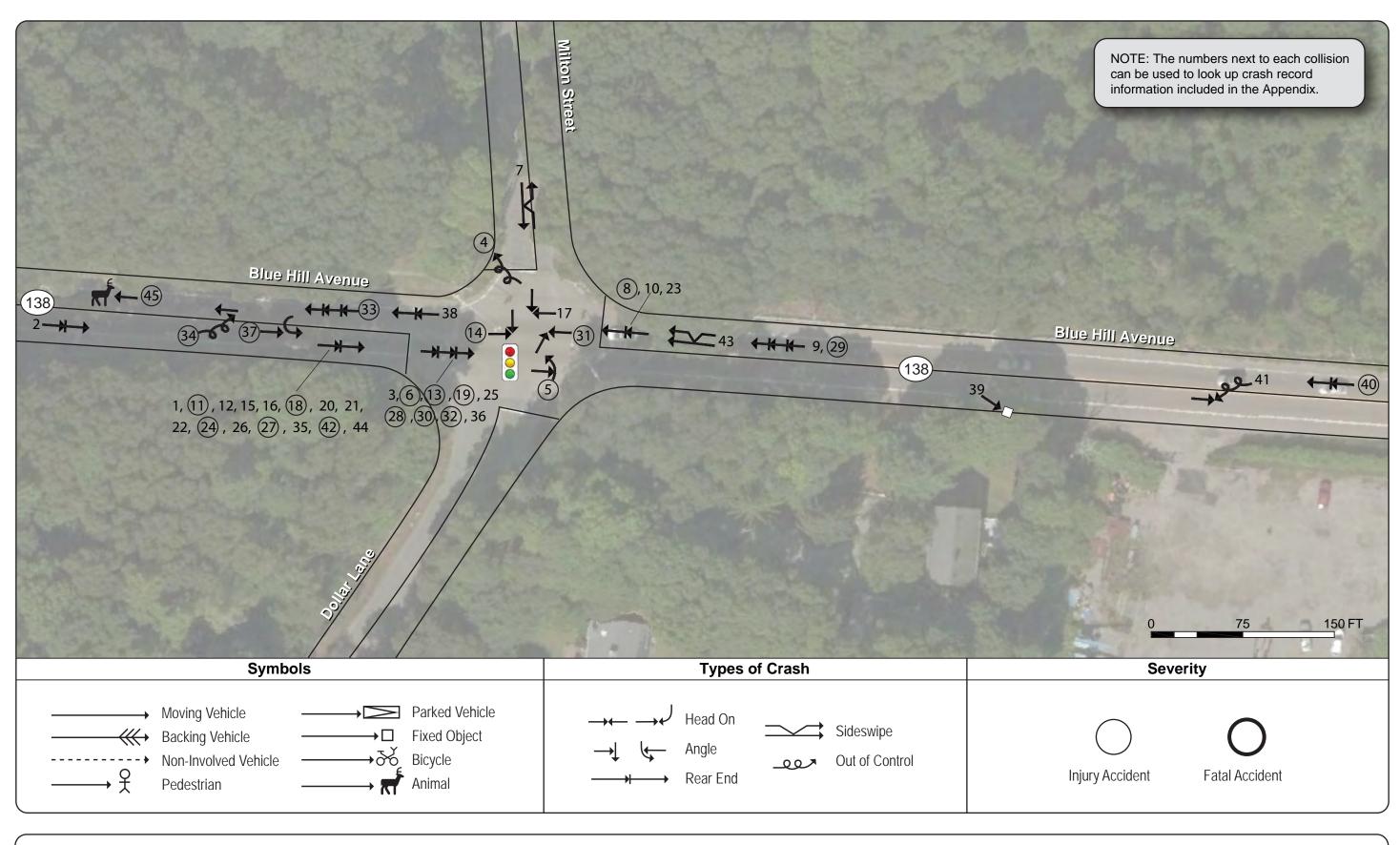




Figure E-4
Collision Diagram 2011-15
Route 138 at Milton Street/Dollar Lane

Table E-4
Route 138 at Milton Street and Dollar Street
Route 138 in Milton

Index	Crash Number	Crash Time	Crash Date	Street Address	Crash Severity	# of Vehicles	Manner of Collision	Condition	Ambient Light Condition	Condition	Bike/ Ped	Vehicle Traveled Direction	Vehicle Action	Driver Contributing Code
1	2743323	9:30 AM	2011-07-06	Blue Hill Avenue Rte 138 / Dollar Lane	Property damage only (no		Rear-end	Dry	Daylight	Clear		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straig	
2	3374276	2:29 PM	2013-01-26	300 Feet S From Intersection Blue Hill Avenue Rte 138 / Milton Street			Rear-end	Dry	Daylight	Clear		V1:Northbound / V2:Northbound	V1: Travelling straight ahead / V2:Slowing or stoppe	
3	3668031	8:25 AM	2013-10-23	300 Feet S From Intersection Blue Hill Avenue Rte 138 N / Dollar Lane	, , , , , ,	4	Rear-end	Dry	Daylight	Clear			Nc V1: Slowing or stopped in traffic / V2:Slowing or stop	
4	3735102	8:10 AM	2013-12-09	Dollar Lane / Blue Hill Avenue	Non-fatal injury	1	Single vehicle crash	Wet	Daylight	Rain/Sleet, hail (V1:Westbound	V1: Turning right	Speeding
5	2711751	9:52 AM	2011-03-30	Blue Hill Avenue / Dollar Lane	· · · · · · · · · · · · · · · · · · ·	2	Angle	Dry	Daylight	Clear		V1:Eastbound / V2:Northbound	V1: Turning left / V2:Travelling straight ahead	
5	2718246	7:27 PM	2011-04-19	Blue Hill Avenue / Dollar Lane	,. ,	3	Rear-to-rear	Wet	Dark - lighted				Nc V1: Slowing or stopped in traffic / V2:Slowing or stop	
7	2731934	7:51 PM	2011-05-29	Milton Street / Blue Hill Avenue	Property damage only (no		Sideswipe, opposite dir	,	Daylight Lighted	Clear		V1:Southbound / V2:Eastbound	V1: Turning right / V2:Slowing or stopped in traffic	Inattention
8	2868516	7:58 PM	2011-12-27	Blue Hill Avenue / Dollar Lane		2	Rear-end	Wet	Dark - lighted			V1:Northbound / V2:Southbound	V1: Travelling straight ahead / V2:Slowing or stopped	
9	3058630	8:09 PM	2012-04-06	Blue Hill Avenue Rte 138 S / Dollar Lane / Milton Street	Property damage only (no		Rear-end	Dry	Dark - lighted				:Si V1: Travelling straight ahead / V2:Slowing or stopped	
10	3408340	1:28 PM	2013-04-02	Rte 138 / Blue Hill Avenue / Milton Street	Property damage only (no		Rear-end	Dry	Daylight	Clear		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straig	
11	3388608	5:10 PM	2013-04-07	Blue Hill Avenue / Dollar Lane		2	Rear-end	Dry	Daylight	Clear		V1:Northbound / V2:Northbound	V1: Travelling straight ahead / V2:Slowing or stoppe	
12	3587383	8:21 PM	2013-07-26	Blue Hill Avenue / Dollar Lane	Property damage only (no		Rear-end	Wet	Dark - lighted			V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Slowing or stop	
13	3612379	11:09 AM	2013-10-10	Blue Hill Avenue / Dollar Lane / Milton Street		3	Rear-end	Dry	Daylight	Clear			Nc V1: Slowing or stopped in traffic / V2:Slowing or stop	
14	3714648	3:50 PM	2013-11-07	Blue Hill Avenue Rte 138 / Dollar Lane		2	Angle	Wet	Dusk	Rain/Rain		V1:Eastbound / V2:Northbound	V1: Travelling straight ahead / V2:Travelling straight	
15	3824837	9:10 AM	2014-03-20	Dollar Lane / Blue Hill Avenue Rte 138 S	Property damage only (no		Rear-end	Dry	Daylight	Clear		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straig	
16	3969059	1:03 PM	2014-10-19	Blue Hill Avenue / Dollar Lane	Property damage only (no		Rear-end	Dry	Daylight	Clear/Clear		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straig	
17	3969775	8:03 AM	2014-10-31	Blue Hill Avenue Rte 138 S / Milton Street	Property damage only (no	2	Angle	Dry	Daylight	Cloudy		V1:Eastbound / V2:Southbound	V1: Travelling straight ahead / V2:Travelling straight	
18	3974986	6:16 AM	2014-11-07	Blue Hill Avenue / Dollar Lane	Non-fatal injury	2	Rear-end	Wet	Daylight	Cloudy/Rain		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Slowing or stop	
19	3982392	4:01 PM	2014-12-03	Blue Hill Avenue Rte 138 / Dollar Lane Rte 138 N / Rte 138	Non-fatal injury	3	Rear-end	Wet	Dark - unknow	w⊦Cloudy/Rain		V1:Northbound / V2:Northbound / V3:	Nc V1: Slowing or stopped in traffic / V2:Slowing or stop	
20	4032177	10:00 PM	2015-03-14	Blue Hill Avenue / Dollar Lane	Property damage only (no	2	Rear-end	Wet	Dark - lighted	l r Rain		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straig	ght ah
21	4117530	4:37 PM	2015-10-02	Blue Hill Avenue / Dollar Lane	Property damage only (no	2	Rear-end	Wet	Daylight	Rain/Cloudy		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straig	ght ah Object in road
22	4117534	7:25 PM	2015-10-02	Blue Hill Avenue / Milton Street	Property damage only (no	2	Rear-end	Wet		l r Cloudy/Rain		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straig	,
23	4117542	3:38 PM	2015-10-04	Blue Hill Avenue / Milton Street	Property damage only (no	2	Rear-end	Dry	Daylight	Clear		V1:Southbound / V2:Southbound	V1: Slowing or stopped in traffic / V2:Travelling straig	ght ah
24	4125341	10:08 PM	2015-10-12	Blue Hill Avenue / Milton Street	Non-fatal injury	2	Rear-end	Dry	Dark - lighted	l r Clear/Clear		V1:Southbound / V2:Southbound	V1: Slowing or stopped in traffic / V2:Travelling straig	ght ah
25	4131777	9:39 PM	2015-10-28	Blue Hill Avenue / Milton Street	Property damage only (no	3	Rear-end	Wet	Dark - lighted	l r Rain		V1:Northbound / V2:Northbound / V3:	Nc V1: Slowing or stopped in traffic / V2:Slowing or stop	pped ir
26	4130805	6:42 PM	2015-12-11	Blue Hill Avenue Rte 138 N / Dollar Lane	Property damage only (no	2	Rear-end	Dry	Dark - lighted	l r Clear/Clear		V1:Northbound / V2:Northbound	V1: Travelling straight ahead / V2:Travelling straight	aheac
27	4130801	11:33 PM	2015-12-22	Blue Hill Avenue Rte 138 / Dollar Lane	Non-fatal injury	2	Rear-end	Wet	Dark - lighted	l r Clear/Clear		V1:Northbound / V2:Not reported	V1: Slowing or stopped in traffic / V2:Not reported	
28	2798636	12:31 PM	2011-11-10	Blue Hill Avenue Rte 138 N / Milton Street	Non-fatal injury	3	Rear-end	Wet	Daylight	Rain		V1:Northbound / V2:Northbound / V3:	Nc V1: Slowing or stopped in traffic / V2:Slowing or stop	pped ir
29	3015357	3:55 PM	2012-03-21	Blue Hill Avenue / Dollar Lane	Not Reported	3	Rear-end	Dry	Daylight	Clear		V1:Southbound / V2:Southbound / V3	:S ₁ V1: Travelling straight ahead / V2:Travelling straight	aheac
30	3288054	5:19 PM	2012-09-20	100 Feet S From Cury College	Non-fatal injury	3	Rear-end	Unknown	Daylight	Clear		V1:Northbound / V2:Northbound / V3:	Nc V1: Slowing or stopped in traffic / V2:Slowing or stop	pped ir
31	3297108	1:34 PM	2012-10-27	Blue Hill Avenue / Milton Street	Non-fatal injury	2	Angle	Dry	Daylight	Clear		V1:Southbound / V2:Westbound	V1: Travelling straight ahead / V2:Travelling straight	aheac
32	3299995	8:08 AM	2012-11-01	Blue Hill Avenue / Dollar Lane	Non-fatal injury	3	Rear-end	Dry	Daylight	Clear		V1:Northbound / V2:Northbound / V3:	Nc V1: Slowing or stopped in traffic / V2:Slowing or stop	pped ir
33	3372355	6:04 PM	2013-01-28	Blue Hill Avenue Rte 138 S / Milton Street Rte 138	Non-fatal injury	3	Rear-end	Wet	Dusk	Clear/Clear		V1:Southbound / V2:Southbound / V3	:N V1: Slowing or stopped in traffic / V2:Slowing or stop	pped ir
34	3727459	10:59 PM	2013-12-14	Blue Hill Avenue / Milton Street	Non-fatal injury	2	Angle	Snow/Ice	Dark - lighted	r Snow/Sleet, hail		V1:Southbound / V2:Southbound	V1: Travelling straight ahead / V2:Travelling straight	aheac
35	3827607	6:20 PM	2014-05-22	Blue Hill Avenue Rte 138 / Dollar Lane	Property damage only (no		Rear-end	Wet	Daylight	Rain/Cloudy		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straig	
36	3936932	5:32 PM	2014-08-14	Blue Hill Avenue / Milton Street	Property damage only (no		Rear-end	Dry	Daylight	Clear/Clear		V1:Northbound / V2:Northbound / V3:	Nc V1: Not reported / V2:Not reported / V3:Travelling st	raight
37	4003505	3:52 PM	2014-12-23	Blue Hill Avenue / Dollar Lane	Non-fatal injury	2	Angle	Wet	Dusk	Cloudy/Rain		V1:Southbound / V2:Northbound	V1: Making U-turn / V2:Travelling straight ahead	Improper turn
38	3956121	3:42 PM	2014-09-20	By Curry	Property damage only (no	2	Rear-end	Dry	Daylight	Clear/Clear		V1:Southbound / V2:Southbound	V1: Travelling straight ahead / V2:Slowing or stoppe	
39	3735111	1:46 PM	2013-12-07	100 Feet N From Intersection Blue Hill Avenue Rte 138 S / Milton Stree	1 7 0 71		Single vehicle crash	Dry	Daylight	Clear		V1:Southbound / V2:Southbound	V1: Slowing or stopped in traffic / V2:Travelling straig	
40	3374308	8:04 AM	2013-01-16	500 Feet N From Intersection 1086 Blue Hill Avenue Rte 138 S / Milton		2	Rear-end		Daylight	Snow		V1:Southbound / V2:Southbound	V1: Slowing or stopped in traffic / V2:Travelling straig	
41	3393021	3:21 PM	2013-02-08	500 Feet N From Intersection 1072 Blue Hill Avenue Rte 138 / Milton S			Angle		Daylight	Snow/Snow		V1:Southbound / V2:Northbound	V1: Travelling straight ahead / V2:Travelling straight	
42	3211376	2:36 PM	2012-05-24	1071 Blue Hill Avenue	, , , , , ,	2	Rear-end	Dry	Daylight	Clear		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Slowing or stop	•
43	3714664	1:35 PM	2013-11-05	Curry College	Property damage only (no		Sideswipe, same direct		Daylight	Clear		V1:Northbound / V2:Southbound	V1: Slowing or stopped in traffic / V2:Overtaking/pas	·
44	4040835	4:06 PM	2015-04-24	1000 Blue Hill Avenue Rte 138	Property damage only (no		Rear-end	Dry	Daylight	Clear/Clear		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straig	_
45	4130803	8:15 AM	2015-12-22	1000 Blue Hill Avenue Rte 138 S	Not Reported	1	Angle	Wet	Daylight	Cloudy/Rain		V1:Southbound	V1: Travelling straight ahead	Defective equipment
10	+100000	0. 10 /\tivi	2010 12-22	1000 Bidd Filli / Worldo Fillo Too O	Hot Hopoitou		7 11910	*****	Dayiigiit	Jioudy/Talli		7 I. Journal out In	VI. Havoling straight ahoad	Dolodino equipment

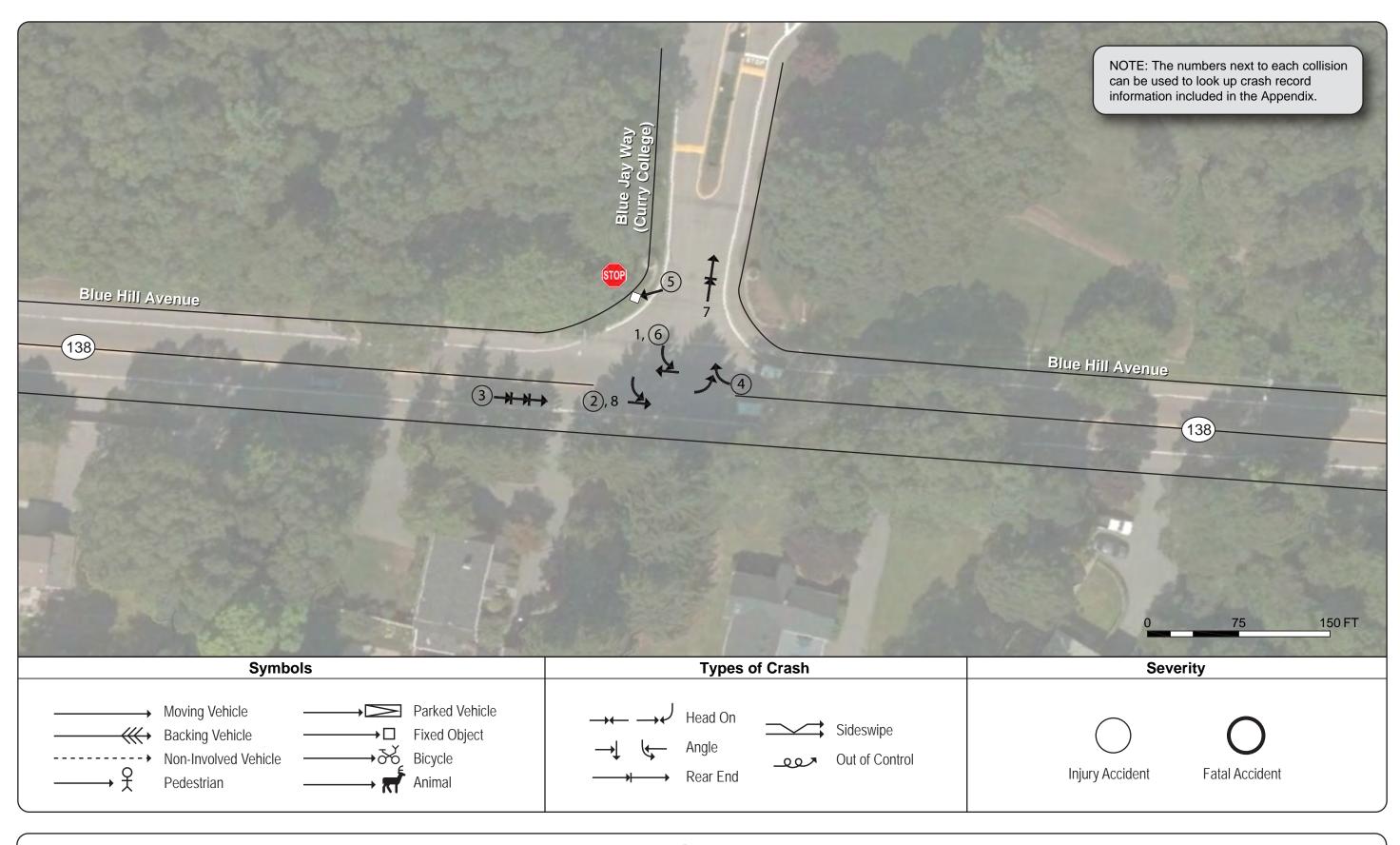




Figure E-5
Collision Diagram 2011-15
Route 138 at Blue Jay Way

Table E–5 Route 138 at Blue Jay Way Route 138 in Milton

Index	Crash Number	Crash Time	Crash Date	Street Address	Crash Severity	# of Vehicles	Manner of Collision		Ambient Light Condition	Weather Condition	Bike/ Ped	Vehicle Traveled Direction	Vehicle Action	Driver Contributing Code
1	3343331	11:54 AM	2012-11-08	1070 Blue Hill Avenue	Property damage only (n	ю 3	Angle	Wet	Daylight	Cloudy/Rain		V1:Southbound / V2:Eastbound / V3:N	lo _l V1: Travelling straight ahead / V2:Turning left / V3:Slow	inç Fail to yield
2	3786416	8:41 PM	2014-02-19	1071 Blue Hill Avenue Rte 138 S	Non-fatal injury	2	Angle	Wet	Dark - lighted	r Sleet, hail (freez	zi	V1:Southbound / V2:Eastbound	V1: Travelling straight ahead / V2:Turning left	Fail to yield
3	2705958	9:52 PM	2011-01-29	1071 Blue Hill Avenue	Non-fatal injury	3	Rear-end	Dry	Dark - lighted	r Clear/Clear		V1:Northbound / V2:Northbound / V3:	Nc V1: Slowing or stopped in traffic / V2:Slowing or stopped	d ir
4	3176330	7:49 PM	2012-05-08	1071 Blue Hill Avenue	Non-fatal injury	3	Angle	Wet	Dark - unknov	/ Rain		V1:Southbound / V2:Southbound / V3	E; V1: Travelling straight ahead / V2:Turning left / V3:Slow	inç Fail to yield
5	3297116	6:29 AM	2012-10-18	1071 Blue Hill Avenue	Non-fatal injury	1	Single vehicle crash	Dry	Daylight	Clear		V1:Southbound	V1: Turning right	
6	3983023	9:51 AM	2014-11-04	Curry College	Non-fatal injury	2	Angle	Dry	Daylight	Clear/Clear		V1:Southbound / V2:Westbound	V1: Travelling straight ahead / V2:Turning left	
7	4038196	4:35 AM	2015-01-24	1071 Blue Hill Avenue	Property damage only (n	10 2	Rear-end	Wet	Dark - lighted	r Snow/Sleet, hai	1	V1:Westbound / V2:Westbound	V1: Slowing or stopped in traffic / V2:Slowing or stopped	d ir Distracted
8	4025266	4:06 PM	2015-01-26	1071 Blue Hill Avenue Rte 138	Property damage only (n	10 2	Angle	Wet	Daylight	Snow		V1:Northbound / V2:Westbound	V1: Travelling straight ahead / V2:Turning left	

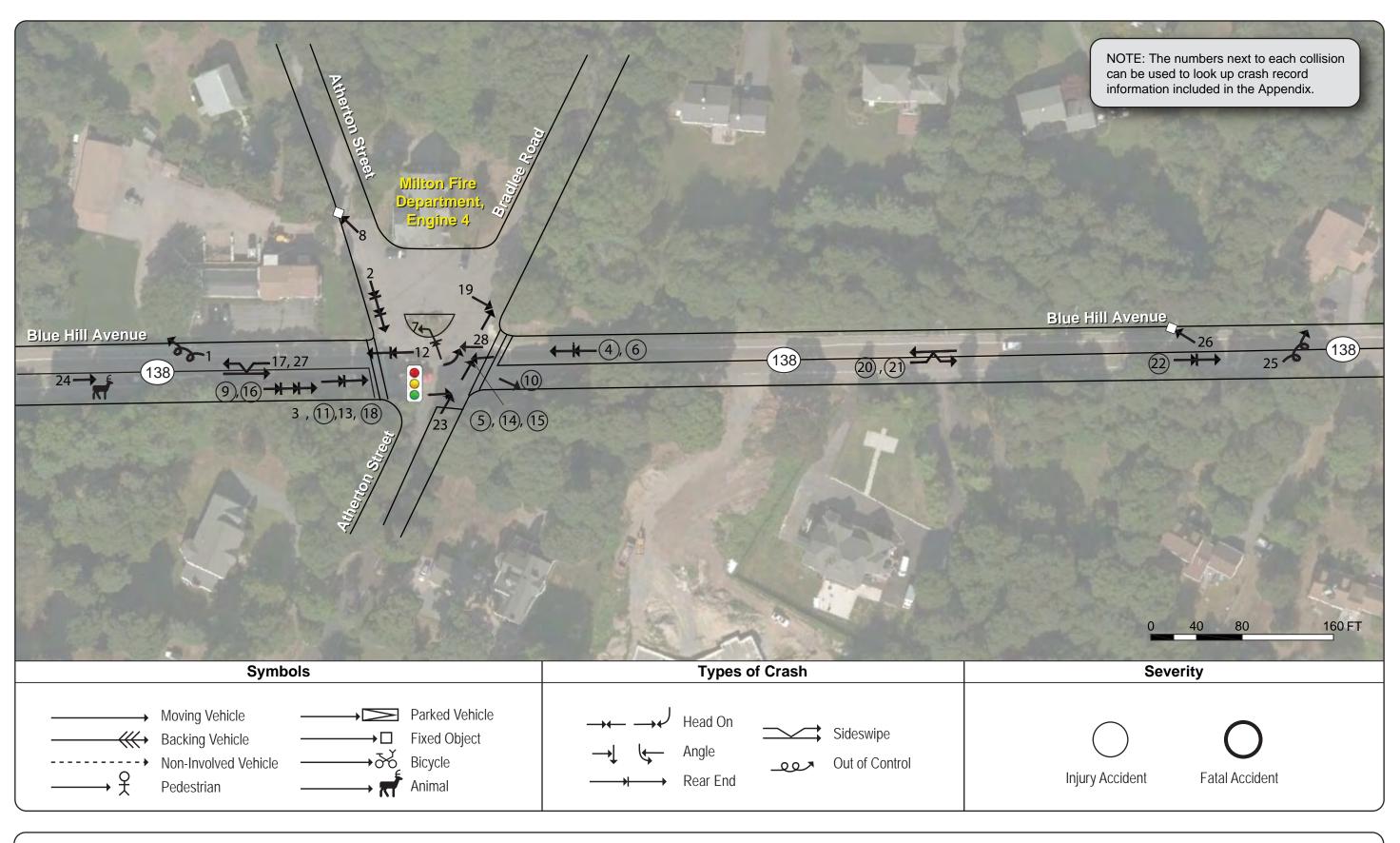




Figure E–6
Collision Diagram 2011–15
Route 138 at Atherton Street and Bradlee Road

Table E–6
Route 138 at Atherton Street and Bradlee Road
Route 138 in Milton

Index	Crash Number	Crash Time	Crash Date	Street Address	Crash Severity	# of Vehicles	Manner of Collision		Ambient Light Condition	Weather Condition	Bike/ Ped	Vehicle Traveled Direction	Vehicle Action	Driver Contributing Code
1	4139833	4:04 PM	2015-11-03	957 Blue Hill Avenue	Property damage only (n	o 1	Single vehicle crash	Dry	Daylight	Clear		V1:Southbound	V1: Travelling straight ahead	
2	2743252	3:39 PM	2011-07-02	Blue Hill Avenue / Atherton Street	Property damage only (n	o 3	Rear-end	Dry	Daylight	Clear		V1:Eastbound / V2:Eastbound / V3:Ea	ast V1: Travelling straight ahead / V2:Slowing or stopped in	n tra
3	2743324	1:47 PM	2011-07-04	Blue Hill Avenue / Atherton Street	Property damage only (n	o 2	Rear-end	Dry	Daylight	Clear		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Slowing or stoppe	d ir
4	2869035	8:57 AM	2011-11-02	Blue Hill Avenue / Bradlee Road	Non-fatal injury	2	Rear-end	Dry	Daylight	Clear		V1:Southbound / V2:Southbound	V1: Travelling straight ahead / V2:Travelling straight ah	ear Fatigue
5	4128339	10:48 AM	2015-10-15	Atherton Street / Blue Hill Avenue	Non-fatal injury	2	Angle	Dry	Daylight	Clear/Clear		V1:Southbound / V2:Westbound	V1: Travelling straight ahead / V2:Travelling straight ah	eac
6	2711344	12:37 PM	2011-03-14	Blue Hill Avenue Rte 138 / Blue Hill Avenue Rte 138 / Bradlee Road	Non-fatal injury	2	Rear-end	Dry	Daylight	Cloudy		V1:Southbound / V2:Southbound	V1: Slowing or stopped in traffic / V2:Travelling straight	ah Inattention
7	2792466	12:34 PM	2011-10-12	Blue Hill Avenue / Blue Hill Avenue / Atherton Street	Property damage only (n	o 2	Rear-end	Dry	Daylight	Cloudy		V1:Southbound / V2:Southbound	V1: Slowing or stopped in traffic / V2:Turning left	
8	3049107	5:09 PM	2012-04-10	Atherton Street / Blue Hill Avenue Rte 138	Property damage only (n	o 1	Single vehicle crash	Dry	Daylight	Clear		V1:Westbound	V1: Backing	
9	3247840	9:22 PM	2012-07-01	Blue Hill Avenue / Atherton Street	Non-fatal injury	3	Rear-end	Dry	Dark - lighted	r Clear		V1:Northbound / V2:Northbound / V3:	Nc V1: Slowing or stopped in traffic / V2:Slowing or stoppe	d ir
10	3282177	6:34 PM	2012-08-16	Blue Hill Avenue / Atherton Street	Non-fatal injury	1	Single vehicle crash	Dry	Daylight	Clear/Clear		V1:Northbound	V1: Travelling straight ahead	
11	3288053	7:25 AM	2012-09-21	Blue Hill Avenue / Atherton Street	Non-fatal injury	2	Rear-end	Dry	Daylight	Clear		V1:Northbound / V2:Northbound	V1: Travelling straight ahead / V2:Slowing or stopped in	n tra
12	3388606	2:40 PM	2013-04-04	Rte 138 N / Blue Hill Avenue / Atherton Street	Property damage only (n	o 2	Rear-end	Dry	Daylight	Clear		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straight	ah Tailgating
13	3403022	1:51 PM	2013-04-12	Bradlee Road / Blue Hill Avenue / Robbins Street	Property damage only (n	o 2	Rear-end	Wet	Daylight	Rain		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Slowing or stoppe	d ir
14	3862378	12:54 PM	2014-06-05	Blue Hill Avenue Rte 138 / Atherton Street / Bradlee Road	Non-fatal injury	2	Angle	Wet	Daylight	Rain		V1:Southbound / V2:Westbound	V1: Travelling straight ahead / V2:Travelling straight ah	ear Disregarding signs
15	3896431	6:27 PM	2014-07-26	Blue Hill Avenue Rte 138 W / Atherton Street	Non-fatal injury	2	Angle	Dry	Daylight	Clear/Clear		V1:Westbound / V2:Southbound	V1: Not reported / V2:Not reported	Inattention
16	4067762	4:15 PM	2015-07-11	Blue Hill Avenue / Atherton Street	Non-fatal injury	3	Rear-end	Dry	Daylight	Clear/Clear		V1:Northbound / V2:Northbound / V3:	Nc V1: Slowing or stopped in traffic / V2:Travelling straight	ah
17	3049098	4:27 PM	2012-04-13	Blue Hill Avenue / Bradlee Road	Property damage only (n	o 1	Sideswipe, opposite dir	e Dry	Daylight	Clear		V1:Northbound	V1: Travelling straight ahead	
18	3282226	1:52 AM	2012-08-05	Blue Hill Avenue / Atherton Street	Non-fatal injury	2	Rear-end	Dry	Dark - lighted	r Clear		V1:Northbound / V2:Northbound	V1: Travelling straight ahead / V2:Slowing or stopped in	n tra
19	4022546	9:58 AM	2015-02-25	Blue Hill Avenue / Bradlee Road	Property damage only (n	o 2	Sideswipe, opposite dir	e Dry	Daylight	Clear/Clear		V1:Westbound / V2:Northbound	V1: Turning left / V2:Travelling straight ahead	Speeding (weather)
20	3739632	1:31 PM	2013-12-29	764 Blue Hill Avenue	Non-fatal injury	2	Sideswipe, opposite dir	e Wet	Daylight	Rain		V1:Southbound / V2:Northbound	V1: Travelling straight ahead / V2:Leaving traffic lane	Leaving lane
21	2696976	10:25 AM	2011-01-15	764 Blue Hill Avenue	Non-fatal injury	3	Sideswipe, opposite dir	e Dry	Daylight	Clear		V1:Northbound / V2:Southbound / V3:	:Sc V1: Leaving traffic lane / V2:Travelling straight ahead /	V3: Leaving lane
22	2829453	8:53 AM	2011-11-06	700 Feet N From Intersection 753 Blue Hill Avenue Rte 138 N / Atherto	r Non-fatal injury	2	Rear-end	Dry	Daylight	Clear		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straight	ah Inattention
23	4151241	6:09 PM	2015-12-16	815 Blue Hill Avenue	Property damage only (n	o 2	Angle	Dry	Dark - lighted	r Clear		V1:Westbound / V2:Northbound	V1: Travelling straight ahead / V2:Slowing or stopped in	n tra
24	4139845	4:45 PM	2015-11-15	805 Blue Hill Avenue	Property damage only (n	o 1	Single vehicle crash	Dry	Daylight	Clear		V1:Northbound	V1: Travelling straight ahead	
25	2739541	7:40 AM	2011-06-17	753 Blue Hill Avenue	Property damage only (n	o 1	Single vehicle crash	Dry	Daylight	Clear		V1:Northbound	V1: Travelling straight ahead	
26	2716451	8:12 AM	2011-04-01	400 Feet N From Intersection 731 Blue Hill Avenue Rte 138 S	Property damage only (n	0 1	Single vehicle crash	Snow/Ice	Daylight	Sleet, hail (free	ezi	V1:Southbound	V1: Travelling straight ahead	Over-correcting
27	3389886	10:28 PM	2013-02-25	Blue Hill Avenue / Concord Avenue	Property damage only (n	o 2	Sideswipe, opposite dir	e Dry	Dark - lighted	r Cloudy/Cloudy		V1:Southbound / V2:Not reported	V1: Travelling straight ahead / V2:Not reported	
28	3391456	9:29 AM	2013-02-18	Blue Hill Avenue Rte Sr138 S / Bradlee Road / Atherton Street	Property damage only (n	o 2	Head-on	Wet	Daylight	Clear		V1:Southbound / V2:Westbound	V1: Travelling straight ahead / V2:Turning left	

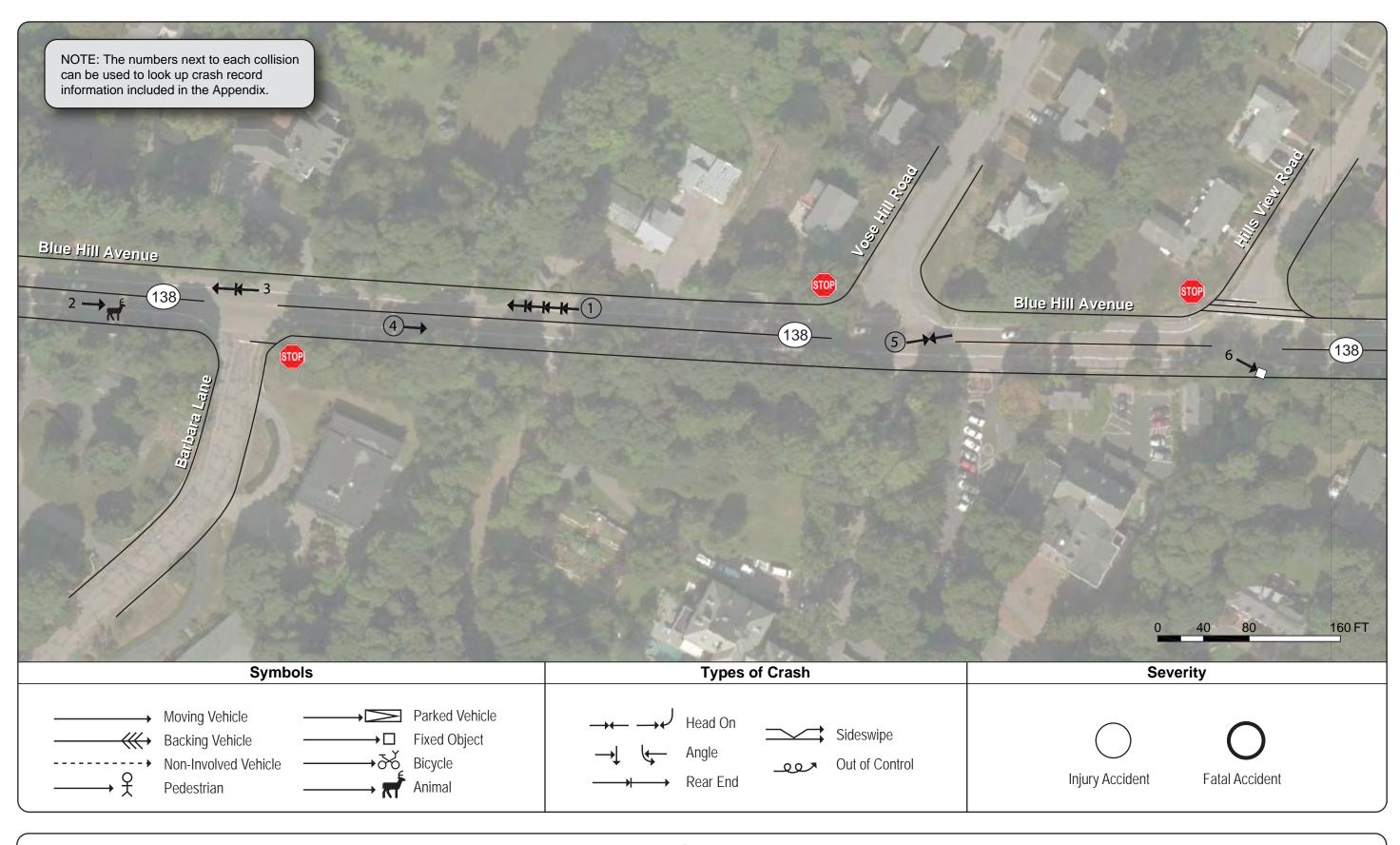




Figure E-7
Collision Diagram 2011-15
Route 138 Between Bradlee Road and Robbins Street

Table E–7
Route 138 between Bradlee Road and Robbins Street
Route 138 in Milton

Index	Crash Number	Crash Time	Crash Date	Street Address	Crash Severity	# of Vehicles	Manner of Collision		Ambient Light Condition	Weather Condition	Bike/ Ped	Vehicle Traveled Direction	Vehicle Action	Driver Contributing Code
1	3292976	6:40 AM	2012-11-13	Canton Avenue Rte 138 S / Blue Hill Avenue	Non-fatal injury	4	Rear-end	Wet	Dawn	Rain		V1:Southbound / V2:Southbound / V3:	Si V1: Slowing or stopped in traffic / V2:Slowing or stopped	d ir
2	3392140	5:44 PM	2013-02-21	680 Blue Hill Avenue	Property damage only (no	1	Single vehicle crash	Dry	Dark - lighted	r Clear		V1:Northbound	V1: Travelling straight ahead	
3	3342732	1:18 AM	2012-11-11	Blue Hill Avenue / Barbara Lane	Property damage only (no	2	Rear-end	Dry	Dark - lighted	r Clear		V1:Southbound / V2:Southbound	V1: Other / V2:Travelling straight ahead	
4	3288052	7:10 PM	2012-09-23	1000 Feet S From Intersection Blue Hill Avenue / Robbins Street	Non-fatal injury	1	Single vehicle crash	Dry	Dark - lighted	r Clear/Clear		V1:Northbound	V1: Travelling straight ahead	
5	3408342	6:28 AM	2013-03-29	Blue Hill Avenue / Vose Hill Road	Non-fatal injury	2	Head-on	Dry	Daylight	Clear/Clear		V1:Northbound / V2:Southbound	V1: Travelling straight ahead / V2:Travelling straight ahe	ear Reckless
6	4132328	4:42 PM	2015-10-24	Hills View Road / Blue Hill Avenue	Property damage only (no	1	Single vehicle crash	Dry	Dark - lighted	r Clear/Clear		V1:Northbound	V1: Travelling straight ahead	Object in road

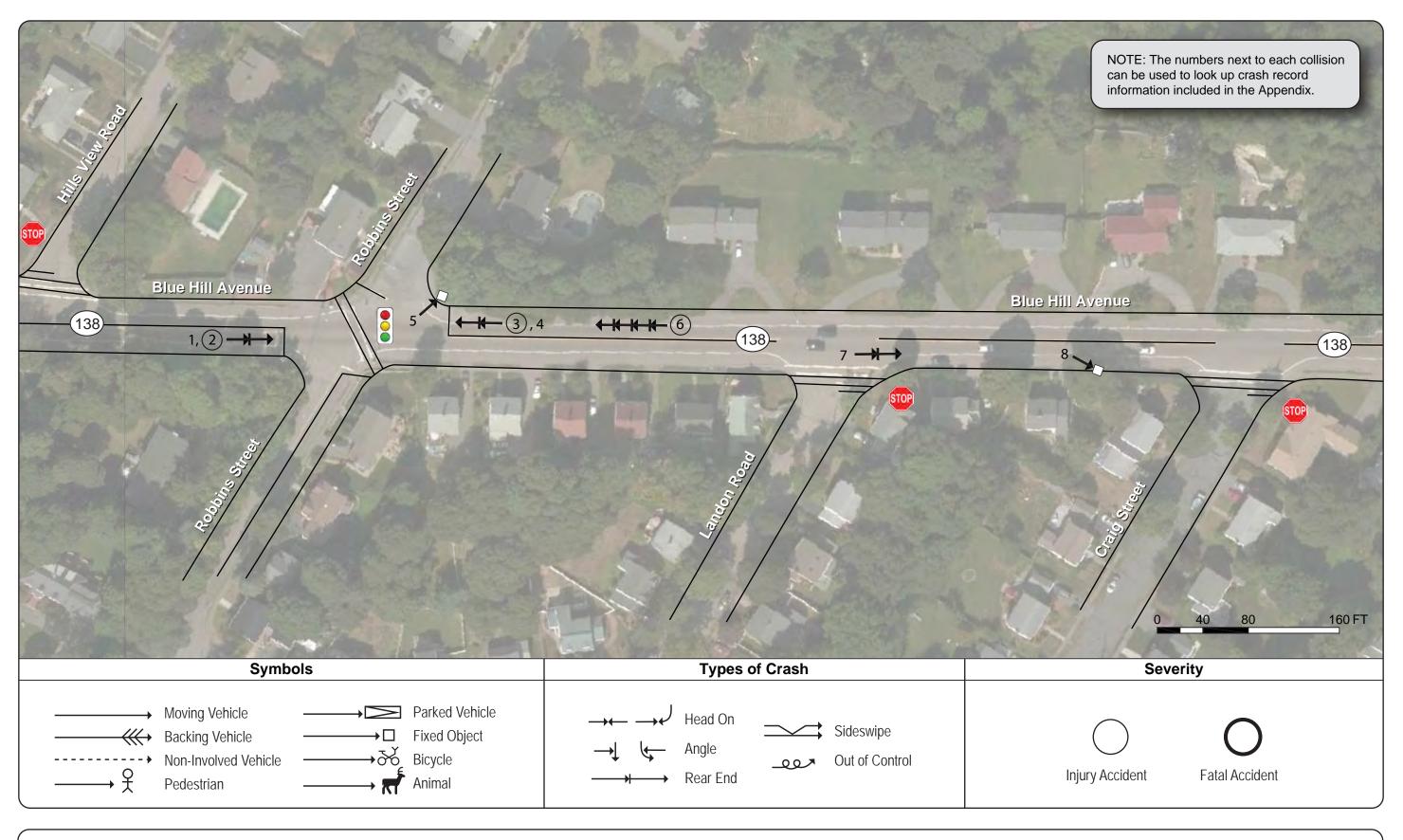




Figure E–8
Collision Diagram 2011–15
Route 138 at Robbins Street

Table E–8 Route 138 at Robbins Street Route 138 in Milton

Index	Crash Number	Crash Time	Crash Date	Street Address	Crash Severity	# of Vehicles	Manner of Collision		Ambient Light Condition	Weather Condition	Bike/ Ped	Vehicle Traveled Direction	Vehicle Action	Driver Contributing Code
1	2703339	6:15 PM	2011-02-16	Blue Hill Avenue Rte 138 N / Robbins Street	Non-fatal injury	2	Rear-end	Dry	Dark - lighted	r Clear/Clear		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straight	ah Tailgating
2	2732053	3:44 PM	2011-06-03	Blue Hill Avenue / Robbins Street	Property damage only (no	o 1	Rear-end	Dry	Daylight	Clear		V1:Northbound	V1: Slowing or stopped in traffic	Inattention
3	3336240	10:22 PM	2012-09-08	Robbins Street / Blue Hill Avenue / Robbins Street	Non-fatal injury	2	Rear-end	Wet	Dark - lighted i	r Rain		V1:Southbound / V2:Southbound	V1: Travelling straight ahead / V2:Slowing or stopped in	tra
4	3588717	6:18 PM	2013-09-09	Blue Hill Avenue / Robbins Street	Property damage only (no	o 2	Rear-end	Dry	Daylight	Clear/Clear		V1:Southbound / V2:Southbound	V1: Travelling straight ahead / V2:Slowing or stopped in	tra Inattention
5	4117103	2:54 AM	2015-09-21	Robbins Street / Blue Hill Avenue	Property damage only (no	o 1	Single vehicle crash	Dry	Dark - lighted i	r Clear/Clear		V1:Northbound	V1: Leaving traffic lane	Object in road
6	4107509	3:37 PM	2015-10-28	Blue Hill Avenue / Robbins Street	Non-fatal injury	4	Rear-end	Wet	Dusk	Rain/Cloudy		V1:Southbound / V2:Southbound / V3:	Si V1: Slowing or stopped in traffic / V2:Slowing or stopped	d ir
7	3392142	9:40 PM	2013-02-20	520 Blue Hill Avenue Rte 138 N	Property damage only (no	o 2	Rear-end	Dry	Dark - lighted i	r Clear/Clear		V1:Northbound / V2:Northbound	V1: Leaving traffic lane / V2:Parked	
8	4078505	10:03 AM	2015-06-09	Blue Hill Avenue / Landon Road / Craig Street	Property damage only (no	o 2	Single vehicle crash	Dry	Daylight	Clear/Clear		V1:Not reported / V2:Northbound	V1: Entering traffic lane / V2:Travelling straight ahead	Inattention

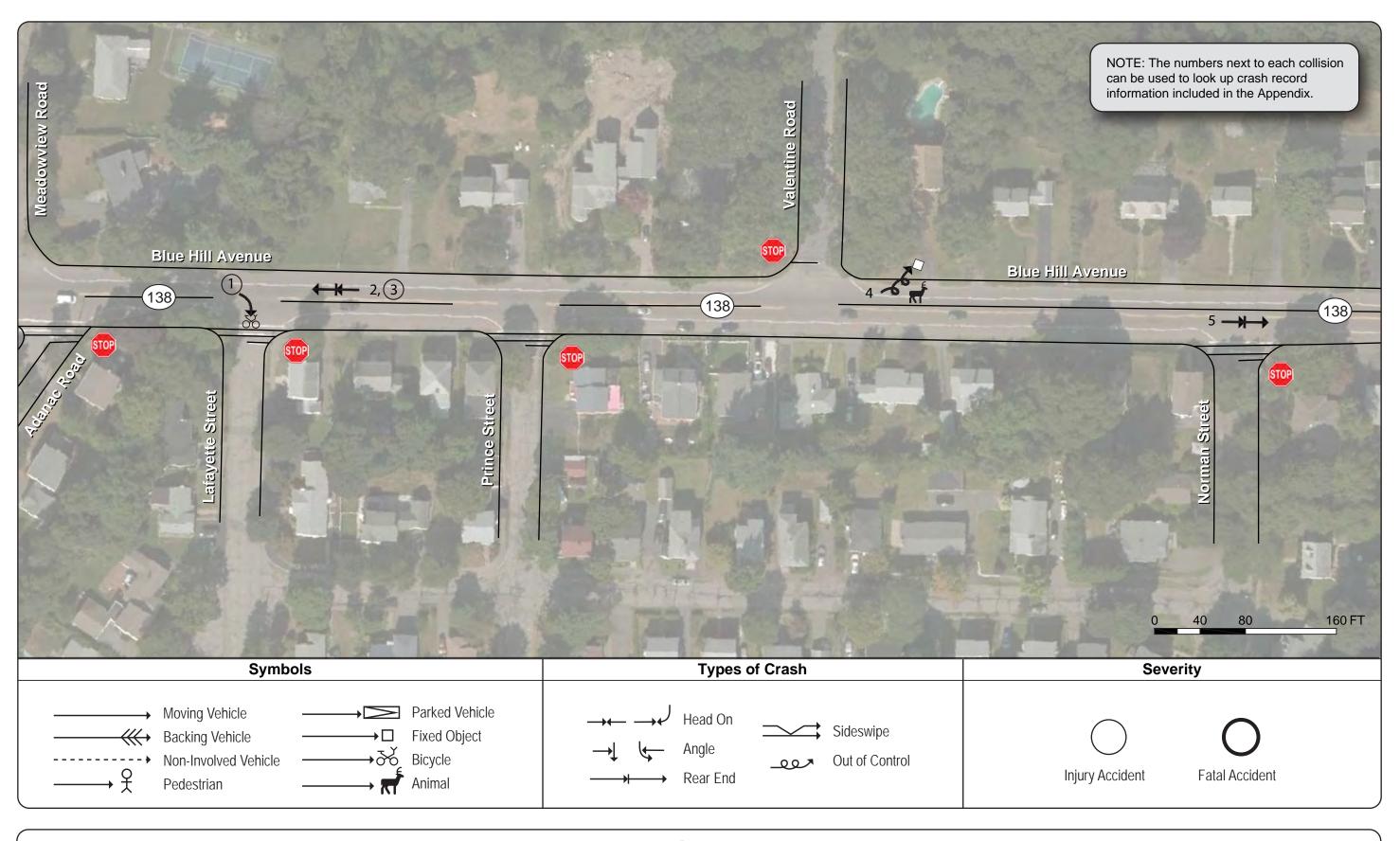




Table E–9
Route 138 Between Robbins Street and Cheever Street
Route 138 in Milton

Index	Crash Number	Crash Time	Crash Date	Street Address	Crash Severity	# of Vehicles	Manner of Collision		Ambient Light Condition	Weather Condition	Bike/ Ped	Vehicle Traveled Direction	Vehicle Action	Driver Contributing Code
1	3288267	11:21 AM	2012-09-16	Blue Hill Lafayette	Non-fatal injury	1	Single vehicle crash	Dry	Daylight	Clear	сус	V1:Northbound	V1: Turning right	Fail to yield
2	3510904	8:16 PM	2013-07-03	Blue Hill Avenue / Lafayette Street	Property damage only (n	o 2	Rear-end	Dry	Dark - lighted	r Clear/Clear		V1:Eastbound / V2:Southbound	V1: Turning left / V2:Travelling straight ahead	Cellphone
3	3928313	4:49 PM	2014-08-22	Blue Hill Avenue / Lafayette Street	Non-fatal injury	2	Rear-end	Dry	Daylight	Clear		V1:Southbound / V2:Southbound	V1: Travelling straight ahead / V2:Not reported	Tailgating
4	3229872	3:07 AM	2012-06-16	351 Blue Hill Avenue	Property damage only (n	o 1	Single vehicle crash	Dry	Dark - lighted	r Clear	•	V1:Northbound	V1: Travelling straight ahead	
5	4131785	5:36 PM	2015-10-28	Blue Hill Avenue / Norman Street	Property damage only (n	o 2	Rear-end	Wet	Dark - lighted	r Rain/Rain		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Slowing or stopped	ir

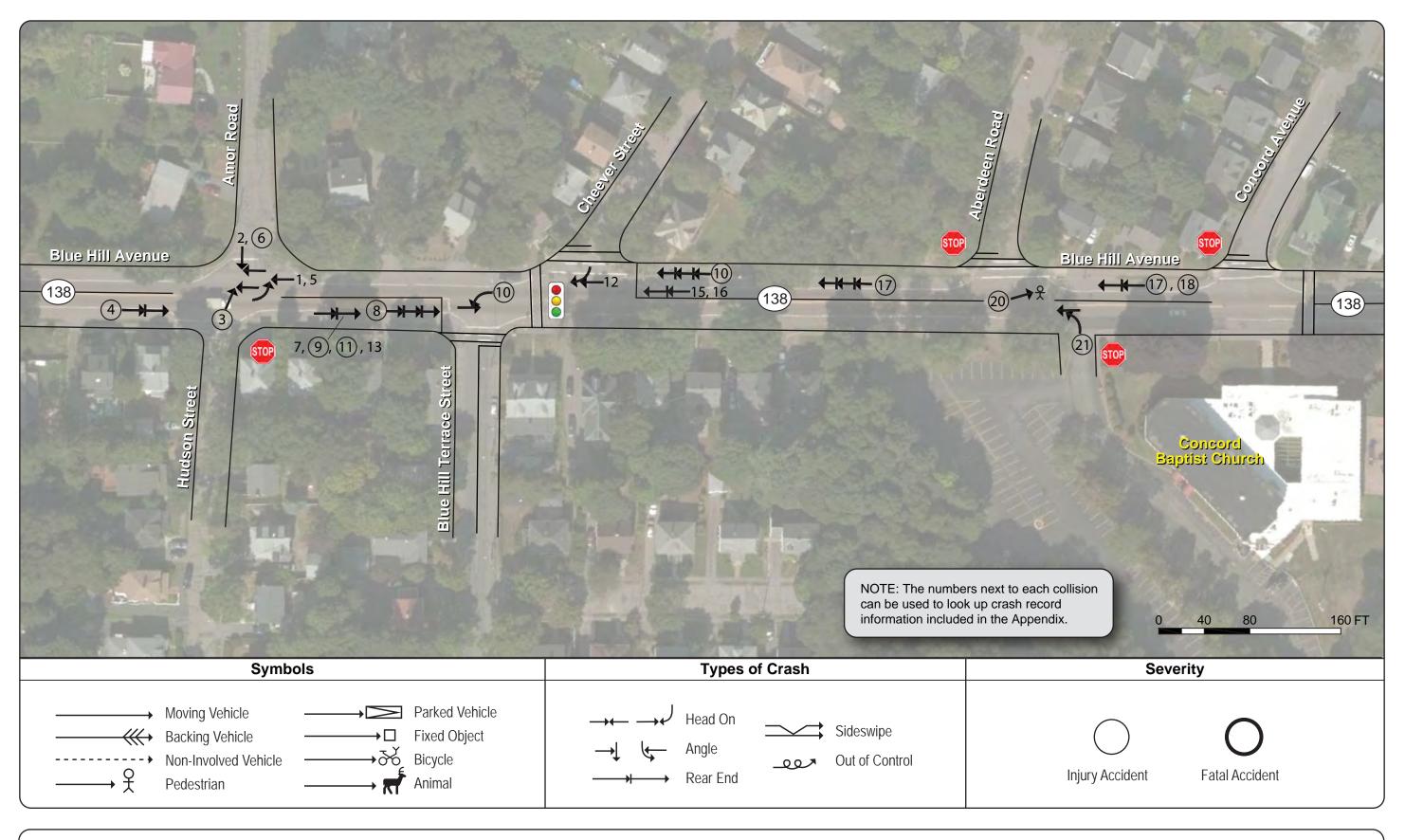




Figure E-10
Collision Diagram 2011-15
Route 138 at Cheever Street and Blue Hill Terrace Street

Table E-10
Route 138 at Cheever Street and Blue Hill Terrace
Route 138 in Milton

Index	Crash Number	Crash Time	Crash Date	Street Address	Crash Severity	# of Vehicles	Manner of Collision		Ambient Light n Condition	Weather Condition	Bike/ Ped	Vehicle Traveled Direction	Vehicle Action	Driver Contributing Code
1	3427631	11:38 PM	2013-04-19	Blue Hill Avenue / Amor Road	Property damage only (no	o 2	Sideswipe, opposite dir	e Wet	Dark - lighted	d r Rain/Rain		V1:Northbound / V2:Southbound	V1: Turning left / V2:Travelling straight ahead	
2	3811331	3:22 AM	2014-04-26	Blue Hill Avenue Rte 138 S / Hudson Street / Amor Road	Property damage only (no	o 3	Sideswipe, opposite dir	e Dry	Dark - lighted	d r Clear/Clear		V1:Southbound / V2:Northbound / V3	:Ea V1: Travelling straight ahead / V2:Travelling straight a	aheac
3	2725481	6:57 AM	2011-05-06	Blue Hill Avenue / Amor Road Rte 138 S	Non-fatal injury	3	Angle	Unknown	Daylight	Clear		V1:Westbound / V2:Southbound / V3:	:Ea V1: Travelling straight ahead / V2:Travelling straight a	hear Fail to yield
4	2725489	7:29 AM	2011-05-06	Blue Hill Avenue Rte 138 N / Amor Road / Hudson Street	Non-fatal injury	2	Rear-end	Dry	Daylight	Clear		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straig	ht ah
5	3612378	8:48 AM	2013-10-11	Blue Hill Avenue Rte 138 / Amor Road / Rte 138	Property damage only (no	o 2	Angle	Dry	Daylight	Clear		V1:Southbound / V2:Westbound	V1: Travelling straight ahead / V2:Turning left	Fail to yield
6	4139819	12:20 PM	2015-11-08	Amor Road / Amor Road / Blue Hill Avenue	Not Reported	2	Angle	Dry	Daylight	Clear		V1:Southbound / V2:Eastbound	V1: Travelling straight ahead / V2:Turning left	
7	3868562	10:51 PM	2014-06-15	248 Blue Hill Avenue	Property damage only (no	o 2	Rear-end	Dry	Dark - lighted	d r Clear		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straig	ht ah Distracted
8	2958031	5:15 AM	2012-03-11	Blue Hill Avenue / Blue Hill Terrace Street	Non-fatal injury	3	Rear-end	Dry	Dark - lighted	d r Clear/Clear		V1:Northbound / V2:Northbound / V3:	:Nc V1: Travelling straight ahead / V2:Slowing or stopped	in tra
9	3794971	10:16 AM	2014-03-12	Blue Hill Avenue Rte 138 N / Blue Hill Avenue Rte 138 N / Blue Hill Ter	าง Non-fatal injury	2	Rear-end	Dry	Daylight	Cloudy		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straig	ht ah Inattention
10	4053466	7:20 PM	2015-03-24	Blue Hill Terrace Street / Blue Hill Avenue / Blue Hill Terrace Street	Not Reported	2	Angle	Dry	Dark - lighted	d r Clear		V1:Northbound / V2:Southbound	V1: Travelling straight ahead / V2:Turning left	
11	3229885	6:07 PM	2012-06-02	240 Blue Hill Avenue	Non-fatal injury	2	Rear-end	Wet	Dark - lighted	d r Rain/Rain		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straig	ht ah
12	2693776	3:20 PM	2011-02-07	Blue Hill Avenue Rte 138 S / Cheever Street	Property damage only (no	o 2	Angle	Dry	Daylight	Cloudy		V1:Eastbound / V2:Southbound	V1: Travelling straight ahead / V2:Travelling straight a	aheac
13	4139852	2:14 PM	2015-11-14	Blue Hill Avenue Rte 138 N / Cheever Street	Property damage only (no	o 2	Rear-end	Dry	Dark - lighted	d r Clear/Clear		V1:Northbound / V2:Northbound	V1: Turning left / V2:Travelling straight ahead	Inattention
14	4093765	2:36 PM	2015-09-26	Cheever Street / Blue Hill Avenue	Non-fatal injury	3	Rear-end	Dry	Daylight	Clear		V1:Southbound / V2:Southbound / V3	3:S ₁ V1: Travelling straight ahead / V2:Travelling straight a	aheac
15	3063149	10:27 AM	2012-03-05	Blue Hill Avenue / Cheever Street	Property damage only (no	o 2	Rear-end	Dry	Daylight	Clear		V1:Southbound / V2:Southbound	V1: Slowing or stopped in traffic / V2:Slowing or stopp	ped ir
16	3297106	6:20 PM	2012-10-30	Blue Hill Avenue / Cheever Street	Property damage only (no	o 2	Rear-end	Wet	Dark - lighted	r Rain/Cloudy		V1:Southbound / V2:Southbound	V1: Slowing or stopped in traffic / V2:Slowing or stopp	ped ir
17	2955657	1:04 PM	2011-12-18	230 Blue Hill Avenue	Non-fatal injury	3	Rear-end	Dry	Daylight	Clear		V1:Southbound / V2:Southbound / V3	3:S(V1: Slowing or stopped in traffic / V2:Slowing or stopped	oed ir
18	3336297	9:21 PM	2012-08-21	Blue Hill Avenue / Blue Hill Avenue Rte 138 / Concord Avenue	Non-fatal injury	2	Rear-end	Dry	Dark - lighted	d r Clear/Clear		V1:Southbound / V2:Southbound	V1: Slowing or stopped in traffic / V2:Travelling straig	ht ah Tailgating
19	3901474	12:18 PM	2014-07-22	Blue Hill Avenue / Concord Avenue	Non-fatal injury	2	Rear-end	Dry	Daylight	Clear/Clear		V1:Southbound / V2:Southbound	V1: Not reported / V2:Not reported	
20	3247671	12:58 PM	2012-06-27	10 Feet N From Intersection 180 Blue Hill Avenue / Concord Avenue	Non-fatal injury	1	Single vehicle crash	Dry	Daylight	Clear/Clear	ped	V1:Northbound	V1: Overtaking/passing	Can't see
21	3965485	1:17 PM	2014-10-05	180 Blue Hill Avenue	Non-fatal injury	2	Sideswipe, same direct	c Dry	Daylight	Clear		V1:Southbound / V2:Southbound	V1: Travelling straight ahead / V2:Turning left	

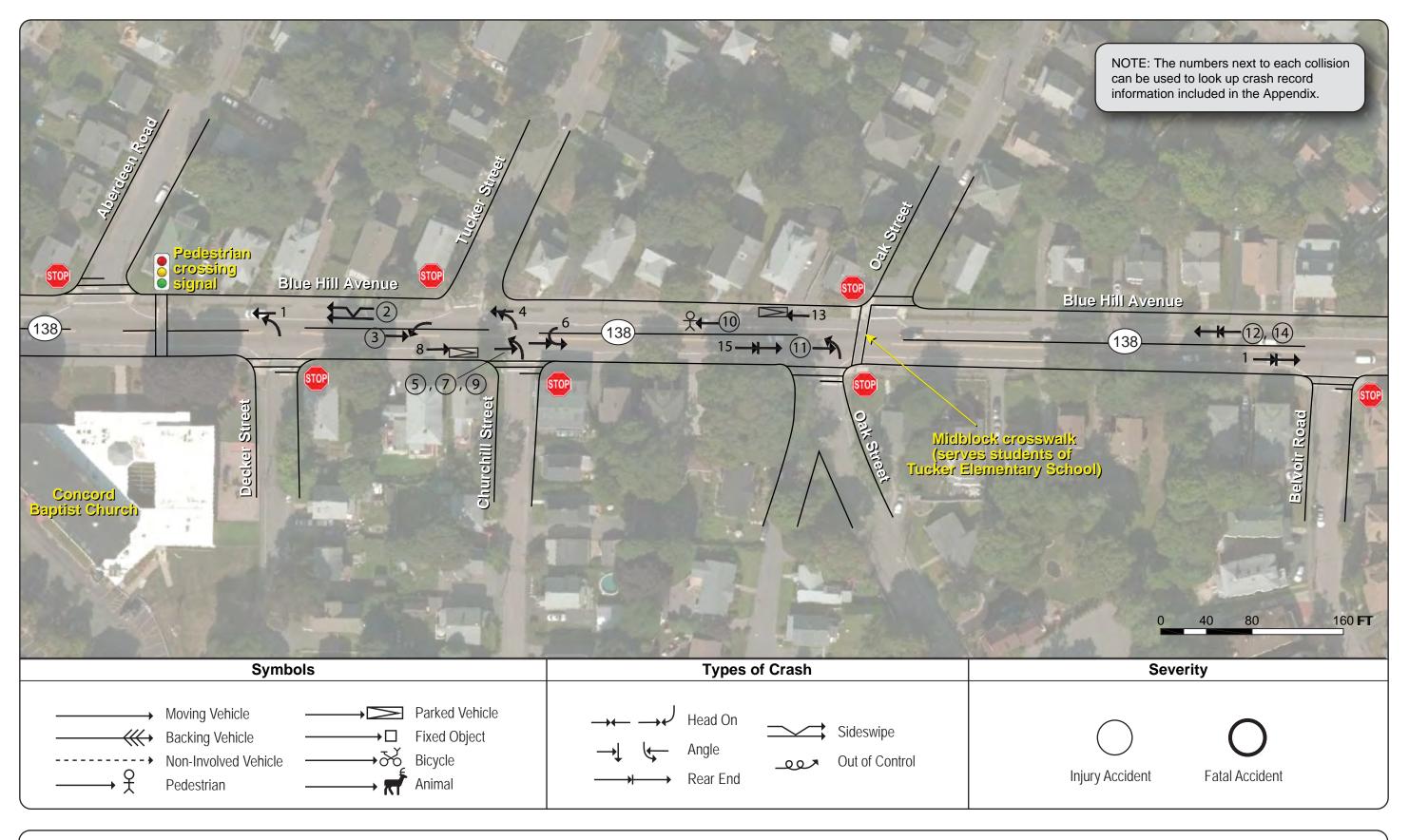




Figure E-11
Collision Diagram 2011-15
Route 138 Oak Street

Table E-11
Route 138 at Oak Street
Route 138 in Milton

Index	Crash Number	Crash Time	Crash Date	Street Address	Crash Severity	# of Vehicles	Manner of Collision		Ambient Light Condition	Weather Condition	Bike/ Ped	Vehicle Traveled Direction	Vehicle Action	Driver Contributing Code
1	2712717	8:06 PM	2011-03-11	Blue Hill Avenue / Decker Street	Property damage only (ne	o 2	Angle	Dry	Dark - lighted	r Clear/Clear		V1:Southbound / V2:Westbound	V1: Travelling straight ahead / V2:Turning left	
2	3202030	2:03 AM	2012-05-11	Blue Hill Avenue / Blue Hill Avenue / Decker Street	Unknown	2	Sideswipe, same direct	ic Dry	Dark - lighted	r Clear		V1:Southbound / V2:Southbound	V1: Turning left / V2:Travelling straight ahead	
3	3937033	3:40 PM	2014-08-03	163 Blue Hill Avenue / Rte 138	Non-fatal injury	2	Angle	Dry	Daylight	Clear		V1:Northbound / V2:Southbound	V1: Not reported / V2:Not reported	
4	2703348	8:08 PM	2011-02-08	Blue Hill Avenue / Churchill Street	Property damage only (ne	o 2	Angle	Dry	Dark - lighted	r Clear		V1:Southbound / V2:Westbound	V1: Travelling straight ahead / V2:Entering traffic lane	Reckless
5	2739637	6:26 PM	2011-06-26	Churchill Street / Blue Hill Avenue Rte 138 S	Non-fatal injury	3	Angle	Dry	Daylight	Cloudy		V1:Northbound / V2:Southbound / V3:	Ea V1: Travelling straight ahead / V2:Parked / V3:Turning le	ft
6	3602797	3:52 PM	2013-09-12	Blue Hill Avenue / Churchill Street / Rte Z	Property damage only (ne	o 2	Angle	Dry	Daylight	Clear		V1:Southbound / V2:Northbound	V1: Making U-turn / V2:Travelling straight ahead	
7	3767843	3:11 PM	2014-01-17	Blue Hill Avenue Rte 138 N / Churchill Street	Non-fatal injury	2	Angle	Dry	Daylight	Clear/Clear		V1:Northbound / V2:Westbound	V1: Travelling straight ahead / V2:Entering traffic lane	Can't see
8	4053899	10:56 PM	2015-03-29	Blue Hill Avenue / Churchill Street	Property damage only (ne	o 3	Single vehicle crash	Dry	Dark - lighted	r Clear/Clear		V1:Northbound / V2:Northbound / V3:I	Nc V1: Travelling straight ahead / V2:Parked / V3:Parked	
9	3336298	5:15 PM	2012-08-18	Blue Hill Avenue / Blue Hill Avenue / Churchill Street	Non-fatal injury	2	Sideswipe, opposite dire	e Wet	Daylight	Rain		V1:Westbound / V2:Northbound	V1: Turning left / V2:Travelling straight ahead	
10	2709440	5:25 PM	2011-01-10	Area Of Union Avenue	Non-fatal injury	1	Single vehicle crash	Dry	Dark - lighted	r Clear/Clear	ped	V1:Southbound	V1: Not reported	
11	3584834	9:35 PM	2013-08-17	Blue Hill Avenue / Blue Hill Avenue / Oak Street	Non-fatal injury	2	Angle	Dry	Dark - lighted	r Clear		V1:Westbound / V2:Northbound	V1: Turning left / V2:Travelling straight ahead	
12	3937034	8:15 AM	2014-07-30	Belvoir	Not Reported	2	Rear-end	Dry	Daylight	Clear		V1:Southbound / V2:Southbound	V1: Not reported / V2:Not reported	
13	4106614	1:08 AM	2015-08-30	Blue Hill Avenue / Oak Street	Property damage only (ne	o 3	Single vehicle crash	Dry	Dark - lighted	r Clear		V1:Southbound / V2:Southbound / V3:	Si V1: Travelling straight ahead / V2:Parked / V3:Parked	Reckless
14	3950629	3:28 PM	2014-09-04	116 Feet S From Intersection Belvoir Road	Non-fatal injury	2	Rear-end	Dry	Daylight	Clear/Clear	•	V1:Southbound / V2:Southbound	V1: Travelling straight ahead / V2:Slowing or stopped in	tri
15	4078624	12:04 PM	2015-06-23	Blue Hill Avenue / Blue Hill Avenue Rte 138 N / Belvoir Road	Property damage only (ne	0 2	Rear-end	Wet	Daylight	Cloudy/Rain		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straight a	h Inattention

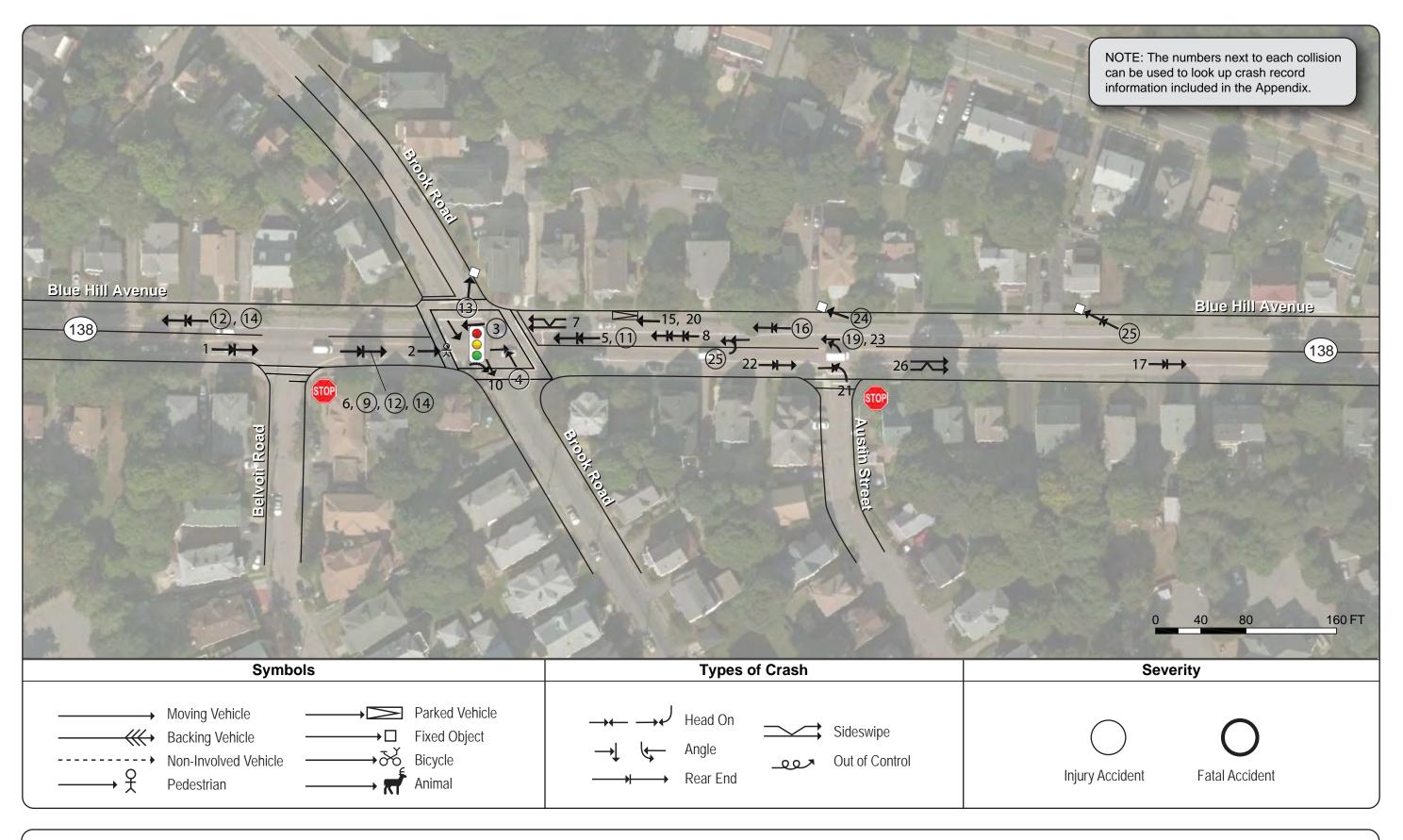




Figure E–12
Collision Diagram 2011–15
Route 138 at Brook Road

Table E-12 Route 138 at Brook Road Route 138 in Milton

Index	Crash Number	Crash Time	Crash Date	Street Address	Crash Severity	# of Vehicles	Manner of Collision		Ambient Light Condition	Weather Condition	Bike/ Ped	Vehicle Traveled Direction	Vehicle Action	Driver Contributing Code
1	2743322	3:50 PM	2011-07-06	Blue Hill Avenue / Belvoir Road	Property damage only (r	no 2	Rear-end	Dry	Daylight	Clear		V1:Northbound / V2:Northbound	V1: Travelling straight ahead / V2:Travelling straight ahea	a(
2	2760767	8:49 PM	2011-08-05	Brook Road / Blue Hill Avenue Rte 138 S	Property damage only (r	no 2	Single vehicle crash	Dry	Dark - lighted	r Clear	ped	V1:Northbound / V2:Eastbound	V1: Travelling straight ahead / V2:Not reported	
3	3176131	7:04 PM	2012-05-10	Brook Road / Blue Hill Avenue	Non-fatal injury	2	Angle	Dry	Daylight	Clear		V1:Eastbound / V2:Southbound	V1: Travelling straight ahead / V2:Travelling straight ahea	a Reckless
4	3288045	12:00 AM	2012-09-29	Blue Hill Avenue Rte 138 / Brook Road	Non-fatal injury	3	Angle	Dry	Daylight	Cloudy		V1:Westbound / V2:Northbound / V3:S	So V1: Travelling straight ahead / V2:Travelling straight ahea	a Disregarding signs
5	3368166	6:32 AM	2013-01-07	Blue Hill Avenue / Brook Road	Property damage only (r	no 2	Rear-end	Dry	Dawn	Clear/Clear		V1:Southbound / V2:Southbound	V1: Travelling straight ahead / V2:Travelling straight ahea	a(
6	3470434	10:05 AM	2013-06-08	Blue Hill Avenue / Brook Road	Property damage only (r	no 2	Rear-end	Wet	Daylight	Clear		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straight a	h
7	3510905	5:20 PM	2013-07-03	Blue Hill Avenue / Brook Road	Property damage only (r	no 2	Sideswipe, same direct	ic Dry	Daylight	Clear/Clear		V1:Southbound / V2:Southbound	V1: Travelling straight ahead / V2:Turning right	
8	3584839	2:58 PM	2013-08-12	Blue Hill Avenue / Blue Hill Avenue Rte 138 S / Brook Road	Property damage only (r	no 3	Rear-end	Dry	Daylight	Clear		V1:Not reported / V2:Southbound / V3	S V1: Slowing or stopped in traffic / V2:Slowing or stopped	ir
9	3771952	7:24 PM	2014-01-01	Blue Hill Avenue / Brook Road	Non-fatal injury	2	Rear-end	Dry	Dark - lighted	r Clear/Clear		V1:Southbound / V2:Southbound	V1: Slowing or stopped in traffic / V2:Travelling straight a	h Fail to yield
10	4055154	6:19 AM	2015-05-28	Brook Road / Blue Hill Avenue Rte 138 N	Property damage only (r	no 2	Sideswipe, same direct	ic Dry	Daylight	Clear/Clear		V1:Eastbound / V2:Eastbound	V1: Travelling straight ahead / V2:Turning right	Inattention
11	4139837	5:49 PM	2015-11-03	Blue Hill Avenue / Brook Road	Non-fatal injury	2	Rear-end	Dry	Dark - lighted	r Clear		V1:Southbound / V2:Southbound	V1: Slowing or stopped in traffic / V2:Travelling straight a	h
12	4139870	2:27 PM	2015-11-14	Blue Hill Avenue / Brook Road	Non-fatal injury	2	Rear-end	Dry	Dark - lighted	r Clear		V1:Southbound / V2:Southbound	V1: Slowing or stopped in traffic / V2:Travelling straight a	h Inattention
13	4148978	8:28 AM	2015-12-03	Blue Hill Avenue Rte 138 / Brook Road	Non-fatal injury	1	Single vehicle crash	Dry	Daylight	Cloudy/Cloudy		V1:Northbound	V1: Other	
14	2766073	2:57 PM	2011-09-16	80 Blue Hill Avenue	Non-fatal injury	2	Rear-end	Dry	Daylight	Clear		V1:Southbound / V2:Southbound	V1: Slowing or stopped in traffic / V2:Slowing or stopped	ir
15	3015364	5:18 AM	2012-03-16	74 Blue Hill Avenue	Property damage only (r	no 2	Rear-end	Dry	Dark - lighted	r Clear/Clear		V1:Southbound / V2:Southbound	V1: Travelling straight ahead / V2:Parked	Distracted
16	3827583	2:44 PM	2014-05-19	10 Feet S From Intersection Blue Hill Avenue / Austin Street	Non-fatal injury	2	Rear-end	Dry	Daylight	Clear/Clear		V1:Southbound / V2:Southbound	V1: Slowing or stopped in traffic / V2:Travelling straight a	h
17	3292240	9:29 AM	2012-10-13	300 Feet N From Intersection 36 Blue Hill Avenue Rte 138 N / Brook R	o Property damage only (r	no 2	Rear-end	Dry	Daylight	Clear		V1:Northbound / V2:Northbound	V1: Travelling straight ahead / V2:Slowing or stopped in t	ri
18	2725478	3:49 AM	2011-05-07	Blue Hill Avenue / Austin Street	Not Reported	2	Rear-end	Dry	Dark - lighted	r Clear		V1:Southbound / V2:Not reported	V1: Travelling straight ahead / V2:Parked	
19	3211383	8:09 AM	2012-05-24	Blue Hill Avenue / Austin Street	Non-fatal injury	2	Angle	Dry	Daylight	Clear		V1:Southbound / V2:Southbound	V1: Travelling straight ahead / V2:Entering traffic lane	
20	3667931	9:19 PM	2013-10-16	77 Blue Hill Avenue / Austin Street	Property damage only (r	no 2	Single vehicle crash	Dry	Dark - lighted	r Clear		V1:Southbound / V2:Not reported	V1: Leaving traffic lane / V2:Parked	
21	3801615	9:27 PM	2014-03-31	Blue Hill Avenue / Austin Street	Property damage only (r	no 2	Sideswipe, opposite dir	e Wet	Dark - lighted	r Rain/Rain		V1:Northbound / V2:Westbound	V1: Travelling straight ahead / V2:Turning left	Fail to yield
22	3811239	8:09 PM	2014-04-26	Blue Hill Avenue Rte 138 N / Austin Street	Property damage only (r	no 2	Rear-end	Dry	Dark - lighted	r Clear/Clear		V1:Northbound / V2:Northbound	V1: Slowing or stopped in traffic / V2:Travelling straight a	h Inattention
23	4094633	7:59 AM	2015-07-21	Blue Hill Avenue Rte 138 S / Austin Street	Property damage only (r	no 2	Angle	Dry	Daylight	Cloudy		V1:Southbound / V2:Westbound	V1: Travelling straight ahead / V2:Not reported	Fail to yield
24	4149466	6:29 AM	2015-09-30	Blue Hill Avenue / Austin Street	Non-fatal injury	1	Single vehicle crash	Wet	Dawn	Rain/Rain		V1:Southbound	V1: Travelling straight ahead	Object in road
25	3928311	5:41 PM	2014-08-23	Blue Hill Avenue / Austin Street	Non-fatal injury	2	Angle	Dry	Daylight	Cloudy/Cloudy		V1:Southbound / V2:Westbound	V1: Not reported / V2:Not reported	
26	4132363	6:50 PM	2015-10-24	56 Blue Hill Avenue	Property damage only (r	no 2	Sideswipe, same direct	ic Dry	Dark - lighted	r Clear/Clear		V1:Northbound / V2:Northbound	V1: Travelling straight ahead / V2:Travelling straight ahea	3(

Part 2: Expected Crashes Analysis

Figure E–13 2011–2015 Observed Crashes

Severity



Peak vs. Off-Peak



Manner of Collision



At or Between Intersections

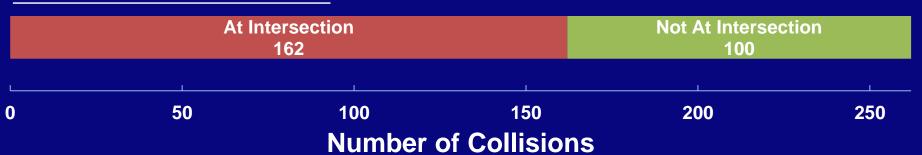
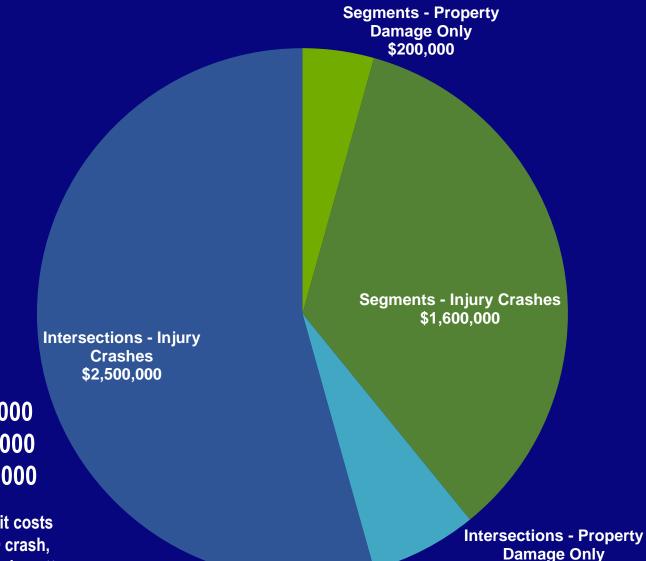


Figure E–14 Annual Comprehensive Cost Estimate: Existing Conditions



\$300,000

Annual Crash Cost:

Fatal and Injury = \$4,100,000

Property Damage Only = \$500,000

Total Cost = \$4,600,000

Note: Recommended comprehensive crash unit costs are \$260,800 per FI crash and \$15,600 per PDO crash, from 2016 FHWA estimates adjusted for Massachusetts.

Figure E–15 Annual Comprehensive Cost Estimate By Facility

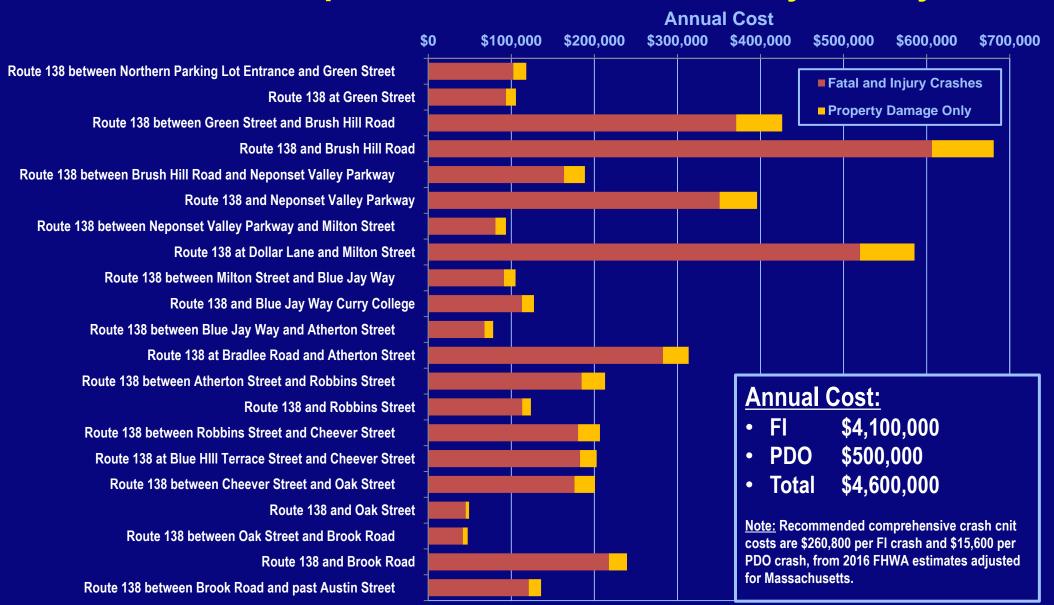


Table E-1
Summary of Expected Crashes Analysis for Existing Conditions
Route 138 in Milton

Site ID	Location	Analysis Type	Total observed crashes	Average observed crashes	Average predicted crashes	•	Average expected crashes	Potential for Safety Improvement (PSI)	High- Risk Site	Observed crashes > Expected Crashes	FI Crash Rate	PDO Crash Rate	FI Cost	PDO Cost	Total Cost
S1	Route 138 between Northern Parking Lot Entrance and Green S	Segment	7	1.4		7.0	1.40	-0.32	-	N	0.39	1.01	\$102,289	\$15,768	\$118,100
11	Route 138 at Green Street	Intersection	3	0.6		5.7		-1.26			0.36	0.77	\$93,589	\$12,035	\$105,600
S2	Route 138 between Green Street and Brush Hill Road	Segment	27	5.4		24.9	4.97	1.94	-	Y	1.42	3.55	\$370,652	\$55,392	\$426,000
14	Route 138 and Brush Hill Road	Intersection	32	6.4		35.4		-0.48			2.33	4.76	\$606,398	\$74,291	\$680,700
S 3	Route 138 between Brush Hill Road and Neponset Valley Parkw	Segment	10	2.0		11.2	2.24	-0.76	-	N	0.63	1.61	\$163,497	\$25,086	\$188,600
16	Route 138 and Neponset Valley Parkway	Intersection	21	4.2		21.2		1.19	Y		1.34	2.89	\$350,710	\$45,098	\$395,800
S4	Route 138 between Neponset Valley Parkway and Milton Street	Segment	6	1.2		5.7	1.13	0.33	-	Y	0.31	0.82	\$80,765	\$12,828	\$93,600
17	Route 138 at Dollar Lane and Milton Street	Intersection	33	6.6		31.0		1.74	Y	Y	1.99	4.20	\$519,812	\$65,535	\$585,300
S5	Route 138 between Milton Street and Blue Jay Way	Segment	6	1.2		6.3	1.25	-0.02	-	N	0.35	0.90	\$91,065	\$14,068	\$105,100
18	Route 138 and Blue Jay Way Curry College	Intersection	9	1.8		6.8		0.36	Y	Y	0.43	0.93	\$112,798	\$14,505	\$127,300
S6	Route 138 between Blue Jay Way and Atherton Street	Segment	4	0.8		4.6	0.93	-0.32	-	N	0.26	0.67	\$67,756	\$10,408	\$78,200
19	Route 138 at Bradlee Road and Atherton Street	Intersection	17	3.4		15.4		0.58	Y	Y	1.08	1.99	\$282,477	\$31,025	\$313,500
S7	Route 138 between Atherton Street and Robbins Street	Segment	13	2.6		12.6	2.52	0.28	-	Y	0.71	1.81	\$184,596	\$28,255	\$212,900
20	Route 138 and Robbins Street	Intersection	5	1.0		5.6		0.09	Y		0.43	0.68	\$113,023	\$10,576	\$123,600
S8	Route 138 between Robbins Street and Cheever Street	Segment	13	2.6		11.9	2.38	0.25	-	Y	0.69	1.69	\$180,289	\$26,391	\$206,700
21	Route 138 at Blue HIII Terrace Street and Cheever Street	Intersection	10	2.0		9.9		0.20	Y	Y	0.70	1.29	\$182,736	\$20,070	\$202,800
S 9	Route 138 between Cheever Street and Oak Street	Segment	16	3.2		11.3	2.25	1.27	-	Y	0.68	1.58	\$176,109	\$24,613	\$200,700
23	Route 138 and Oak Street	Intersection	2	0.4		2.2		0.04	Y		0.17	0.27	\$44,985	\$4,210	\$49,200
S10	Route 138 between Oak Street and Brook Road	Segment	3	0.6		2.7	0.54	0.08	-	Y	0.16	0.38	\$41,569	\$5,922	\$47,500
24	Route 138 and Brook Road	Intersection	12	2.4		11.2		0.56	Y	Y	0.83	1.40	\$217,390	\$21,852	\$239,200
S11	Route 138 between Brook Road and past Austin Street	Segment	13	2.6		7.1	1.42	0.87	-	Y	0.46	0.96	\$120,873	\$14,969	\$135,800
Total	Entire Route 138 Corridor		262	52.4	43.28	249.5	49.89	15 of 21	8 of 10	12 of 21	15.7	34.2	\$4,103,377	\$532,896	\$4,636,200

General Information	n	Location Information			
Analyst	Ben Erban	Intersection	Route 138 at Green Street		
Agency or Company	CTPS	Intersection Type	3ST		
Date Performed	May-18	Jurisdiction	MassDOT Highwat District 6		
City	Milton	Analysis Year	2018		

	Input Information												
Year	Oserved MV crashes	Observed total crashes	Predicted MV crashes	Predicted total crashes	Combined CMF for veh-ped crashes								
2015	0	0	2.07	2.46	1.00								
2014	1	1	2.05	2.43	1.00								
2013	2	2	2.02	2.39	1.00								
2012	0	0	1.98	2.35	1.00								
2011	0	0	1.95	2.31	1.00								

	Output Information												
Observed MV crashes	Average observed total crashes	Total predicted MV crashes	Average predicted total crashes	Standard deviation of predicted total crashes	Weight	Total expected MV crashes	No of expected total crashes	Average expected total crashes	High-risk Intersection (Y/N)	Safety	If avg observed total crashes > avg expected crashes		
3.00	0.60	10.07	2.39	0.06	0.29	5.07	1.16 1.15 1.13 1.11 1.09	1.13	N	-1.26	M		

	Legend	
Required Manual Input	Select from Drop-Down List	Optional Input

	Worksheet 2A General Information and Inpu	t Data for Urban and Suburban A	Arterial Intersections			
Genera	I Information		Location Information			
Analyst	Ben Erban	City	Milton			
Agency or Company	CTPS	Intersection	Route 138 at Green Street			
Date Performed	05/01/18	Jurisdiction	MassDOT Highwat District 6			
	·	Analysis Year	2015			
Inj	put Data	Base Conditions	Site Conditions			
Intersection type (3ST, 3SG, 4ST, 4SG)			3ST			
AADT _{major} (veh/day)	$AADT_{MAX} = 45,700 (veh/day)$		35,500			
AADT _{minor} (veh/day)	$AADT_{MAX} = 9,300$ (veh/day)		400			
Intersection lighting (present/not present)		Not Present	Not Present			
Calibration factor, C _i		1.00	1.13			
Data for unsignalized intersections only:						
Number of major-road approaches with left	-turn lanes (0,1,2)		0			
Number of major-road approaches with right	nt-turn lanes (0,1,2)		0			

	SUMMARY OF INTERSECTION & CRASHES												
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF ₄i	CMF 5i	N _{bi_total}	N _{pedi_total}	N _{bikei_total}	N predictedint					
1.00	1.00	1.00	1.00	1.00	2.319	0.053	0.089	2.462					

ADDITIONAL COMMENTS

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections												
(1)	(2)	(3)	(4)	(5)	(6)	(7)							
CMF for Left-Turn Lanes	CMF for Left-Turn Signal	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF							
	Phasing												
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}							
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)							
1.00	1.00	1.00	1.00	1.00	1.00	1.00							

(1)	(1) (2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)	
Crash Severity Level SPF Coefficients		Overdispersion Parameter, k Initial N _{bimv}		Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Adjusted N _{bimv}		
	a fro	om Table 12-10 b) C	from Table 12-10	from Equation 12- 21		(4) _{TOTAL} *(5)	(7) from Worksheet 2B		(6)*(7)*(8)
Total	-20.02	1.66	0.54	0.24	1.836	1.000	1.836	1.00	1.13	2.075
Fatal and Injury (FI)					0.510		0.510	1.00	1.13	0.576
Property Damage Only (PDO)					1.327		1.327	1.00	1.13	1.499

	Worksheet 2D Multiple-V	ehicle Collisions by Collis	ion Type for Urban and Suburb	an Arterial Intersections	
(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C
Total	1.000	0.576	1.000	1.499	2.075
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Rear-end collision	0.411	0.237	0.337	0.505	0.742
Head-on collision	0.014	0.008	0.011	0.016	0.024
Angle collision	0.562	0.323	0.579	0.868	1.191
Sideswipe	0.014	0.008	0.074	0.110	0.118
Other multiple-vehicle collision	0.000	0.000	0.000	0.000	0.000

	Worksheet 2E Single-Vehicle Collisions by Severity Level for Urban and Suburban Arterial Intersections											
(1)	(1) (2)			(3) (4)		(5)	(6)	(7)	(8)	(9)		
	SPF Coefficients from Table 12-12		Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}			
Crash Severity Level						(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)			
	a	h	С	from Table 1A	(3) * (4) From		(')TOTAL (O)	Worksheet 2B		(5) (.) (0)		
	<u> </u>	b	Ŭ		Worksheet 2C							
Total				0.118	0.216	1.000	0.216	1.00	1.13	0.245		
Fatal and Injury (FI)				0.258	0.056		0.056	1.00	1.13	0.063		
r didi dila injuly (i i)				0.200	0.000		0.000	1.00	1.10	0.000		
Property Damage Only (PDO)				0.742	0.161		0.161	1.00	1.13	0.181		

Worksheet 2F -- Single-Vehicle Collisions by Collision Type for Urban and Suburban Arterial Intersections

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv (FI)} (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.000	0.063	1.000	0.181	0.245
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.000	0.000	0.043	0.008	0.008
Collision with animal	0.000	0.000	0.043	0.008	0.008
Collision with fixed object	0.625	0.039	0.870	0.158	0.197
Collision with other object	0.125	0.008	0.043	0.008	0.016
Other single-vehicle collision	0.000	0.000	0.000	0.000	0.000
Single-vehicle noncollision	0.250	0.016	0.000	0.000	0.016

Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Stop-Controlled Intersections											
(1)	(1) (2) (3) (4) (5) (6) (7)										
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh- Ped ω Calibration		Predicted N _{pedi}					
	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	from Table 1B	factor, C _i	(4)*(5)*(6)					
Total	2.075	0.245	2.319	0.020	1.13	0.053					
Fatal and injury (FI)					1.13	0.053					

	Worksheet 2J Vehicle	e-Bicycle Collisions for Urb	an and Suburban A	Arterial Intersections							
(1)	(1) (2) (3) (4) (5) (6) (7)										
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω Calibration factor, C _i		Predicted N _{bikei}					
	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)			(4)*(5)*(6)					
Total	2.075	0.245	2.319	0.034	1.13	0.089					
Fatal and injury (FI)					1.13	0.089					

Worksh	eet 2K Crash Severity Distribution for Urban an	d Suburban Arterial Intersections	
(1)	(2)	(3)	(4)
, ,	Fatal and injury (FI)	Property damage only (PDO)	Total
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;
	(7) from Worksheet 2H and 2J		(7) from Worksheet 2H and 2J
	MULTIPLE-VEHICLE	·	
Rear-end collisions (from Worksheet 2D)	0.237	0.505	0.742
Head-on collisions (from Worksheet 2D)	0.008	0.016	0.024
Angle collisions (from Worksheet 2D)	0.323	0.868	1.191
Sideswipe (from Worksheet 2D)	0.008	0.110	0.118
Other multiple-vehicle collision (from Worksheet 2D)	0.000	0.000	0.000
Subtotal	0.576	1.499	2.075
	SINGLE-VEHICLE		
Collision with parked vehicle (from Worksheet 2F)	0.000	0.008	0.008
Collision with animal (from Worksheet 2F)	0.000	0.008	0.008
Collision with fixed object (from Worksheet 2F)	0.039	0.158	0.197
Collision with other object (from Worksheet 2F)	0.008	0.008	0.016
Other single-vehicle collision (from Worksheet 2F)	0.000	0.000	0.000
Single-vehicle noncollision (from Worksheet 2F)	0.016	0.000	0.016
Collision with pedestrian (from Worksheet 2G or 2I)	0.053	0.000	0.053
Collision with bicycle (from Worksheet 2J)	0.089	0.000	0.089
Subtotal	0.206	0.181	0.387
Total	0.782	1.680	2.462

Worksheet 2L Summary R	Worksheet 2L Summary Results for Urban and Suburban Arterial Intersections						
(1)	(2)						
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)						
	(Total) from Worksheet 2K						
Total	2.462						
Fatal and injury (FI)	0.782						
Property damage only (PDO)	1.680						

	Legend						
Required Manual Input	Select from Drop-Down List		Optional Input				

	Worksheet 2A General Information ar	nd Input D	ata for Urban and Suburban A	rterial Interse	ctions	
Gener	al Information	Location Information				
Analyst	Ben Erban		City		Milton	
Agency or Company	CTPS		Intersection		Route 138 at Green Street	
Date Performed	05/01/18		Jurisdiction		MassDOT Highwat District 6	
			Analysis Year	2014		
I	nput Data		Base Conditions		Site Conditions	
Intersection type (3ST, 3SG, 4ST, 4SG)				3ST		
AADT _{major} (veh/day)	$AADT_{MAX} = 45,700 (vet)$	h/day)			35,200	
AADT _{minor} (veh/day)	$AADT_{MAX} = 9,300 (vet)$	h/day)			400	
Intersection lighting (present/not present)			Not Present		Not Present	
Calibration factor, C _i			1.00		1.13	
Data for unsignalized intersections only:						
Number of major-road approaches with le	ft-turn lanes (0,1,2)			0		
Number of major-road approaches with ri	ght-turn lanes (0,1,2)				0	

	SUMMARY OF INTERSECTION & CRASHES									
CMF _{1i}	CMF 1i CMF 2i CMF 3i CMF 4i CMF 5i N bi_total N pedi_total N bikei_total N predictedint									
1.00										

ADDITIONAL COMMENTS

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections										
(1) (2) (3) (4) (5) (6) (7)											
CMF for Left-Turn Lanes	CMF for Left-Turn Signal	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF					
	Phasing										
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}					
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)					
1.00	1.00	1.00	1.00	1.00	1.00	1.00					

(1)	(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)	
Crash Severity Level	SPF Coefficients		Overdispersion Parameter, k	Initial N _{bimv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Adjusted N _{bimv}	
	a	om Table 12-10 b	С С	from Table 12-10	from Equation 12- 21		(4) _{TOTAL} *(5)	(7) from Worksheet 2B		(6)*(7)*(8)
Total	-20.02	1.66	0.54	0.24	1.810	1.000	1.810	1.00	1.13	2.046
Fatal and Injury (FI)					0.503		0.503	1.00	1.13	0.568
Property Damage Only (PDO)					1.308		1.308	1.00	1.13	1.478

(1)	(2)	(3)	ion Type for Urban and Suburba (4)	(6)		
Collision Type	Proportion of Collision Type _(Fi)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)	
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C	
Total	1.000	0.568	1.000	1.478	2.046	
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)	
Rear-end collision	0.411	0.233	0.337	0.498	0.731	
Head-on collision	0.014	0.008	0.011	0.016	0.023	
Angle collision	0.562	0.319	0.579	0.856	1.175	
Sideswipe	0.014	0.008	0.074	0.109	0.117	
Other multiple-vehicle collision	0.000	0.000	0.000	0.000	0.000	

		Worksheet	2E Single-V	ehicle Collisions by Seve	ity Level for Urban a	and Suburban Arterial Int	ersections			
(1)	(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)	
	SPF Coefficients		Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}	
Crash Severity Level	fr	om Table 12-12	2				(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	h	С	from Table 1A	(3) * (4) From		() TOTAL (O)	Worksheet 2B		(0) (1) (0)
	u u	Ü	•		Worksheet 2C					
Total				0.118	0.213	1.000	0.213	1.00	1.13	0.241
Fatal and Injury (FI)				0.258	0.055		0.055	1.00	1.13	0.062
ratar and injury (i i)				0.200	0.000		0.000	1.00	1.10	0.002
Property Damage Only (PDO)				0.742	0.158		0.158	1.00	1.13	0.179

Worksheet 2F -- Single-Vehicle Collisions by Collision Type for Urban and Suburban Arterial Intersections

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv (FI)} (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.000	0.062	1.000	0.179	0.241
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.000	0.000	0.043	0.008	0.008
Collision with animal	0.000	0.000	0.043	0.008	0.008
Collision with fixed object	0.625	0.039	0.870	0.156	0.194
Collision with other object	0.125	0.008	0.043	0.008	0.016
Other single-vehicle collision	0.000	0.000	0.000	0.000	0.000
Single-vehicle noncollision	0.250	0.016	0.000	0.000	0.016

Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Stop-Controlled Intersections						
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh- Ped ω	Calibration factor, C _i	Predicted N _{pedi}
	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	from Table 1B		(4)*(5)*(6)
Total	2.046	0.241	2.287	0.020	1.13	0.053
atal and injury (FI)					1.13	0.053

Worksheet 2J Vehicle-Bicycle Collisions for Urban and Suburban Arterial Intersections						
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω Calibration factor, C _i	Calibration	Predicted N _{bikei}
	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)		(4)*(5)*(6)	
Total	2.046	0.241	2.287	0.034	1.13	0.088
Fatal and injury (FI)					1.13	0.088

Worksheet 2K Crash Severity Distribution for Urban and Suburban Arterial Intersections					
(1)	(2)	(3)	(4)		
	Fatal and injury (FI)	Property damage only (PDO)	Total		
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;		
	(7) from Worksheet 2H and 2J		(7) from Worksheet 2H and 2J		
	MULTIPLE-VEHICLE	·			
Rear-end collisions (from Worksheet 2D)	0.233	0.498	0.731		
Head-on collisions (from Worksheet 2D)	0.008	0.016	0.023		
Angle collisions (from Worksheet 2D)	0.319	0.856	1.175		
Sideswipe (from Worksheet 2D)	0.008	0.109	0.117		
Other multiple-vehicle collision (from Worksheet 2D)	0.000	0.000	0.000		
Subtotal	0.568	1.478	2.046		
	SINGLE-VEHICLE	·	•		
Collision with parked vehicle (from Worksheet 2F)	0.000	0.008	0.008		
Collision with animal (from Worksheet 2F)	0.000	0.008	0.008		
Collision with fixed object (from Worksheet 2F)	0.039	0.156	0.194		
Collision with other object (from Worksheet 2F)	0.008	0.008	0.016		
Other single-vehicle collision (from Worksheet 2F)	0.000	0.000	0.000		
Single-vehicle noncollision (from Worksheet 2F)	0.016	0.000	0.016		
Collision with pedestrian (from Worksheet 2G or 2I)	0.053	0.000	0.053		
Collision with bicycle (from Worksheet 2J)	0.088	0.000	0.088		
Subtotal	0.203	0.179	0.382		
Total	0.771	1.657	2.428		

Worksheet 2L Summary Results for Urban and Suburban Arterial Intersections		
(1)	(2)	
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)	
	(Total) from Worksheet 2K	
Total	2.428	
Fatal and injury (FI)	0.771	
Property damage only (PDO)	1.657	

	Leg	gend	
Required Manual Input	Selec	t from Drop-Down List	Optional Input

	Worksheet 2A General Information and In	out Data for Urban and Suburban	terial Intersections			
General	Information		Location Information			
Analyst	Ben Erban	City		Milton		
Agency or Company	CTPS	Intersection	Ro	ute 138 at Green Street		
Date Performed	05/01/18	Jurisdiction	Mass	DOT Highwat District 6		
				2013		
Inp	Base Conditions	Site Conditions				
Intersection type (3ST, 3SG, 4ST, 4SG)				3ST		
AADT _{major} (veh/day)	$AADT_{MAX} = 45,700 (veh/day)$			34,900		
AADT _{minor} (veh/day)	AADT _{MAX} = 9,300 (veh/day			400		
Intersection lighting (present/not present)		Not Present	No	t Present		
Calibration factor, C _i	1.00		1.13			
Data for unsignalized intersections only:						
Number of major-road approaches with left-		0				
Number of major-road approaches with righ			0			

SUMMARY OF INTERSECTION & CRASHES								
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF ₄i	CMF 5i	N _{bi_total}	N _{pedi_total}	N _{bikei_total}	N predictedint
1.00	1.00	1.00	1.00	1.00	2.255	0.052	0.087	2.393

Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections						
(1)	(2)	(3)	(4)	(5)	(6)	(7)
CMF for Left-Turn Lanes	CMF for Left-Turn Signal	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF
	Phasing					
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)
1.00	1.00	1.00	1.00	1.00	1.00	1.00

(1)		(2)	O Manapie-	Vehicle Collisions by Seve	(4)	(5)	(6)	(7)	(8)	(9)
Crash Severity Level	SI	SPF Coefficients		Overdispersion Parameter, k	Initial N _{bimv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Adjusted N _{bimv}
	a fro	om Table 12-10 b) C	from Table 12-10	from Equation 12- 21		(4) _{TOTAL} *(5)	(7) from Worksheet 2B		(6)*(7)*(8)
Total	-20.02	1.66	0.54	0.24	1.785	1.000	1.785	1.00	1.13	2.017
Fatal and Injury (FI)					0.495		0.495	1.00	1.13	0.560
Property Damage Only (PDO)					1.290		1.290	1.00	1.13	1.457

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(Fi)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C
Total	1.000	0.560	1.000	1.457	2.017
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Rear-end collision	0.411	0.230	0.337	0.491	0.721
Head-on collision	0.014	0.008	0.011	0.015	0.023
Angle collision	0.562	0.314	0.579	0.844	1.158
Sideswipe	0.014	0.008	0.074	0.107	0.115
Other multiple-vehicle collision	0.000	0.000	0.000	0.000	0.000

	Worksheet 2E Single-Vehicle Collisions by Severity Level for Urban and Suburban Arterial Intersections									
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
	SI	SPF Coefficients		Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}
Crash Severity Level	fr	om Table 12-12	2				(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	from Table 1A	(3) * (4) From		() TOTAL (O)	Worksheet 2B		(0) (1) (0)
	~	~	•		Worksheet 2C					
Total				0.118	0.210	1.000	0.210	1.00	1.13	0.238
Fatal and Injury (FI)				0.258	0.054		0.054	1.00	1.13	0.061
ratarana mjary (11)				0.200	0.001		0.001	1.00	1.10	0.001
Property Damage Only (PDO)				0.742	0.156		0.156	1.00	1.13	0.176

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv (FI)} (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.000	0.061	1.000	0.176	0.238
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.000	0.000	0.043	0.008	0.008
Collision with animal	0.000	0.000	0.043	0.008	0.008
Collision with fixed object	0.625	0.038	0.870	0.153	0.192
Collision with other object	0.125	0.008	0.043	0.008	0.015
Other single-vehicle collision	0.000	0.000	0.000	0.000	0.000
Single-vehicle noncollision	0.250	0.015	0.000	0.000	0.015

Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Stop-Controlled Intersections						
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Crash Savarity Laval	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh- Ped ω	Calibration	Predicted N _{pedi}
Crash Severity Level	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	from Table 1B	factor, C _i	(4)*(5)*(6)
Total	2.017	0.238	2.255	0.020	1.13	0.052
Fatal and injury (FI)					1.13	0.052

	Worksheet 2J Vehicle	e-Bicycle Collisions for Urb	an and Suburban A	Arterial Intersections		
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Crach Savarity Laval	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration	Predicted N _{bikei}
Crash Severity Level	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	ven-Bike ω factor, C _i		(4)*(5)*(6)
Total	2.017	0.238	2.255	0.034	1.13	0.087
Fatal and injury (FI)					1.13	0.087

Worksh	eet 2K Crash Severity Distribution for Urban ar	nd Suburban Arterial Intersections	
(1)	(2)	(3)	(4)
	Fatal and injury (FI)	Property damage only (PDO)	Total
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;
	(7) from Worksheet 2H and 2J		(7) from Worksheet 2H and 2J
	MULTIPLE-VEHICLE	·	• • •
Rear-end collisions (from Worksheet 2D)	0.230	0.491	0.721
Head-on collisions (from Worksheet 2D)	0.008	0.015	0.023
Angle collisions (from Worksheet 2D)	0.314	0.844	1.158
Sideswipe (from Worksheet 2D)	0.008	0.107	0.115
Other multiple-vehicle collision (from Worksheet 2D)	0.000	0.000	0.000
Subtotal	0.560	1.457	2.017
	SINGLE-VEHICLE	·	·
Collision with parked vehicle (from Worksheet 2F)	0.000	0.008	0.008
Collision with animal (from Worksheet 2F)	0.000	0.008	0.008
Collision with fixed object (from Worksheet 2F)	0.038	0.153	0.192
Collision with other object (from Worksheet 2F)	0.008	0.008	0.015
Other single-vehicle collision (from Worksheet 2F)	0.000	0.000	0.000
Single-vehicle noncollision (from Worksheet 2F)	0.015	0.000	0.015
Collision with pedestrian (from Worksheet 2G or 2I)	0.052	0.000	0.052
Collision with bicycle (from Worksheet 2J)	0.087	0.000	0.087
Subtotal	0.200	0.176	0.376
Total	0.760	1.634	2.393

Worksheet 2L Summary Results for Urban and Suburban Arterial Intersections					
(1)	(2)				
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)				
	(Total) from Worksheet 2K				
Total	2.393				
Fatal and injury (FI)	0.760				
Property damage only (PDO)	1.634				

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	Leg	gend	
Required Manual Input	Selec	t from Drop-Down List	Optional Input

	Worksheet 2A General Information an	nd Input Da	ata for Urban and Suburban A	rterial Interse	ctions	
Genera	l Information	•	Location Information			
Analyst	Ben Erban	Ben Erban			Milton	
Agency or Company	CTPS		Intersection		Route 138 at Green Street	
Date Performed	05/01/18		Jurisdiction		MassDOT Highwat District 6	
	·				2012	
Input Data			Base Conditions	Site Conditions		
Intersection type (3ST, 3SG, 4ST, 4SG)	<u> </u>				3ST	
AADT _{major} (veh/day)	$AADT_{MAX} = 45,700 \text{(veh}$	n/day)			34,550	
AADT _{minor} (veh/day)	$AADT_{MAX} = 9,300$ (veh	n/day)			400	
Intersection lighting (present/not present)			Not Present		Not Present	
Calibration factor, C _i			1.00		1.13	
Data for unsignalized intersections only:						
Number of major-road approaches with lef	t-turn lanes (0,1,2)			0		
Number of major-road approaches with right-turn lanes (0,1,2)					0	

	SUMMARY OF INTERSECTION & CRASHES							
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF ₄i	CMF 5i	N _{bi_total}	N _{pedi_total}	N _{bikei_total}	N predictedint
1.00	1.00	1.00	1.00	1.00	2.217	0.051	0.085	2.354

Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections						
(1)	(2)	(3)	(4)	(5)	(6)	(7)
CMF for Left-Turn Lanes	CMF for Left-Turn Signal	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF
	Phasing					
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)
1.00	1.00	1.00	1.00	1.00	1.00	1.00

(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
Crash Severity Level	SF	PF Coefficient	s	Overdispersion Parameter, k	Initial N _{bimv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Adjusted N _{bimv}
	a fro	om Table 12-10 b) c	from Table 12-10	from Equation 12- 21		(4) _{TOTAL} *(5)	(7) from Worksheet 2B		(6)*(7)*(8)
Total	-20.02	1.66	0.54	0.24	1.755	1.000	1.755	1.00	1.13	1.984
Fatal and Injury (FI)					0.487		0.487	1.00	1.13	0.551
Property Damage Only (PDO)					1.268		1.268	1.00	1.13	1.433

(1)	(2)	(3)	ion Type for Urban and Suburba (4)	(5)	(6)
Collision Type	Proportion of Collision Type _(Fi)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C
Total	1.000	0.551	1.000	1.433	1.984
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Rear-end collision	0.411	0.226	0.337	0.483	0.709
Head-on collision	0.014	0.008	0.011	0.015	0.023
Angle collision	0.562	0.309	0.579	0.830	1.139
Sideswipe	0.014	0.008	0.074	0.106	0.113
Other multiple-vehicle collision	0.000	0.000	0.000	0.000	0.000

		Worksheet	2E Single-V	ehicle Collisions by Seve	ity Level for Urban a	and Suburban Arterial Int	ersections			
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
	SI	PF Coefficients	S	Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}
Crash Severity Level	fre	om Table 12-12	2				(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	h	С	from Table 1A	(3) * (4) From		(I)TOTAL (O)	Worksheet 2B		(0) (1) (0)
	ű	S	9		Worksheet 2C					
Total				0.118	0.207	1.000	0.207	1.00	1.13	0.234
Fatal and Injury (FI)				0.258	0.053		0.053	1.00	1.13	0.060
r didi dila injury (i i)				0.200	0.000		0.000	1.00	11.10	0.000
Property Damage Only (PDO)	-		-	0.742	0.154		0.154	1.00	1.13	0.173

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv (FI)} (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.000	0.060	1.000	0.173	0.234
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.000	0.000	0.043	0.008	0.008
Collision with animal	0.000	0.000	0.043	0.008	0.008
Collision with fixed object	0.625	0.038	0.870	0.151	0.189
Collision with other object	0.125	0.008	0.043	0.008	0.015
Other single-vehicle collision	0.000	0.000	0.000	0.000	0.000
Single-vehicle noncollision	0.250	0.015	0.000	0.000	0.015

Work	sheet 2G Vehicle-Pedestri	an Collisions for Urban and	l Suburban Arteria	Stop-Controlled Interse	ctions	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Crach Soverity Lovel	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh- Ped ω	Calibration	Predicted N _{pedi}
Crash Severity Level	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	from Table 1B	factor, C _i	(4)*(5)*(6)
Total	1.984	0.234	2.217	0.020	1.13	0.051
Fatal and injury (FI)					1.13	0.051

	Worksheet 2J Vehicle	e-Bicycle Collisions for Urb	an and Suburban A	Arterial Intersections		
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Crach Savarity Laval	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration	Predicted N _{bikei}
Crash Severity Level	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	ven-Biκe ω factor, C _i		(4)*(5)*(6)
Total	1.984	0.234	2.217	0.034	1.13	0.085
Fatal and injury (FI)					1.13	0.085

Worksh	eet 2K Crash Severity Distribution for Urban an	d Suburban Arterial Intersections	
(1)	(2)	(3)	(4)
	Fatal and injury (FI)	Property damage only (PDO)	Total
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;
	(7) from Worksheet 2H and 2J		(7) from Worksheet 2H and 2J
	MULTIPLE-VEHICLE	·	
Rear-end collisions (from Worksheet 2D)	0.226	0.483	0.709
Head-on collisions (from Worksheet 2D)	0.008	0.015	0.023
Angle collisions (from Worksheet 2D)	0.309	0.830	1.139
Sideswipe (from Worksheet 2D)	0.008	0.106	0.113
Other multiple-vehicle collision (from Worksheet 2D)	0.000	0.000	0.000
Subtotal	0.551	1.433	1.984
	SINGLE-VEHICLE		
Collision with parked vehicle (from Worksheet 2F)	0.000	0.008	0.008
Collision with animal (from Worksheet 2F)	0.000	0.008	0.008
Collision with fixed object (from Worksheet 2F)	0.038	0.151	0.189
Collision with other object (from Worksheet 2F)	0.008	0.008	0.015
Other single-vehicle collision (from Worksheet 2F)	0.000	0.000	0.000
Single-vehicle noncollision (from Worksheet 2F)	0.015	0.000	0.015
Collision with pedestrian (from Worksheet 2G or 2I)	0.051	0.000	0.051
Collision with bicycle (from Worksheet 2J)	0.085	0.000	0.085
Subtotal	0.197	0.173	0.370
Total	0.747	1.606	2.354

Worksheet 2L Summary R	esults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)
	(Total) from Worksheet 2K
Total	2.354
Fatal and injury (FI)	0.747
Property damage only (PDO)	1.606

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	Legend	
Required Manual Input	Select from Drop-Down List	Optional Input

	Worksheet 2A General Information and Inpu	ut Data for Urban and Suburban A	rterial Intersections			
General	Information		Location Information			
Analyst	Ben Erban	City		Milton		
Agency or Company	CTPS	Intersection		Route 138 at Green Street		
Date Performed	05/01/18	Jurisdiction		MassDOT Highwat District 6		
		Analysis Year		2011		
Inp	Base Conditions	Site Conditions				
Intersection type (3ST, 3SG, 4ST, 4SG)				3ST		
AADT _{major} (veh/day)	$AADT_{MAX} = 45,700 (veh/day)$			34,200		
AADT _{minor} (veh/day)	$AADT_{MAX} = 9,300 (veh/day)$			400		
Intersection lighting (present/not present)		Not Present		Not Present		
Calibration factor, C _i	1.00		1.13			
Data for unsignalized intersections only:						
Number of major-road approaches with left-	Number of major-road approaches with left-turn lanes (0,1,2)			0		
Number of major-road approaches with righ	t-turn lanes (0,1,2)			0		

SUMMARY OF INTERSECTION & CRASHES								
CMF ₁i	CMF _{2i}	CMF _{3i}	CMF ₄i	CMF ₅i	N _{bi_total}	N _{pedi_total}	N _{bikei_total}	N predictedint
1.00	1.00	1.00	1.00	1.00	2.180	0.050	0.084	2.314

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections						
(1)	(2)	(3)	(4)	(5)	(6)	(7)	
CMF for Left-Turn Lanes	CMF for Left-Turn Signal	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF	
	Phasing						
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}	
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)	
1.00	1.00	1.00	1.00	1.00	1.00	1.00	

(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
Crash Severity Level	SF	PF Coefficient	s	Overdispersion Parameter, k	Initial N _{bimv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Adjusted N _{bimv}
	fro	om Table 12-10	0	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from Worksheet 2B		(6)*(7)*(8)
Total	-20.02	1.66	0.54	0.24	1.726	1.000	1.726	1.00	1.13	1.950
Fatal and Injury (FI)					0.479		0.479	1.00	1.13	0.541
Property Damage Only (PDO)					1.247		1.247	1.00	1.13	1.409

	Worksheet 2D Multiple-V	ehicle Collisions by Collis	ion Type for Urban and Suburba	an Arterial Intersections	
(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(Fi)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbirnv} from Worksheet 2C
Total	1.000	0.541	1.000	1.409	1.950
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Rear-end collision	0.411	0.222	0.337	0.475	0.697
Head-on collision	0.014	0.007	0.011	0.015	0.022
Angle collision	0.562	0.304	0.579	0.816	1.120
Sideswipe	0.014	0.007	0.074	0.104	0.111
Other multiple-vehicle collision	0.000	0.000	0.000	0.000	0.000

		Worksheet 2	2E Single-Vo	ehicle Collisions by Sever	ity Level for Urban a	and Suburban Arterial Int	ersections			
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
	SF	PF Coefficients	8	Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}
Crash Severity Level	fro	om Table 12-12	-				(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	h	С	from Table 1A	(3) * (4) From		(4)TOTAL (0)	Worksheet 2B		(0) (1) (0)
	a	Б	C		Worksheet 2C					
Total				0.118	0.203	1.000	0.203	1.00	1.13	0.230
Fatal and Injury (FI)				0.258	0.052		0.052	1.00	1.13	0.059
ratar and injury (i i)				0.200	0.002		0.002	1.00	1.10	0.000
Property Damage Only (PDO)				0.742	0.151		0.151	1.00	1.13	0.171

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv} (FI) (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.000	0.059	1.000	0.171	0.230
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.000	0.000	0.043	0.007	0.007
Collision with animal	0.000	0.000	0.043	0.007	0.007
Collision with fixed object	0.625	0.037	0.870	0.148	0.185
Collision with other object	0.125	0.007	0.043	0.007	0.015
Other single-vehicle collision	0.000	0.000	0.000	0.000	0.000
Single-vehicle noncollision	0.250	0.015	0.000	0.000	0.015

V	Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Stop-Controlled Intersections						
(1)	(2)	(3)	(4)	(5)	(6)	(7)	
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh- Ped ω	Calibration	Predicted N _{pedi}	
Crash Seventy Level	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	from Table 1B	factor, C _i	(4)*(5)*(6)	
Total	1.950	0.230	2.180	0.020	1.13	0.050	
Fatal and injury (FI)					1.13	0.050	

	Worksheet 2J Vehicle-Bicycle Collisions for Urban and Suburban Arterial Intersections						
(1)	(2)	(3)	(4)	(5)	(6)	(7)	
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Vob Bike Calibration		
Crash Seventy Level	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	ven-bike ω	factor, C _i	(4)*(5)*(6)	
Total	1.950	0.230	2.180	0.034	1.13	0.084	
Fatal and injury (FI)					1.13	0.084	

Worksh	eet 2K Crash Severity Distribution for Urban an	nd Suburban Arterial Intersections	
(1)	(2)	(3)	(4)
	Fatal and injury (FI)	Property damage only (PDO)	Total
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;
	(7) from Worksheet 2H and 2J		(7) from Worksheet 2H and 2J
	MULTIPLE-VEHICLE	·	
Rear-end collisions (from Worksheet 2D)	0.222	0.475	0.697
Head-on collisions (from Worksheet 2D)	0.007	0.015	0.022
Angle collisions (from Worksheet 2D)	0.304	0.816	1.120
Sideswipe (from Worksheet 2D)	0.007	0.104	0.111
Other multiple-vehicle collision (from Worksheet 2D)	0.000	0.000	0.000
Subtotal	0.541	1.409	1.950
	SINGLE-VEHICLE		
Collision with parked vehicle (from Worksheet 2F)	0.000	0.007	0.007
Collision with animal (from Worksheet 2F)	0.000	0.007	0.007
Collision with fixed object (from Worksheet 2F)	0.037	0.148	0.185
Collision with other object (from Worksheet 2F)	0.007	0.007	0.015
Other single-vehicle collision (from Worksheet 2F)	0.000	0.000	0.000
Single-vehicle noncollision (from Worksheet 2F)	0.015	0.000	0.015
Collision with pedestrian (from Worksheet 2G or 2I)	0.050	0.000	0.050
Collision with bicycle (from Worksheet 2J)	0.084	0.000	0.084
Subtotal	0.193	0.171	0.364
Total	0.735	1.580	2.314

Worksheet 2L Summary Results for Urban and Suburban Arterial Intersections					
(1)	(2)				
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)				
	(Total) from Worksheet 2K				
Total	2.314				
Fatal and injury (FI)	0.735				
Property damage only (PDO)	1.580				

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General Information	n	Location Information				
Analyst	Ben Erban	Intersection	Route 138 and Brush Hill Road			
Agency or Company	CTPS	Intersection Type	4SG			
Date Performed	May-18	Jurisdiction	MassDOT Highwat District 6			
City	Milton	Analysis Year	2018			

		Input Inf	ormation		
Year	Oserved MV crashes	Observed total crashes	Predicted MV crashes	Predicted total crashes	Combined CMF for veh-ped crashes
2015	9	9	6.86	7.78	1.35
2014	7	8	6.78	7.69	1.35
2013	7	7	6.67	7.57	1.35
2012	4	4	6.58	7.46	1.35
2011	4	4	6.49	7.36	1.35

	Output Information										
Observed MV crashes	Average observed total crashes	Total predicted MV crashes	Average predicted total crashes	Standard deviation of predicted total crashes	Weight	Total expected MV crashes	No of expected total crashes	Average expected total crashes		Potential for Safety Improvement (PSI)	If avg observed total crashes > avg expected crashes
							7.28				
							7.20				
31.00	6.40	33.38	7.57	0.17	0.24	31.57	7.08	7.09	N	-0.48	M
							6.98				
							6.89				

General Information	tion		Location Information		
Analyst	Ben Erban	City	Milton		
Agency or Company	CTPS	Intersection	Route 138 and Brush Hill Road		
Date Performed	5/1/2018	Jurisdiction	MassDOT Highwat District 6		
		Analysis Year	2015		
Input Data		Base Conditions	Site Conditions		
Intersection type (3ST, 3SG, 4ST, 4SG)			4SG		
AADT _{major} (veh/day)	$AADT_{MAX} = 67,700 (veh/day)$		24,600		
AADT _{minor} (veh/day)	$AADI_{MAX} = 33,400 (veh/day)$		6,050		
Intersection lighting (present/not present)	_	Not Present	Not Present		
Calibration factor, C _i		1.00	1.00		
Data for signalized intersections only:					
Number of approaches with left-turn lanes (0,1,2,3,4)	[for 3SG, use maximum value of 3]		1		
Number of approaches with right-turn lanes (0,1,2,3,4	4) [for 3SG, use maximum value of 3]		0		
Number of approaches with left-turn signal phasing [for 3SG, use maximum value of 3]		0		
Type of left-turn signal phasing for Leg #1		Permissive	Permissive		
Type of left-turn signal phasing for Leg #2			Permissive		
Type of left-turn signal phasing for Leg #3			Permissive		
Type of left-turn signal phasing for Leg #4 (if applicate	ole)		Permissive		
Number of approaches with right-turn-on-red prohibit	ed [for 3SG, use maximum value of 3]	0	0		
Intersection red light cameras (present/not present)		Not Present	Not Present		
Sum of all pedestrian crossing volumes (PedVol) 3	Signalized intersections only		0		
Maximum number of lanes crossed by a pedestrian (n _{lanesx})		2		
Number of bus stops within 300 m (1,000 ft) of the in	tersection	0	0		
Schools within 300 m (1,000 ft) of the intersection (pr	resent/not present)	Not Present	Present		
Number of alcohol sales establishments within 300 n	n (1,000 ft) of the intersection	0	0		

	SUMMARY OF INTERSECTION & CRASHES										
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF 4i	CMF 5i	CMF _{1p}	CMF _{2p}	CMF 3p	N _{bi_total}	N_{pedi_total}	N _{bikei_total}	N predictedint
0.900	1.000	1.000	1.000	1.000	1.000	1.350	1.000	7.198	0.278	0.305	7.781

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections										
(1)	(1) (2) (3) (4) (5) (6) (7)										
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF					
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}					
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)					
0.90	1.00	1.00	1.00	1.00	1.00	0.90					

		Worksheet 2	C Multiple-	ehicle Collisions by Seve	rity Level for Urban	and Suburban Arterial Ir	tersections	•		
(1)	(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)	
Crash Severity Level	SPF Coefficients		Overdispersion Parameter, k	Initial N _{bimv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bimv}	
	fro	om Table 12-10)	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	110111 14510 12 10	21		(')TOTAL (')	Worksheet 2B		(0) (1) (0)
Total	-11.79	0.92	0.52	0.10	7.62	1.00	7.62	0.90	1.00	6.86
Fatal and Injury (FI)					2.12		2.12	0.90	1.00	1.91
i atai and injury (i i)					2.12		2.12	0.90	1.00	1.91
Property Damage Only (PDO)					5.50		5.50	0.90	1.00	4.95

	Worksheet 2D Multiple-V	ehicle Collisions by Collis	ion Type for Urban and Suburba	an Arterial Intersections	
(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C
Total	1.00	1.91	1.00	4.95	6.86
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Rear-end collision	0.45	0.86	0.42	2.09	2.95
Head-on collision	0.04	0.07	0.04	0.19	0.26
Angle collision	0.49	0.94	0.43	2.11	3.05
Sideswipe	0.02	0.04	0.11	0.56	0.60
Other multiple-vehicle collision	0.00	0.00	0.00	0.01	0.01

	Worksheet 2E Single-Vehicle Collisions by Severity Level for Urban and Suburban Arterial Intersections											
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)		
	SPF Coefficients		Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bisv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}			
Crash Severity Level	fro	om Table 12-12	2				(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)		
	а	b	С	from Table 1A	(3) * (4) From Worksheet 2C			Worksheet 2B		.,,,,,		
Total				0.05	0.37	1.00	0.37	0.90	1.00	0.34		
Fatal and Injury (FI)				0.18	0.07		0.07	0.90	1.00	0.06		
Property Damage Only (PDO)			1	0.82	0.31		0.31	0.90	1.00	0.27		

	Worksheet 2F Single-Ve	ehicle Collisions by Collisi	on Type for Urban and Suburba	n Arterial Intersections		
(1)	(2)	(3)	(4)	(5)	(6)	
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv (FI)} (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)	
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E	
Total	1.00	0.06	1.00	0.27	0.34	
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)	
Collision with parked vehicle	0.00	0.00	0.03	0.01	0.01	
Collision with animal	0.00	0.00	0.03	0.01	0.01	
Collision with fixed object	0.63	0.04	0.86	0.24	0.27	
Collision with other object	0.00	0.00	0.06	0.02	0.02	
Other single-vehicle collision	0.13	0.01	0.03	0.01	0.02	
Single-vehicle noncollision	0.25	0.02	0.00	0.00	0.02	

Worksheet 2G-1 Crash Modif	Worksheet 2G-1 Crash Modification Factors for Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Signalized Intersections									
(1)	(2)	(3)	(4)							
CMF for Bus Stops	CMF for Schools	CMF for Alcohol Sales Establishments	Combined CMF							
CMF _{1p}	CMF _{2p}	CMF _{3p}	Combined Civil							
from Table 12-28	from Table 12-29	from Table 12-30	(1)*(2)*(3)							
1.00	1.35	1.00	1.35							

	Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Intersections										
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)				
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	- Veh-Ped ω	No. II acc	Calibration factor, C _i	Predicted N _{pedi}				
	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	ven-rea ω	Npedbase		(4) from Worksheet 2H*(6)*(7)				
Total	6.86	0.34	7.20	0.03	0.21	1.00	0.28				
Fatal and injury (FI)						1.00	0.28				

······································							
(1)	(2) (3)		(4)	(5)	(6)	(7)	
Crash Soverity Lovel	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration factor, C _i	Predicted N _{bikei}	
Crash Severity Level	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3) Cambration factor, C _i			(4)*(5)*(6)	
Total	6.86	0.34	7.20	0.04	1.00	0.31	
Fatal and injury (FI)					1.00	0.31	

Worksh	Worksheet 2K Crash Severity Distribution for Urban and Suburban Arterial Intersections								
(1)	(2)	(3)	(4)						
	Fatal and injury (FI)	Property damage only (PDO)	Total						
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;						
	(8) from Worksheet 2G and (7) from 2J		(8) from Worksheet 2G and (7) from 2J						
	MULTIPLE-VEHICLE		•						
Rear-end collisions (from Worksheet 2D)	0.86	2.09	2.95						
Head-on collisions (from Worksheet 2D)	0.07	0.19	0.26						
Angle collisions (from Worksheet 2D)	0.94	2.11	3.05						
Sideswipe (from Worksheet 2D)	0.04	0.56	0.60						
Other multiple-vehicle collision (from Worksheet 2D)	0.00	0.01	0.01						
Subtotal	1.91	4.95	6.86						
	SINGLE-VEHICLE								
Collision with parked vehicle (from Worksheet 2F)	0.00	0.01	0.01						
Collision with animal (from Worksheet 2F)	0.00	0.01	0.01						
Collision with fixed object (from Worksheet 2F)	0.04	0.24	0.27						
Collision with other object (from Worksheet 2F)	0.00	0.02	0.02						
Other single-vehicle collision (from Worksheet 2F)	0.01	0.01	0.02						
Single-vehicle noncollision (from Worksheet 2F)	0.02	0.00	0.02						
Collision with pedestrian (from Worksheet 2G or 2I)	0.28	0.00	0.28						
Collision with bicycle (from Worksheet 2J)	0.31	0.00	0.31						
Subtotal	0.64	0.27	0.92						
Total	2.55	5.23	7.78						

Worksheet 2L Summary R	esults for Urban and Suburban Arterial Intersections			
(1)	(2)			
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)			
	Total			
Total	7.78			
Fatal and injury (FI)	2.55			
Property damage only (PDO)	5.23			

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Worksh	eet 2A General Information and Input	Data for Urban and Suburban A	rterial Intersections		
General Informati	on		Location Information		
Analyst	Ben Erban	City	Milton		
Agency or Company	CTPS	Intersection	Route 138 and Brush Hill Road		
Date Performed	5/1/2018	Jurisdiction	MassDOT Highwat District 6		
		Analysis Year	2014		
Input Data		Base Conditions	Site Conditions		
Intersection type (3ST, 3SG, 4ST, 4SG)			4SG		
AADT _{major} (veh/day)	$AADT_{MAX} = 67,700 (veh/day)$		24,400		
AADT _{minor} (veh/day)	$AADI_{MAX} = 33,400 (veh/day)$		6,000		
Intersection lighting (present/not present)		Not Present	Not Present		
Calibration factor, C _i		1.00	1.00		
Data for signalized intersections only:					
Number of approaches with left-turn lanes (0,1,2,3,4)	[for 3SG, use maximum value of 3]		1		
Number of approaches with right-turn lanes (0,1,2,3,4			0		
Number of approaches with left-turn signal phasing [fo	or 3SG, use maximum value of 3]		0		
Type of left-turn signal phasing for Leg #1		Permissive	Permissive		
Type of left-turn signal phasing for Leg #2			Permissive		
Type of left-turn signal phasing for Leg #3			Permissive		
Type of left-turn signal phasing for Leg #4 (if applicable	e)		Permissive		
Number of approaches with right-turn-on-red prohibite	d [for 3SG, use maximum value of 3]	0	0		
Intersection red light cameras (present/not present)		Not Present	Not Present		
Sum of all pedestrian crossing volumes (PedVol) S	ignalized intersections only		0		
Maximum number of lanes crossed by a pedestrian (r	lanesx)		2		
Number of bus stops within 300 m (1,000 ft) of the into	ersection	0	0		
Schools within 300 m (1,000 ft) of the intersection (pre	esent/not present)	Not Present	Present		
Number of alcohol sales establishments within 300 m	(1,000 ft) of the intersection	0	0		

	SUMMARY OF INTERSECTION & CRASHES										
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF 4i	CMF 5i	CMF _{1p}	CMF _{2p}	CMF 3p	N _{bi_total}	N_{pedi_total}	N _{bikei_total}	N predictedint
0.900										7.690	

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections									
(1) (2) (3) (4) (5) (6) (7)										
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF				
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}				
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)				
0.90	1.00	1.00	1.00	1.00	1.00	0.90				

Worksheet 2C Multiple-Vehicle Collisions by Severity Level for Urban and Suburban Arterial Intersections										
(1)	(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)	
Crash Severity Level	SPF Coefficients		Overdispersion Parameter, k	Initial N _{bimv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bimv}	
	fro	om Table 12-10)	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	110111 14510 12 10	21		(')TOTAL (')	Worksheet 2B		
Total	-11.79	0.92	0.52	0.10	7.53	1.00	7.53	0.90	1.00	6.78
Fatal and Injury (FI)					2.10		2.10	0.90	1.00	1.89
i atai and injury (i i)					2.10		2.10	0.90	1.00	1.09
Property Damage Only (PDO)					5.44		5.44	0.90	1.00	4.90

	Worksheet 2D Multiple-V	ehicle Collisions by Collis	ion Type for Urban and Suburba	an Arterial Intersections		
(1)	(2)	(3)	(4)	(5)	(6)	
Collision Type	Proportion of Collision Type _(FI)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)	
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C	
Total	1.00	1.89	1.00	4.90	6.78	
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)	
Rear-end collision	0.45	0.85	0.42	2.07	2.92	
Head-on collision	0.04	0.07	0.04	0.19	0.26	
Angle collision	0.49	0.93	0.43	2.08	3.01	
Sideswipe	0.02	0.04	0.11	0.55	0.59	
Other multiple-vehicle collision	0.00	0.00	0.00	0.01	0.01	

(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
	SPF Coefficients		Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bisv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}	
Crash Severity Level	from Table 12-12		from Toble 4A		(4) _{TOTAL} *(5)		(7) from		(6)*(7)*(8)	
	а	b	С	from Table 1A	(3) * (4) From Worksheet 2C			Worksheet 2B		
Total				0.05	0.37	1.00	0.37	0.90	1.00	0.33
Fatal and Injury (FI)				0.18	0.07		0.07	0.90	1.00	0.06
Property Damage Only (PDO)				0.82	0.30		0.30	0.90	1.00	0.27

	Worksheet 2F Single-Ve	ehicle Collisions by Collision	on Type for Urban and Suburba	n Arterial Intersections	
(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N bisv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.00	0.06	1.00	0.27	0.33
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.00	0.00	0.03	0.01	0.01
Collision with animal	0.00	0.00	0.03	0.01	0.01
Collision with fixed object	0.63	0.04	0.86	0.23	0.27
Collision with other object	0.00	0.00	0.06	0.02	0.02
Other single-vehicle collision	0.13	0.01	0.03	0.01	0.02
Single-vehicle noncollision	0.25	0.02	0.00	0.00	0.02

Worksheet 2G-1 Crash Modifie	Worksheet 2G-1 Crash Modification Factors for Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Signalized Intersections							
(1)	(2)	(3)	(4)					
CMF for Bus Stops	CMF for Schools	CMF for Alcohol Sales Establishments	Combined CMF					
CMF _{1p}	CMF _{2p}	CMF _{3p}	Combined Civil					
from Table 12-28	from Table 12-29	from Table 12-30	(1)*(2)*(3)					
1.00	1.35	1.00	1.35					

	Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)			
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Ped ω	N	Calibration	Predicted N _{pedi}			
	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	ven-rea ω	Npedbase	factor, C _i	(4) from Worksheet 2H*(6)*(7)			
Total	6.78	0.33	7.11	0.03	0.20	1.00	0.27			
Fatal and injury (FI)						1.00	0.27			

	· · · · · · · · · · · · · · · · · · ·								
(1)	(2)	(3)	(4)	(5)	(6)	(7)			
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration factor, C _i	Predicted N _{bikei}			
	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	Vell-blke w	(4)*(5)*(6)				
Total	6.78	0.33	7.11	0.04	1.00	0.30			
Fatal and injury (FI)					1.00	0.30			

Worksheet 2K Crash Severity Distribution for Urban and Suburban Arterial Intersections								
(1)	(2)	(3)	(4)					
	Fatal and injury (FI)	Property damage only (PDO)	Total					
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;					
	(8) from Worksheet 2G and (7) from 2J		(8) from Worksheet 2G and (7) from 2J					
	MULTIPLE-VEHICLE	•	•					
Rear-end collisions (from Worksheet 2D)	0.85	2.07	2.92					
Head-on collisions (from Worksheet 2D)	0.07	0.19	0.26					
Angle collisions (from Worksheet 2D)	0.93	2.08	3.01					
Sideswipe (from Worksheet 2D)	0.04	0.55	0.59					
Other multiple-vehicle collision (from Worksheet 2D)	0.00	0.01	0.01					
Subtotal	1.89	4.90	6.78					
	SINGLE-VEHICLE							
Collision with parked vehicle (from Worksheet 2F)	0.00	0.01	0.01					
Collision with animal (from Worksheet 2F)	0.00	0.01	0.01					
Collision with fixed object (from Worksheet 2F)	0.04	0.23	0.27					
Collision with other object (from Worksheet 2F)	0.00	0.02	0.02					
Other single-vehicle collision (from Worksheet 2F)	0.01	0.01	0.02					
Single-vehicle noncollision (from Worksheet 2F)	0.02	0.00	0.02					
Collision with pedestrian (from Worksheet 2G or 2I)	0.27	0.00	0.27					
Collision with bicycle (from Worksheet 2J)	0.30	0.00	0.30					
Subtotal	0.64	0.27	0.91					
Total	2.52	5.17	7.69					

Worksheet 2L Summary Re	Worksheet 2L Summary Results for Urban and Suburban Arterial Intersections					
(1)	(2)					
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)					
	Total					
Total	7.69					
Fatal and injury (FI)	2.52					
Property damage only (PDO)	5.17					

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General Inform	sheet 2A General Information and Input		Location Information		
Analyst	Ben Erban	City	Milton		
Agency or Company	CTPS	Intersection Route 138 and Brush Hill F			
Date Performed	5/1/2018	Jurisdiction MassDOT Highwat Dist			
		Analysis Year	2013		
Input Data		Base Conditions	Site Conditions		
Intersection type (3ST, 3SG, 4ST, 4SG)			4SG		
AADT _{major} (veh/day)	$AADT_{MAX} = 67,700 (veh/day)$		24,200		
AADT _{minor} (veh/day)	$AADI_{MAX} = 33,400 (veh/day)$		5,900		
Intersection lighting (present/not present)		Not Present	Not Present		
Calibration factor, C _i		1.00	1.00		
Data for signalized intersections only:					
Number of approaches with left-turn lanes (0,1,2,3,	4) [for 3SG, use maximum value of 3]		1		
Number of approaches with right-turn lanes (0,1,2,3			0		
Number of approaches with left-turn signal phasing	[for 3SG, use maximum value of 3]		0		
Type of left-turn signal phasing for Leg #1		Permissive	Permissive		
Type of left-turn signal phasing for Leg #2			Permissive		
Type of left-turn signal phasing for Leg #3			Permissive		
Type of left-turn signal phasing for Leg #4 (if applications)	able)		Permissive		
Number of approaches with right-turn-on-red prohib	ited [for 3SG, use maximum value of 3]	0	0		
Intersection red light cameras (present/not present)		Not Present	Not Present		
Sum of all pedestrian crossing volumes (PedVol) -			0		
Maximum number of lanes crossed by a pedestrian			2		
Number of bus stops within 300 m (1,000 ft) of the	ntersection	0	0		
Schools within 300 m (1,000 ft) of the intersection (Not Present	Present		
Number of alcohol sales establishments within 300		0	0		

	SUMMARY OF INTERSECTION & CRASHES										
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF 4i	CMF 5i	CMF _{1p}	CMF _{2p}	CMF 3p	N _{bi_total}	N_{pedi_total}	N _{bikei_total}	N predictedint
0.900											

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections									
(1) (2) (3) (4) (5) (6) (7)										
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF				
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}				
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)				
0.90	1.00	1.00	1.00	1.00	1.00	0.90				

	Worksheet 2C Multiple-Vehicle Collisions by Severity Level for Urban and Suburban Arterial Intersections										
(1)	(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)		
Crash Severity Level	SPF Coefficients		Overdispersion Parameter, k	Initial N _{bimv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bimv}		
	fro	om Table 12-10)	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)	
	а	b	С	Hom rable 12 10	21		(· / IOTAL (• /	Worksheet 2B		(0) (1) (0)	
Total	-11.79	0.92	0.52	0.10	7.41	1.00	7.41	0.90	1.00	6.67	
Fatal and Injury (FI)					2.06		2.06	0.90	1.00	1.86	
r atai and injury (i i)	2.00		2.00	0.90	1.00	1.00					
Property Damage Only (PDO)					5.35		5.35	0.90	1.00	4.82	

	Worksheet 2D Multiple-V	ehicle Collisions by Collis	ion Type for Urban and Suburba	an Arterial Intersections		
(1)	(2)	(3)	(4)	(5)	(6)	
Collision Type	Proportion of Collision Type _(FI)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)	
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C	
Total	1.00	1.86	1.00	4.82	6.67	
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)	
Rear-end collision	0.45	0.84	0.42	2.03	2.87	
Head-on collision	0.04	0.07	0.04	0.19	0.25	
Angle collision	0.49	0.91	0.43	2.05	2.96	
Sideswipe	0.02	0.04	0.11	0.54	0.58	
Other multiple-vehicle collision	0.00	0.00	0.00	0.01	0.01	

	Worksheet 2E Single-Vehicle Collisions by Severity Level for Urban and Suburban Arterial Intersections									
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
	SPF Coefficients		Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bisv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}	
Crash Severity Level	from Table 12-12					(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)	
	а	b	С	from Table 1A	(3) * (4) From Worksheet 2C			Worksheet 2B		.,,,,,
Total				0.05	0.36	1.00	0.36	0.90	1.00	0.33
Fatal and Injury (FI)				0.18	0.07		0.07	0.90	1.00	0.06
Property Damage Only (PDO)			1	0.82	0.30		0.30	0.90	1.00	0.27

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	Worksheet 2F Single-Ve	ehicle Collisions by Collision	on Type for Urban and Suburba	n Arterial Intersections		
(1)	(2)	(3)	(4)	(5)	(6)	
Collision Type	Proportion of Collision Type _(FI)			Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)	
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E	
Total	1.00	0.06	1.00	0.27	0.33	
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)	
Collision with parked vehicle	0.00	0.00	0.03	0.01	0.01	
Collision with animal	0.00	0.00	0.03	0.01	0.01	
Collision with fixed object	0.63	0.04	0.86	0.23	0.27	
Collision with other object	0.00	0.00	0.06	0.01	0.01	
Other single-vehicle collision	0.13	0.01	0.03	0.01	0.01	
Single-vehicle noncollision	0.25	0.01	0.00	0.00	0.01	

Worksheet 2G-1 Crash Modi	Worksheet 2G-1 Crash Modification Factors for Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Signalized Intersections							
(1)	(2)	(3)	(4)					
CMF for Bus Stops	CMF for Schools	CMF for Alcohol Sales Establishments	Combined CMF					
CMF _{1p}	CMF _{2p}	CMF _{3p}	Combined Civil					
from Table 12-28	from Table 12-29	from Table 12-30	(1)*(2)*(3)					
1.00	1.35	1.00	1.35					

	Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Intersections								
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)		
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	- Veh-Ped ω	N	Calibration	Predicted N _{pedi}		
	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	ven-rea ω	Npedbase	factor, C _i	(4) from Worksheet 2H*(6)*(7)		
Total	6.67	0.33	7.00	0.03	0.20	1.00	0.27		
Fatal and injury (FI)						1.00	0.27		

(1)	(2)	(3)	(4)	(5)	(6)	(7)	
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration factor, C	Predicted N _{bikei}	
	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	Veii-bike ω	Cambration factor, C _i	(4)*(5)*(6)	
Total	6.67	0.33	7.00	0.04	1.00	0.30	
Fatal and injury (FI)					1.00	0.30	

Worksheet 2K Crash Severity Distribution for Urban and Suburban Arterial Intersections								
(1)	(2)	(3)	(4)					
	Fatal and injury (FI)	Property damage only (PDO)	Total					
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;					
	(8) from Worksheet 2G and (7) from 2J		(8) from Worksheet 2G and (7) from 2J					
	MULTIPLE-VEHICLE	•	•					
Rear-end collisions (from Worksheet 2D)	0.84	2.03	2.87					
Head-on collisions (from Worksheet 2D)	0.07	0.19	0.25					
Angle collisions (from Worksheet 2D)	0.91	2.05	2.96					
Sideswipe (from Worksheet 2D)	0.04	0.54	0.58					
Other multiple-vehicle collision (from Worksheet 2D)	0.00	0.01	0.01					
Subtotal	1.86	4.82	6.67					
	SINGLE-VEHICLE							
Collision with parked vehicle (from Worksheet 2F)	0.00	0.01	0.01					
Collision with animal (from Worksheet 2F)	0.00	0.01	0.01					
Collision with fixed object (from Worksheet 2F)	0.04	0.23	0.27					
Collision with other object (from Worksheet 2F)	0.00	0.01	0.01					
Other single-vehicle collision (from Worksheet 2F)	0.01	0.01	0.01					
Single-vehicle noncollision (from Worksheet 2F)	0.01	0.00	0.01					
Collision with pedestrian (from Worksheet 2G or 2I)	0.27	0.00	0.27					
Collision with bicycle (from Worksheet 2J)	0.30	0.00	0.30					
Subtotal	0.63	0.27	0.89					
Total	2.48	5.08	7.57					

Worksheet 2L Summary R	Worksheet 2L Summary Results for Urban and Suburban Arterial Intersections						
(1)	(2)						
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)						
	Total						
Total	7.57						
Fatal and injury (FI)	2.48						
Property damage only (PDO)	5.08						

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Works	heet 2A General Information and Input	Data for Urban and Suburban A	rterial Intersections		
General Informa	tion	Location Information			
Analyst	Ben Erban	City		Milton	
Agency or Company	CTPS	Intersection Route 138 and Brush Hil			
Date Performed	5/1/2018	Jurisdiction		MassDOT Highwat District 6	
	· ·			2012	
Input Data		Base Conditions		Site Conditions	
ntersection type (3ST, 3SG, 4ST, 4SG)				4SG	
AADT _{major} (veh/day)	$AADT_{MAX} = 67,700 (veh/day)$			23,950	
AADT _{minor} (veh/day)	$AADI_{MAX} = 33,400 (veh/day)$			5,850	
ntersection lighting (present/not present)	_	Not Present	Not Present		
Calibration factor, C _i		1.00		1.00	
Data for signalized intersections only:					
Number of approaches with left-turn lanes (0,1,2,3,4) [for 3SG, use maximum value of 3]			1	
Number of approaches with right-turn lanes (0,1,2,3,				0	
Number of approaches with left-turn signal phasing [for 3SG, use maximum value of 3]		0		
Type of left-turn signal phasing for Leg #1		Permissive		Permissive	
Type of left-turn signal phasing for Leg #2			Permissive		
Type of left-turn signal phasing for Leg #3				Permissive	
Type of left-turn signal phasing for Leg #4 (if applical	ole)		Permissive		
Number of approaches with right-turn-on-red prohibit	ed [for 3SG, use maximum value of 3]	0		0	
Intersection red light cameras (present/not present)	-	Not Present		Not Present	
Sum of all pedestrian crossing volumes (PedVol)	Signalized intersections only			0	
Maximum number of lanes crossed by a pedestrian			2		
Number of bus stops within 300 m (1,000 ft) of the in	tersection	0	0		
Schools within 300 m (1,000 ft) of the intersection (p	resent/not present)	Not Present	Present		
Number of alcohol sales establishments within 300 r	n (1.000 ft) of the intersection	0		0	

	SUMMARY OF INTERSECTION & CRASHES										
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF 4i	CMF 5i	CMF _{1p}	CMF _{2p}	CMF 3p	N _{bi_total}	N_{pedi_total}	N _{bikei_total}	N predictedint
0.900										7.461	

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections									
(1) (2) (3) (4) (5) (6) (7)										
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF				
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}				
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)				
0.90	1.00	1.00	1.00	1.00	1.00	0.90				

	Worksheet 2C Multiple-Vehicle Collisions by Severity Level for Urban and Suburban Arterial Intersections										
(1)	(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)		
Crash Severity Level	SPF Coefficients		Overdispersion Parameter, k	Initial N _{bimv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bimv}		
	fro	om Table 12-10)	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)	
	а	b	С	110111 14510 12 10	21		(')TOTAL (')	Worksheet 2B		(0) (1) (0)	
Total	-11.79	0.92	0.52	0.10	7.31	1.00	7.31	0.90	1.00	6.58	
Fatal and Injury (FI)					2.03		2.03	0.90	1.00	1.83	
r atai and injury (i i)	2.03		2.00	0.90	1.00	1.03					
Property Damage Only (PDO)					5.28		5.28	0.90	1.00	4.75	

Worksheet 2D Multiple-Vehicle Collisions by Collision Type for Urban and Suburban Arterial Intersections								
(1)	(2)	(3)	(4)	(5)	(6)			
Collision Type	Proportion of Collision Type _(FI)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)			
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C			
Total	1.00	1.83	1.00	4.75	6.58			
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)			
Rear-end collision	0.45	0.83	0.42	2.01	2.83			
Head-on collision	0.04	0.07	0.04	0.18	0.25			
Angle collision	0.49	0.90	0.43	2.02	2.92			
Sideswipe	0.02	0.04	0.11	0.53	0.57			
Other multiple-vehicle collision	0.00	0.00	0.00	0.01	0.01			

	Worksheet 2E Single-Vehicle Collisions by Severity Level for Urban and Suburban Arterial Intersections										
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)	
	SPF Coefficients		Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bisv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}		
Crash Severity Level	fro	om Table 12-12	2	frame Table 4.6			(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)	
	а	b	С	from Table 1A	(3) * (4) From Worksheet 2C			Worksheet 2B			
Total				0.05	0.36	1.00	0.36	0.90	1.00	0.32	
Fatal and Injury (FI)				0.18	0.07		0.07	0.90	1.00	0.06	
Property Damage Only (PDO)			ł	0.82	0.29		0.29	0.90	1.00	0.26	

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	Worksheet 2F Single-Vehicle Collisions by Collision Type for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)	(5)	(6)					
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv (FI)} (crashes/year)	` '		Predicted N _{bisv (TOTAL)} (crashes/year)					
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E					
Total	1.00	0.06	1.00	0.26	0.32					
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)					
Collision with parked vehicle	0.00	0.00	0.03	0.01	0.01					
Collision with animal	0.00	0.00	0.03	0.01	0.01					
Collision with fixed object	0.63	0.04	0.86	0.23	0.26					
Collision with other object	0.00	0.00	0.06	0.01	0.01					
Other single-vehicle collision	0.13	0.01	0.03	0.01	0.01					
Single-vehicle noncollision	0.25	0.01	0.00	0.00	0.01					

Worksheet 2G-1 Crash Modifie	Worksheet 2G-1 Crash Modification Factors for Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Signalized Intersections								
(1)	(2)	(3)	(4)						
CMF for Bus Stops	CMF for Schools	CMF for Alcohol Sales Establishments	Combined CMF						
CMF _{1p}	CMF _{2p}	CMF _{3p}	Combined Civil						
from Table 12-28	from Table 12-29	from Table 12-30	(1)*(2)*(3)						
1.00	1.35	1.00	1.35						

	Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)			
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Ped ω	N	Calibration	Predicted N _{pedi}			
	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	ven-rea ω	Npedbase	factor, C _i	(4) from Worksheet 2H*(6)*(7)			
Total	6.58	0.32	6.90	0.03	0.20	1.00	0.27			
Fatal and injury (FI)						1.00	0.27			

(1)	(2)	(3)	(4)	(5)	(6)	(7)
Crash Severity Level	Predicted N _{bimv}		Predicted N _{bi}	Veh-Bike ω	Calibration factor, C	Predicted N _{bikei}
	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	Veii-bike ω	Cambration factor, C _i	(4)*(5)*(6)
Total	6.58	0.32	6.90	0.04	1.00	0.29
Fatal and injury (FI)					1.00	0.29

Worksheet 2K Crash Severity Distribution for Urban and Suburban Arterial Intersections								
(1)	(2)	(3)	(4)					
	Fatal and injury (FI)	Property damage only (PDO)	Total					
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;					
	(8) from Worksheet 2G and (7) from 2J		(8) from Worksheet 2G and (7) from 2J					
	MULTIPLE-VEHICLE	•	•					
Rear-end collisions (from Worksheet 2D)	0.83	2.01	2.83					
Head-on collisions (from Worksheet 2D)	0.07	0.18	0.25					
Angle collisions (from Worksheet 2D)	0.90	2.02	2.92					
Sideswipe (from Worksheet 2D)	0.04	0.53	0.57					
Other multiple-vehicle collision (from Worksheet 2D)	0.00	0.01	0.01					
Subtotal	1.83	4.75	6.58					
	SINGLE-VEHICLE							
Collision with parked vehicle (from Worksheet 2F)	0.00	0.01	0.01					
Collision with animal (from Worksheet 2F)	0.00	0.01	0.01					
Collision with fixed object (from Worksheet 2F)	0.04	0.23	0.26					
Collision with other object (from Worksheet 2F)	0.00	0.01	0.01					
Other single-vehicle collision (from Worksheet 2F)	0.01	0.01	0.01					
Single-vehicle noncollision (from Worksheet 2F)	0.01	0.00	0.01					
Collision with pedestrian (from Worksheet 2G or 2I)	0.27	0.00	0.27					
Collision with bicycle (from Worksheet 2J)	0.29	0.00	0.29					
Subtotal	0.62	0.26	0.88					
Total	2.45	5.01	7.46					

Worksheet 2L Summary Results for Urban and Suburban Arterial Intersections					
(1)	(2)				
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)				
	Total				
Total	7.46				
Fatal and injury (FI)	2.45				
Property damage only (PDO)	5.01				

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W	orksheet 2A General Information and Input	Data for Urban and Suburban Arteria	al Intersections		
General Inf	ormation	Location Information			
Analyst	Ben Erban	City	Milton		
Agency or Company	CTPS	Intersection Route 138 and Brush H			
Date Performed	5/1/2018	Jurisdiction	MassDOT Highwat District 6		
		Analysis Year	2011		
Input I	Data	Base Conditions	Site Conditions		
Intersection type (3ST, 3SG, 4ST, 4SG)			4SG		
AADT _{major} (veh/day)	$AADT_{MAX} = 67,700 (veh/day)$		23,700		
AADT _{minor} (veh/day)	$AADI_{MAX} = 33,400 (veh/day)$		5,800		
Intersection lighting (present/not present)		Not Present	Not Present		
Calibration factor, C _i		1.00	1.00		
Data for signalized intersections only:					
Number of approaches with left-turn lanes (0,1,	2,3,4) [for 3SG, use maximum value of 3]		1		
Number of approaches with right-turn lanes (0,			0		
Number of approaches with left-turn signal pha	sing [for 3SG, use maximum value of 3]		0		
Type of left-turn signal phasing for Leg #1		Permissive	Permissive		
Type of left-turn signal phasing for Leg #2			Permissive		
Type of left-turn signal phasing for Leg #3			Permissive		
Type of left-turn signal phasing for Leg #4 (if ap	plicable)		Permissive		
Number of approaches with right-turn-on-red pr	ohibited [for 3SG, use maximum value of 3]	0	0		
Intersection red light cameras (present/not pres		Not Present	Not Present		
Sum of all pedestrian crossing volumes (PedV	ol) Signalized intersections only		0		
Maximum number of lanes crossed by a pedes			2		
Number of bus stops within 300 m (1,000 ft) of	the intersection	0	0		
Schools within 300 m (1,000 ft) of the intersecti	on (present/not present)	Not Present	Present		
Number of alcohol sales establishments within	300 m (1,000 ft) of the intersection	0	0		

	SUMMARY OF INTERSECTION & CRASHES										
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF 4i	CMF 5i	CMF _{1p}	CMF _{2p}	CMF 3p	N _{bi_total}	N_{pedi_total}	N _{bikei_total}	N predictedint
0.900											

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections									
(1) (2) (3) (4) (5) (6) (7)										
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF				
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}				
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)				
0.90	1.00	1.00	1.00	1.00	1.00	0.90				

Worksheet 2C Multiple-Vehicle Collisions by Severity Level for Urban and Suburban Arterial Intersections										
(1)	(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)	
Crash Severity Level	SPF Coefficients		Overdispersion Parameter, k	Initial N _{bimv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bimv}	
	fro	om Table 12-10)	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	110111 14510 12 10	21		(')TOTAL (')	Worksheet 2B		(0) (1) (0)
Total	-11.79	0.92	0.52	0.10	7.21	1.00	7.21	0.90	1.00	6.49
Fatal and Injury (FI)					2.00		2.00	0.90	1.00	1.80
i atai and injury (i i)					2.00		2.00	0.90	1.00	1.00
Property Damage Only (PDO)					5.20		5.20	0.90	1.00	4.68

	Worksheet 2D Multiple-V	ehicle Collisions by Collis	ion Type for Urban and Suburba	an Arterial Intersections		
(1)	(2)	(3)	(4)	(5)	(6)	
Collision Type	Proportion of Collision Type _(FI)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)	
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C	
Total	1.00	1.80	1.00	4.68	6.49	
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)	
Rear-end collision	0.45	0.82	0.42	1.98	2.79	
Head-on collision	0.04	0.06	0.04	0.18	0.25	
Angle collision	0.49	0.89	0.43	1.99	2.88	
Sideswipe	0.02	0.04	0.11	0.53	0.56	
Other multiple-vehicle collision	0.00	0.00	0.00	0.01	0.01	

	Worksheet 2E Single-Vehicle Collisions by Severity Level for Urban and Suburban Arterial Intersections									
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
	SPF Coefficients		Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bisv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}	
Crash Severity Level	fro	om Table 12-12	2				(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	from Table 1A	(3) * (4) From Worksheet 2C		() 10 1/12 ()	Worksheet 2B		
Total				0.05	0.35	1.00	0.35	0.90	1.00	0.32
Fatal and Injury (FI)				0.18	0.06		0.06	0.90	1.00	0.06
Property Damage Only (PDO)			1	0.82	0.29		0.29	0.90	1.00	0.26

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	Worksheet 2F Single-Ve	ehicle Collisions by Collision	on Type for Urban and Suburba	n Arterial Intersections	
(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv (FI)} (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.00	0.06	1.00	0.26	0.32
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.00	0.00	0.03	0.01	0.01
Collision with animal	0.00	0.00	0.03	0.01	0.01
Collision with fixed object	0.63	0.04	0.86	0.22	0.26
Collision with other object	0.00	0.00	0.06	0.01	0.01
Other single-vehicle collision	0.13	0.01	0.03	0.01	0.01
Single-vehicle noncollision	0.25	0.01	0.00	0.00	0.01

Worksheet 2G-1 Crash Modifie	Worksheet 2G-1 Crash Modification Factors for Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Signalized Intersections							
(1)	(2)	(3)	(4)					
CMF for Bus Stops	CMF for Schools	CMF for Alcohol Sales Establishments	Combined CMF					
CMF _{1p}	CMF _{2p}	CMF _{3p}	Combined Civil					
from Table 12-28	from Table 12-29	from Table 12-30	(1)*(2)*(3)					
1.00	1.35	1.00	1.35					

	Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Intersections								
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)		
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Ped ω	N	Calibration	Predicted N _{pedi}		
	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	ven-rea ω	Npedbase	factor, C _i	(4) from Worksheet 2H*(6)*(7)		
Total	6.49	0.32	6.80	0.03	0.19	1.00	0.26		
Fatal and injury (FI)						1.00	0.26		

(1)	(2)	(3)	(4)	(5)	(6)	(7)
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	ed N _{bisv} Predicted N _{bi} Veh-Bike ω		Calibration factor, C	Predicted N _{bikei}
Crasii Severily Lever	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	Veii-bike ω	Cambration factor, C _i	(4)*(5)*(6)
Total	6.49	0.32	6.80	0.04	1.00	0.29
Fatal and injury (FI)					1.00	0.29

Worksh	neet 2K Crash Severity Distribution for Urban an	d Suburban Arterial Intersections	
(1)	(2)	(3)	(4)
	Fatal and injury (FI)	Property damage only (PDO)	Total
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;
	(8) from Worksheet 2G and (7) from 2J		(8) from Worksheet 2G and (7) from 2J
	MULTIPLE-VEHICLE		•
Rear-end collisions (from Worksheet 2D)	0.82	1.98	2.79
Head-on collisions (from Worksheet 2D)	0.06	0.18	0.25
Angle collisions (from Worksheet 2D)	0.89	1.99	2.88
Sideswipe (from Worksheet 2D)	0.04	0.53	0.56
Other multiple-vehicle collision (from Worksheet 2D)	0.00	0.01	0.01
Subtotal	1.80	4.68	6.49
	SINGLE-VEHICLE		
Collision with parked vehicle (from Worksheet 2F)	0.00	0.01	0.01
Collision with animal (from Worksheet 2F)	0.00	0.01	0.01
Collision with fixed object (from Worksheet 2F)	0.04	0.22	0.26
Collision with other object (from Worksheet 2F)	0.00	0.01	0.01
Other single-vehicle collision (from Worksheet 2F)	0.01	0.01	0.01
Single-vehicle noncollision (from Worksheet 2F)	0.01	0.00	0.01
Collision with pedestrian (from Worksheet 2G or 2I)	0.26	0.00	0.26
Collision with bicycle (from Worksheet 2J)	0.29	0.00	0.29
Subtotal	0.61	0.26	0.87
Total	2.41	4.94	7.36

Worksheet 2L Summary Results for Urban and Suburban Arterial Intersections						
(1)	(2)					
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)					
	Total					
Total	7.36					
Fatal and injury (FI)	2.41					
Property damage only (PDO)	4.94					

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General Information	n	Location Information			
Analyst	Ben Erban	Intersection	Route 138 and Neponset Valley Parkway		
Agency or Company	CTPS	Intersection Type	3ST		
Date Performed	May-18	Jurisdiction	MassDOT Highwat District 6		
City	Milton	Analysis Year	2018		

		Input Inf	ormation		
Year	Oserved MV crashes	Observed total crashes	Predicted MV crashes	Predicted total crashes	Combined CMF for veh-ped crashes
2015	5	5	2.68	3.17	1.00
2014	5	5	2.60	3.09	1.00
2013	2	2	2.56	3.04	1.00
2012	2	2	2.52	2.99	1.00
2011	7	7	2.47	2.93	1.00

	Output Information										
Observed MV crashes	Average observed total crashes	Total predicted MV crashes	Average predicted total crashes	Standard deviation of predicted total crashes	Weight	Total expected MV crashes	No of expected total crashes	Average expected total crashes	High-risk Intersection (Y/N)	Safety	If avg observed total crashes > avg expected crashes
21.00	4.20	12.83	3.04	0.09	0.25	19.00	4.42 4.29 4.23 4.16 4.08	4.24	Y	1.19	,,

	Legend	
Required Manual Input	Select from Drop-Down List	Optional Input

	Worksheet 2A General Information	on and Input	Data for Urban and Suburban A	rterial Interse	ctions	
Gener	al Information		Location Information			
Analyst	Ben Erban		City	Milton		
Agency or Company	CTPS		Intersection		Route 138 and Neponset Valley Parkway	
Date Performed	05/01/18		Jurisdiction		MassDOT Highwat District 6	
			Analysis Year		2015	
İr	nput Data	Base Conditions	Site Conditions			
Intersection type (3ST, 3SG, 4ST, 4SG)	<u> </u>				3ST	
AADT _{major} (veh/day)	$AADT_{MAX} = 45,700$	(veh/day)			15,000	
AADT _{minor} (veh/day)	$AADT_{MAX} = 9,300$	(veh/day)			9,050	
Intersection lighting (present/not present)			Not Present		Not Present	
Calibration factor, C _i			1.00		1.13	
Data for unsignalized intersections only:						
Number of major-road approaches with le	ft-turn lanes (0,1,2)				0	
Number of major-road approaches with rig	ght-turn lanes (0,1,2)				0	

	SUMMARY OF INTERSECTION & CRASHES									
CMF _{1i}	CMF 1i CMF 2i CMF 3i CMF 4i CMF 5i N bi total N pedi total N bikei total N predictedint									
1.00	1.00	1.00	1.00	1.00	2.991	0.069	0.115	3.175		

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections										
(1)	(2)	(2) (3) (4) (5) (6)									
CMF for Left-Turn Lanes	CMF for Left-Turn Signal	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF					
	Phasing										
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}					
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)					
1.00	1.00	1.00	1.00	1.00	1.00	1.00					

(1)	F	(2)	C Wullipie-	Vehicle Collisions by Seve		anu Suburban Artenai in		(7)	(0)	(0)
Crash Severity Level	SI	PF Coefficient	S	Overdispersion Parameter, k	(4) Initial N _{bimv}	Proportion of Total Crashes	(6) Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	(9) Adjusted N _{bimv}
	a fro	om Table 12-10 b) C	from Table 12-10	from Equation 12- 21		(4) _{TOTAL} *(5)	(7) from Worksheet 2B		(6)*(7)*(8)
Total	-20.02	1.66	0.54	0.24	2.368	1.000	2.368	1.00	1.13	2.675
Fatal and Injury (FI)					0.657		0.657	1.00	1.13	0.743
Property Damage Only (PDO)					1.711		1.711	1.00	1.13	1.933

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(Fi)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C
Total	1.000	0.743	1.000	1.933	2.675
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Rear-end collision	0.411	0.305	0.337	0.651	0.956
Head-on collision	0.014	0.010	0.011	0.020	0.031
Angle collision	0.562	0.417	0.579	1.119	1.536
Sideswipe	0.014	0.010	0.074	0.142	0.153
Other multiple-vehicle collision	0.000	0.000	0.000	0.000	0.000

		Worksheet	2E Single-V	ehicle Collisions by Sever	ity Level for Urban a	and Suburban Arterial Int	ersections			
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
	SI	PF Coefficients	3	Historical Data		Proportion of Total	Adjusted	Combined	Calibration	Predicted
					Initial N _{bisv}	Crashes	N_{bimv}	CMFs	Factor, C _i	N_{bisv}
Crash Severity Level	fre	om Table 12-12	2				(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	a	h	С	from Table 1A	(3) * (4) From		(·/TOTAL (°/	Worksheet 2B		(0) (1) (0)
	a	, b	C		Worksheet 2C					
Total				0.118	0.279	1.000	0.279	1.00	1.13	0.315
Fatal and Injury (FI)				0.258	0.072		0.072	1.00	1.13	0.081
ratar and injury (i i)				0.200	0.012		0.072	1.00	11.10	0.001
Property Damage Only (PDO)				0.742	0.207		0.207	1.00	1.13	0.234

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv (FI)} (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.000	0.081	1.000	0.234	0.315
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.000	0.000	0.043	0.010	0.010
Collision with animal	0.000	0.000	0.043	0.010	0.010
Collision with fixed object	0.625	0.051	0.870	0.203	0.254
Collision with other object	0.125	0.010	0.043	0.010	0.020
Other single-vehicle collision	0.000	0.000	0.000	0.000	0.000
Single-vehicle noncollision	0.250	0.020	0.000	0.000	0.020

Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Stop-Controlled Intersections										
(1) (2) (3) (4) (5) (6) (7)										
Crash Savarity Laval	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh- Ped ω	Calibration	Predicted N _{pedi}				
Crash Severity Level	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	from Table 1B	factor, C _i	(4)*(5)*(6)				
Total	2.675	0.315	2.991	0.020	1.13	0.069				
Fatal and injury (FI)		-			1.13	0.069				

Worksheet 2J Vehicle-Bicycle Collisions for Urban and Suburban Arterial Intersections										
(1)	(1) (2) (3) (4) (5)									
Crock Soverity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Voh Pika	Calibration	Predicted N _{bikei}				
Crash Severity Level	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	Veh-Bike ω factor, C _i		(4)*(5)*(6)				
Total	2.675	0.315	2.991	0.034	1.13	0.115				
Fatal and injury (FI)					1.13	0.115				

Worksheet 2K Crash Severity Distribution for Urban and Suburban Arterial Intersections								
(1)	(2)	(3)	(4)					
• •	Fatal and injury (FI)	Property damage only (PDO)	Total					
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;					
	(7) from Worksheet 2H and 2J		(7) from Worksheet 2H and 2J					
	MULTIPLE-VEHICLE	·						
Rear-end collisions (from Worksheet 2D)	0.305	0.651	0.956					
Head-on collisions (from Worksheet 2D)	0.010	0.020	0.031					
Angle collisions (from Worksheet 2D)	0.417	1.119	1.536					
Sideswipe (from Worksheet 2D)	0.010	0.142	0.153					
Other multiple-vehicle collision (from Worksheet 2D)	0.000	0.000	0.000					
Subtotal	0.743	1.933	2.675					
	SINGLE-VEHICLE	·						
Collision with parked vehicle (from Worksheet 2F)	0.000	0.010	0.010					
Collision with animal (from Worksheet 2F)	0.000	0.010	0.010					
Collision with fixed object (from Worksheet 2F)	0.051	0.203	0.254					
Collision with other object (from Worksheet 2F)	0.010	0.010	0.020					
Other single-vehicle collision (from Worksheet 2F)	0.000	0.000	0.000					
Single-vehicle noncollision (from Worksheet 2F)	0.020	0.000	0.020					
Collision with pedestrian (from Worksheet 2G or 2I)	0.069	0.000	0.069					
Collision with bicycle (from Worksheet 2J)	0.115	0.000	0.115					
Subtotal	0.265	0.234	0.499					
Total	1.008	2.167	3.175					

Worksheet 2L Summary R	esults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)
	(Total) from Worksheet 2K
Total	3.175
Fatal and injury (FI)	1.008
Property damage only (PDO)	2.167

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	Legend					
Required Manual Input	Select from Drop-Down List		Optional Input			

	Worksheet 2A General Informat	ion and Input	Data for Urban and Suburban A	rterial Interse	ctions	
Genera	al Information	•	Location Information			
Analyst	Ben Erban		City		Milton	
Agency or Company	CTPS		Intersection		Route 138 and Neponset Valley Parkway	
Date Performed	05/01/18		Jurisdiction		MassDOT Highwat District 6	
			Analysis Year		2014	
In	Base Conditions	Site Conditions				
Intersection type (3ST, 3SG, 4ST, 4SG)					3ST	
AADT _{major} (veh/day)	$AADT_{MAX} = 45,700$	(veh/day)			14,800	
AADT _{minor} (veh/day)	$AADT_{MAX} = 9,300$	(veh/day)			8,950	
Intersection lighting (present/not present)			Not Present		Not Present	
Calibration factor, C _i			1.00		1.13	
Data for unsignalized intersections only:						
Number of major-road approaches with lef	ft-turn lanes (0,1,2)			0		
Number of major-road approaches with rig	ht-turn lanes (0,1,2)				0	

	SUMMARY OF INTERSECTION & CRASHES									
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF ₄i	CMF 5i	N _{bi_total}	N _{pedi_total}	N _{bikei_total}	N predictedint		
1.00	1.00	1.00	1.00	1.00	2.907	0.067	0.112	3.086		

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)				
CMF for Left-Turn Lanes	CMF for Left-Turn Signal	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF				
	Phasing									
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}				
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)				
1.00	1.00	1.00	1.00	1.00	1.00	1.00				

(1)	(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)	
Crash Severity Level	SF	SPF Coefficients		Overdispersion Parameter, k	Initial N _{bimv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Adjusted N _{bimv}
	a	om Table 12-10 b	С С	from Table 12-10	from Equation 12- 21		(4) _{TOTAL} *(5)	(7) from Worksheet 2B		(6)*(7)*(8)
Total	-20.02	1.66	0.54	0.24	2.302	1.000	2.302	1.00	1.13	2.601
Fatal and Injury (FI)					0.639		0.639	1.00	1.13	0.722
Property Damage Only (PDO)					1.663		1.663	1.00	1.13	1.879

	Worksheet 2D Multiple-V	ehicle Collisions by Collis	ion Type for Urban and Suburb	an Arterial Intersections	
(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C
Total	1.000	0.722	1.000	1.879	2.601
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Rear-end collision	0.411	0.297	0.337	0.633	0.930
Head-on collision	0.014	0.010	0.011	0.020	0.030
Angle collision	0.562	0.405	0.579	1.088	1.493
Sideswipe	0.014	0.010	0.074	0.138	0.148
Other multiple-vehicle collision	0.000	0.000	0.000	0.000	0.000

		Worksheet	2E Single-V	ehicle Collisions by Seve	ity Level for Urban a	and Suburban Arterial Int	ersections			
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
	SI	PF Coefficients	S	Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}
Crash Severity Level	fr	om Table 12-12	2				(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	from Table 1A	(3) * (4) From		() TOTAL (O)	Worksheet 2B		(0) (1) (0)
	~	~	,		Worksheet 2C					
Total				0.118	0.271	1.000	0.271	1.00	1.13	0.307
Fatal and Injury (FI)				0.258	0.070		0.070	1.00	1.13	0.079
ratar and injury (i i)				0.200	0.010		0.070	1.00	1.10	0.070
Property Damage Only (PDO)				0.742	0.201		0.201	1.00	1.13	0.227

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv (FI)} (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.000	0.079	1.000	0.227	0.307
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.000	0.000	0.043	0.010	0.010
Collision with animal	0.000	0.000	0.043	0.010	0.010
Collision with fixed object	0.625	0.049	0.870	0.198	0.247
Collision with other object	0.125	0.010	0.043	0.010	0.020
Other single-vehicle collision	0.000	0.000	0.000	0.000	0.000
Single-vehicle noncollision	0.250	0.020	0.000	0.000	0.020

Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Stop-Controlled Intersections										
(1)	(2)	(3)	(4)	(5)	(6)	(7)				
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh- Ped ω	Calibration	Predicted N _{pedi}				
Crash Severity Level	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	from Table 1B	factor, C _i	(4)*(5)*(6)				
Total	2.601	0.307	2.907	0.020	1.13	0.067				
Fatal and injury (FI)					1.13	0.067				

Worksheet 2J Vehicle-Bicycle Collisions for Urban and Suburban Arterial Intersections										
(1)	(1) (2) (3) (4) (5)									
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration	Predicted N _{bikei}				
Crash Severity Level	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	ven-bike ω	factor, C _i	(4)*(5)*(6)				
Total	2.601	0.307	2.907	0.034	1.13	0.112				
Fatal and injury (FI)					1.13	0.112				

Worksh	eet 2K Crash Severity Distribution for Urban an	d Suburban Arterial Intersections	
(1)	(2)	(3)	(4)
	Fatal and injury (FI)	Property damage only (PDO)	Total
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;
	(7) from Worksheet 2H and 2J		(7) from Worksheet 2H and 2J
	MULTIPLE-VEHICLE	·	
Rear-end collisions (from Worksheet 2D)	0.297	0.633	0.930
Head-on collisions (from Worksheet 2D)	0.010	0.020	0.030
Angle collisions (from Worksheet 2D)	0.405	1.088	1.493
Sideswipe (from Worksheet 2D)	0.010	0.138	0.148
Other multiple-vehicle collision (from Worksheet 2D)	0.000	0.000	0.000
Subtotal	0.722	1.879	2.601
	SINGLE-VEHICLE	·	·
Collision with parked vehicle (from Worksheet 2F)	0.000	0.010	0.010
Collision with animal (from Worksheet 2F)	0.000	0.010	0.010
Collision with fixed object (from Worksheet 2F)	0.049	0.198	0.247
Collision with other object (from Worksheet 2F)	0.010	0.010	0.020
Other single-vehicle collision (from Worksheet 2F)	0.000	0.000	0.000
Single-vehicle noncollision (from Worksheet 2F)	0.020	0.000	0.020
Collision with pedestrian (from Worksheet 2G or 2I)	0.067	0.000	0.067
Collision with bicycle (from Worksheet 2J)	0.112	0.000	0.112
Subtotal	0.258	0.227	0.485
Total	0.980	2.106	3.086

Worksheet 2L Summary R	esults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)
	(Total) from Worksheet 2K
Total	3.086
Fatal and injury (FI)	0.980
Property damage only (PDO)	2.106

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	Legend					
Required Manual Input	Select from Drop-Down List		Optional Input			

	Worksheet 2A General Information	on and Input	Data for Urban and Suburban A	rterial Interse	ctions	
Gener	al Information	Location Information				
Analyst	Ben Erban		City	Milton		
Agency or Company	CTPS		Intersection		Route 138 and Neponset Valley Parkway	
Date Performed	05/01/18		Jurisdiction		MassDOT Highwat District 6	
	·		Analysis Year		2013	
li	Base Conditions	Site Conditions				
Intersection type (3ST, 3SG, 4ST, 4SG)	<u> </u>				3ST	
AADT _{major} (veh/day)	$AADT_{MAX} = 45,700$	(veh/day)			14,700	
AADT _{minor} (veh/day)	$AADT_{MAX} = 9,300$	(veh/day)			8,900	
Intersection lighting (present/not present)			Not Present		Not Present	
Calibration factor, C _i			1.00		1.13	
Data for unsignalized intersections only:						
Number of major-road approaches with le	ft-turn lanes (0,1,2)				0	
Number of major-road approaches with right-turn lanes (0,1,2)				0		

	SUMMARY OF INTERSECTION & CRASHES									
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF ₄i	CMF 5i	N _{bi_total}	N_{pedi_total}	N _{bikei_total}	N predictedint		
1.00	1.00	1.00	1.00	1.00	2.866	0.066	0.110	3.042		

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections										
(1)	(2)	(3)	(4)	(5)	(6)	(7)					
CMF for Left-Turn Lanes	CMF for Left-Turn Signal	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF					
	Phasing										
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}					
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)					
1.00	1.00	1.00	1.00	1.00	1.00	1.00					

(1)		(2)	C Wultiple-	Vehicle Collisions by Seve	(4)	/5\	(6)	(7)	(8)	(9)
Crash Severity Level	SI	PF Coefficient	S	Overdispersion Parameter, k	Initial N _{bimv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Adjusted N _{bimv}
	fro a	om Table 12-10 b	С С	from Table 12-10	from Equation 12- 21		(4) _{TOTAL} *(5)	(7) from Worksheet 2B		(6)*(7)*(8)
Total	-20.02	1.66	0.54	0.24	2.269	1.000	2.269	1.00	1.13	2.564
Fatal and Injury (FI)					0.630		0.630	1.00	1.13	0.712
Property Damage Only (PDO)					1.639		1.639	1.00	1.13	1.852

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(Fi)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C
Total	1.000	0.712	1.000	1.852	2.564
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Rear-end collision	0.411	0.292	0.337	0.624	0.916
Head-on collision	0.014	0.010	0.011	0.019	0.029
Angle collision	0.562	0.400	0.579	1.072	1.472
Sideswipe	0.014	0.010	0.074	0.136	0.146
Other multiple-vehicle collision	0.000	0.000	0.000	0.000	0.000

	Worksheet 2E Single-Vehicle Collisions by Severity Level for Urban and Suburban Arterial Intersections									
(1)	(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)	
	SI	SPF Coefficients		Historical Data		Proportion of Total	Adjusted	Combined	Calibration	Predicted
					Initial N _{bisv}	Crashes	N_{bimv}	CMFs	Factor, C _i	N_{bisv}
Crash Severity Level	fr	om Table 12-12	2				(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	a	h	С	from Table 1A	(3) * (4) From		(·/TOTAL (°/	Worksheet 2B		(0) (1) (0)
	a	, b	C		Worksheet 2C					
Total				0.118	0.267	1.000	0.267	1.00	1.13	0.302
Fatal and Injury (FI)				0.258	0.069		0.069	1.00	1.13	0.078
ratar and injury (i i)				0.200	0.000		0.000	1.00	11.10	0.070
Property Damage Only (PDO)				0.742	0.198		0.198	1.00	1.13	0.224

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv (FI)} (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.000	0.078	1.000	0.224	0.302
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.000	0.000	0.043	0.010	0.010
Collision with animal	0.000	0.000	0.043	0.010	0.010
Collision with fixed object	0.625	0.049	0.870	0.195	0.244
Collision with other object	0.125	0.010	0.043	0.010	0.019
Other single-vehicle collision	0.000	0.000	0.000	0.000	0.000
Single-vehicle noncollision	0.250	0.019	0.000	0.000	0.019

Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Stop-Controlled Intersections									
(1) (2) (3) (4) (5) (6) (7)									
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh- Ped ω	Calibration	Predicted N _{pedi}			
	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	from Table 1B	factor, C _i	(4)*(5)*(6)			
Total	2.564	0.302	2.866	0.020	1.13	0.066			
Fatal and injury (FI)					1.13	0.066			

Worksheet 2J Vehicle-Bicycle Collisions for Urban and Suburban Arterial Intersections									
(1)	(1) (2) (3) (4) (5)								
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω Calibrati factor, 0		Voh Biko Calibration		Predicted N _{bikei}	
	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)			(4)*(5)*(6)			
Total	2.564	0.302	2.866	0.034	1.13	0.110			
Fatal and injury (FI)					1.13	0.110			

Worksheet 2K Crash Severity Distribution for Urban and Suburban Arterial Intersections								
(1)	(2)	(3)	(4)					
	Fatal and injury (FI)	Property damage only (PDO)	Total					
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;					
	(7) from Worksheet 2H and 2J		(7) from Worksheet 2H and 2J					
	MULTIPLE-VEHICLE	·						
Rear-end collisions (from Worksheet 2D)	0.292	0.624	0.916					
Head-on collisions (from Worksheet 2D)	0.010	0.019	0.029					
Angle collisions (from Worksheet 2D)	0.400	1.072	1.472					
Sideswipe (from Worksheet 2D)	0.010	0.136	0.146					
Other multiple-vehicle collision (from Worksheet 2D)	0.000	0.000	0.000					
Subtotal	0.712	1.852	2.564					
	SINGLE-VEHICLE							
Collision with parked vehicle (from Worksheet 2F)	0.000	0.010	0.010					
Collision with animal (from Worksheet 2F)	0.000	0.010	0.010					
Collision with fixed object (from Worksheet 2F)	0.049	0.195	0.244					
Collision with other object (from Worksheet 2F)	0.010	0.010	0.019					
Other single-vehicle collision (from Worksheet 2F)	0.000	0.000	0.000					
Single-vehicle noncollision (from Worksheet 2F)	0.019	0.000	0.019					
Collision with pedestrian (from Worksheet 2G or 2I)	0.066	0.000	0.066					
Collision with bicycle (from Worksheet 2J)	0.110	0.000	0.110					
Subtotal	0.254	0.224	0.478					
Total	0.966	2.077	3.042					

Worksheet 2L Summary R	esults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)
	(Total) from Worksheet 2K
Total	3.042
Fatal and injury (FI)	0.966
Property damage only (PDO)	2.077

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	Legend						
Required Manual Input	Select from Drop-Down List		Optional Input				

	Worksheet 2A General Information and Inpu	it Data for Urban and Suburban A	Arterial Intersec	etions		
Gene	eral Information		Location Information			
Analyst	Ben Erban	City		Milton		
Agency or Company	CTPS	Intersection		Route 138 and Neponset Valley Parkway		
Date Performed	05/01/18	Jurisdiction		MassDOT Highwat District 6		
		Analysis Year 2012		2012		
	Base Conditions		Site Conditions			
Intersection type (3ST, 3SG, 4ST, 4SG)				3ST		
AADT _{major} (veh/day)	$AADT_{MAX} = 45,700 (veh/day)$			14,600		
AADT _{minor} (veh/day)	$AADT_{MAX} = 9,300 (veh/day)$			8,800		
Intersection lighting (present/not present)		Not Present		Not Present		
Calibration factor, C _i	1.00		1.13			
Data for unsignalized intersections only:						
Number of major-road approaches with I		0				
Number of major-road approaches with r	right-turn lanes (0,1,2)		0			

SUMMARY OF INTERSECTION & CRASHES									
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF ₄i	CMF 5i	N _{bi_total}	N _{pedi_total}	N _{bikei_total}	N predictedint	
1.00	1.00	1.00	1.00	1.00	2.817	0.065	0.108	2.990	

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections										
(1) (2) (3) (4) (5) (6) (7)											
CMF for Left-Turn Lanes	CMF for Left-Turn Signal	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF					
	Phasing										
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}					
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)					
1.00	1.00	1.00	1.00	1.00	1.00	1.00					

(1)		(2)	C Waltiple-	Vehicle Collisions by Seve	(4)	(5)	(6)	(7)	(8)	(9)
Crash Severity Level	SPF Coefficients		Overdispersion Parameter, k	Initial N _{bimv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Adjusted N _{bimv}	
	fro a	om Table 12-10 b) C	from Table 12-10	from Equation 12- 21		(4) _{TOTAL} *(5)	(7) from Worksheet 2B		(6)*(7)*(8)
Total	-20.02	1.66	0.54	0.24	2.230	1.000	2.230	1.00	1.13	2.520
Fatal and Injury (FI)					0.619		0.619	1.00	1.13	0.699
Property Damage Only (PDO)					1.611		1.611	1.00	1.13	1.820

	Worksheet 2D Multiple-V	ehicle Collisions by Collis	ion Type for Urban and Suburb	an Arterial Intersections	
(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type(FI)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C
Total	1.000	0.699	1.000	1.820	2.520
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Rear-end collision	0.411	0.287	0.337	0.613	0.901
Head-on collision	0.014	0.010	0.011	0.019	0.029
Angle collision	0.562	0.393	0.579	1.054	1.447
Sideswipe	0.014	0.010	0.074	0.134	0.144
Other multiple-vehicle collision	0.000	0.000	0.000	0.000	0.000

	Worksheet 2E Single-Vehicle Collisions by Severity Level for Urban and Suburban Arterial Intersections										
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)	
	SI	PF Coefficients	S	Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}	
Crash Severity Level	fre	om Table 12-12	2				(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)	
	а	h	С	from Table 1A	(3) * (4) From		(I)TOTAL (O)	Worksheet 2B		(0) (1) (0)	
	ű	S	9		Worksheet 2C						
Total				0.118	0.263	1.000	0.263	1.00	1.13	0.297	
Fatal and Injury (FI)				0.258	0.068		0.068	1.00	1.13	0.077	
ratar and injury (i i)				0.200	0.000		0.000	1.00	1.10	0.077	
Property Damage Only (PDO)	-			0.742	0.195		0.195	1.00	1.13	0.220	

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type(FI)	Predicted N _{bisv (FI)} (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.000	0.077	1.000	0.220	0.297
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.000	0.000	0.043	0.010	0.010
Collision with animal	0.000	0.000	0.043	0.010	0.010
Collision with fixed object	0.625	0.048	0.870	0.192	0.240
Collision with other object	0.125	0.010	0.043	0.010	0.019
Other single-vehicle collision	0.000	0.000	0.000	0.000	0.000
Single-vehicle noncollision	0.250	0.019	0.000	0.000	0.019

Work	Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Stop-Controlled Intersections										
(1)	(2) (3) (4) (5)										
Croch Soverity Lovel	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh- Ped ω	Calibration	Predicted N _{pedi}					
Crash Severity Level	(9) from Worksheet 2C	rksheet 2C (9) from Worksheet 2E (2) + (3) from Table 1B factor, C _i		factor, C _i	(4)*(5)*(6)						
Total	2.520	0.297	2.817	0.020	1.13	0.065					
Fatal and injury (FI)					1.13	0.065					

Worksheet 2J Vehicle-Bicycle Collisions for Urban and Suburban Arterial Intersections									
(1)	(2)	(6)	(7)						
Crach Savarity Laval	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration	Predicted N _{bikei}			
Crash Severity Level	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	ven-Βικε ω factor, C _i		(4)*(5)*(6)			
Total	2.520	0.297	2.817	0.034	1.13	0.108			
Fatal and injury (FI)					1.13	0.108			

Worksheet 2K Crash Severity Distribution for Urban and Suburban Arterial Intersections								
(1)	(2)	(3)	(4)					
	Fatal and injury (FI)	Property damage only (PDO)	Total					
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;					
	(7) from Worksheet 2H and 2J		(7) from Worksheet 2H and 2J					
	MULTIPLE-VEHICLE	•	• • •					
Rear-end collisions (from Worksheet 2D)	0.287	0.613	0.901					
Head-on collisions (from Worksheet 2D)	0.010	0.019	0.029					
Angle collisions (from Worksheet 2D)	0.393	1.054	1.447					
Sideswipe (from Worksheet 2D)	0.010	0.134	0.144					
Other multiple-vehicle collision (from Worksheet 2D)	0.000	0.000	0.000					
Subtotal	0.699	1.820	2.520					
	SINGLE-VEHICLE							
Collision with parked vehicle (from Worksheet 2F)	0.000	0.010	0.010					
Collision with animal (from Worksheet 2F)	0.000	0.010	0.010					
Collision with fixed object (from Worksheet 2F)	0.048	0.192	0.240					
Collision with other object (from Worksheet 2F)	0.010	0.010	0.019					
Other single-vehicle collision (from Worksheet 2F)	0.000	0.000	0.000					
Single-vehicle noncollision (from Worksheet 2F)	0.019	0.000	0.019					
Collision with pedestrian (from Worksheet 2G or 2I)	0.065	0.000	0.065					
Collision with bicycle (from Worksheet 2J)	0.108	0.000	0.108					
Subtotal	0.250	0.220	0.470					
Total	0.949	2.041	2.990					

Worksheet 2L Summary R	esults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)
	(Total) from Worksheet 2K
Total	2.990
Fatal and injury (FI)	0.949
Property damage only (PDO)	2.041

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	Legend	
Required Manual Input	Select from Drop-Down List	Optional Input

	Worksheet 2A General Informati	on and Input	Data for Urban and Suburban A	rterial Interse	ctions	
Genera	Il Information	•	Location Information			
Analyst	Ben Erban		City		Milton	
Agency or Company	CTPS		Intersection		Route 138 and Neponset Valley Parkway	
Date Performed	05/01/18		Jurisdiction		MassDOT Highwat District 6	
			Analysis Year		2011	
In	Base Conditions	Site Conditions				
Intersection type (3ST, 3SG, 4ST, 4SG)	_				3ST	
AADT _{major} (veh/day)	$AADT_{MAX} = 45,700$	(veh/day)			14,450	
AADT _{minor} (veh/day)	$AADT_{MAX} = 9,300$	(veh/day)			8,750	
Intersection lighting (present/not present)			Not Present		Not Present	
Calibration factor, C _i	Calibration factor, C _i				1.13	
Data for unsignalized intersections only:						
Number of major-road approaches with lef	t-turn lanes (0,1,2)			0		
Number of major-road approaches with right-turn lanes (0,1,2)				0		

	SUMMARY OF INTERSECTION & CRASHES										
CMF _{1i}	CMF _{1i} CMF _{2i} CMF _{3i} CMF _{4i} CMF _{5i} N _{bi_total} N _{pedi_total} N _{bikei_total} N _{predictedint}										
1.00											

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections										
(1)	(2)	(3)	(4)	(5)	(6)	(7)					
CMF for Left-Turn Lanes	CMF for Left-Turn Signal	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF					
	Phasing										
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}					
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)					
1.00	1.00	1.00	1.00	1.00	1.00	1.00					

		Worksheet 2	C Multiple-	Vehicle Collisions by Seve	rity Level for Urban	and Suburban Arterial In	tersections			
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
Crash Severity Level	SF	SPF Coefficients		Overdispersion Parameter, k	Initial N _{bimv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Adjusted N _{bimv}
	fro	om Table 12-10)	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from Worksheet 2B		(6)*(7)*(8)
Total	-20.02	1.66	0.54	0.24	2.185	1.000	2.185	1.00	1.13	2.469
Fatal and Injury (FI)					0.607		0.607	1.00	1.13	0.685
Property Damage Only (PDO)					1.579		1.579	1.00	1.13	1.784

(1)	(2)	(3)	ion Type for Urban and Suburba (4)	(5)	(6)
Collision Type	Proportion of Collision Type _(Fi)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C
Total	1.000	0.685	1.000	1.784	2.469
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Rear-end collision	0.411	0.282	0.337	0.601	0.883
Head-on collision	0.014	0.009	0.011	0.019	0.028
Angle collision	0.562	0.385	0.579	1.033	1.418
Sideswipe	0.014	0.009	0.074	0.131	0.141
Other multiple-vehicle collision	0.000	0.000	0.000	0.000	0.000

	Worksheet 2E Single-Vehicle Collisions by Severity Level for Urban and Suburban Arterial Intersections											
(1)	(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)			
	SPF Coefficients		Historical Data		Proportion of Total	Adjusted	Combined	Calibration	Predicted			
					Initial N _{bisv}	Crashes	N_{bimv}	CMFs	Factor, C _i	N_{bisv}		
Crash Severity Level	fro	om Table 12-12	2				$(4)_{TOTAL}^*(5)$ (7) from	(7) from		(6)*(7)*(8)		
	a	h	С	from Table 1A	(3) * (4) From		(·/TOTAL (°/	Worksheet 2B		(0) (1) (0)		
	a	D	C		Worksheet 2C							
Total				0.118	0.258	1.000	0.258	1.00	1.13	0.291		
Fatal and Injury (FI)				0.258	0.066		0.066	1.00	1.13	0.075		
r didi dila injury (i i)				0.200	0.000		0.000	1.00	11.10	0.070		
Property Damage Only (PDO)				0.742	0.191		0.191	1.00	1.13	0.216		

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv (FI)} (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.000	0.075	1.000	0.216	0.291
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.000	0.000	0.043	0.009	0.009
Collision with animal	0.000	0.000	0.043	0.009	0.009
Collision with fixed object	0.625	0.047	0.870	0.188	0.235
Collision with other object	0.125	0.009	0.043	0.009	0.019
Other single-vehicle collision	0.000	0.000	0.000	0.000	0.000
Single-vehicle noncollision	0.250	0.019	0.000	0.000	0.019

Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Stop-Controlled Intersections										
(1)	(2)	(3)	(4)	(5)	(6)	(7)				
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh- Ped ω	Calibration	Predicted N _{pedi}				
Crash Seventy Level	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	from Table 1B	factor, C _i	(4)*(5)*(6)				
Total	2.469	0.291	2.760	0.020	1.13	0.064				
Fatal and injury (FI)					1.13	0.064				

Worksheet 2J Vehicle-Bicycle Collisions for Urban and Suburban Arterial Intersections										
(1)	(2)	(3)	(4)	(5)	(6)	(7)				
Creah Cayarity Layel	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω Calibration		Predicted N _{bikei}				
Crash Severity Level	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	ven-bike ω	factor, C _i	(4)*(5)*(6)				
Total	2.469	0.291	2.760	0.034	1.13	0.106				
Fatal and injury (FI)					1.13	0.106				

Worksheet 2K Crash Severity Distribution for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)						
	Fatal and injury (FI)	Property damage only (PDO)	Total						
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;						
	(7) from Worksheet 2H and 2J		(7) from Worksheet 2H and 2J						
	MULTIPLE-VEHICLE	·							
Rear-end collisions (from Worksheet 2D)	0.282	0.601	0.883						
Head-on collisions (from Worksheet 2D)	0.009	0.019	0.028						
Angle collisions (from Worksheet 2D)	0.385	1.033	1.418						
Sideswipe (from Worksheet 2D)	0.009	0.131	0.141						
Other multiple-vehicle collision (from Worksheet 2D)	0.000	0.000	0.000						
Subtotal	0.685	1.784	2.469						
	SINGLE-VEHICLE	·	·						
Collision with parked vehicle (from Worksheet 2F)	0.000	0.009	0.009						
Collision with animal (from Worksheet 2F)	0.000	0.009	0.009						
Collision with fixed object (from Worksheet 2F)	0.047	0.188	0.235						
Collision with other object (from Worksheet 2F)	0.009	0.009	0.019						
Other single-vehicle collision (from Worksheet 2F)	0.000	0.000	0.000						
Single-vehicle noncollision (from Worksheet 2F)	0.019	0.000	0.019						
Collision with pedestrian (from Worksheet 2G or 2I)	0.064	0.000	0.064						
Collision with bicycle (from Worksheet 2J)	0.106	0.000	0.106						
Subtotal	0.245	0.216	0.461						
Total	0.930	2.000	2.930						

Worksheet 2L Summary Results for Urban and Suburban Arterial Intersections						
(1)	(2)					
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)					
	(Total) from Worksheet 2K					
Total	2.930					
Fatal and injury (FI)	0.930					
Property damage only (PDO)	2.000					

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General Information	n	Location Information		
Analyst	Ben Erban	Intersection	Route 138 at Dollar Lane and Milton Street	
Agency or Company	CTPS	Intersection Type	4SG	
Date Performed	May-18	Jurisdiction	MassDOT Highwat District 6	
City	Milton	Analysis Year	2018	

	Input Information										
Year	Oserved MV crashes	Observed total crashes	Predicted MV crashes	Predicted total crashes	Combined CMF for veh-ped crashes						
2015	9	9	4.08	4.58	1.00						
2014	7	7	4.03	4.52	1.00						
2013	6	7	3.95	4.44	1.00						
2012	4	4	3.93	4.41	1.00						
2011	6	6	3.84	4.32	1.00						

	Output Information										
Observed MV crashes	Average observed total crashes	Total predicted MV crashes	Average predicted total crashes	Standard deviation of predicted total crashes	Weight	Total expected MV crashes	No of expected total crashes	Average expected total crashes	INTERSECTION	Potential for Safety Improvement (PSI)	If avg observed total crashes > avg expected crashes
32.00	6.60	19.83	4.46	0.10	0.35	27.78	6.37 6.29 6.17 6.14 6.00	6.19	Y	1.74	Y

Works	sheet 2A General Information and Input	Data for Urban and Suburban A	rterial Intersect	ions		
General Informa	ation		Location	on Information		
Analyst	Ben Erban	City		Milton		
Agency or Company	CTPS	Intersection		Route 138 at Dollar Lane and Milton Street		
Date Performed	43221	Jurisdiction		MassDOT Highwat District 6		
		Analysis Year		2015		
Input Data		Base Conditions		Site Conditions		
ntersection type (3ST, 3SG, 4ST, 4SG)				4SG		
AADT _{major} (veh/day)	$AADT_{MAX} = 67700 (veh/day)$			15,000		
AADT _{minor} (veh/day)	$AAD1_{MAX} = 33400 (veh/day)$			4,350		
ntersection lighting (present/not present)		Not Present	Not Present			
Calibration factor, C _i		1		1		
Data for signalized intersections only:						
Number of approaches with left-turn lanes (0,1,2,3,4) [for 3SG, use maximum value of 3]			0		
Number of approaches with right-turn lanes (0,1,2,3	,4) [for 3SG, use maximum value of 3]			0		
Number of approaches with left-turn signal phasing	[for 3SG, use maximum value of 3]		0			
Type of left-turn signal phasing for Leg #1		Permissive		Permissive		
Type of left-turn signal phasing for Leg #2				Permissive		
Type of left-turn signal phasing for Leg #3				Permissive		
Type of left-turn signal phasing for Leg #4 (if applica	able)			Permissive		
Number of approaches with right-turn-on-red prohib	ited [for 3SG, use maximum value of 3]	0		0		
Intersection red light cameras (present/not present)		Not Present		Not Present		
Sum of all pedestrian crossing volumes (PedVol)	Signalized intersections only			6		
Maximum number of lanes crossed by a pedestrian	(n _{lanesx})			2		
Number of bus stops within 300 m (1,000 ft) of the i	ntersection	0		0		
Schools within 300 m (1,000 ft) of the intersection (p	present/not present)	Not Present		Not Present		
Number of alcohol sales establishments within 300	m (1.000 ft) of the intersection	0		0		

	SUMMARY OF INTERSECTION & CRASHES										
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF 4i	CMF 5i	CMF _{1p}	CMF _{2p}	CMF 3p	N _{bi_total}	N_{pedi_total}	N _{bikei_total}	N predictedint
1.000									4.580		

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)				
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF				
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}				
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)				
1.00	1.00	1.00	1.00	1.00	1.00	1.00				

	1	worksheet 2	C Multiple-	ehicle Collisions by Seve	rity Level for Urban	and Suburban Arterial In	itersections			
(1)	(1) (2)			(3)	(4)	(5)	(6)	(7)	(8)	(9)
Crash Severity Level	SPF Coefficients		Overdispersion Parameter, k	Initial N _{bimv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bimv}	
from Table 12-10)	from Table 12-10 from Equation 12-		(4) _{TOTAL} *(5) (7) from			(6)*(7)*(8)		
	а	b	С	110111 14510 12 10	21		() TOTAL ()	Worksheet 2B	(0)	(6) (1) (6)
Total	-11.79	0.92	0.52	0.10	4.08	1.00	4.08	1.00	1.00	4.08
Fatal and Injury (FI)					1.13		1.13	1.00	1.00	1.13
r atai and injury (i i)					1.10		1.10	1.00	1.00	1.13
Property Damage Only (PDO)					2.94		2.94	1.00	1.00	2.94

(1)	(2)	(3)	(4)	(5)	(6)	
Collision Type	Proportion of Collision Type _(FI)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)	
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C	
Total	1.00	1.13	1.00	2.94	4.08	
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)	
Rear-end collision	0.45	0.51	0.42	1.24	1.75	
Head-on collision	0.04	0.04	0.04	0.11	0.15	
Angle collision	0.49	0.56	0.43	1.25	1.81	
Sideswipe	0.02	0.02	0.11	0.33	0.35	
Other multiple-vehicle collision	0.00	0.00	0.00	0.00	0.00	

	•	Worksheet	2E Single-V	ehicle Collisions by Seve	rity Level for Urban a	and Suburban Arterial Int	ersections		•	•
(1)	(2)			(3)	(4)	(5)	(6)	(7)	(8)	(9)
	SPF Coefficients		Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bisv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}	
Crash Severity Level	from Table 12-12						(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	from Table 1A	(3) * (4) From Worksheet 2C			Worksheet 2B		.,,,,,
Total				0.05	0.20	1.00	0.20	1.00	1.00	0.20
Fatal and Injury (FI)				0.18	0.04		0.04	1.00	1.00	0.04
Property Damage Only (PDO)				0.82	0.16		0.16	1.00	1.00	0.16

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	Worksheet 2F Single-Ve	ehicle Collisions by Collisi	on Type for Urban and Suburba	n Arterial Intersections	_	
(1)	(2)	(3)	(4)	(5)	(6)	
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv} (FI) (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)	
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E	
Total	1.00	0.04	1.00	0.16	0.20	
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)	
Collision with parked vehicle	0.00	0.00	0.03	0.00	0.00	
Collision with animal	0.00	0.00	0.03	0.00	0.00	
Collision with fixed object	0.63	0.02	0.86	0.14	0.16	
Collision with other object	0.00	0.00	0.06	0.01	0.01	
Other single-vehicle collision	0.13	0.00	0.03	0.00	0.01	
Single-vehicle noncollision	0.25	0.01	0.00	0.00	0.01	

Worksheet 2G-1 Crash Modif	Worksheet 2G-1 Crash Modification Factors for Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Signalized Intersections									
(1)	(2)	(3)	(4)							
CMF for Bus Stops	CMF for Schools	CMF for Alcohol Sales Establishments	Combined CMF							
CMF _{1p}	CMF _{2p}	CMF _{3p}	Combined Civil							
from Table 12-28	from Table 12-29	from Table 12-30	(1)*(2)*(3)							
1.00	1.00	1.00	1.00							

	Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Intersections										
(1)	(2)		(4)	(5)	(6)	(7)	(8)				
Creek Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	- Veh-Ped ω	Npedbase	Calibration	Predicted N _{pedi}				
Crash Severity Level	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	ven-rea ω	Npedbase	factor, C _i	(4) from Worksheet 2H*(6)*(7)				
Total	4.08	0.20	4.28	0.03	0.12	1.00	0.12				
Fatal and injury (FI)						1.00	0.12				

(1)	(2)	(3)	(4)	(5)	(6)	(7)
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration factor, C _i	Predicted N _{bikei}
Crash Seventy Level	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	Ven-bike ω	Cambration factor, C _i	(4)*(5)*(6)
Total	4.08	0.20	4.28	0.04	1.00	0.18
Fatal and injury (FI)					1.00	0.18

Worksh	Worksheet 2K Crash Severity Distribution for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)							
	Fatal and injury (FI)	Property damage only (PDO)	Total							
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;							
	(8) from Worksheet 2G and (7) from 2J		(8) from Worksheet 2G and (7) from 2J							
	MULTIPLE-VEHICLE	•	•							
Rear-end collisions (from Worksheet 2D)	0.51	1.24	1.75							
Head-on collisions (from Worksheet 2D)	0.04	0.11	0.15							
Angle collisions (from Worksheet 2D)	0.56	1.25	1.81							
Sideswipe (from Worksheet 2D)	0.02	0.33	0.35							
Other multiple-vehicle collision (from Worksheet 2D)	0.00	0.00	0.00							
Subtotal	1.13	2.94	4.08							
	SINGLE-VEHICLE									
Collision with parked vehicle (from Worksheet 2F)	0.00	0.00	0.00							
Collision with animal (from Worksheet 2F)	0.00	0.00	0.00							
Collision with fixed object (from Worksheet 2F)	0.02	0.14	0.16							
Collision with other object (from Worksheet 2F)	0.00	0.01	0.01							
Other single-vehicle collision (from Worksheet 2F)	0.00	0.00	0.01							
Single-vehicle noncollision (from Worksheet 2F)	0.01	0.00	0.01							
Collision with pedestrian (from Worksheet 2G or 2I)	0.12	0.00	0.12							
Collision with bicycle (from Worksheet 2J)	0.18	0.00	0.18							
Subtotal	0.34	0.16	0.50							
Total	1.47	3.11	4.58							

Worksheet 2L Summary Re	esults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)
	Total
Total	4.58
Fatal and injury (FI)	1.47
Property damage only (PDO)	3.11

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Worksl	heet 2A General Information and Input	Data for Urban and Suburban A	rterial Intersec	tions		
General Information	tion		Locati	on Information		
Analyst	Ben Erban	City		Milton		
Agency or Company	CTPS	Intersection		Route 138 at Dollar Lane and Milton Street		
Date Performed	43221	Jurisdiction		MassDOT Highwat District 6		
		Analysis Year		2014		
Input Data		Base Conditions		Site Conditions		
Intersection type (3ST, 3SG, 4ST, 4SG)				4SG		
AADT _{major} (veh/day)	$AADT_{MAX} = 67700 (veh/day)$			14,800		
AADT _{minor} (veh/day)	$AADI_{MAX} = 33400 (veh/day)$			4,350		
Intersection lighting (present/not present)	Not Present	Not Present				
Calibration factor, C _i		1		1		
Data for signalized intersections only:						
Number of approaches with left-turn lanes (0,1,2,3,4)	[for 3SG, use maximum value of 3]			0		
Number of approaches with right-turn lanes (0,1,2,3,4	4) [for 3SG, use maximum value of 3]			0		
Number of approaches with left-turn signal phasing [f	or 3SG, use maximum value of 3]		0			
Type of left-turn signal phasing for Leg #1		Permissive		Permissive		
Type of left-turn signal phasing for Leg #2				Permissive		
Type of left-turn signal phasing for Leg #3				Permissive		
Type of left-turn signal phasing for Leg #4 (if applicab	ole)			Permissive		
Number of approaches with right-turn-on-red prohibite	ed [for 3SG, use maximum value of 3]	0		0		
Intersection red light cameras (present/not present)		Not Present		Not Present		
Sum of all pedestrian crossing volumes (PedVol) \$	Signalized intersections only			6		
Maximum number of lanes crossed by a pedestrian (n _{lanesx})			2		
Number of bus stops within 300 m (1,000 ft) of the in	tersection	0		0		
Schools within 300 m (1,000 ft) of the intersection (pr	resent/not present)	Not Present		Not Present		
Number of alcohol sales establishments within 300 m	(1.000 ft) of the intersection	0		0		

	SUMMARY OF INTERSECTION & CRASHES										
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF 4i	CMF 5i	CMF _{1p}	CMF _{2p}	CMF 3p	N _{bi_total}	N_{pedi_total}	N _{bikei_total}	N predictedint
1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	4.224	0.121	0.179	4.524

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections										
(1) (2) (3) (4) (5) (6) (7)											
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF					
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}					
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)					
1.00	1.00	1.00	1.00	1.00	1.00	1.00					

		worksheet 2	C Multiple-	ehicle Collisions by Seve	rity Level for Urban	and Suburban Arterial Ir	tersections			
(1)	(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)	
Crash Severity Level	ash Severity Level SPF Coefficients from Table 12-10		Overdispersion Parameter, k	Initial N _{bimv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bimv}	
			0	from Table 12-10 from Equation 12-		(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)	
	а	b	С		21		()TOTAL (-)	Worksheet 2B		(0) (1) (0)
Total	-11.79	0.92	0.52	0.10	4.03	1.00	4.03	1.00	1.00	4.03
Fatal and Injury (FI)					1.12		1.12	1.00	1.00	1.12
ratarana mjary (r 1)					1.12		1.12	1.00	1.00	1.12
Property Damage Only (PDO)					2.91		2.91	1.00	1.00	2.91

	Worksheet 2D Multiple-\	ehicle Collisions by Collis	ion Type for Urban and Suburba	an Arterial Intersections		
(1)	(2)	(3)	(4)	(5)	(6)	
Collision Type	Proportion of Collision Type _(FI)	· · · · · · · · · · · · · · · ·		Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)	
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C	
Total	1.00	1.12	1.00	2.91	4.03	
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)	
Rear-end collision	0.45	0.51	0.42	1.23	1.73	
Head-on collision	0.04	0.04	0.04	0.11	0.15	
Angle collision	0.49	0.55	0.43	1.24	1.79	
Sideswipe	0.02	0.02	0.11	0.33	0.35	
Other multiple-vehicle collision	0.00	0.00	0.00	0.00	0.00	

(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
	SPF Coefficients		Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bisv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}	
Crash Severity Level	from Table 12-12		frame Table 4.6			(4) _{TOTAL} *(5) (7) from			(6)*(7)*(8)	
	а	b	С	from Table 1A	(3) * (4) From Worksheet 2C			Worksheet 2B		
Total				0.05	0.20	1.00	0.20	1.00	1.00	0.20
Fatal and Injury (FI)				0.18	0.04		0.04	1.00	1.00	0.04
Property Damage Only (PDO)				0.82	0.16		0.16	1.00	1.00	0.16

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	Worksheet 2F Single-Ve	ehicle Collisions by Collisi	on Type for Urban and Suburba	n Arterial Intersections		
(1)	(2)	(3)	(4)	(5)	(6)	
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv (FI)} (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv} _(TOTAL) (crashes/year)	
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E	
Total	1.00	0.04	1.00	0.16	0.20	
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)	
Collision with parked vehicle	0.00	0.00	0.03	0.00	0.00	
Collision with animal	0.00	0.00	0.03	0.00	0.00	
Collision with fixed object	0.63	0.02	0.86	0.14	0.16	
Collision with other object	0.00	0.00	0.06	0.01	0.01	
Other single-vehicle collision	0.13	0.00	0.03	0.00	0.01	
Single-vehicle noncollision	0.25	0.01	0.00	0.00	0.01	

Worksheet 2G-1 Crash Modif	ication Factors for Vehicle-Pe	edestrian Collisions for Urban and Suburban	Arterial Signalized Intersections
(1)	(2)	(3)	(4)
CMF for Bus Stops	CMF for Schools	CMF for Alcohol Sales Establishments	Combined CMF
CMF _{1p}	CMF _{2p}	CMF _{3p}	Combined Civil
from Table 12-28	from Table 12-29	from Table 12-30	(1)*(2)*(3)
1.00	1.00	1.00	1.00

	Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)			
Crack Soverity Loyal	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	- Veh-Ped ω	Needbass	Calibration	Predicted N _{pedi}			
Crash Severity Level	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	ven-rea ω	Npedbase	factor, C _i	(4) from Worksheet 2H*(6)*(7)			
Total	4.03	0.20	4.22	0.03	0.12	1.00	0.12			
Fatal and injury (FI)		-				1.00	0.12			

(1)	(2)	(3)	(4)	(5)	(6)	(7)
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration factor, C	Predicted N _{bikei}
Crasii Severily Lever	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	Veii-bike ω	Cambration factor, C _i	(4)*(5)*(6)
Total	4.03	0.20	4.22	0.04	1.00	0.18
Fatal and injury (FI)					1.00	0.18

Worksheet 2K Crash Severity Distribution for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)						
	Fatal and injury (FI)	Property damage only (PDO)	Total						
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;						
	(8) from Worksheet 2G and (7) from 2J		(8) from Worksheet 2G and (7) from 2J						
	MULTIPLE-VEHICLE	•	•						
Rear-end collisions (from Worksheet 2D)	0.51	1.23	1.73						
Head-on collisions (from Worksheet 2D)	0.04	0.11	0.15						
Angle collisions (from Worksheet 2D)	0.55	1.24	1.79						
Sideswipe (from Worksheet 2D)	0.02	0.33	0.35						
Other multiple-vehicle collision (from Worksheet 2D)	0.00	0.00	0.00						
Subtotal	1.12	2.91	4.03						
	SINGLE-VEHICLE								
Collision with parked vehicle (from Worksheet 2F)	0.00	0.00	0.00						
Collision with animal (from Worksheet 2F)	0.00	0.00	0.00						
Collision with fixed object (from Worksheet 2F)	0.02	0.14	0.16						
Collision with other object (from Worksheet 2F)	0.00	0.01	0.01						
Other single-vehicle collision (from Worksheet 2F)	0.00	0.00	0.01						
Single-vehicle noncollision (from Worksheet 2F)	0.01	0.00	0.01						
Collision with pedestrian (from Worksheet 2G or 2I)	0.12	0.00	0.12						
Collision with bicycle (from Worksheet 2J)	0.18	0.00	0.18						
Subtotal	0.34	0.16	0.50						
Total	1.46	3.07	4.52						

Worksheet 2L Summary R	esults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)
	Total
Total	4.52
Fatal and injury (FI)	1.46
Property damage only (PDO)	3.07

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Worksl	neet 2A General Information and Input	Data for Urban and Suburban A	Arterial Intersect	tions		
General Information	ion		Locati	on Information		
Analyst	Ben Erban	City		Milton		
Agency or Company	CTPS	Intersection		Route 138 at Dollar Lane and Milton Street		
Date Performed	43221	Jurisdiction		MassDOT Highwat District 6		
		Analysis Year		2013		
Input Data		Base Conditions		Site Conditions		
Intersection type (3ST, 3SG, 4ST, 4SG)				4SG		
AADT _{major} (veh/day)	$AADT_{MAX} = 67700$ (veh/day)			14,700		
AADT _{minor} (veh/day)	$AAD1_{MAX} = 33400 (veh/day)$			4,250		
Intersection lighting (present/not present)	ersection lighting (present/not present)			Not Present		
Calibration factor, C _i		1		1		
Data for signalized intersections only:						
Number of approaches with left-turn lanes (0,1,2,3,4)	[for 3SG, use maximum value of 3]			0		
Number of approaches with right-turn lanes (0,1,2,3,4	1) [for 3SG, use maximum value of 3]			0		
Number of approaches with left-turn signal phasing [f	or 3SG, use maximum value of 3]			0		
Type of left-turn signal phasing for Leg #1		Permissive		Permissive		
Type of left-turn signal phasing for Leg #2				Permissive		
Type of left-turn signal phasing for Leg #3				Permissive		
Type of left-turn signal phasing for Leg #4 (if applicab	ole)			Permissive		
Number of approaches with right-turn-on-red prohibite	ed [for 3SG, use maximum value of 3]	0		0		
Intersection red light cameras (present/not present)		Not Present		Not Present		
Sum of all pedestrian crossing volumes (PedVol) \$	Signalized intersections only			6		
Maximum number of lanes crossed by a pedestrian (n _{lanesx})			2		
Number of bus stops within 300 m (1,000 ft) of the in	tersection	0		0		
Schools within 300 m (1,000 ft) of the intersection (pr	esent/not present)	Not Present		Not Present		
Number of alcohol sales establishments within 300 m	(1.000 ft) of the intersection	0		0		

	SUMMARY OF INTERSECTION & CRASHES										
CMF ₁i	CMF _{2i}	CMF _{3i}	CMF 4i	CMF 5i	CMF _{1p}	CMF _{2p}	CMF 3p	N _{bi_total}	N_{pedi_total}	N _{bikei_total}	N predictedint
1.000	11 21 31 The specifical prediction of the spe									4.442	

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections										
(1)	(1) (2) (3) (4) (5) (6) (7)										
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF					
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}					
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)					
1.00	1.00	1.00	1.00	1.00	1.00	1.00					

	Г	worksneet 2	C Multiple-	ehicle Collisions by Seve	erity Level for Orban	and Suburban Arterial in	tersections	T	1	1
(1)	(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)	
Crash Severity Level	SPF Coefficients		Overdispersion		Proportion of Total	Adjusted	Combined	Calibration	Predicted	
				Parameter, k	Initial N _{bimv}	Crashes	N_{bimv}	CMFs	Factor, C _i	N_{bimv}
from Table 12-10)	from Table 12-10	from Table 12-10 from Equation 12-		(4) _{TOTAL} *(5)	(7) from	(6)*(7)*(8)		
	а	b	С	110111 14510 12 10	21		(· / IOTAL (· /	Worksheet 2B		(0) (1) (0)
Total	-11.79	0.92	0.52	0.10	3.95	1.00	3.95	1.00	1.00	3.95
Fatal and Injury (FI)					1.10		1.10	1.00	1.00	1.10
r atai and injury (i i)					1.10		1.10	1.00	1.00	1.10
Property Damage Only (PDO)					2.85		2.85	1.00	1.00	2.85

	Worksheet 2D Multiple-V	ehicle Collisions by Collis	ion Type for Urban and Suburba	an Arterial Intersections		
(1)	(2)	(3)	(4)	(5)	(6)	
Collision Type	Proportion of Collision Type _(FI)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)	
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C	
Total	1.00	1.10	1.00	2.85	3.95	
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)	
Rear-end collision	0.45	0.50	0.42	1.20	1.70	
Head-on collision	0.04	0.04	0.04	0.11	0.15	
Angle collision	0.49	0.54	0.43	1.21	1.75	
Sideswipe	0.02	0.02	0.11	0.32	0.34	
Other multiple-vehicle collision	0.00	0.00	0.00	0.00	0.00	

	Worksheet 2E Single-Vehicle Collisions by Severity Level for Urban and Suburban Arterial Intersections											
(1)	(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)			
	SPF Coefficients Historical Data Proportion of Total Initial N _{bisv} Crashes		Adjusted N _{bisv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}						
Crash Severity Level	from Table 12-12					(4) _{TOTAL} *(5)	(7) from	,	(6)*(7)*(8)			
	а	b	С	from Table 1A	(3) * (4) From Worksheet 2C		. , , , , , , ,	Worksheet 2B				
Total				0.05	0.19	1.00	0.19	1.00	1.00	0.19		
Fatal and Injury (FI)				0.18	0.04		0.04	1.00	1.00	0.04		
Property Damage Only (PDO)			1	0.82	0.16		0.16	1.00	1.00	0.16		

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	Worksheet 2F Single-Ve	ehicle Collisions by Collisi	on Type for Urban and Suburba	n Arterial Intersections	_	
(1)	(2)	(3)	(4)	(5)	(6)	
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv} (FI) (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)	
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E	
Total	1.00	0.04	1.00	0.16	0.19	
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)	
Collision with parked vehicle	0.00	0.00	0.03	0.00	0.00	
Collision with animal	0.00	0.00	0.03	0.00	0.00	
Collision with fixed object	0.63	0.02	0.86	0.14	0.16	
Collision with other object	0.00	0.00	0.06	0.01	0.01	
Other single-vehicle collision	0.13	0.00	0.03	0.00	0.01	
Single-vehicle noncollision	0.25	0.01	0.00	0.00	0.01	

Worksheet 2G-1 Crash Modifie	Worksheet 2G-1 Crash Modification Factors for Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Signalized Intersections								
(1)	(2)	(3)	(4)						
CMF for Bus Stops	CMF for Schools	CMF for Alcohol Sales Establishments	Combined CMF						
CMF _{1p}	CMF _{2p}	CMF _{3p}	Combined Civil						
from Table 12-28	from Table 12-29	from Table 12-30	(1)*(2)*(3)						
1.00	1.00	1.00	1.00						

	Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)			
Creak Savarity Laval	Predicted N _{bimv}		Predicted N _{bi}	- Veh-Ped ω	Calibration		Predicted N _{pedi}			
Crash Severity Level	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	ven-rea ω	Npedbase factor, C _i		(4) from Worksheet 2H*(6)*(7)			
Total	3.95	0.19	4.15	0.03	0.12	1.00	0.12			
Fatal and injury (FI)						1.00	0.12			

(1)	(2)	(3)	(4)	(5)	(6)	(7)
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration factor, C	Predicted N _{bikei}
Crash Severity Level	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	- ven-Bike ω Calibration factor, C _i -		(4)*(5)*(6)
Total	3.95	0.19	4.15	0.04	1.00	0.18
Fatal and injury (FI)					1.00	0.18

Workshe	Worksheet 2K Crash Severity Distribution for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)							
	Fatal and injury (FI)	Property damage only (PDO)	Total							
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;							
	(8) from Worksheet 2G and (7) from 2J		(8) from Worksheet 2G and (7) from 2J							
	MULTIPLE-VEHICLE									
Rear-end collisions (from Worksheet 2D)	0.50	1.20	1.70							
Head-on collisions (from Worksheet 2D)	0.04	0.11	0.15							
Angle collisions (from Worksheet 2D)	0.54	1.21	1.75							
Sideswipe (from Worksheet 2D)	0.02	0.32	0.34							
Other multiple-vehicle collision (from Worksheet 2D)	0.00	0.00	0.00							
Subtotal	1.10	2.85	3.95							
	SINGLE-VEHICLE									
Collision with parked vehicle (from Worksheet 2F)	0.00	0.00	0.00							
Collision with animal (from Worksheet 2F)	0.00	0.00	0.00							
Collision with fixed object (from Worksheet 2F)	0.02	0.14	0.16							
Collision with other object (from Worksheet 2F)	0.00	0.01	0.01							
Other single-vehicle collision (from Worksheet 2F)	0.00	0.00	0.01							
Single-vehicle noncollision (from Worksheet 2F)	0.01	0.00	0.01							
Collision with pedestrian (from Worksheet 2G or 2I)	0.12	0.00	0.12							
Collision with bicycle (from Worksheet 2J)	0.18	0.00	0.18							
Subtotal	0.33	0.16	0.49							
Total	1.43	3.01	4.44							

Worksheet 2L Summary R	esults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)
	Total
Total	4.44
Fatal and injury (FI)	1.43
Property damage only (PDO)	3.01

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Works	sheet 2A General Information and Input	Data for Urban and Suburban A	rterial Intersecti	ons		
General Informa	ation		Locatio	n Information		
Analyst	Ben Erban	City		Milton		
Agency or Company	CTPS	Intersection		Route 138 at Dollar Lane and Milton Street		
Date Performed	43221	Jurisdiction		MassDOT Highwat District 6		
		Analysis Year		2012		
Input Data		Base Conditions		Site Conditions		
ntersection type (3ST, 3SG, 4ST, 4SG)				4SG		
AADT _{major} (veh/day)	$AADT_{MAX} = 67700$ (veh/day)			14,600		
AADT _{minor} (veh/day)	$AADI_{MAX} = 33400 (veh/day)$			4,250		
ntersection lighting (present/not present)	Not Present	Not Present				
Calibration factor, C _i		1		1		
Data for signalized intersections only:						
Number of approaches with left-turn lanes (0,1,2,3,4) [for 3SG, use maximum value of 3]			0		
Number of approaches with right-turn lanes (0,1,2,3	(4) [for 3SG, use maximum value of 3]			0		
Number of approaches with left-turn signal phasing	[for 3SG, use maximum value of 3]			0		
Type of left-turn signal phasing for Leg #1		Permissive		Permissive		
Type of left-turn signal phasing for Leg #2				Permissive		
Type of left-turn signal phasing for Leg #3				Permissive		
Type of left-turn signal phasing for Leg #4 (if applica	ble)			Permissive		
Number of approaches with right-turn-on-red prohibi	ted [for 3SG, use maximum value of 3]	0		0		
Intersection red light cameras (present/not present)		Not Present		Not Present		
Sum of all pedestrian crossing volumes (PedVol)	Signalized intersections only			6		
Maximum number of lanes crossed by a pedestrian				2		
Number of bus stops within 300 m (1,000 ft) of the in	ntersection	0		0		
Schools within 300 m (1,000 ft) of the intersection (p	present/not present)	Not Present		Not Present		
Number of alcohol sales establishments within 300	m (1.000 ft) of the intersection	0		0		

	SUMMARY OF INTERSECTION & CRASHES										
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF 4i	CMF 5i	CMF _{1p}	CMF _{2p}	CMF 3p	N _{bi_total}	N_{pedi_total}	N _{bikei_total}	N predictedint
1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	4.121	0.118	0.175	4.414

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections										
(1)	(2)	(3)	(4)	(5)	(6)	(7)					
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF					
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}					
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)					
1.00	1.00	1.00	1.00	1.00	1.00	1.00					

		worksheet 2	C Multiple-	Vehicle Collisions by Seve	rity Level for Urban	and Suburban Arterial In	tersections			
(1)	(1) (2) Crash Severity Level SPF Coefficients from Table 12-10			(3) Overdispersion Parameter, k from Table 12-10	(4) Initial N _{bimv} from Equation 12-	(5)	(6) Adjusted N _{bimv} (4) _{TOTAL} *(5)	(7) Combined CMFs (7) from	(8) Calibration Factor, C _i	(9) Predicted N _{bimv} (6)*(7)*(8)
Crash Severity Level						Proportion of Total Crashes				
	а	b	С	110111 14510 12 10	21		(')TOTAL (')	Worksheet 2B		(0) (1) (0)
Total	-11.79	0.92	0.52	0.10	3.93	1.00	3.93	1.00	1.00	3.93
Fatal and Injury (FI)					1.09		1.09	1.00	1.00	1.09
Property Damage Only (PDO)					2.84		2.84	1.00	1.00	2.84

Worksheet 2D Multiple-Vehicle Collisions by Collision Type for Urban and Suburban Arterial Intersections								
(1)	(2)	(3)	(4)	(5)	(6)			
Collision Type	Proportion of Collision Type _(FI)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)			
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C			
Total	1.00	1.09	1.00	2.84	3.93			
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)			
Rear-end collision	0.45	0.49	0.42	1.20	1.69			
Head-on collision	0.04	0.04	0.04	0.11	0.15			
Angle collision	0.49	0.54	0.43	1.21	1.74			
Sideswipe	0.02	0.02	0.11	0.32	0.34			
Other multiple-vehicle collision	0.00	0.00	0.00	0.00	0.00			

		Worksheet	2E Single-V	ehicle Collisions by Seve	ity Level for Urban a	and Suburban Arterial Int	ersections			
(1)	(2)			(3)	(4)	(5)	(6)	(7)	(8)	(9)
Crash Severity Level	SPF Coefficients		Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bisv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}	
	from Table 12-12			· · · · ·			(4) _{TOTAL} *(5)	(7) from]	(6)*(7)*(8)
	а	b	С	from Table 1A	(3) * (4) From Worksheet 2C			Worksheet 2B		.,,,,,
Total				0.05	0.19	1.00	0.19	1.00	1.00	0.19
Fatal and Injury (FI)				0.18	0.03		0.03	1.00	1.00	0.03
Property Damage Only (PDO)			1	0.82	0.16		0.16	1.00	1.00	0.16

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	Worksheet 2F Single-Ve	ehicle Collisions by Collision	on Type for Urban and Suburba	n Arterial Intersections		
(1)	(2)	(3)	(4)	(5)	(6)	
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv (FI)} (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv} (TOTAL) (crashes/year)	
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E	
Total	1.00	0.03	1.00	0.16	0.19	
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)	
Collision with parked vehicle	0.00	0.00	0.03	0.00	0.00	
Collision with animal	0.00	0.00	0.03	0.00	0.00	
Collision with fixed object	0.63	0.02	0.86	0.14	0.16	
Collision with other object	0.00	0.00	0.06	0.01	0.01	
Other single-vehicle collision	0.13	0.00	0.03	0.00	0.01	
Single-vehicle noncollision	0.25	0.01	0.00	0.00	0.01	

Worksheet 2G-1 Crash Modif	Worksheet 2G-1 Crash Modification Factors for Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Signalized Intersections									
(1)	(2)	(3)	(4)							
CMF for Bus Stops	CMF for Schools	CMF for Alcohol Sales Establishments	Combined CMF							
CMF _{1p}	CMF _{2p}	CMF _{3p}	Combined Civil							
from Table 12-28	from Table 12-29	from Table 12-30	(1)*(2)*(3)							
1.00	1.00	1.00	1.00							

	Worksheet 2G Vo	ehicle-Pedestrian Collisions	for Urban and Su	burban Arterial Intersecti	ons		
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)
Creak Coverity Lovel	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Vob Bod	No. II acc	Calibration	Predicted N _{pedi}
Crash Severity Level	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	Veh-Ped ω Npedbase Calibration factor, C_i (4) from Worksh $2H^*(6)^*(7)$	(4) from Worksheet 2H*(6)*(7)		
Total	3.93	0.19	4.12	0.03	0.12	1.00	0.12
Fatal and injury (FI)						1.00	0.12

(1)	(2)	(3)	(4)	(5)	(6)	(7)				
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration factor, C	Predicted N _{bikei}				
	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	Vell-blke w	Cambration factor, o	(4)*(5)*(6)				
Total	3.93	0.19	4.12	0.04	1.00	0.17				
Fatal and injury (FI)					1.00	0.17				

Worksh	eet 2K Crash Severity Distribution for Urban and	Suburban Arterial Intersections	
(1)	(2)	(3)	(4)
	Fatal and injury (FI)	Property damage only (PDO)	Total
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;
	(8) from Worksheet 2G and (7) from 2J		(8) from Worksheet 2G and (7) from 2J
	MULTIPLE-VEHICLE	•	•
Rear-end collisions (from Worksheet 2D)	0.49	1.20	1.69
Head-on collisions (from Worksheet 2D)	0.04	0.11	0.15
Angle collisions (from Worksheet 2D)	0.54	1.21	1.74
Sideswipe (from Worksheet 2D)	0.02	0.32	0.34
Other multiple-vehicle collision (from Worksheet 2D)	0.00	0.00	0.00
Subtotal	1.09	2.84	3.93
	SINGLE-VEHICLE		
Collision with parked vehicle (from Worksheet 2F)	0.00	0.00	0.00
Collision with animal (from Worksheet 2F)	0.00	0.00	0.00
Collision with fixed object (from Worksheet 2F)	0.02	0.14	0.16
Collision with other object (from Worksheet 2F)	0.00	0.01	0.01
Other single-vehicle collision (from Worksheet 2F)	0.00	0.00	0.01
Single-vehicle noncollision (from Worksheet 2F)	0.01	0.00	0.01
Collision with pedestrian (from Worksheet 2G or 2I)	0.12	0.00	0.12
Collision with bicycle (from Worksheet 2J)	0.17	0.00	0.17
Subtotal	0.33	0.16	0.49
Total	1.42	2.99	4.41

Worksheet 2L Summary Re	esults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, $N_{predicted\ int}$ (crashes/year)
	Total
Total	4.41
Fatal and injury (FI)	1.42
Property damage only (PDO)	2.99

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Worksh	eet 2A General Information a	nd Input D	ata for Urban and Suburban A	Arterial Interse	ections
General Informati	ion			Loca	tion Information
Analyst	Ben Erban		City		Milton
Agency or Company	CTPS		Intersection	Route 138 at Dollar Lane and Milton Street	
Date Performed	43221		Jurisdiction		MassDOT Highwat District 6
			Analysis Year		2011
Input Data			Base Conditions		Site Conditions
Intersection type (3ST, 3SG, 4ST, 4SG)					4SG
AADT _{major} (veh/day)	******	h/day)			14,450
AADT _{minor} (veh/day)	$AADI_{MAX} = 33400 (ve$	h/day)			4,150
Intersection lighting (present/not present)	ersection lighting (present/not present)				Not Present
Calibration factor, C _i			1		1
Data for signalized intersections only:					
Number of approaches with left-turn lanes (0,1,2,3,4)	[for 3SG, use maximum value of	3]			0
Number of approaches with right-turn lanes (0,1,2,3,4					0
Number of approaches with left-turn signal phasing [for	or 3SG, use maximum value of 3]				0
Type of left-turn signal phasing for Leg #1			Permissive		Permissive
Type of left-turn signal phasing for Leg #2					Permissive
Type of left-turn signal phasing for Leg #3					Permissive
Type of left-turn signal phasing for Leg #4 (if applicable	le)				Permissive
Number of approaches with right-turn-on-red prohibite	ed [for 3SG, use maximum value of	of 3]	0		0
Intersection red light cameras (present/not present)			Not Present		Not Present
Sum of all pedestrian crossing volumes (PedVol) S	signalized intersections only				6
Maximum number of lanes crossed by a pedestrian (r	n _{lanesx})				2
Number of bus stops within 300 m (1,000 ft) of the interest of	ersection	•	0		0
Schools within 300 m (1,000 ft) of the intersection (pre	esent/not present)	-	Not Present		Not Present
Number of alcohol sales establishments within 300 m	(1,000 ft) of the intersection		0		0

	SUMMARY OF INTERSECTION & CRASHES										
CMF ₁i	CMF _{2i}	CMF _{3i}	CMF 4i	CMF 5i	CMF _{1p}	CMF _{2p}	CMF 3p	N _{bi_total}	N_{pedi_total}	N _{bikei_total}	N predictedint
1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	4.032	0.115	0.171	4.318

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections										
(1)	(2)	(3)	(4)	(5)	(6)	(7)					
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF					
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}					
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)					
1.00	1.00	1.00	1.00	1.00	1.00	1.00					

	Worksheet 2C Multiple-Vehicle Collisions by Severity Level for Urban and Suburban Arterial Intersections											
(1)	(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)			
Crash Severity Level	SPF Coefficients		Overdispersion Parameter, k	Initial N _{bimv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bimv}			
	fro	om Table 12-10)	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)		
	а	b	С	110111 14510 12 10	21		(')TOTAL (')	Worksheet 2B		(0) (1) (0)		
Total	-11.79	0.92	0.52	0.10	3.84	1.00	3.84	1.00	1.00	3.84		
Fatal and Injury (FI)					1.07		1.07	1.00	1.00	1.07		
i atai and injury (i i)					1.07		1.07	1.00	1.00	1.07		
Property Damage Only (PDO)					2.77		2.77	1.00	1.00	2.77		

	Worksheet 2D Multiple-V	ehicle Collisions by Collis	ion Type for Urban and Suburba	an Arterial Intersections		
(1)	(2)	(3)	(4)	(5)	(6)	
Collision Type	Proportion of Collision Type _(FI)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)	
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C	
Total	1.00	1.07	1.00	2.77	3.84	
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)	
Rear-end collision	0.45	0.48	0.42	1.17	1.65	
Head-on collision	0.04	0.04	0.04	0.11	0.15	
Angle collision	0.49	0.53	0.43	1.18	1.71	
Sideswipe	0.02	0.02	0.11	0.31	0.33	
Other multiple-vehicle collision	0.00	0.00	0.00	0.00	0.00	

	Worksheet 2E Single-Vehicle Collisions by Severity Level for Urban and Suburban Arterial Intersections											
(1)	(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)			
	SF	PF Coefficients	cients Historical Data		Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bisv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}		
Crash Severity Level	fro	from Table 12-12					(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)		
	а	b	С	from Table 1A	(3) * (4) From Worksheet 2C			Worksheet 2B		.,,,,,		
Total				0.05	0.19	1.00	0.19	1.00	1.00	0.19		
Fatal and Injury (FI)				0.18	0.03		0.03	1.00	1.00	0.03		
Property Damage Only (PDO)			1	0.82	0.15		0.15	1.00	1.00	0.15		

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Worksheet 2F Single-Vehicle Collisions by Collision Type for Urban and Suburban Arterial Intersections										
(1)	(2)	(2) (3) (4)		(5)	(6)					
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv (FI)} (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv} (TOTAL) (crashes/year)					
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E					
Total	1.00	0.03	1.00	0.15	0.19					
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)					
Collision with parked vehicle	0.00	0.00	0.03	0.00	0.00					
Collision with animal	0.00	0.00	0.03	0.00	0.00					
Collision with fixed object	0.63	0.02	0.86	0.13	0.15					
Collision with other object	0.00	0.00	0.06	0.01	0.01					
Other single-vehicle collision	0.13	0.00	0.03	0.00	0.01					
Single-vehicle noncollision	0.25	0.01	0.00	0.00	0.01					

Worksheet 2G-1 Crash Modi	Worksheet 2G-1 Crash Modification Factors for Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Signalized Intersections									
(1)	(2)	(3)	(4)							
CMF for Bus Stops	CMF for Schools	CMF for Alcohol Sales Establishments	Combined CMF							
CMF _{1p}	CMF _{2p}	CMF _{3p}	Combined Civil							
from Table 12-28	from Table 12-29	from Table 12-30	(1)*(2)*(3)							
1.00	1.00	1.00	1.00							

Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Intersections										
(1) (2)		(2) (3) (4)		(5)	(6)	(7)	(8)			
Crash Severity Level	Predicted N _{bimv}		Predicted N _{bi}	- Veh-Ped ω	Npedbase	Calibration	Predicted N _{pedi}			
Crash Severity Level	(9) from Worksheet 2C		ven-red w	Npedbase	factor, C _i	(4) from Worksheet 2H*(6)*(7)				
Total	3.84	0.19	4.03	0.03	0.12	1.00	0.12			
Fatal and injury (FI)						1.00	0.12			

(1)	(2)	(3)	(4)	(5)	(6)	(7)
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration factor, C	Predicted N _{bikei}
Crasii Severily Lever	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	Veii-bike ω	Cambration factor, C _i	(4)*(5)*(6)
Total	3.84	0.19	4.03	0.04	1.00	0.17
Fatal and injury (FI)					1.00	0.17

Worksheet 2K Crash Severity Distribution for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)						
	Fatal and injury (FI)	Property damage only (PDO)	Total						
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;						
	(8) from Worksheet 2G and (7) from 2J		(8) from Worksheet 2G and (7) from 2J						
	MULTIPLE-VEHICLE	•	•						
Rear-end collisions (from Worksheet 2D)	0.48	1.17	1.65						
Head-on collisions (from Worksheet 2D)	0.04	0.11	0.15						
Angle collisions (from Worksheet 2D)	0.53	1.18	1.71						
Sideswipe (from Worksheet 2D)	0.02	0.31	0.33						
Other multiple-vehicle collision (from Worksheet 2D)	0.00	0.00	0.00						
Subtotal	1.07	2.77	3.84						
	SINGLE-VEHICLE								
Collision with parked vehicle (from Worksheet 2F)	0.00	0.00	0.00						
Collision with animal (from Worksheet 2F)	0.00	0.00	0.00						
Collision with fixed object (from Worksheet 2F)	0.02	0.13	0.15						
Collision with other object (from Worksheet 2F)	0.00	0.01	0.01						
Other single-vehicle collision (from Worksheet 2F)	0.00	0.00	0.01						
Single-vehicle noncollision (from Worksheet 2F)	0.01	0.00	0.01						
Collision with pedestrian (from Worksheet 2G or 2I)	0.12	0.00	0.12						
Collision with bicycle (from Worksheet 2J)	0.17	0.00	0.17						
Subtotal	0.32	0.15	0.47						
Total	1.39	2.93	4.32						

Worksheet 2L Summary Results for Urban and Suburban Arterial Intersections					
(1)	(2)				
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)				
	Total				
Total	4.32				
Fatal and injury (FI)	1.39				
Property damage only (PDO)	2.93				

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General Information	n	Location Information		
Analyst	Ben Erban	Intersection	Route 138 and Blue Jay Way Curry College	
Agency or Company	CTPS	Intersection Type	3ST	
Date Performed	May-18	Jurisdiction	MassDOT Highwat District 6	
City	Milton	Analysis Year	2018	

	Input Information											
Year	Oserved MV crashes	Observed total crashes	Predicted MV crashes	Predicted total crashes	Combined CMF for veh-ped crashes							
2015	2	2	0.87	1.03	1.00							
2014	2	2	0.85	1.01	1.00							
2013	0	0	0.84	1.00	1.00							
2012	2	3	0.83	0.98	1.00							
2011	2	2	0.81	0.96	1.00							

	Output Information											
Observed MV crashes	Average observed total crashes	Total predicted MV crashes	Average predicted total crashes	Standard deviation of predicted total crashes	Weight	Total expected MV crashes	No of expected total crashes	Average expected total crashes	High-risk Intersection (Y/N)	Safety	If avg observed total crashes > avg expected crashes	
8.00	1.80	4.20	1.00	0.03	0.50	6.11	1.41 1.38 1.36 1.34 1.32	1.36	Y	0.36	Y	

Legend						
Required Manual Input	Select from Drop-Down List	Optional Input				

	Worksheet 2A General Information	on and Input	Data for Urban and Suburban A	rterial Interse	ctions
Genera	General Information				tion Information
Analyst	Ben Erban		City		Milton
Agency or Company	CTPS		Intersection		Route 138 and Blue Jay Way Curry College
Date Performed	05/01/18		Jurisdiction		MassDOT Highwat District 6
					2015
In	Base Conditions		Site Conditions		
Intersection type (3ST, 3SG, 4ST, 4SG)					3ST
AADT _{major} (veh/day)	$AADT_{MAX} = 45,700$	(veh/day)			16,750
AADT _{minor} (veh/day)	$AADT_{MAX} = 9,300$	(veh/day)			800
Intersection lighting (present/not present)			Not Present		Not Present
Calibration factor, C _i			1.00		1.13
Data for unsignalized intersections only:					
Number of major-road approaches with left			0		
Number of major-road approaches with rigl	nt-turn lanes (0,1,2)	•			0

SUMMARY OF INTERSECTION & CRASHES									
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF ₄i	CMF 5i	N _{bi_total}	N_{pedi_total}	N _{bikei_total}	N predictedint	
1.00	1.00	1.00	1.00	1.00	0.969	0.022	0.037	1.029	

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)				
CMF for Left-Turn Lanes	CMF for Left-Turn Signal	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF				
	Phasing									
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}				
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)				
1.00	1.00	1.00	1.00	1.00	1.00	1.00				

(1)	(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)	
Crash Severity Level	SPF Coefficients		Overdispersion Parameter, k	Initial N _{bimv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Adjusted N _{bimv}	
	a fro	om Table 12-10 b	С С	from Table 12-10	from Equation 12- 21		(4) _{TOTAL} *(5)	(7) from Worksheet 2B		(6)*(7)*(8)
Total	-20.02	1.66	0.54	0.24	0.767	1.000	0.767	1.00	1.13	0.867
Fatal and Injury (FI)					0.213		0.213	1.00	1.13	0.241
Property Damage Only (PDO)					0.554		0.554	1.00	1.13	0.626

	Worksheet 2D Multiple-V	ehicle Collisions by Collis	ion Type for Urban and Suburba	an Arterial Intersections	
(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(Fi)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C
Total	1.000	0.241	1.000	0.626	0.867
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Rear-end collision	0.411	0.099	0.337	0.211	0.310
Head-on collision	0.014	0.003	0.011	0.007	0.010
Angle collision	0.562	0.135	0.579	0.363	0.498
Sideswipe	0.014	0.003	0.074	0.046	0.049
Other multiple-vehicle collision	0.000	0.000	0.000	0.000	0.000

		Worksheet	2E Single-V	ehicle Collisions by Sever	ity Level for Urban a	and Suburban Arterial Int	ersections			
(1)	(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)	
	SPF Coefficients		Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}	
Crash Severity Level	fro	om Table 12-12	2				(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	a	h	С	from Table 1A	(3) * (4) From		(')TOTAL (O)	Worksheet 2B		(0) (1) (0)
	u	, and the second	•		Worksheet 2C					
Total				0.118	0.090	1.000	0.090	1.00	1.13	0.102
Fatal and Injury (FI)				0.258	0.023		0.023	1.00	1.13	0.026
ratarana injary (i i)				0.200	0.020		0.020	1.00	1.10	0.020
Property Damage Only (PDO)				0.742	0.067		0.067	1.00	1.13	0.076

Worksheet 2F -- Single-Vehicle Collisions by Collision Type for Urban and Suburban Arterial Intersections

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Collision Type Proportion of Collision Type _(FI) Pred (cra		Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.000	0.026	1.000	0.076	0.102
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.000	0.000	0.043	0.003	0.003
Collision with animal	0.000	0.000	0.043	0.003	0.003
Collision with fixed object	0.625	0.016	0.870	0.066	0.082
Collision with other object	0.125	0.003	0.043	0.003	0.007
Other single-vehicle collision	0.000	0.000	0.000	0.000	0.000
Single-vehicle noncollision	0.250	0.007	0.000	0.000	0.007

Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Stop-Controlled Intersections									
(1)	(1) (2) (3) (4) (5)								
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh- Ped ω	Calibration	Predicted N _{pedi}			
Crash Seventy Level	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	from Table 1B	factor, C _i	(4)*(5)*(6)			
Total	0.867	0.102	0.969	0.020	1.13	0.022			
Fatal and injury (FI)					1.13	0.022			

	Worksheet 2J Vehicle-Bicycle Collisions for Urban and Suburban Arterial Intersections									
(1)	(2)	(2) (3) (4) (5) (6) (7)								
Swaah Carravitud arral	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration	Predicted N _{bikei}				
Crash Severity Level	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	veii-bike ω	factor, C _i	(4)*(5)*(6)				
Total	0.867	0.102	0.969	0.034	1.13	0.037				
Fatal and injury (FI)					1.13	0.037				

Worksheet 2K Crash Severity Distribution for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)						
	Fatal and injury (FI)	Property damage only (PDO)	Total						
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;						
	(7) from Worksheet 2H and 2J		(7) from Worksheet 2H and 2J						
	MULTIPLE-VEHICLE	·							
Rear-end collisions (from Worksheet 2D)	0.099	0.211	0.310						
Head-on collisions (from Worksheet 2D)	0.003	0.007	0.010						
Angle collisions (from Worksheet 2D)	0.135	0.363	0.498						
Sideswipe (from Worksheet 2D)	0.003	0.046	0.049						
Other multiple-vehicle collision (from Worksheet 2D)	0.000	0.000	0.000						
Subtotal	0.241	0.626	0.867						
	SINGLE-VEHICLE	·	•						
Collision with parked vehicle (from Worksheet 2F)	0.000	0.003	0.003						
Collision with animal (from Worksheet 2F)	0.000	0.003	0.003						
Collision with fixed object (from Worksheet 2F)	0.016	0.066	0.082						
Collision with other object (from Worksheet 2F)	0.003	0.003	0.007						
Other single-vehicle collision (from Worksheet 2F)	0.000	0.000	0.000						
Single-vehicle noncollision (from Worksheet 2F)	0.007	0.000	0.007						
Collision with pedestrian (from Worksheet 2G or 2I)	0.022	0.000	0.022						
Collision with bicycle (from Worksheet 2J)	0.037	0.000	0.037						
Subtotal	0.086	0.076	0.162						
Total	0.327	0.702	1.029						

Worksheet 2L Summary R	esults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)
	(Total) from Worksheet 2K
Total	1.029
Fatal and injury (FI)	0.327
Property damage only (PDO)	0.702

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	Legend						
Required Manual Input	Select from Drop-Down List	Optional Input					

	Worksheet 2A General Infor	mation and Input	Data for Urban and Suburban A	Arterial Interse	ections		
(General Information		Location Information				
Analyst	Ben E	rban	City		Milton		
Agency or Company	CTF	PS .	Intersection		Route 138 and Blue Jay Way Curry College		
Date Performed	05/01	/18	Jurisdiction		MassDOT Highwat District 6		
			Analysis Year	2014			
Input Data			Base Conditions		Site Conditions		
Intersection type (3ST, 3SG, 4ST, 4SG)	<u></u>				3ST		
AADT _{major} (veh/day)	$AADT_{MAX} = 45,7$	00 (veh/day)			16,600		
AADT _{minor} (veh/day)	$AADT_{MAX} = 9,30$	00 (veh/day)			800		
ntersection lighting (present/not present)	-		Not Present		Not Present		
Calibration factor, C _i			1.00		1.13		
ata for unsignalized intersections only:							
Number of major-road approaches v	with left-turn lanes (0,1,2)				0		
Number of major-road approaches v	with right-turn lanes (0,1,2)				0		

SUMMARY OF INTERSECTION & CRASHES								
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF ₄i	CMF ₅i	N _{bi_total}	N_{pedi_total}	N _{bikei_total}	N predictedint
1.00	1.00	1.00	1.00	1.00	0.955	0.022	0.037	1.014

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections							
(1) (2) (3) (4) (5) (6) (7)								
CMF for Left-Turn Lanes	CMF for Left-Turn Signal	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF		
	Phasing							
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}		
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)		
1.00	1.00	1.00	1.00	1.00	1.00	1.00		

(1)	(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)	
Crash Severity Level	SPF Coefficients		Severity Level SPF Coefficients Overdispersion Parameter, k Initial N _{bimv}		Initial N _{bimv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Adjusted N _{bimv}
	a	om Table 12-10 b	С С	from Table 12-10	from Equation 12- 21		(4) _{TOTAL} *(5)	(7) from Worksheet 2B		(6)*(7)*(8)
Total	-20.02	1.66	0.54	0.24	0.756	1.000	0.756	1.00	1.13	0.854
Fatal and Injury (FI)					0.210		0.210	1.00	1.13	0.237
Property Damage Only (PDO)					0.546		0.546	1.00	1.13	0.617

(4)	(-)	(=)	ion Type for Urban and Suburba	(=)	(0)	
(1)	(2)	(3)	(4)	(5)	(6)	
Collision Type	Proportion of Collision Type _(FI)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)	
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C	
Total	1.000	0.237	1.000	0.617	0.854	
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)	
Rear-end collision	0.411	0.097	0.337	0.208	0.305	
Head-on collision	0.014	0.003	0.011	0.006	0.010	
Angle collision	0.562	0.133	0.579	0.357	0.490	
Sideswipe	0.014	0.003	0.074	0.045	0.049	
Other multiple-vehicle collision	0.000	0.000	0.000	0.000	0.000	

		Worksheet	2E Single-V	ehicle Collisions by Sever	ity Level for Urban a	and Suburban Arterial Int	ersections			
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
	SPF Coefficients		Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}	
Crash Severity Level						(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)	
	a	h	С	from Table 1A	(3) * (4) From		(')TOTAL (O)	Worksheet 2B		(0) (1) (0)
	u	, and the second	•		Worksheet 2C					
Total				0.118	0.089	1.000	0.089	1.00	1.13	0.101
Fatal and Injury (FI)				0.258	0.023		0.023	1.00	1.13	0.026
ratarana injary (i i)				0.200	0.020		0.020	1.00	1.10	0.020
Property Damage Only (PDO)				0.742	0.066		0.066	1.00	1.13	0.075

Worksheet 2F -- Single-Vehicle Collisions by Collision Type for Urban and Suburban Arterial Intersections

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	llision Type Proportion of Collision Type _(F1)		Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.000	0.026	1.000	0.075	0.101
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.000	0.000	0.043	0.003	0.003
Collision with animal	0.000	0.000	0.043	0.003	0.003
Collision with fixed object	0.625	0.016	0.870	0.065	0.081
Collision with other object	0.125	0.003	0.043	0.003	0.006
Other single-vehicle collision	0.000	0.000	0.000	0.000	0.000
Single-vehicle noncollision	0.250	0.006	0.000	0.000	0.006

Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Stop-Controlled Intersections								
(1) (2) (3) (4)					(6)	(7)		
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh- Ped ω	Calibration	Predicted N _{pedi}		
Crash Seventy Level	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	from Table 1B	factor, C _i	(4)*(5)*(6)		
Total	0.854	0.101	0.955	0.020	1.13	0.022		
Fatal and injury (FI)		-			1.13	0.022		

Worksheet 2J Vehicle-Bicycle Collisions for Urban and Suburban Arterial Intersections								
(1) (2) (3) (4) (5) (6) (7)								
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration	Predicted N _{bikei}		
	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3) Ven-Bike ω fa		factor, C _i	(4)*(5)*(6)		
Total	0.854	0.101	0.955	0.034	1.13	0.037		
Fatal and injury (FI)					1.13	0.037		

Worksh	Worksheet 2K Crash Severity Distribution for Urban and Suburban Arterial Intersections							
(1)	(2)	(3)	(4)					
	Fatal and injury (FI)	Property damage only (PDO)	Total					
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;					
	(7) from Worksheet 2H and 2J		(7) from Worksheet 2H and 2J					
	MULTIPLE-VEHICLE	·						
Rear-end collisions (from Worksheet 2D)	0.097	0.208	0.305					
Head-on collisions (from Worksheet 2D)	0.003	0.006	0.010					
Angle collisions (from Worksheet 2D)	0.133	0.357	0.490					
Sideswipe (from Worksheet 2D)	0.003	0.045	0.049					
Other multiple-vehicle collision (from Worksheet 2D)	0.000	0.000	0.000					
Subtotal	0.237	0.617	0.854					
	SINGLE-VEHICLE	·	•					
Collision with parked vehicle (from Worksheet 2F)	0.000	0.003	0.003					
Collision with animal (from Worksheet 2F)	0.000	0.003	0.003					
Collision with fixed object (from Worksheet 2F)	0.016	0.065	0.081					
Collision with other object (from Worksheet 2F)	0.003	0.003	0.006					
Other single-vehicle collision (from Worksheet 2F)	0.000	0.000	0.000					
Single-vehicle noncollision (from Worksheet 2F)	0.006	0.000	0.006					
Collision with pedestrian (from Worksheet 2G or 2I)	0.022	0.000	0.022					
Collision with bicycle (from Worksheet 2J)	0.037	0.000	0.037					
Subtotal	0.085	0.075	0.159					
Total	0.322	0.692	1.014					

Worksheet 2L Summary R	esults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)
	(Total) from Worksheet 2K
Total	1.014
Fatal and injury (FI)	0.322
Property damage only (PDO)	0.692

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	Legend						
Required Manual Input	Select from Drop-Down List		Optional Input				

	Worksheet 2A General Information and Input	Data for Urban and Suburban A	rterial Intersec	ctions	
General	Information	Location Information			
Analyst	Ben Erban	City		Milton	
Agency or Company	CTPS	Intersection		Route 138 and Blue Jay Way Curry College	
Date Performed	05/01/18	Jurisdiction		MassDOT Highwat District 6	
				2013	
Inp	Base Conditions		Site Conditions		
Intersection type (3ST, 3SG, 4ST, 4SG)				3ST	
AADT _{major} (veh/day)	$AADT_{MAX} = 45,700 \text{(veh/day)}$			16,450	
AADT _{minor} (veh/day)	$AADT_{MAX} = 9,300 (veh/day)$			800	
Intersection lighting (present/not present)		Not Present		Not Present	
Calibration factor, C _i		1.00		1.13	
Data for unsignalized intersections only:					
Number of major-road approaches with left-	urn lanes (0,1,2)			0	
Number of major-road approaches with right	-turn lanes (0,1,2)		0		

	SUMMARY OF INTERSECTION & CRASHES									
CMF _{1i}	CMF 1i CMF 2i CMF 3i CMF 4i CMF 5i N bi_total N pedi_total N bikei_total N predictedin									
1.00										

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)				
CMF for Left-Turn Lanes	CMF for Left-Turn Signal	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF				
	Phasing									
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}				
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)				
1.00	1.00	1.00	1.00	1.00	1.00	1.00				

(1)	(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)	
Crash Severity Level	SPF Coefficients from Table 12-10		Overdispersion Parameter, k	Initial N _{bimv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Adjusted N _{bimv}	
			from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from	,		
	а	b	С		21		() 101/12 ()	Worksheet 2B		(6)*(7)*(8)
Total	-20.02	1.66	0.54	0.24	0.745	1.000	0.745	1.00	1.13	0.841
Fatal and Injury (FI)					0.207		0.207	1.00	1.13	0.234
rataranu injury (ri)					0.201		0.201	1.00	1.13	0.204
Property Damage Only (PDO)					0.538		0.538	1.00	1.13	0.608

(1)	(2)	(3)	ion Type for Urban and Suburba (4)	(5)	(6)
Collision Type	Proportion of Collision Type _(Fi)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C
Total	1.000	0.234	1.000	0.608	0.841
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Rear-end collision	0.411	0.096	0.337	0.205	0.301
Head-on collision	0.014	0.003	0.011	0.006	0.010
Angle collision	0.562	0.131	0.579	0.352	0.483
Sideswipe	0.014	0.003	0.074	0.045	0.048
Other multiple-vehicle collision	0.000	0.000	0.000	0.000	0.000

		Worksheet	2E Single-V	ehicle Collisions by Seve	ity Level for Urban a	and Suburban Arterial Int	ersections			
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
SPF Coefficients		ents Historical Data		Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}	
Crash Severity Level	from Table 12-12		2				(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	a	h	С	from Table 1A	(3) * (4) From		(I)TOTAL (O)	Worksheet 2B		(0) (1) (0)
	u u	۵	9		Worksheet 2C					
Total				0.118	0.088	1.000	0.088	1.00	1.13	0.099
Fatal and Injury (FI)				0.258	0.023		0.023	1.00	1.13	0.026
· atai and injury (i i)				0.200	0.020		0.020	1.00	1.10	0.020
Property Damage Only (PDO)			-	0.742	0.065		0.065	1.00	1.13	0.074

Worksheet 2F -- Single-Vehicle Collisions by Collision Type for Urban and Suburban Arterial Intersections

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Collision Type Proportion of Collision Type _(Fi)		Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.000	0.026	1.000	0.074	0.099
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.000	0.000	0.043	0.003	0.003
Collision with animal	0.000	0.000	0.043	0.003	0.003
Collision with fixed object	0.625	0.016	0.870	0.064	0.080
Collision with other object	0.125	0.003	0.043	0.003	0.006
Other single-vehicle collision	0.000	0.000	0.000	0.000	0.000
Single-vehicle noncollision	0.250	0.006	0.000	0.000	0.006

V	Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Stop-Controlled Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)				
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh- Ped ω	Calibration	Predicted N _{pedi}				
	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	from Table 1B	factor, C _i	(4)*(5)*(6)				
Total	0.841	0.099	0.941	0.020	1.13	0.022				
Fatal and injury (FI)					1.13	0.022				

	Worksheet 2J Vehicle-Bicycle Collisions for Urban and Suburban Arterial Intersections								
(1) (2) (3) (4) (5) (6) (7)									
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	redicted N _{bi} Veh-Bike ω Calibration		Predicted N _{bikei}			
	(9) from Worksheet 2C	(9) from Worksheet 2E	$(2) + (3)$ ven-Bike ω factor, C_i		(4)*(5)*(6)				
Total	0.841	0.099	0.941	0.034	1.13	0.036			
Fatal and injury (FI)					1.13	0.036			

Worksh	eet 2K Crash Severity Distribution for Urban an	d Suburban Arterial Intersections	
(1)	(2)	(3)	(4)
	Fatal and injury (FI)	Property damage only (PDO)	Total
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;
	(7) from Worksheet 2H and 2J		(7) from Worksheet 2H and 2J
	MULTIPLE-VEHICLE	·	
Rear-end collisions (from Worksheet 2D)	0.096	0.205	0.301
Head-on collisions (from Worksheet 2D)	0.003	0.006	0.010
Angle collisions (from Worksheet 2D)	0.131	0.352	0.483
Sideswipe (from Worksheet 2D)	0.003	0.045	0.048
Other multiple-vehicle collision (from Worksheet 2D)	0.000	0.000	0.000
Subtotal	0.234	0.608	0.841
	SINGLE-VEHICLE		
Collision with parked vehicle (from Worksheet 2F)	0.000	0.003	0.003
Collision with animal (from Worksheet 2F)	0.000	0.003	0.003
Collision with fixed object (from Worksheet 2F)	0.016	0.064	0.080
Collision with other object (from Worksheet 2F)	0.003	0.003	0.006
Other single-vehicle collision (from Worksheet 2F)	0.000	0.000	0.000
Single-vehicle noncollision (from Worksheet 2F)	0.006	0.000	0.006
Collision with pedestrian (from Worksheet 2G or 2I)	0.022	0.000	0.022
Collision with bicycle (from Worksheet 2J)	0.036	0.000	0.036
Subtotal	0.083	0.074	0.157
Total	0.317	0.681	0.998

Worksheet 2L Summary R	esults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)
	(Total) from Worksheet 2K
Total	0.998
Fatal and injury (FI)	0.317
Property damage only (PDO)	0.681

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	Legend							
Required Manual Input	Select from Drop-Down List		Optional Input					

	Worksheet 2A General Informatio	n and Input	Data for Urban and Suburban A	rterial Interse	ctions		
Gener	al Information	•	Location Information				
Analyst	Ben Erban	Ben Erban City		Milton			
Agency or Company	CTPS		Intersection		Route 138 and Blue Jay Way Curry College		
Date Performed	05/01/18		Jurisdiction MassDOT Highwat District 6				
			Analysis Year		2012		
Input Data			Base Conditions		Site Conditions		
Intersection type (3ST, 3SG, 4ST, 4SG)					3ST		
AADT _{major} (veh/day)	$AADT_{MAX} = 45,700$	(veh/day)			16,300		
AADT _{minor} (veh/day)	$AADT_{MAX} = 9,300$	(veh/day)			800		
Intersection lighting (present/not present)			Not Present		Not Present		
Calibration factor, C _i			1.00		1.13		
Data for unsignalized intersections only:							
Number of major-road approaches with le	ft-turn lanes (0,1,2)				0		
Number of major-road approaches with right	ght-turn lanes (0,1,2)				0		

	SUMMARY OF INTERSECTION & CRASHES										
CMF _{1i}	CMF 1i CMF 2i CMF 3i CMF 4i CMF 5i N bi_total N pedi_total N bikei_total N predicted										
1.00											

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections										
(1)	(2)	(3)	(4)	(5)	(6)	(7)					
CMF for Left-Turn Lanes	CMF for Left-Turn Signal	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF					
	Phasing										
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}					
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)					
1.00	1.00	1.00	1.00	1.00	1.00	1.00					

(1)	(2) y Level SPF Coefficients from Table 12-10		(2)		(4)	(5)	(6)	(7)	(8)	(9)
Crash Severity Level					Initial N _{bimv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Adjusted N _{bimv}
			from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)			(6)*(7)*(8)	
	a	b	С		21		()TOTAL (-)	Worksheet 2B		(-) (-)
Total	-20.02	1.66	0.54	0.24	0.733	1.000	0.733	1.00	1.13	0.829
Fatal and Injury (FI)					0.204		0.204	1.00	1.13	0.230
ratarand injury (ri)					0.204		0.204	1.00	1.15	0.230
Property Damage Only (PDO)					0.530		0.530	1.00	1.13	0.599

	Worksheet 2D Multiple-V	ehicle Collisions by Collis	ion Type for Urban and Suburba	an Arterial Intersections	
(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(Fi)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C
Total	1.000	0.230	1.000	0.599	0.829
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Rear-end collision	0.411	0.095	0.337	0.202	0.296
Head-on collision	0.014	0.003	0.011	0.006	0.009
Angle collision	0.562	0.129	0.579	0.347	0.476
Sideswipe	0.014	0.003	0.074	0.044	0.047
Other multiple-vehicle collision	0.000	0.000	0.000	0.000	0.000

		Worksheet	2E Single-V	ehicle Collisions by Seve	ity Level for Urban a	and Suburban Arterial Int	ersections			
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
	SPF Coefficients from Table 12-12		Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}	
Crash Severity Level			2				(4) _{TOTAL} *(5) (7) from	` '] ' ' [(6)*(7)*(8)
	a	h	С	from Table 1A	(3) * (4) From		(I)TOTAL (O)	Worksheet 2B		(0) (1) (0)
	a a	۵	9		Worksheet 2C					
Total				0.118	0.086	1.000	0.086	1.00	1.13	0.098
Fatal and Injury (FI)				0.258	0.022		0.022	1.00	1.13	0.025
· atai and injury (i i)				0.200	0.022		0.022	1.00	1.10	0.020
Property Damage Only (PDO)				0.742	0.064		0.064	1.00	1.13	0.072

Worksheet 2F -- Single-Vehicle Collisions by Collision Type for Urban and Suburban Arterial Intersections

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv (FI)} (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.000	0.025	1.000	0.072	0.098
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.000	0.000	0.043	0.003	0.003
Collision with animal	0.000	0.000	0.043	0.003	0.003
Collision with fixed object	0.625	0.016	0.870	0.063	0.079
Collision with other object	0.125	0.003	0.043	0.003	0.006
Other single-vehicle collision	0.000	0.000	0.000	0.000	0.000
Single-vehicle noncollision	0.250	0.006	0.000	0.000	0.006

Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Stop-Controlled Intersections										
(1)	(2)	(3)	(4)	(5)	(6)	(7)				
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh- Ped ω	Calibration	Predicted N _{pedi}				
	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	from Table 1B	factor, C _i	(4)*(5)*(6)				
Total	0.829	0.098	0.926	0.020	1.13	0.021				
Fatal and injury (FI)					1.13	0.021				

Worksheet 2J Vehicle-Bicycle Collisions for Urban and Suburban Arterial Intersections									
(1) (2) (3) (4) (5) (6) (7)									
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv} Predicted N _{bi}		Vob Piko	Calibration	Predicted N _{bikei}			
	(9) from Worksheet 2C	(9) from Worksheet 2E	Veh-Bike ω facto		factor, C _i	(4)*(5)*(6)			
Total	0.829	0.098	0.926	0.034	1.13	0.036			
Fatal and injury (FI)					1.13	0.036			

Worksheet 2K	Worksheet 2K Crash Severity Distribution for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)							
	Fatal and injury (FI)	Property damage only (PDO)	Total							
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;							
	(7) from Worksheet 2H and 2J		(7) from Worksheet 2H and 2J							
MULTIPLE-VEHICLE										
Rear-end collisions (from Worksheet 2D)	0.095	0.202	0.296							
Head-on collisions (from Worksheet 2D)	0.003	0.006	0.009							
Angle collisions (from Worksheet 2D)	0.129	0.347	0.476							
Sideswipe (from Worksheet 2D)	0.003	0.044	0.047							
Other multiple-vehicle collision (from Worksheet 2D)	0.000	0.000	0.000							
Subtotal	0.230	0.599	0.829							
	SINGLE-VEHICLE									
Collision with parked vehicle (from Worksheet 2F)	0.000	0.003	0.003							
Collision with animal (from Worksheet 2F)	0.000	0.003	0.003							
Collision with fixed object (from Worksheet 2F)	0.016	0.063	0.079							
Collision with other object (from Worksheet 2F)	0.003	0.003	0.006							
Other single-vehicle collision (from Worksheet 2F)	0.000	0.000	0.000							
Single-vehicle noncollision (from Worksheet 2F)	0.006	0.000	0.006							
Collision with pedestrian (from Worksheet 2G or 2I)	0.021	0.000	0.021							
Collision with bicycle (from Worksheet 2J)	0.036	0.000	0.036							
Subtotal	0.082	0.072	0.155							
Total	0.312	0.671	0.983							

Worksheet 2L Summary R	esults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)
	(Total) from Worksheet 2K
Total	0.983
Fatal and injury (FI)	0.312
Property damage only (PDO)	0.671

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	Legend						
Required Manual Input	Select from Drop-Down List	Optional Input					

	Worksheet 2A General Information a	and Input I	Data for Urban and Suburban A	rterial Interse	ctions	
Genera	I Information	•	Location Information			
Analyst	Ben Erban		City		Milton	
Agency or Company	CTPS		Intersection		Route 138 and Blue Jay Way Curry College	
Date Performed	05/01/18		Jurisdiction MassDOT Highwat District 6			
			Analysis Year		2011	
Input Data			Base Conditions	Site Conditions		
Intersection type (3ST, 3SG, 4ST, 4SG)	<u></u>				3ST	
AADT _{major} (veh/day)	$AADT_{MAX} = 45,700 (vertex)$	eh/day)			16,100	
AADT _{minor} (veh/day)	$AADT_{MAX} = 9,300 (vertex)$	eh/day)			800	
Intersection lighting (present/not present)			Not Present		Not Present	
Calibration factor, C _i			1.00		1.13	
Data for unsignalized intersections only:						
Number of major-road approaches with left	-turn lanes (0,1,2)			0		
Number of major-road approaches with right	nt-turn lanes (0,1,2)			0		

	SUMMARY OF INTERSECTION & CRASHES									
CMF _{1i}	CMF _{1i} CMF _{2i} CMF _{3i} CMF _{4i} CMF _{5i} N _{bi_total} N _{pedi_total} N _{bikei_total} N _{predictedint}									
1.00										

Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)			
CMF for Left-Turn Lanes	CMF for Left-Turn Signal	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF			
	Phasing								
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}			
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)			
1.00	1.00	1.00	1.00	1.00	1.00	1.00			

(1)			(2)		(3) (4)		(5)	(6)	(7)	(8)	(9)
Crash Severity Level			Overdispersion Properties Propert		Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Adjusted N _{bimv}		
	a	om Table 12-10 h)	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from Worksheet 2B		(6)*(7)*(8)	
Total	-20.02	1.66	0.54	0.24	0.718	1.000	0.718	1.00	1.13	0.812	
Fatal and Injury (FI)					0.199		0.199	1.00	1.13	0.225	
Property Damage Only (PDO)					0.519		0.519	1.00	1.13	0.587	

	Worksheet 2D Multiple-V	ehicle Collisions by Collis	ion Type for Urban and Suburb	an Arterial Intersections	_	
(1)	(2)	(3)	(4)	(5)	(6)	
Collision Type	Proportion of Collision Type _(Fi)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)	
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C	
Total	1.000	0.225	1.000	0.587	0.812	
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)	
Rear-end collision	0.411	0.093	0.337	0.198	0.290	
Head-on collision	0.014	0.003	0.011	0.006	0.009	
Angle collision	0.562	0.127	0.579	0.340	0.466	
Sideswipe	0.014	0.003	0.074	0.043	0.046	
Other multiple-vehicle collision	0.000	0.000	0.000	0.000	0.000	

	Worksheet 2E Single-Vehicle Collisions by Severity Level for Urban and Suburban Arterial Intersections											
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)		
	SPF Coefficients from Table 12-12		Historical Data	Initial N _{bisv}	Proportion of Total Adjusted Crashes N _{bimv}		Combined CMFs	Calibration Factor, C _i				
Crash Severity Level						(4) _{TOTAL} *(5) (7) from		(6	(6)*(7)*(8)			
	а	h	С	from Table 1A	(3) * (4) From		(')TOTAL (O)	Worksheet 2B		(0) (1) (0)		
	u	b	Ü		Worksheet 2C							
Total				0.118	0.085	1.000	0.085	1.00	1.13	0.096		
Fatal and Injury (FI)				0.258	0.022		0.022	1.00	1.13	0.025		
ratarana injary (i i)				0.200	0.022		0.022	1.00	1.10	0.020		
Property Damage Only (PDO)				0.742	0.063		0.063	1.00	1.13	0.071		

Worksheet 2F -- Single-Vehicle Collisions by Collision Type for Urban and Suburban Arterial Intersections

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv (FI)} (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.000	0.025	1.000	0.071	0.096
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.000	0.000	0.043	0.003	0.003
Collision with animal	0.000	0.000	0.043	0.003	0.003
Collision with fixed object	0.625	0.015	0.870	0.062	0.077
Collision with other object	0.125	0.003	0.043	0.003	0.006
Other single-vehicle collision	0.000	0.000	0.000	0.000	0.000
Single-vehicle noncollision	0.250	0.006	0.000	0.000	0.006

Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Stop-Controlled Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)			
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh- Ped ω	Calibration	Predicted N _{pedi}			
Crasii Severity Lever	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	from Table 1B	factor, C _i	(4)*(5)*(6)			
Total	0.812	0.096	0.908	0.020	1.13	0.021			
Fatal and injury (FI)					1.13	0.021			

Worksheet 2J Vehicle-Bicycle Collisions for Urban and Suburban Arterial Intersections								
(1) (2) (3) (4) (5) (6)								
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration	Predicted N _{bikei}		
	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	ven-bike ω	factor, C _i	(4)*(5)*(6)		
Total	0.812	0.096	0.908	0.034	1.13	0.035		
Fatal and injury (FI)					1.13	0.035		

Worksh	eet 2K Crash Severity Distribution for Urban an	nd Suburban Arterial Intersections	
(1)	(2)	(3)	(4)
	Fatal and injury (FI)	Property damage only (PDO)	Total
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;
	(7) from Worksheet 2H and 2J		(7) from Worksheet 2H and 2J
	MULTIPLE-VEHICLE	·	
Rear-end collisions (from Worksheet 2D)	0.093	0.198	0.290
Head-on collisions (from Worksheet 2D)	0.003	0.006	0.009
Angle collisions (from Worksheet 2D)	0.127	0.340	0.466
Sideswipe (from Worksheet 2D)	0.003	0.043	0.046
Other multiple-vehicle collision (from Worksheet 2D)	0.000	0.000	0.000
Subtotal	0.225	0.587	0.812
	SINGLE-VEHICLE	·	·
Collision with parked vehicle (from Worksheet 2F)	0.000	0.003	0.003
Collision with animal (from Worksheet 2F)	0.000	0.003	0.003
Collision with fixed object (from Worksheet 2F)	0.015	0.062	0.077
Collision with other object (from Worksheet 2F)	0.003	0.003	0.006
Other single-vehicle collision (from Worksheet 2F)	0.000	0.000	0.000
Single-vehicle noncollision (from Worksheet 2F)	0.006	0.000	0.006
Collision with pedestrian (from Worksheet 2G or 2I)	0.021	0.000	0.021
Collision with bicycle (from Worksheet 2J)	0.035	0.000	0.035
Subtotal	0.081	0.071	0.152
Total	0.306	0.658	0.963

Worksheet 2L Summary R	esults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, $N_{predicted\ int}$ (crashes/year)
	(Total) from Worksheet 2K
Total	0.963
Fatal and injury (FI)	0.306
Property damage only (PDO)	0.658

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General Information	n	Location Information			
Analyst	Analyst Ben Erban		oute 138 at Bradlee Road and Atherton Stre		
Agency or Company CTPS		Intersection Type	4SG		
Date Performed	May-18	Jurisdiction	MassDOT Highwat District 6		
City Milton		Analysis Year	2018		

		Input Inf	ormation		
Year	Oserved MV crashes	Observed total crashes	Predicted MV crashes	Predicted total crashes	Combined CMF for veh-ped crashes
2015	4	4	2.19	2.57	2.78
2014	2	2	2.16	2.55	2.78
2013	3	3	2.12	2.49	2.78
2012	3	4	2.08	2.44	2.78
2011	4	4	2.06	2.42	2.78

					Outp	out Inform	ation				
Observed MV crashes	Average observed total crashes	Total predicted MV crashes	Average predicted total crashes	Standard deviation of predicted total crashes	Weight	Total expected MV crashes	No of expected total crashes	Average expected total crashes	INTARGACTION	Potential for Safety Improvement (PSI)	If avg observed total crashes > avg expected crashes
16.00	3.40	10.61	2.50	0.06	0.50	13.31	3.17 3.13 3.07 3.01 2.98	3.07	Y	0.58	Y

Works	heet 2A General Information and Input	Data for Urban and Suburban A	rterial Intersection	ons	
General Informa	tion	Location Information			
Analyst	Ben Erban	City	City Milton		
Agency or Company	CTPS	Intersection		Route 138 at Bradlee Road and Atherton Street	
Date Performed	5/1/2018	Jurisdiction		MassDOT Highwat District 6	
	Analysis Year		2015		
Input Data	Base Conditions		Site Conditions		
Intersection type (3ST, 3SG, 4ST, 4SG)			4SG		
AADT _{major} (veh/day)	$AADT_{MAX} = 67700 (veh/day)$			12,850	
AADT _{minor} (veh/day)	$AADI_{MAX} = 33400 (veh/day)$			1,950	
Intersection lighting (present/not present)	Not Present	Not Present			
Calibration factor, C _i		1	1		
Data for signalized intersections only:					
Number of approaches with left-turn lanes (0,1,2,3,4	[for 3SG, use maximum value of 3]			0	
Number of approaches with right-turn lanes (0,1,2,3,	4) [for 3SG, use maximum value of 3]			0	
Number of approaches with left-turn signal phasing [for 3SG, use maximum value of 3]		1		
Type of left-turn signal phasing for Leg #1		Permissive	Protected		
Type of left-turn signal phasing for Leg #2			Permissive		
Type of left-turn signal phasing for Leg #3				Permissive	
Type of left-turn signal phasing for Leg #4 (if applical	ole)		Permissive		
Number of approaches with right-turn-on-red prohibit	ed [for 3SG, use maximum value of 3]	0	0		
Intersection red light cameras (present/not present)		Not Present	Not Present		
Sum of all pedestrian crossing volumes (PedVol)			14		
Maximum number of lanes crossed by a pedestrian		2			
Number of bus stops within 300 m (1,000 ft) of the in	tersection	0	2		
Schools within 300 m (1,000 ft) of the intersection (p	resent/not present)	Not Present		Not Present	
Number of alcohol sales establishments within 300 r	(1.000 ft) of the intersection	0		0	

	SUMMARY OF INTERSECTION & CRASHES										
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF ₄i	CMF 5i	CMF _{1p}	CMF _{2p}	CMF 3p	N _{bi_total}	N_{pedi_total}	N _{bikei_total}	N predictedint
1.000	0.940	1.000	1.000	1.000	2.780	1.000	1.000	2.295	0.183	0.097	2.575

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections								
(1)	(2)	(3)	(4)	(5)	(6)	(7)			
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF			
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}			
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)			
1.00	0.94	1.00	1.00	1.00	1.00	0.94			

	1	worksheet 2	C Multiple-	ehicle Collisions by Seve	rity Level for Urban	and Suburban Arterial In	itersections			1
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
Crash Severity Level	SF	SPF Coefficients		Overdispersion Parameter, k	Initial N _{bimv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bimv}
	fro	om Table 12-10)	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	110111 14510 12 10	21		() TOTAL ()	Worksheet 2B		(0) (1) (0)
Total	-11.79	0.92	0.52	0.10	2.33	1.00	2.33	0.94	1.00	2.19
Fatal and Injury (FI)					0.65		0.65	0.94	1.00	0.61
i atai and injury (i i)					0.03		0.03	0.34	1.00	0.01
Property Damage Only (PDO)					1.68		1.68	0.94	1.00	1.58

	Worksheet 2D Multiple-V	ehicle Collisions by Collis	ion Type for Urban and Suburba	an Arterial Intersections	
(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C
Total	1.00	0.61	1.00	1.58	2.19
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Rear-end collision	0.45	0.27	0.42	0.67	0.94
Head-on collision	0.04	0.02	0.04	0.06	0.08
Angle collision	0.49	0.30	0.43	0.67	0.97
Sideswipe	0.02	0.01	0.11	0.18	0.19
Other multiple-vehicle collision	0.00	0.00	0.00	0.00	0.00

	Worksheet 2E Single-Vehicle Collisions by Severity Level for Urban and Suburban Arterial Intersections									
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
	SF	PF Coefficients	S	Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bisv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}
Crash Severity Level	fro	om Table 12-12	2	ć -			(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	from Table 1A	(3) * (4) From Worksheet 2C			Worksheet 2B		.,,,,,
Total				0.05	0.11	1.00	0.11	0.94	1.00	0.11
Fatal and Injury (FI)				0.18	0.02		0.02	0.94	1.00	0.02
Property Damage Only (PDO)			1	0.82	0.09		0.09	0.94	1.00	0.09

	Worksheet 2F Single-Ve	ehicle Collisions by Collision	on Type for Urban and Suburba	n Arterial Intersections	
(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv (FI)} (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.00	0.02	1.00	0.09	0.11
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.00	0.00	0.03	0.00	0.00
Collision with animal	0.00	0.00	0.03	0.00	0.00
Collision with fixed object	0.63	0.01	0.86	0.08	0.09
Collision with other object	0.00	0.00	0.06	0.00	0.00
Other single-vehicle collision	0.13	0.00	0.03	0.00	0.00
Single-vehicle noncollision	0.25	0.00	0.00	0.00	0.00

Worksheet 2G-1 Crash Modific	Worksheet 2G-1 Crash Modification Factors for Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Signalized Intersections							
(1)	(2)	(3)	(4)					
CMF for Bus Stops	CMF for Schools	CMF for Alcohol Sales Establishments	Combined CMF					
CMF _{1p}	CMF _{2p}	CMF _{3p}	Combined Civil					
from Table 12-28	from Table 12-29	from Table 12-30	(1)*(2)*(3)					
2.78	1.00	1.00	2.78					

	Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Intersections							
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	
Creek Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Ped ω	Valo Dad		Predicted N _{pedi}	
Crash Severity Level	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	ven-red ω	Npedbase	factor, C _i	(4) from Worksheet 2H*(6)*(7)	
Total	2.19	0.11	2.29	0.03	0.07	1.00	0.18	
Fatal and injury (FI)						1.00	0.18	

(1)	(2)	(3)	(4)	(5)	(6)	(7)
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bisv} Predicted N _{bi}		Calibration factor, C	Predicted N _{bikei}
Crash Severny Level	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	- Veh-Bike ω	Cambration factor, C _i	(4)*(5)*(6)
Total	2.19	0.11	2.29	0.04	1.00	0.10
Fatal and injury (FI)					1.00	0.10

Worksh	eet 2K Crash Severity Distribution for Urban and	Suburban Arterial Intersections	
(1)	(2)	(3)	(4)
	Fatal and injury (FI)	Property damage only (PDO)	Total
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;
	(8) from Worksheet 2G and (7) from 2J		(8) from Worksheet 2G and (7) from 2J
	MULTIPLE-VEHICLE	•	•
Rear-end collisions (from Worksheet 2D)	0.27	0.67	0.94
Head-on collisions (from Worksheet 2D)	0.02	0.06	0.08
Angle collisions (from Worksheet 2D)	0.30	0.67	0.97
Sideswipe (from Worksheet 2D)	0.01	0.18	0.19
Other multiple-vehicle collision (from Worksheet 2D)	0.00	0.00	0.00
Subtotal	0.61	1.58	2.19
	SINGLE-VEHICLE		
Collision with parked vehicle (from Worksheet 2F)	0.00	0.00	0.00
Collision with animal (from Worksheet 2F)	0.00	0.00	0.00
Collision with fixed object (from Worksheet 2F)	0.01	0.08	0.09
Collision with other object (from Worksheet 2F)	0.00	0.00	0.00
Other single-vehicle collision (from Worksheet 2F)	0.00	0.00	0.00
Single-vehicle noncollision (from Worksheet 2F)	0.00	0.00	0.00
Collision with pedestrian (from Worksheet 2G or 2I)	0.18	0.00	0.18
Collision with bicycle (from Worksheet 2J)	0.10	0.00	0.10
Subtotal	0.30	0.09	0.39
Total	0.91	1.67	2.57

Worksheet 2L Summary Re	Worksheet 2L Summary Results for Urban and Suburban Arterial Intersections					
(1)	(2)					
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)					
	Total					
Total	2.57					
Fatal and injury (FI)	0.91					
Property damage only (PDO)	1.67					

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Works	sheet 2A General Information and Input	Data for Urban and Suburban A	terial Intersections		
General Informa	ation	Location Information			
Analyst	Ben Erban	City		Milton	
Agency or Company	CTPS	Intersection	Ro	oute 138 at Bradlee Road and Atherton Street	
Date Performed	43221	Jurisdiction		MassDOT Highwat District 6	
		Analysis Year		2014	
Input Data	Base Conditions		Site Conditions		
Intersection type (3ST, 3SG, 4ST, 4SG)				4SG	
AADT _{major} (veh/day)	$AADT_{MAX} = 67700 (veh/day)$			12,700	
AADT _{minor} (veh/day)	$AADI_{MAX} = 33400 (veh/day)$			1,950	
Intersection lighting (present/not present)	Not Present	Not Present			
Calibration factor, C _i		1	1		
Data for signalized intersections only:					
Number of approaches with left-turn lanes (0,1,2,3,4) [for 3SG, use maximum value of 3]			0	
Number of approaches with right-turn lanes (0,1,2,3	,4) [for 3SG, use maximum value of 3]		0		
Number of approaches with left-turn signal phasing	[for 3SG, use maximum value of 3]		1		
Type of left-turn signal phasing for Leg #1		Permissive	Protected		
Type of left-turn signal phasing for Leg #2			Permissive		
Type of left-turn signal phasing for Leg #3			Permissive		
Type of left-turn signal phasing for Leg #4 (if applica	ble)		Permissive		
Number of approaches with right-turn-on-red prohibi	ted [for 3SG, use maximum value of 3]	0	0		
Intersection red light cameras (present/not present)		Not Present	Not Present		
Sum of all pedestrian crossing volumes (PedVol)	Signalized intersections only			14	
Maximum number of lanes crossed by a pedestrian			2		
Number of bus stops within 300 m (1,000 ft) of the in	ntersection	0	2		
Schools within 300 m (1,000 ft) of the intersection (p	present/not present)	Not Present	Not Present		
Number of alcohol sales establishments within 300	m (1.000 ft) of the intersection	0		0	

	SUMMARY OF INTERSECTION & CRASHES										
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF ₄i	CMF 5i	CMF _{1p}	CMF _{2p}	CMF 3p	N _{bi_total}	N_{pedi_total}	N _{bikei_total}	N predictedint
1.000	0.940	1.000	1.000	1.000	2.780	1.000	1.000	2.270	0.181	0.096	2.547

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections							
(1)	(2)	(3)	(4)	(5)	(6)	(7)		
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF		
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}		
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)		
1.00	0.94	1.00	1.00	1.00	1.00	0.94		

		worksheet 2	C Multiple-	ehicle Collisions by Seve	rity Level for Urban	and Suburban Arterial In	tersections			
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
Crash Severity Level	SF	PF Coefficient	s	Overdispersion Parameter, k	Initial N _{bimv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bimv}
	fro	om Table 12-10)	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	110111 14510 12 10	21		(')TOTAL (')	Worksheet 2B		(0) (1) (0)
Total	-11.79	0.92	0.52	0.10	2.30	1.00	2.30	0.94	1.00	2.16
Fatal and Injury (FI)					0.64		0.64	0.94	1.00	0.60
i atai and injury (i i)					0.04		0.04	0.94	1.00	0.00
Property Damage Only (PDO)					1.66		1.66	0.94	1.00	1.56

	Worksheet 2D Multiple-V	ehicle Collisions by Collis	ion Type for Urban and Suburba	an Arterial Intersections	
(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C
Total	1.00	0.60	1.00	1.56	2.16
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Rear-end collision	0.45	0.27	0.42	0.66	0.93
Head-on collision	0.04	0.02	0.04	0.06	0.08
Angle collision	0.49	0.30	0.43	0.66	0.96
Sideswipe	0.02	0.01	0.11	0.18	0.19
Other multiple-vehicle collision	0.00	0.00	0.00	0.00	0.00

		Worksheet	2E Single-V	ehicle Collisions by Seve	ity Level for Urban a	and Suburban Arterial Int	ersections			
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
	SF	PF Coefficients	S	Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bisv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}
Crash Severity Level	fro	om Table 12-12	2	ć -			(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	from Table 1A	(3) * (4) From Worksheet 2C			Worksheet 2B		.,,,,,
Total				0.05	0.11	1.00	0.11	0.94	1.00	0.11
Fatal and Injury (FI)				0.18	0.02		0.02	0.94	1.00	0.02
Property Damage Only (PDO)			1	0.82	0.09		0.09	0.94	1.00	0.09

	Worksheet 2F Single-Ve	ehicle Collisions by Collision	on Type for Urban and Suburba	n Arterial Intersections	
(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv} (FI) (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.00	0.02	1.00	0.09	0.11
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.00	0.00	0.03	0.00	0.00
Collision with animal	0.00	0.00	0.03	0.00	0.00
Collision with fixed object	0.63	0.01	0.86	0.07	0.09
Collision with other object	0.00	0.00	0.06	0.00	0.00
Other single-vehicle collision	0.13	0.00	0.03	0.00	0.00
Single-vehicle noncollision	0.25	0.00	0.00	0.00	0.00

Worksheet 2G-1 Crash Mod	Worksheet 2G-1 Crash Modification Factors for Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Signalized Intersections							
(1)	(2)	(3)	(4)					
CMF for Bus Stops	CMF for Schools	CMF for Alcohol Sales Establishments	Combined CMF					
CMF _{1p}	CMF _{2p}	CMF _{3p}	Combined Civil					
from Table 12-28	from Table 12-29	from Table 12-30	(1)*(2)*(3)					
2.78	1.00	1.00	2.78					

	Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Intersections							
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	
Crack Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	- Veh-Ped ω	Needbass	Calibration	Predicted N _{pedi}	
Crash Severity Level	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	- ven-Ped ω	Npedbase	factor, C _i	(4) from Worksheet 2H*(6)*(7)	
Total	2.16	0.11	2.27	0.03	0.07	1.00	0.18	
Fatal and injury (FI)						1.00	0.18	

(1)	(2)	(3)	(4)	(5)	(6)	(7)
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	5.0.		Calibration factor, C	Predicted N _{bikei}
Crash Severny Level	(9) from Worksheet 2C	(4) from Worksheet 2E			Cambration factor, C _i	(4)*(5)*(6)
Total	2.16	0.11	2.27	0.04	1.00	0.10
Fatal and injury (FI)					1.00	0.10

Worksh	eet 2K Crash Severity Distribution for Urban an	d Suburban Arterial Intersections	
(1)	(2)	(3)	(4)
	Fatal and injury (FI)	Property damage only (PDO)	Total
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;
	(8) from Worksheet 2G and (7) from 2J		(8) from Worksheet 2G and (7) from 2J
	MULTIPLE-VEHICLE	•	•
Rear-end collisions (from Worksheet 2D)	0.27	0.66	0.93
Head-on collisions (from Worksheet 2D)	0.02	0.06	0.08
Angle collisions (from Worksheet 2D)	0.30	0.66	0.96
Sideswipe (from Worksheet 2D)	0.01	0.18	0.19
Other multiple-vehicle collision (from Worksheet 2D)	0.00	0.00	0.00
Subtotal	0.60	1.56	2.16
	SINGLE-VEHICLE		
Collision with parked vehicle (from Worksheet 2F)	0.00	0.00	0.00
Collision with animal (from Worksheet 2F)	0.00	0.00	0.00
Collision with fixed object (from Worksheet 2F)	0.01	0.07	0.09
Collision with other object (from Worksheet 2F)	0.00	0.00	0.00
Other single-vehicle collision (from Worksheet 2F)	0.00	0.00	0.00
Single-vehicle noncollision (from Worksheet 2F)	0.00	0.00	0.00
Collision with pedestrian (from Worksheet 2G or 2I)	0.18	0.00	0.18
Collision with bicycle (from Worksheet 2J)	0.10	0.00	0.10
Subtotal	0.30	0.09	0.38
Total	0.90	1.65	2.55

Worksheet 2L Summary Results for Urban and Suburban Arterial Intersections					
(1)	(2)				
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)				
	Total				
Total	2.55				
Fatal and injury (FI)	0.90				
Property damage only (PDO)	1.65				

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Works	sheet 2A General Information and Input	Data for Urban and Suburban A	rterial Intersect	ions	
General Informa	ation		Location	on Information	
Analyst	Ben Erban	City Milton		Milton	
Agency or Company	CTPS	Intersection		Route 138 at Bradlee Road and Atherton Street	
Date Performed	43221	Jurisdiction		MassDOT Highwat District 6	
·		Analysis Year		2013	
Input Data	Base Conditions		Site Conditions		
Intersection type (3ST, 3SG, 4ST, 4SG)				4SG	
AADT _{major} (veh/day)	$AADT_{MAX} = 67700 (veh/day)$			12,600	
AADT _{minor} (veh/day)	$AADI_{MAX} = 33400 (veh/day)$			1,900	
Intersection lighting (present/not present)	Not Present	Not Present			
Calibration factor, C _i	1		1		
Data for signalized intersections only:					
Number of approaches with left-turn lanes (0,1,2,3,4) [for 3SG, use maximum value of 3]			0	
Number of approaches with right-turn lanes (0,1,2,3	,4) [for 3SG, use maximum value of 3]		0		
Number of approaches with left-turn signal phasing	[for 3SG, use maximum value of 3]		1		
Type of left-turn signal phasing for Leg #1		Permissive	Protected		
Type of left-turn signal phasing for Leg #2				Permissive	
Type of left-turn signal phasing for Leg #3				Permissive	
Type of left-turn signal phasing for Leg #4 (if applica	ble)			Permissive	
Number of approaches with right-turn-on-red prohibi	ted [for 3SG, use maximum value of 3]	0	0		
Intersection red light cameras (present/not present)	Not Present		Not Present		
Sum of all pedestrian crossing volumes (PedVol)			14		
Maximum number of lanes crossed by a pedestrian		2			
Number of bus stops within 300 m (1,000 ft) of the in	ntersection	0	2		
Schools within 300 m (1,000 ft) of the intersection (p	present/not present)	Not Present	Not Present		
Number of alcohol sales establishments within 300	m (1.000 ft) of the intersection	0		0	

	SUMMARY OF INTERSECTION & CRASHES										
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF ₄i	CMF 5i	CMF _{1p}	CMF _{2p}	CMF 3p	N _{bi_total}	N_{pedi_total}	N _{bikei_total}	N predictedint
1.000	0.940	1.000	1.000	1.000	2.780	1.000	1.000	2.223	0.177	0.094	2.495

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections							
(1)	(2)	(3)	(4)	(5)	(6)	(7)		
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF		
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}		
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)		
1.00	0.94	1.00	1.00	1.00	1.00	0.94		

		Worksheet 2	C Multiple-	ehicle Collisions by Seve	rity Level for Urban	and Suburban Arterial In	tersections			
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
Crash Severity Level	SF	PF Coefficient	s	Overdispersion Parameter, k	Initial N _{bimv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bimv}
	fro	om Table 12-10)	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	Hem rable 12 16	21		(')TOTAL (')	Worksheet 2B		(0) (1) (0)
Total	-11.79	0.92	0.52	0.10	2.26	1.00	2.26	0.94	1.00	2.12
Fatal and Injury (FI)					0.63		0.63	0.94	1.00	0.59
i atai and injury (i i)					0.03		0.03	0.94	1.00	0.59
Property Damage Only (PDO)					1.63		1.63	0.94	1.00	1.53

	Worksheet 2D Multiple-V	ehicle Collisions by Collis	ion Type for Urban and Suburb	an Arterial Intersections	
(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C
Total	1.00	0.59	1.00	1.53	2.12
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Rear-end collision	0.45	0.27	0.42	0.65	0.91
Head-on collision	0.04	0.02	0.04	0.06	0.08
Angle collision	0.49	0.29	0.43	0.65	0.94
Sideswipe	0.02	0.01	0.11	0.17	0.18
Other multiple-vehicle collision	0.00	0.00	0.00	0.00	0.00

(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
	SF	PF Coefficients	3	Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bisv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}
Crash Severity Level	fro	om Table 12-12	2	frame Table 4.6			(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	from Table 1A	(3) * (4) From Worksheet 2C			Worksheet 2B		
Total				0.05	0.11	1.00	0.11	0.94	1.00	0.10
Fatal and Injury (FI)				0.18	0.02		0.02	0.94	1.00	0.02
Property Damage Only (PDO)				0.82	0.09		0.09	0.94	1.00	0.08

	Worksheet 2F Single-Ve	ehicle Collisions by Collisi	on Type for Urban and Suburba	n Arterial Intersections	_
(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv} (FI) (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.00	0.02	1.00	0.08	0.10
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.00	0.00	0.03	0.00	0.00
Collision with animal	0.00	0.00	0.03	0.00	0.00
Collision with fixed object	0.63	0.01	0.86	0.07	0.08
Collision with other object	0.00	0.00	0.06	0.00	0.00
Other single-vehicle collision	0.13	0.00	0.03	0.00	0.00
Single-vehicle noncollision	0.25	0.00	0.00	0.00	0.00

Worksheet 2G-1 Crash Modific	cation Factors for Vehicle-Pe	edestrian Collisions for Urban and Suburban	Arterial Signalized Intersections
(1)	(2)	(3)	(4)
CMF for Bus Stops	CMF for Schools	CMF for Alcohol Sales Establishments	Combined CMF
CMF _{1p}	CMF _{2p}	CMF _{3p}	Combined Civil
from Table 12-28	from Table 12-29	from Table 12-30	(1)*(2)*(3)
2.78	1.00	1.00	2.78

	Worksheet 2G Ve	ehicle-Pedestrian Collisions	for Urban and Su	burban Arterial Intersecti	ons		
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)
Creek Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Ped ω Npedbase		Calibration	Predicted N _{pedi}
Crash Severity Level	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	ven-rea ω	Npedbase	factor, C _i	(4) from Worksheet 2H*(6)*(7)
Total	2.12	0.10	2.22	0.03	0.06	1.00	0.18
Fatal and injury (FI)						1.00	0.18

(1)	(2)	(3)	(4)	(5)	(6)	(7)
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration factor, C	Predicted N _{bikei}
Crasii Severily Lever	(9) from Worksheet 2C			Veii-bike ω	Cambration factor, C _i	(4)*(5)*(6)
Total	2.12	0.10	2.22	0.04	1.00	0.09
Fatal and injury (FI)					1.00	0.09

Workshe	et 2K Crash Severity Distribution for Urban and	Suburban Arterial Intersections	
(1)	(2)	(3)	(4)
	Fatal and injury (FI)	Property damage only (PDO)	Total
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;
	(8) from Worksheet 2G and (7) from 2J		(8) from Worksheet 2G and (7) from 2J
	MULTIPLE-VEHICLE		
Rear-end collisions (from Worksheet 2D)	0.27	0.65	0.91
Head-on collisions (from Worksheet 2D)	0.02	0.06	0.08
Angle collisions (from Worksheet 2D)	0.29	0.65	0.94
Sideswipe (from Worksheet 2D)	0.01	0.17	0.18
Other multiple-vehicle collision (from Worksheet 2D)	0.00	0.00	0.00
Subtotal	0.59	1.53	2.12
	SINGLE-VEHICLE		
Collision with parked vehicle (from Worksheet 2F)	0.00	0.00	0.00
Collision with animal (from Worksheet 2F)	0.00	0.00	0.00
Collision with fixed object (from Worksheet 2F)	0.01	0.07	0.08
Collision with other object (from Worksheet 2F)	0.00	0.00	0.00
Other single-vehicle collision (from Worksheet 2F)	0.00	0.00	0.00
Single-vehicle noncollision (from Worksheet 2F)	0.00	0.00	0.00
Collision with pedestrian (from Worksheet 2G or 2I)	0.18	0.00	0.18
Collision with bicycle (from Worksheet 2J)	0.09	0.00	0.09
Subtotal	0.29	0.08	0.38
Total	0.88	1.62	2.49

Worksheet 2L Summary R	esults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)
	Total
Total	2.49
Fatal and injury (FI)	0.88
Property damage only (PDO)	1.62

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Worl	sheet 2A General Information and Input	Data for Urban and Suburban Arter	rial Intersections		
General Inform	nation	Location Information			
Analyst	Ben Erban	City	Milton		
Agency or Company	CTPS	Intersection	Route 138 at Bradlee Road and Atherton Street		
Date Performed	43221	Jurisdiction	MassDOT Highwat District 6		
		Analysis Year	2012		
Input Date	a	Base Conditions	Site Conditions		
Intersection type (3ST, 3SG, 4ST, 4SG)			4SG		
AADT _{major} (veh/day)	$AADT_{MAX} = 67700$ (veh/day)		12,500		
AADT _{minor} (veh/day)	$AAD1_{MAX} = 33400 (veh/day)$		1,850		
Intersection lighting (present/not present)		Not Present	Not Present		
Calibration factor, C _i		1	1		
Data for signalized intersections only:					
Number of approaches with left-turn lanes (0,1,2,3	,4) [for 3SG, use maximum value of 3]		0		
Number of approaches with right-turn lanes (0,1,2,	3,4) [for 3SG, use maximum value of 3]		0		
Number of approaches with left-turn signal phasing	g [for 3SG, use maximum value of 3]		1		
Type of left-turn signal phasing for Leg #1		Permissive	Protected		
Type of left-turn signal phasing for Leg #2			Permissive		
Type of left-turn signal phasing for Leg #3			Permissive		
Type of left-turn signal phasing for Leg #4 (if applic	cable)		Permissive		
Number of approaches with right-turn-on-red prohi	bited [for 3SG, use maximum value of 3]	0	0		
Intersection red light cameras (present/not present		Not Present	Not Present		
Sum of all pedestrian crossing volumes (PedVol)	Signalized intersections only		14		
Maximum number of lanes crossed by a pedestrial	n (n _{lanesx})		2		
Number of bus stops within 300 m (1,000 ft) of the	intersection	0	2		
Schools within 300 m (1,000 ft) of the intersection	(present/not present)	Not Present	Not Present		
Number of alcohol sales establishments within 300	m (1,000 ft) of the intersection	0	0		

	SUMMARY OF INTERSECTION & CRASHES										
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF ₄i	CMF 5i	CMF _{1p}	CMF _{2p}	CMF 3p	N _{bi_total}	$N_{\text{pedi_total}}$	N _{bikei_total}	N predictedint
1.000	0.940	1.000	1.000	1.000	2.780	1.000	1.000	2.177	0.173	0.092	2.442

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections							
(1)	(2)	(3)	(4)	(5)	(6)	(7)		
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF		
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}		
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)		
1.00	0.94	1.00	1.00	1.00	1.00	0.94		

		worksneet 2	C Multiple-	ehicle Collisions by Seve	erity Level for Urban	and Suburban Arterial in	tersections	1	T	
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
Crash Severity Level	SF	SPF Coefficients		Overdispersion		Proportion of Total	Adjusted	Combined	Calibration	Predicted
-				Parameter, k	Initial N _{bimv}	Crashes	N_{bimv}	CMFs	Factor, C _i	N_{bimv}
	fro	om Table 12-10)	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	110111 14510 12 10	21		(· / IOTAL (· /	Worksheet 2B		(0) (1) (0)
Total	-11.79	0.92	0.52	0.10	2.21	1.00	2.21	0.94	1.00	2.08
Fatal and Injury (FI)					0.61		0.61	0.94	1.00	0.58
i atai and injury (i i)					0.01		0.01	0.94	1.00	0.50
Property Damage Only (PDO)					1.59		1.59	0.94	1.00	1.50

	Worksheet 2D Multiple-V	ehicle Collisions by Collis	ion Type for Urban and Suburba	an Arterial Intersections	
(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C
Total	1.00	0.58	1.00	1.50	2.08
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Rear-end collision	0.45	0.26	0.42	0.63	0.89
Head-on collision	0.04	0.02	0.04	0.06	0.08
Angle collision	0.49	0.28	0.43	0.64	0.92
Sideswipe	0.02	0.01	0.11	0.17	0.18
Other multiple-vehicle collision	0.00	0.00	0.00	0.00	0.00

		Worksheet	2E Single-V	ehicle Collisions by Seve	ity Level for Urban a	and Suburban Arterial Int	ersections			
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
	SF	PF Coefficients	S	Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bisv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}
Crash Severity Level	fro	om Table 12-12	2				(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	from Table 1A	(3) * (4) From Worksheet 2C			Worksheet 2B		.,,,,,
Total				0.05	0.11	1.00	0.11	0.94	1.00	0.10
Fatal and Injury (FI)				0.18	0.02		0.02	0.94	1.00	0.02
Property Damage Only (PDO)			1	0.82	0.09		0.09	0.94	1.00	0.08

	Worksheet 2F Single-Ve	ehicle Collisions by Collisi	on Type for Urban and Suburba	n Arterial Intersections	_
(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv} (FI) (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.00	0.02	1.00	0.08	0.10
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.00	0.00	0.03	0.00	0.00
Collision with animal	0.00	0.00	0.03	0.00	0.00
Collision with fixed object	0.63	0.01	0.86	0.07	0.08
Collision with other object	0.00	0.00	0.06	0.00	0.00
Other single-vehicle collision	0.13	0.00	0.03	0.00	0.00
Single-vehicle noncollision	0.25	0.00	0.00	0.00	0.00

Worksheet 2G-1 Crash Modif	Worksheet 2G-1 Crash Modification Factors for Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Signalized Intersections							
(1)	(2)	(3)	(4)					
CMF for Bus Stops	CMF for Schools	CMF for Alcohol Sales Establishments	Combined CMF					
CMF _{1p}	CMF _{2p}	CMF _{3p}	Combined Civil					
from Table 12-28	from Table 12-29	from Table 12-30	(1)*(2)*(3)					
2.78	1.00	1.00	2.78					

	Worksheet 2G Ve	hicle-Pedestrian Collisions	for Urban and Su	burban Arterial Intersecti	ons		
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)
Crach Savarity Laval	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	- Veh-Ped ω	Needbass	Calibration	Predicted N _{pedi}
Crash Severity Level	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	ven-rea ω	Npedbase	factor, C _i	(4) from Worksheet 2H*(6)*(7)
Total	2.08	0.10	2.18	0.03	0.06	1.00	0.17
Fatal and injury (FI)		-				1.00	0.17

(1)	(2)	(3)	(4)	(5)	(6)	(7)
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration factor, C _i	Predicted N _{bikei}
Crash Seventy Level	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	ven-bike ω	Cambration factor, C _i	(4)*(5)*(6)
Total	2.08	0.10	2.18	0.04	1.00	0.09
Fatal and injury (FI)					1.00	0.09

Worksh	eet 2K Crash Severity Distribution for Urban and	d Suburban Arterial Intersections	
(1)	(2)	(3)	(4)
	Fatal and injury (FI)	Property damage only (PDO)	Total
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;
	(8) from Worksheet 2G and (7) from 2J		(8) from Worksheet 2G and (7) from 2J
	MULTIPLE-VEHICLE	•	•
Rear-end collisions (from Worksheet 2D)	0.26	0.63	0.89
Head-on collisions (from Worksheet 2D)	0.02	0.06	0.08
Angle collisions (from Worksheet 2D)	0.28	0.64	0.92
Sideswipe (from Worksheet 2D)	0.01	0.17	0.18
Other multiple-vehicle collision (from Worksheet 2D)	0.00	0.00	0.00
Subtotal	0.58	1.50	2.08
	SINGLE-VEHICLE		
Collision with parked vehicle (from Worksheet 2F)	0.00	0.00	0.00
Collision with animal (from Worksheet 2F)	0.00	0.00	0.00
Collision with fixed object (from Worksheet 2F)	0.01	0.07	0.08
Collision with other object (from Worksheet 2F)	0.00	0.00	0.00
Other single-vehicle collision (from Worksheet 2F)	0.00	0.00	0.00
Single-vehicle noncollision (from Worksheet 2F)	0.00	0.00	0.00
Collision with pedestrian (from Worksheet 2G or 2I)	0.17	0.00	0.17
Collision with bicycle (from Worksheet 2J)	0.09	0.00	0.09
Subtotal	0.28	0.08	0.37
Total	0.86	1.58	2.44

Worksheet 2L Summary R	esults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)
	Total
Total	2.44
Fatal and injury (FI)	0.86
Property damage only (PDO)	1.58

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We	orksheet 2A General Information and Input	Data for Urban and Suburban Arte	erial Intersections		
General Info	rmation		Location Information		
Analyst	Ben Erban	City	Milton		
Agency or Company	CTPS	Intersection	Route 138 at Bradlee Road and Atherton Street		
Date Performed	43221	Jurisdiction	MassDOT Highwat District 6		
		Analysis Year	2011		
Input D	ata	Base Conditions	Site Conditions		
ntersection type (3ST, 3SG, 4ST, 4SG)			4SG		
AADT _{major} (veh/day)	$AADT_{MAX} = 67700 (veh/day)$		12,400		
AADT _{minor} (veh/day)	$AADI_{MAX} = 33400 (veh/day)$		1,850		
Intersection lighting (present/not present)		Not Present	Not Present		
Calibration factor, C _i		1	1		
Data for signalized intersections only:					
Number of approaches with left-turn lanes (0,1,2	2,3,4) [for 3SG, use maximum value of 3]		0		
Number of approaches with right-turn lanes (0,1	,2,3,4) [for 3SG, use maximum value of 3]		0		
Number of approaches with left-turn signal phas	ing [for 3SG, use maximum value of 3]		1		
Type of left-turn signal phasing for Leg #1		Permissive	Protected		
Type of left-turn signal phasing for Leg #2			Permissive		
Type of left-turn signal phasing for Leg #3			Permissive		
Type of left-turn signal phasing for Leg #4 (if app	olicable)		Permissive		
Number of approaches with right-turn-on-red pro	hibited [for 3SG, use maximum value of 3]	0	0		
Intersection red light cameras (present/not prese		Not Present	Not Present		
Sum of all pedestrian crossing volumes (PedVo	l) Signalized intersections only		14		
Maximum number of lanes crossed by a pedesti	ian (n _{lanesx})		2		
Number of bus stops within 300 m (1,000 ft) of the	he intersection	0	2		
Schools within 300 m (1,000 ft) of the intersection	n (present/not present)	Not Present	Not Present		
Number of alcohol sales establishments within 3	00 m (1 000 ft) of the intersection	0	0		

	SUMMARY OF INTERSECTION & CRASHES										
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF ₄i	CMF 5i	CMF _{1p}	CMF _{2p}	CMF _{3p}	N _{bi_total}	N _{pedi_total}	N _{bikei_total}	N predictedint
1.000	0.940	1.000	1.000	1.000	2.780	1.000	1.000	2.161	0.172	0.092	2.424

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections							
(1)	(2)	(3)	(4)	(5)	(6)	(7)		
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF		
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}		
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)		
1.00	0.94	1.00	1.00	1.00	1.00	0.94		

	T	worksneet 2	C Multiple-	ehicle Collisions by Seve	erity Level for Orban	and Suburban Arterial in	tersections	T	1	
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
Crash Severity Level	SF	SPF Coefficients		Overdispersion		Proportion of Total	Adjusted	Combined	Calibration	Predicted
				Parameter, k	Initial N _{bimv}	Crashes	N_{bimv}	CMFs	Factor, C _i	N_{bimv}
	fro	om Table 12-10)	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	110111 14510 12 10	21		(· / IOTAL (· /	Worksheet 2B		(0) (1) (0)
Total	-11.79	0.92	0.52	0.10	2.19	1.00	2.19	0.94	1.00	2.06
Fatal and Injury (FI)					0.61		0.61	0.94	1.00	0.57
i atai and injury (i i)					0.01	==	0.01	0.94	1.00	0.57
Property Damage Only (PDO)					1.58		1.58	0.94	1.00	1.49

	Worksheet 2D Multiple-V	ehicle Collisions by Collis	ion Type for Urban and Suburba	an Arterial Intersections	
(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C
Total	1.00	0.57	1.00	1.49	2.06
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Rear-end collision	0.45	0.26	0.42	0.63	0.89
Head-on collision	0.04	0.02	0.04	0.06	0.08
Angle collision	0.49	0.28	0.43	0.63	0.91
Sideswipe	0.02	0.01	0.11	0.17	0.18
Other multiple-vehicle collision	0.00	0.00	0.00	0.00	0.00

(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
	SF	PF Coefficients	3	Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bisv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}
Crash Severity Level	fro	om Table 12-12	2	frame Table 4.6			(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	from Table 1A	(3) * (4) From Worksheet 2C			Worksheet 2B		
Total				0.05	0.11	1.00	0.11	0.94	1.00	0.10
Fatal and Injury (FI)				0.18	0.02		0.02	0.94	1.00	0.02
Property Damage Only (PDO)				0.82	0.09		0.09	0.94	1.00	0.08

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	Worksheet 2F Single-Ve	ehicle Collisions by Collisi	on Type for Urban and Suburba	n Arterial Intersections	_
(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv} (FI) (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.00	0.02	1.00	0.08	0.10
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.00	0.00	0.03	0.00	0.00
Collision with animal	0.00	0.00	0.03	0.00	0.00
Collision with fixed object	0.63	0.01	0.86	0.07	0.08
Collision with other object	0.00	0.00	0.06	0.00	0.00
Other single-vehicle collision	0.13	0.00	0.03	0.00	0.00
Single-vehicle noncollision	0.25	0.00	0.00	0.00	0.00

Worksheet 2G-1 Crash Modific	Worksheet 2G-1 Crash Modification Factors for Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Signalized Intersections						
(1)	(2)	(3)	(4)				
CMF for Bus Stops	CMF for Schools	CMF for Alcohol Sales Establishments	Combined CMF				
CMF _{1p}	CMF _{2p}	CMF _{3p}	Combined Civil				
from Table 12-28	from Table 12-29	from Table 12-30	(1)*(2)*(3)				
2.78	1.00	1.00	2.78				

	Worksheet 2G Ve	ehicle-Pedestrian Collisions	s for Urban and Su	burban Arterial Intersecti	ons			
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	
Creek Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	– Veh-Ped ω Npedbase	Veh-Ped ()	No. II acc	Calibration	Predicted N _{pedi}
Crash Severity Level	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)		Npedbase	factor, C _i	(4) from Worksheet 2H*(6)*(7)	
Total	2.06	0.10	2.16	0.03	0.06	1.00	0.17	
Fatal and injury (FI)						1.00	0.17	

	,						
(1)	(2)	(3)	(4)	(5)	(6)	(7)	
Crash Severity Level	Predicted N _{bimv}		Predicted N _{bi}	Veh-Bike ω	Calibration factor, C _i	Predicted N _{bikei}	
Crash Seventy Level	(9) from Worksheet 2C	(4) from Worksheet 2E	4) from Worksheet 2E (2) + (3)		Cambration factor, O	(4)*(5)*(6)	
Total	2.06	0.10	2.16	0.04	1.00	0.09	
Fatal and injury (FI)					1.00	0.09	

Worksh	eet 2K Crash Severity Distribution for Urban and	Suburban Arterial Intersections	
(1)	(2)	(3)	(4)
	Fatal and injury (FI)	Property damage only (PDO)	Total
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;
	(8) from Worksheet 2G and (7) from 2J		(8) from Worksheet 2G and (7) from 2J
	MULTIPLE-VEHICLE	•	•
Rear-end collisions (from Worksheet 2D)	0.26	0.63	0.89
Head-on collisions (from Worksheet 2D)	0.02	0.06	0.08
Angle collisions (from Worksheet 2D)	0.28	0.63	0.91
Sideswipe (from Worksheet 2D)	0.01	0.17	0.18
Other multiple-vehicle collision (from Worksheet 2D)	0.00	0.00	0.00
Subtotal	0.57	1.49	2.06
	SINGLE-VEHICLE		
Collision with parked vehicle (from Worksheet 2F)	0.00	0.00	0.00
Collision with animal (from Worksheet 2F)	0.00	0.00	0.00
Collision with fixed object (from Worksheet 2F)	0.01	0.07	0.08
Collision with other object (from Worksheet 2F)	0.00	0.00	0.00
Other single-vehicle collision (from Worksheet 2F)	0.00	0.00	0.00
Single-vehicle noncollision (from Worksheet 2F)	0.00	0.00	0.00
Collision with pedestrian (from Worksheet 2G or 2I)	0.17	0.00	0.17
Collision with bicycle (from Worksheet 2J)	0.09	0.00	0.09
Subtotal	0.28	0.08	0.36
Total	0.85	1.57	2.42

Worksheet 2L Summary Re	esults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)
	Total
Total	2.42
Fatal and injury (FI)	0.85
Property damage only (PDO)	1.57

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General Information	n	Locati	on Information
Analyst	Ben Erban	Intersection	Route 138 and Robbins Street
Agency or Company	CTPS	Intersection Type	4ST
Date Performed	May-18	Jurisdiction	MassDOT Highwat District 6
City	Milton	Analysis Year	2018

		Input Inf	ormation		
Year	Oserved MV crashes	Observed total crashes	Predicted MV crashes	Predicted total crashes	Combined CMF for veh-ped crashes
2015	1	1	1.02	1.05	1.00
2014	0	0	1.01	1.04	1.00
2013	1	1	1.01	1.04	1.00
2012	1	1	0.96	0.99	1.00
2011	2	2	0.96	0.99	1.00

					Outp	out Inform	ation				
Observed MV crashes	Average observed total crashes	Total predicted MV crashes	Average predicted total crashes	Standard deviation of predicted total crashes	Weight	Total expected MV crashes	No of expected total crashes	Average expected total crashes	INTERSECTION	Potential for Safety Improvement (PSI)	If avg observed total crashes > avg expected crashes
5.00	1.00	4.96	1.02	0.03	0.36	4.98	1.14 1.14 1.13 1.08 1.07	1.11	Y	0.09	W

	Workshe	et 2A General Informati	on and Input	Data for Urban and Suburban A	Arterial Interse	ections		
G	eneral Information	n		Location Information				
Analyst		Ben Erban		City				
Agency or Company		CTPS		Intersection		Route 138 and Robbins Street		
Date Performed		5/1/2018		Jurisdiction				
				Analysis Year		2015		
	Input Data			Base Conditions		Site Conditions		
Intersection type (3ST, 3SG, 4ST, 4SG)	_					4ST		
AADT _{major} (veh/day)		$AADT_{MAX} = 46800.00$	(veh/day)			10,250		
AADT _{minor} (veh/day)		$AADT_{MAX} = 5900.00$	(veh/day)			900		
Intersection lighting (present/not present)				Not Present		Not Present		
Calibration factor, C _i				1.00		1.04		
Data for unsignalized intersections only:								
Number of major-road approaches wi	0,1,2)			0				
Number of major-road approaches wi	th right-turn lanes	(0.1.2)	·			0		

		(SUMMARY OF	INTERSECTION	ON & CRASHES			
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF 4i	CMF 5i	N _{bi_total}	N_{pedi_total}	N _{bikei_total}	N predictedint
1.00	1.00	1.00	1.00	1.00	1.05	0.02	0.01	1.07

	Wo	rksheet 2B Crash Modifica	ation Factors for Urban and Sub	urban Arterial Intersectio	ns	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)
1.00	1.00	1.00	1.00	1.00	1.00	1.00

		Worksheet 2	C Multiple-	ehicle Collisions by Seve	rity Level for Urban	and Suburban Arterial In	tersections			
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
Crash Severity Level	SF	PF Coefficient	S	Overdispersion		Proportion of Total	Adjusted	Combined	Calibration	Predicted
				Parameter, k	Initial N _{bimv}	Crashes	N_{bimv}	CMFs	Factor, C _i	N_{bimv}
	fro	om Table 12-10)	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	Hom rable 12-10	21		(T)TOTAL (O)	Worksheet 2B		(0) (1) (0)
Total	-8.70	0.31	0.86	0.36	0.98	1.00	0.98	1.00	1.04	1.02
Fatal and Injury (FI)					0.37		0.37	1.00	1.04	0.38
r atai ana mjary (1 1)					0.01		0.07	1.00	1.01	0.00
Property Damage Only (PDO)					0.61		0.61	1.00	1.04	0.63

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(Fi)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C
Total	1.00	0.38	1.00	0.63	1.02
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Rear-end collision	0.16	0.06	0.19	0.12	0.18
Head-on collision	0.01	0.00	0.02	0.01	0.01
Angle collision	0.82	0.31	0.72	0.46	0.77
Sideswipe	0.02	0.01	0.08	0.05	0.06
Other multiple-vehicle collision	0.00	0.00	0.00	0.00	0.00

(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
	S	PF Coefficients	6	Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C	Predicted N _{bisv}
Crash Severity Level	fı	rom Table 12-12)				(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	from Table 1A	(3) * (4) From Worksheet 2C		()TOTAL ()	Worksheet 2B		.,,,,
Total				0.03	0.03	1.00	0.03	1.00	1.04	0.03
Fatal and Injury (FI)				0.30	0.01		0.01	1.00	1.04	0.01
Property Damage Only (PDO)				0.70	0.02		0.02	1.00	1.04	0.02

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N bisv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.00	0.01	1.00	0.02	0.03
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.00	0.00	0.14	0.00	0.00
Collision with animal	0.00	0.00	0.29	0.01	0.01
Collision with fixed object	0.68	0.01	0.57	0.01	0.02
Collision with other object	0.09	0.00	0.00	0.00	0.00
Other single-vehicle collision	0.05	0.00	0.00	0.00	0.00
Single-vehicle noncollision	0.18	0.00	0.00	0.00	0.00

	Worksheet 2G Vehicle-Pedestri	an Collisions for Urban and	Suburban Arteria	Stop-Controlled Interse	ctions	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh- Ped ω	Calibration	Predicted N _{pedi}
	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	from Table 1B	factor, C _i	(4)*(5)*(6)
Total	1.02	0.03	1.05	0.02	1.04	0.02
Fatal and injury (FI)					1.04	0.02

	Worksheet 2J Vehicle	e-Bicycle Collisions for Urba	an and Suburban A	Arterial Intersections		
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration	Predicted N _{bikei}
Crasii Severity Lever	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	(4) * (5)	factor, C _i	(4)*(5)*(6)
Total	1.02	0.03	1.05	0.01	1.04	0.01
Fatal and injury (FI)					1.04	0.01

(1)	(2)	(3)	(4)
	Fatal and injury (FI)	Property damage only (PDO)	Total
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;
	(7) from Worksheet 2G and 2J	, ,	(7) from Worksheet 2G and 2J
	MULTIPLE-VEHICLE	·	•
Rear-end collisions (from Worksheet 2D)	0.06	0.12	0.18
Head-on collisions (from Worksheet 2D)	0.00	0.01	0.01
Angle collisions (from Worksheet 2D)	0.31	0.46	0.77
Sideswipe (from Worksheet 2D)	0.01	0.05	0.06
Other multiple-vehicle collision (from Worksheet 2D)	0.00	0.00	0.00
Subtotal	0.38	0.63	1.02
	SINGLE-VEHICLE		
Collision with parked vehicle (from Worksheet 2F)	0.00	0.00	0.00
Collision with animal (from Worksheet 2F)	0.00	0.01	0.01
Collision with fixed object (from Worksheet 2F)	0.01	0.01	0.02
Collision with other object (from Worksheet 2F)	0.00	0.00	0.00
Other single-vehicle collision (from Worksheet 2F)	0.00	0.00	0.00
Single-vehicle noncollision (from Worksheet 2F)	0.00	0.00	0.00
Collision with pedestrian (from Worksheet 2G or 2I)	0.02	0.00	0.02
Collision with bicycle (from Worksheet 2J)	0.01	0.00	0.01
Subtotal	0.04	0.02	0.06
Total	0.42	0.66	1.07

Worksheet 2L Summary R	esults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)
	(Total) from Worksheet 2K
Total	1.07
Fatal and injury (FI)	0.42
Property damage only (PDO)	0.66

	Workshe	eet 2A General Informati	on and Input	Data for Urban and Suburban A	Arterial Interse	ections	
G	eneral Information	on		Location Information			
Analyst		Ben Erban		City			
Agency or Company		CTPS		Intersection		Route 138 and Robbins Street	
Date Performed		5/1/2018		Jurisdiction			
				Analysis Year		2014	
Input Data				Base Conditions	Site Conditions		
Intersection type (3ST, 3SG, 4ST, 4SG)	_					4ST	
AADT _{major} (veh/day)		$AADT_{MAX} = 46800.00$	(veh/day)			10,150	
AADT _{minor} (veh/day)		$AADT_{MAX} = 5900.00$	(veh/day)			900	
Intersection lighting (present/not present)	=			Not Present		Not Present	
Calibration factor, C _i	_	1.00		1.04			
Data for unsignalized intersections only:							
Number of major-road approaches wi	(0,1,2)		0				
Number of major-road approaches wi	th right-turn lanes	(0.1.2)				0	

	SUMMARY OF INTERSECTION & CRASHES									
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF 4i	CMF 5i	N _{bi_total}	N _{pedi_total}	N _{bikei_total}	N predictedint		
1.00	1.00	1.00	1.00	1.00	1.04	0.02	0.01	1.07		

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)				
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF				
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}				
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)				
1.00	1.00	1.00	1.00	1.00	1.00	1.00				

		Worksheet 2	C Multiple-	Vehicle Collisions by Seve	rity Level for Urban	and Suburban Arterial In	tersections			
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
Crash Severity Level	SF	SPF Coefficients		Overdispersion		Proportion of Total	Adjusted	Combined	Calibration	Predicted
				Parameter, k	Initial N _{bimv}	Crashes	N_{bimv}	CMFs	Factor, C _i	N_{bimv}
	fro	om Table 12-1)	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	Hom rable 12-10	21		(T)TOTAL (O)	Worksheet 2B		(0) (1) (0)
Total	-8.70	0.31	0.86	0.36	0.97	1.00	0.97	1.00	1.04	1.01
Fatal and Injury (FI)					0.37		0.37	1.00	1.04	0.38
r atai ana mjary (1 1)					0.01		0.07	1.00	1.01	0.00
Property Damage Only (PDO)					0.61		0.61	1.00	1.04	0.63

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(Fi)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C
Total	1.00	0.38	1.00	0.63	1.01
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Rear-end collision	0.16	0.06	0.19	0.12	0.18
Head-on collision	0.01	0.00	0.02	0.01	0.01
Angle collision	0.82	0.31	0.72	0.45	0.77
Sideswipe	0.02	0.01	0.08	0.05	0.06
Other multiple-vehicle collision	0.00	0.00	0.00	0.00	0.00

(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
	SI	PF Coefficients	s	Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}
Crash Severity Level	fr	om Table 12-12	2				(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	from Table 1A	(3) * (4) From Worksheet 2C		()TOTAL (-)	Worksheet 2B		(3) (1) (3)
Total				0.03	0.03	1.00	0.03	1.00	1.04	0.03
Fatal and Injury (FI)				0.30	0.01		0.01	1.00	1.04	0.01
Property Damage Only				0.70	0.02		0.02	1.00	1.04	0.02

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N bisv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.00	0.01	1.00	0.02	0.03
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.00	0.00	0.14	0.00	0.00
Collision with animal	0.00	0.00	0.29	0.01	0.01
Collision with fixed object	0.68	0.01	0.57	0.01	0.02
Collision with other object	0.09	0.00	0.00	0.00	0.00
Other single-vehicle collision	0.05	0.00	0.00	0.00	0.00
Single-vehicle noncollision	0.18	0.00	0.00	0.00	0.00

Works	sheet 2G Vehicle-Pedestri	an Collisions for Urban and	l Suburban Arterial	Stop-Controlled Interse	ctions	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh- Ped ω	Calibration	Predicted N _{pedi}
Crasii Severity Lever	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	from Table 1B	factor, C _i	(4)*(5)*(6)
Total	1.01	0.03	1.04	0.02	1.04	0.02
Fatal and injury (FI)					1.04	0.02

	Worksheet 2J Vehicle-Bicycle Collisions for Urban and Suburban Arterial Intersections							
(1)	(2)	(3)	(4)	(5)	(6)	(7)		
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration	Predicted N _{bikei}		
Crash Gevenly Level	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	(4) * (5)	factor, C _i	(4)*(5)*(6)		
Total	1.01	0.03	1.04	0.01	1.04	0.01		
Fatal and injury (FI)					1.04	0.01		

Worksh	eet 2K Crash Severity Distribution for Urban an	d Suburban Arterial Intersections	
(1)	(2)	(3)	(4)
	Fatal and injury (FI)	Property damage only (PDO)	Total
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;
	(7) from Worksheet 2G and 2J		(7) from Worksheet 2G and 2J
	MULTIPLE-VEHICLE		
Rear-end collisions (from Worksheet 2D)	0.06	0.12	0.18
Head-on collisions (from Worksheet 2D)	0.00	0.01	0.01
Angle collisions (from Worksheet 2D)	0.31	0.45	0.77
Sideswipe (from Worksheet 2D)	0.01	0.05	0.06
Other multiple-vehicle collision (from Worksheet 2D)	0.00	0.00	0.00
Subtotal	0.38	0.63	1.01
	SINGLE-VEHICLE		
Collision with parked vehicle (from Worksheet 2F)	0.00	0.00	0.00
Collision with animal (from Worksheet 2F)	0.00	0.01	0.01
Collision with fixed object (from Worksheet 2F)	0.01	0.01	0.02
Collision with other object (from Worksheet 2F)	0.00	0.00	0.00
Other single-vehicle collision (from Worksheet 2F)	0.00	0.00	0.00
Single-vehicle noncollision (from Worksheet 2F)	0.00	0.00	0.00
Collision with pedestrian (from Worksheet 2G or 2I)	0.02	0.00	0.02
Collision with bicycle (from Worksheet 2J)	0.01	0.00	0.01
Subtotal	0.04	0.02	0.06
Total	0.42	0.65	1.07

Worksheet 2L Summary Ro	esults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)
	(Total) from Worksheet 2K
Total	1.07
Fatal and injury (FI)	0.42
Property damage only (PDO)	0.65

	Worksheet 2A General Information and Input	t Data for Orban and Suburban /			
Ge	neral Information	Location Information			
Analyst	Ben Erban	City			
Agency or Company	CTPS	Intersection	Route 138 and Robbins Street		
Date Performed	5/1/2018	Jurisdiction			
	·	Analysis Year	2013		
	Input Data	Base Conditions	Site Conditions		
ntersection type (3ST, 3SG, 4ST, 4SG)			4ST		
AADT _{major} (veh/day)	$AADT_{MAX} = 46800.00 \text{ (veh/day)}$		10,050		
AADT _{minor} (veh/day)	AADT _{MAX} = 5900.00 (veh/day)		900		
ntersection lighting (present/not present)		Not Present	Not Present		
Calibration factor, C _i		1.00	1.04		
Data for unsignalized intersections only:					
Number of major-road approaches wit	n left-turn lanes (0,1,2)		0		
Number of major-road approaches wit	n right-turn lanes (0.1.2)		0		

	SUMMARY OF INTERSECTION & CRASHES							
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF 4i	CMF 5i	N _{bi_total}	N _{pedi_total}	N _{bikei_total}	N predictedint
1.00	1.00	1.00	1.00	1.00	1.04	0.02	0.01	1.07

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections					
(1)	(2)	(3)	(4)	(5)	(6)	(7)
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)
1.00	1.00	1.00	1.00	1.00	1.00	1.00

		Worksheet 2	C Multiple-	Vehicle Collisions by Seve	rity Level for Urban	and Suburban Arterial In	tersections			
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
Crash Severity Level	SF	PF Coefficient	S	Overdispersion		Proportion of Total	Adjusted	Combined	Calibration	Predicted
				Parameter, k	Initial N _{bimv}	Crashes	N_{bimv}	CMFs	Factor, C _i	N_{bimv}
	fro	om Table 12-1)	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	Hom rable 12-10	21		(T)TOTAL (O)	Worksheet 2B		(0) (1) (0)
Total	-8.70	0.31	0.86	0.36	0.97	1.00	0.97	1.00	1.04	1.01
Fatal and Injury (FI)					0.37		0.37	1.00	1.04	0.38
r atai ana mjary (1 1)					0.01		0.07	1.00	1.01	0.00
Property Damage Only (PDO)					0.61		0.61	1.00	1.04	0.63

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(Fi)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C
Total	1.00	0.38	1.00	0.63	1.01
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Rear-end collision	0.16	0.06	0.19	0.12	0.18
Head-on collision	0.01	0.00	0.02	0.01	0.01
Angle collision	0.82	0.31	0.72	0.45	0.76
Sideswipe	0.02	0.01	0.08	0.05	0.06
Other multiple-vehicle collision	0.00	0.00	0.00	0.00	0.00

(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
, ,	SF	PF Coefficient	s	Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C	Predicted N _{bisv}
Crash Severity Level	fro	om Table 12-12	2		(-) (() -		(4) _{TOTAL} *(5)	(7) from	, ,	(6)*(7)*(8)
	а	b	С	from Table 1A	(3) * (4) From Worksheet 2C		()TOTAL ()	Worksheet 2B		(-) () (-)
Γotal				0.03	0.03	1.00	0.03	1.00	1.04	0.03
atal and Injury (FI)				0.30	0.01		0.01	1.00	1.04	0.01
Property Damage Only PDO)				0.70	0.02		0.02	1.00	1.04	0.02

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(Fi)	Predicted N _{bisv} (FI) (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.00	0.01	1.00	0.02	0.03
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.00	0.00	0.14	0.00	0.00
Collision with animal	0.00	0.00	0.29	0.01	0.01
Collision with fixed object	0.68	0.01	0.57	0.01	0.02
Collision with other object	0.09	0.00	0.00	0.00	0.00
Other single-vehicle collision	0.05	0.00	0.00	0.00	0.00
Single-vehicle noncollision	0.18	0.00	0.00	0.00	0.00

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Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Stop-Controlled Intersections						
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh- Ped ω	Calibration	Predicted N _{pedi}
Crasii Severity Level	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	from Table 1B	factor, C _i	(4)*(5)*(6)
Total	1.01	0.03	1.04	0.02	1.04	0.02
atal and injury (FI)					1.04	0.02

	Worksheet 2J Vehicle-Bicycle Collisions for Urban and Suburban Arterial Intersections						
(1)	(2)	(3)	(4)	(5)	(6)	(7)	
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration	Predicted N _{bikei}	
Crasii Severity Lever	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	(4) * (5)	factor, C _i	(4)*(5)*(6)	
Total	1.01	0.03	1.04	0.01	1.04	0.01	
Fatal and injury (FI)					1.04	0.01	

Worksheet 2K Crash Severity Distribution for Urban and Suburban Arterial Intersections						
(1)	(2)	(3)	(4)			
	Fatal and injury (FI)	Property damage only (PDO)	Total			
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;			
	(7) from Worksheet 2G and 2J		(7) from Worksheet 2G and 2J			
	MULTIPLE-VEHICLE					
Rear-end collisions (from Worksheet 2D)	0.06	0.12	0.18			
Head-on collisions (from Worksheet 2D)	0.00	0.01	0.01			
Angle collisions (from Worksheet 2D)	0.31	0.45	0.76			
Sideswipe (from Worksheet 2D)	0.01	0.05	0.06			
Other multiple-vehicle collision (from Worksheet 2D)	0.00	0.00	0.00			
Subtotal	0.38	0.63	1.01			
	SINGLE-VEHICLE					
Collision with parked vehicle (from Worksheet 2F)	0.00	0.00	0.00			
Collision with animal (from Worksheet 2F)	0.00	0.01	0.01			
Collision with fixed object (from Worksheet 2F)	0.01	0.01	0.02			
Collision with other object (from Worksheet 2F)	0.00	0.00	0.00			
Other single-vehicle collision (from Worksheet 2F)	0.00	0.00	0.00			
Single-vehicle noncollision (from Worksheet 2F)	0.00	0.00	0.00			
Collision with pedestrian (from Worksheet 2G or 2I)	0.02	0.00	0.02			
Collision with bicycle (from Worksheet 2J)	0.01	0.00	0.01			
Subtotal	0.04	0.02	0.06			
Total	0.42	0.65	1.07			

Worksheet 2L Summary Ro	esults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)
	(Total) from Worksheet 2K
Total	1.07
Fatal and injury (FI)	0.42
Property damage only (PDO)	0.65

	Workshe	et 2A General Informati	on and Input	Data for Urban and Suburban A	Arterial Interse	ections		
G	eneral Informatio	on		Location Information				
Analyst		Ben Erban		City				
Agency or Company		CTPS		Intersection		Route 138 and Robbins Street		
Date Performed		5/1/2018		Jurisdiction				
				Analysis Year		2012		
Input Data			Base Conditions	Site Conditions				
Intersection type (3ST, 3SG, 4ST, 4SG)	_					4ST		
AADT _{major} (veh/day)		$AADT_{MAX} = 46800.00$	(veh/day)			10,000		
AADT _{minor} (veh/day)		$AADT_{MAX} = 5900.00$	(veh/day)			850		
Intersection lighting (present/not present)				Not Present		Not Present		
Calibration factor, C _i			1.00		1.04			
Data for unsignalized intersections only:								
Number of major-road approaches w	ith left-turn lanes ((0,1,2)			0			
Number of major-road approaches w	ith right-turn lanes	(0.1.2)	·			0		

	SUMMARY OF INTERSECTION & CRASHES									
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF 4i	CMF 5i	N _{bi_total}	N_{pedi_total}	N bikei_total	N predictedint		
1.00	1.00	1.00	1.00	1.00	0.99	0.02	0.01	1.02		

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)				
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF				
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}				
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)				
1.00	1.00	1.00	1.00	1.00	1.00	1.00				

		Worksheet 2	C Multiple-	Vehicle Collisions by Seve	rity Level for Urban	and Suburban Arterial In	tersections			
(1)) (2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)	
Crash Severity Level	SPF Coefficients		Overdispersion		Proportion of Total	Adjusted	Combined	Calibration	Predicted	
				Parameter, k	Initial N _{bimv}	Crashes	N_{bimv}	CMFs	Factor, C _i	N_{bimv}
	from Table 12-10		from Table 12-10 from Equation 12-		(4) *(5)	$(4)_{TOTAL}^*(5)$ (7) from		(6)*(7)*(8)		
	а	b	С	Hom rable 12-10	21		(T)TOTAL (O)	Worksheet 2B		(0) (1) (0)
Total	-8.70	0.31	0.86	0.36	0.92	1.00	0.92	1.00	1.04	0.96
Fatal and Injury (FI)					0.35		0.35	1.00	1.04	0.36
ratarana nijary (11)					0.00		0.00	1.00	1.01	0.00
Property Damage Only (PDO)					0.58		0.58	1.00	1.04	0.60

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(Fi)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C
Total	1.00	0.36	1.00	0.60	0.96
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Rear-end collision	0.16	0.06	0.19	0.11	0.17
Head-on collision	0.01	0.00	0.02	0.01	0.01
Angle collision	0.82	0.30	0.72	0.43	0.73
Sideswipe	0.02	0.01	0.08	0.05	0.05
Other multiple-vehicle collision	0.00	0.00	0.00	0.00	0.00

(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
, ,	SF	PF Coefficient	s	Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C	Predicted N _{bisv}
Crash Severity Level	fro	om Table 12-12	2		(-) (() -		(4) _{TOTAL} *(5)	(7) from	, ,	(6)*(7)*(8)
	а	b	С	from Table 1A	(3) * (4) From Worksheet 2C		()TOTAL ()	Worksheet 2B		(-) () (-)
Γotal				0.03	0.03	1.00	0.03	1.00	1.04	0.03
atal and Injury (FI)				0.30	0.01		0.01	1.00	1.04	0.01
Property Damage Only PDO)				0.70	0.02		0.02	1.00	1.04	0.02

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N bisv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.00	0.01	1.00	0.02	0.03
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.00	0.00	0.14	0.00	0.00
Collision with animal	0.00	0.00	0.29	0.01	0.01
Collision with fixed object	0.68	0.01	0.57	0.01	0.02
Collision with other object	0.09	0.00	0.00	0.00	0.00
Other single-vehicle collision	0.05	0.00	0.00	0.00	0.00
Single-vehicle noncollision	0.18	0.00	0.00	0.00	0.00

Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Stop-Controlled Intersections							
(1)	(2)	(3)	(4)	(5)	(6)	(7)	
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh- Ped ω	Calibration	Predicted N _{pedi}	
	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	from Table 1B	factor, C _i	(4)*(5)*(6)	
Total	0.96	0.03	0.99	0.02	1.04	0.02	
Fatal and injury (FI)					1.04	0.02	

	Worksheet 2J Vehicle	e-Bicycle Collisions for Urba	an and Suburban A	Arterial Intersections		
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration	Predicted N _{bikei}
Crash Seventy Level	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	(4) * (5)	factor, C _i	(4)*(5)*(6)
Total	0.96	0.03	0.99	0.01	1.04	0.01
Fatal and injury (FI)					1.04	0.01

Worksh	eet 2K Crash Severity Distribution for Urban an	d Suburban Arterial Intersections	
(1)	(2)	(3)	(4)
	Fatal and injury (FI)	Property damage only (PDO)	Total
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;
	(7) from Worksheet 2G and 2J		(7) from Worksheet 2G and 2J
	MULTIPLE-VEHICLE		
Rear-end collisions (from Worksheet 2D)	0.06	0.11	0.17
Head-on collisions (from Worksheet 2D)	0.00	0.01	0.01
Angle collisions (from Worksheet 2D)	0.30	0.43	0.73
Sideswipe (from Worksheet 2D)	0.01	0.05	0.05
Other multiple-vehicle collision (from Worksheet 2D)	0.00	0.00	0.00
Subtotal	0.36	0.60	0.96
	SINGLE-VEHICLE		
Collision with parked vehicle (from Worksheet 2F)	0.00	0.00	0.00
Collision with animal (from Worksheet 2F)	0.00	0.01	0.01
Collision with fixed object (from Worksheet 2F)	0.01	0.01	0.02
Collision with other object (from Worksheet 2F)	0.00	0.00	0.00
Other single-vehicle collision (from Worksheet 2F)	0.00	0.00	0.00
Single-vehicle noncollision (from Worksheet 2F)	0.00	0.00	0.00
Collision with pedestrian (from Worksheet 2G or 2I)	0.02	0.00	0.02
Collision with bicycle (from Worksheet 2J)	0.01	0.00	0.01
Subtotal	0.03	0.02	0.05
Total	0.40	0.62	1.02

Worksheet 2L Summary R	lesults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)
	(Total) from Worksheet 2K
Total	1.02
Fatal and injury (FI)	0.40
Property damage only (PDO)	0.62

	Worksheet 2A General Information and Input	Data for Urban and Suburban A	Arterial Interse	ections	
Ge	neral Information	Location Information			
Analyst	Ben Erban	City			
Agency or Company	CTPS	Intersection		Route 138 and Robbins Street	
Date Performed	5/1/2018	Jurisdiction			
				2011	
	Base Conditions		Site Conditions		
Intersection type (3ST, 3SG, 4ST, 4SG)				4ST	
AADT _{major} (veh/day)	$AADT_{MAX} = 46800.00 \text{ (veh/day)}$			9,850	
AADT _{minor} (veh/day)	AADT _{MAX} = 5900.00 (veh/day)			850	
Intersection lighting (present/not present)		Not Present		Not Present	
Calibration factor, C _i		1.00		1.04	
Data for unsignalized intersections only:					
Number of major-road approaches with	n left-turn lanes (0,1,2)			0	
Number of major-road approaches with	right-turn lanes (0,1,2)			0	

SUMMARY OF INTERSECTION & CRASHES								
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF 4i	CMF 5i	N _{bi_total}	N _{pedi_total}	N _{bikei_total}	N predictedint
1.00	1.00	1.00	1.00	1.00	0.99	0.02	0.01	1.01

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections							
(1)	(2)	(3)	(4)	(5)	(6)	(7)		
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF		
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}		
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)		
1.00	1.00	1.00	1.00	1.00	1.00	1.00		

		Worksheet 2	C Multiple-	ehicle Collisions by Seve	rity Level for Urban	and Suburban Arterial In	tersections			
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
Crash Severity Level	SF	PF Coefficient	s	Overdispersion		Proportion of Total	Adjusted	Combined	Calibration	Predicted
				Parameter, k	Initial N _{bimv}	Crashes	N_{bimv}	CMFs	Factor, C _i	N _{bimv}
	fro	om Table 12-1)	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	Hom rable 12-10	21		(T)TOTAL (O)	Worksheet 2B		(0) (1) (0)
Total	-8.70	0.31	0.86	0.36	0.92	1.00	0.92	1.00	1.04	0.96
Fatal and Injury (FI)					0.35		0.35	1.00	1.04	0.36
ratarana nijary (11)					0.00		0.00	1.00	1.01	0.00
Property Damage Only (PDO)					0.57		0.57	1.00	1.04	0.60

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(Fi)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C
Total	1.00	0.36	1.00	0.60	0.96
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Rear-end collision	0.16	0.06	0.19	0.11	0.17
Head-on collision	0.01	0.00	0.02	0.01	0.01
Angle collision	0.82	0.29	0.72	0.43	0.72
Sideswipe	0.02	0.01	0.08	0.05	0.05
Other multiple-vehicle collision	0.00	0.00	0.00	0.00	0.00

(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
	SF	PF Coefficient	s	Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C	Predicted N _{bisv}
Crash Severity Level	fro	om Table 12-12	2		(-) (() -		(4) _{TOTAL} *(5)	(7) from	, ,	(6)*(7)*(8)
	а	b c	from Table 1A	(3) * (4) From Worksheet 2C		V / TOTAL V /	Worksheet 2B		(-) () (-)	
Γotal				0.03	0.03	1.00	0.03	1.00	1.04	0.03
atal and Injury (FI)				0.30	0.01		0.01	1.00	1.04	0.01
Property Damage Only PDO)				0.70	0.02		0.02	1.00	1.04	0.02

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N bisv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.00	0.01	1.00	0.02	0.03
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.00	0.00	0.14	0.00	0.00
Collision with animal	0.00	0.00	0.29	0.01	0.01
Collision with fixed object	0.68	0.01	0.57	0.01	0.02
Collision with other object	0.09	0.00	0.00	0.00	0.00
Other single-vehicle collision	0.05	0.00	0.00	0.00	0.00
Single-vehicle noncollision	0.18	0.00	0.00	0.00	0.00

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Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Stop-Controlled Intersections							
(1)	(2)	(3)	(4)	(5)	(6)	(7)	
Crach Soverity Lavel	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh- Ped ω	Calibration	Predicted N _{pedi}	
Crash Severity Level	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	from Table 1B	factor, C _i	(4)*(5)*(6)	
Total	0.96	0.03	0.99	0.02	1.04	0.02	
atal and injury (FI)					1.04	0.02	

Worksheet 2J Vehicle-Bicycle Collisions for Urban and Suburban Arterial Intersections								
(1)	(2)	(3)	(4)	(5)	(6)	(7)		
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration	Predicted N _{bikei}		
Crash Severity Level	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	(4) * (5)	factor, C _i	(4)*(5)*(6)		
Total	0.96	0.03	0.99	0.01	1.04	0.01		
Fatal and injury (FI)					1.04	0.01		

Worksh	eet 2K Crash Severity Distribution for Urban an	d Suburban Arterial Intersections	
(1)	(2)	(3)	(4)
	Fatal and injury (FI)	Property damage only (PDO)	Total
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;
	(7) from Worksheet 2G and 2J		(7) from Worksheet 2G and 2J
	MULTIPLE-VEHICLE		
Rear-end collisions (from Worksheet 2D)	0.06	0.11	0.17
Head-on collisions (from Worksheet 2D)	0.00	0.01	0.01
Angle collisions (from Worksheet 2D)	0.29	0.43	0.72
Sideswipe (from Worksheet 2D)	0.01	0.05	0.05
Other multiple-vehicle collision (from Worksheet 2D)	0.00	0.00	0.00
Subtotal	0.36	0.60	0.96
	SINGLE-VEHICLE		
Collision with parked vehicle (from Worksheet 2F)	0.00	0.00	0.00
Collision with animal (from Worksheet 2F)	0.00	0.01	0.01
Collision with fixed object (from Worksheet 2F)	0.01	0.01	0.02
Collision with other object (from Worksheet 2F)	0.00	0.00	0.00
Other single-vehicle collision (from Worksheet 2F)	0.00	0.00	0.00
Single-vehicle noncollision (from Worksheet 2F)	0.00	0.00	0.00
Collision with pedestrian (from Worksheet 2G or 2I)	0.02	0.00	0.02
Collision with bicycle (from Worksheet 2J)	0.01	0.00	0.01
Subtotal	0.03	0.02	0.05
Total	0.39	0.62	1.01

Worksheet 2L Summary Results for Urban and Suburban Arterial Intersections					
(1)	(2)				
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)				
	(Total) from Worksheet 2K				
Total	1.01				
Fatal and injury (FI)	0.39				
Property damage only (PDO)	0.62				

General Information	n	Location Information				
Analyst	Ben Erban	Intersection	138 at Blue HIII Terrace Street and Cheever			
Agency or Company	CTPS	Intersection Type	4SG			
Date Performed	May-18	Jurisdiction	MassDOT Highwat District 6			
City	City Milton		2018			

		Input Inf	ormation		
Year	Oserved MV crashes	Observed total crashes	Predicted MV crashes	Predicted total crashes	Combined CMF for veh-ped crashes
2015	3	3	1.56	1.84	2.78
2014	2	2	1.55	1.82	2.78
2013	0	0	1.53	1.80	2.78
2012	4	4	1.50	1.77	2.78
2011	1	1	1.46	1.72	2.78

	Output Information											
Observed MV crashes	Average observed total crashes	Total predicted MV crashes	Average predicted total crashes	Standard deviation of predicted total crashes	Weight	Total expected MV crashes	No of expected total crashes	Average expected total crashes	High-risk Intersection (Y/N)	Safety	If avg observed total crashes > avg expected crashes	
10.00	2.00	7.61	1.79	0.05	0.58	8.61	2.04 2.02 2.00 1.96 1.91	1.09	Y	0.20	Y	

General Informati	on		Location Information
Analyst	Ben Erban	City	Milton
Agency or Company	CTPS	Intersection	Route 138 at Blue HIII Terrace Street and Cheever Street
Date Performed	5/1/2018	Jurisdiction	MassDOT Highwat District 6
		Analysis Year	2015
Input Data		Base Conditions	Site Conditions
Intersection type (3ST, 3SG, 4ST, 4SG)			4SG
AADT _{major} (veh/day)	$AADT_{MAX} = 67,700 (veh/day)$		10,250
AADT _{minor} (veh/day)	$AADI_{MAX} = 33,400 (veh/day)$		1,850
Intersection lighting (present/not present)		Not Present	Not Present
Calibration factor, C _i		1.00	1.00
Data for signalized intersections only:			
Number of approaches with left-turn lanes (0,1,2,3,4)	[for 3SG, use maximum value of 3]		0
Number of approaches with right-turn lanes (0,1,2,3,4) [for 3SG, use maximum value of 3]		0
Number of approaches with left-turn signal phasing [fo	or 3SG, use maximum value of 3]		2
Type of left-turn signal phasing for Leg #1		Permissive	Permissive
Type of left-turn signal phasing for Leg #2			Permissive
Type of left-turn signal phasing for Leg #3			Protected
Type of left-turn signal phasing for Leg #4 (if applicable	le)		Protected
Number of approaches with right-turn-on-red prohibite	d [for 3SG, use maximum value of 3]	0	2
Intersection red light cameras (present/not present)		Not Present	Not Present
Sum of all pedestrian crossing volumes (PedVol) S	ignalized intersections only		0
Maximum number of lanes crossed by a pedestrian (r			2
Number of bus stops within 300 m (1,000 ft) of the inter-	ersection	0	2
Schools within 300 m (1,000 ft) of the intersection (pre	esent/not present)	Not Present	Not Present
Number of alcohol sales establishments within 300 m	(1.000 ft) of the intersection	0	0

	SUMMARY OF INTERSECTION & CRASHES										
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF ₄i	CMF 5i	CMF _{1p}	CMF _{2p}	CMF _{3p}	N _{bi_total}	N _{pedi_total}	N _{bikei_total}	N predictedint
1.000	0.884	1.000	0.960	1.000	2.780	1.000	1.000	1.638	0.130	0.069	1.838

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections										
(1)	(2)	(3)	(4)	(5)	(6)	(7)					
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF					
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}					
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)					
1.00	0.88	1.00	0.96	1.00	1.00	0.85					

	1	worksneet 2	C Multiple-	ehicle Collisions by Seve	rity Level for Urban	and Suburban Arterial in	tersections		_	
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
Crash Severity Level	SPF Coefficients		Overdispersion Parameter, k	Initial N _{bimv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bimv}	
	fro	om Table 12-10)	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	110111 14510 12 10	21		() TOTAL ()	Worksheet 2B		(6) (1) (6)
Total	-11.79	0.92	0.52	0.10	1.84	1.00	1.84	0.85	1.00	1.56
Fatal and Injury (FI)					0.51		0.51	0.85	1.00	0.43
i atai and injury (i i)					0.51		0.51	0.65	1.00	0.43
Property Damage Only (PDO)					1.33		1.33	0.85	1.00	1.13

	Worksheet 2D Multiple-V	ehicle Collisions by Collis	ion Type for Urban and Suburba	an Arterial Intersections	
(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C
Total	1.00	0.43	1.00	1.13	1.56
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Rear-end collision	0.45	0.20	0.42	0.48	0.67
Head-on collision	0.04	0.02	0.04	0.04	0.06
Angle collision	0.49	0.21	0.43	0.48	0.69
Sideswipe	0.02	0.01	0.11	0.13	0.14
Other multiple-vehicle collision	0.00	0.00	0.00	0.00	0.00

	Worksheet 2E Single-Vehicle Collisions by Severity Level for Urban and Suburban Arterial Intersections											
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)		
	SPF Coefficients His		Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bisv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}			
Crash Severity Level	fro	om Table 12-12	2	ć -			(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)		
	а	b	С	from Table 1A	(3) * (4) From Worksheet 2C			Worksheet 2B		.,,,,,		
Total				0.05	0.09	1.00	0.09	0.85	1.00	0.08		
Fatal and Injury (FI)				0.18	0.02		0.02	0.85	1.00	0.01		
Property Damage Only (PDO)			1	0.82	0.07		0.07	0.85	1.00	0.06		

	Worksheet 2F Single-Ve	ehicle Collisions by Collision	on Type for Urban and Suburba	n Arterial Intersections	
(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N bisv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.00	0.01	1.00	0.06	0.08
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.00	0.00	0.03	0.00	0.00
Collision with animal	0.00	0.00	0.03	0.00	0.00
Collision with fixed object	0.63	0.01	0.86	0.05	0.06
Collision with other object	0.00	0.00	0.06	0.00	0.00
Other single-vehicle collision	0.13	0.00	0.03	0.00	0.00
Single-vehicle noncollision	0.25	0.00	0.00	0.00	0.00

Worksheet 2G-1 Crash Modific	Worksheet 2G-1 Crash Modification Factors for Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Signalized Intersections								
(1)	(2)	(3)	(4)						
CMF for Bus Stops	CMF for Schools	CMF for Alcohol Sales Establishments	Combined CMF						
CMF _{1p}	CMF _{2p}	CMF _{3p}	Combined Civil						
from Table 12-28	from Table 12-29	from Table 12-30	(1)*(2)*(3)						
2.78	1.00	1.00	2.78						

	Worksheet 2G Ve	ehicle-Pedestrian Collisions	for Urban and Su	burban Arterial Intersecti	ons		
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)
Crack Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	- Veh-Ped ω	Needbass	Calibration	Predicted N _{pedi}
Crash Severity Level	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	ven-rea ω	Npedbase	factor, C _i	(4) from Worksheet 2H*(6)*(7)
Total	1.56	0.08	1.64	0.03	0.05	1.00	0.13
Fatal and injury (FI)						1.00	0.13

(1)	(2)	(3)	(4)	(5)	(6)	(7)
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration factor, C	Predicted N _{bikei}
	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	Veii-bike ω	(4)*(5)*(6)	
Total	1.56	0.08	1.64	0.04	1.00	0.07
Fatal and injury (FI)					1.00	0.07

Worksh	Worksheet 2K Crash Severity Distribution for Urban and Suburban Arterial Intersections							
(1)	(2)	(3)	(4)					
	Fatal and injury (FI)	Property damage only (PDO)	Total					
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;					
	(8) from Worksheet 2G and (7) from 2J		(8) from Worksheet 2G and (7) from 2J					
	MULTIPLE-VEHICLE	•	•					
Rear-end collisions (from Worksheet 2D)	0.20	0.48	0.67					
Head-on collisions (from Worksheet 2D)	0.02	0.04	0.06					
Angle collisions (from Worksheet 2D)	0.21	0.48	0.69					
Sideswipe (from Worksheet 2D)	0.01	0.13	0.14					
Other multiple-vehicle collision (from Worksheet 2D)	0.00	0.00	0.00					
Subtotal	0.43	1.13	1.56					
	SINGLE-VEHICLE							
Collision with parked vehicle (from Worksheet 2F)	0.00	0.00	0.00					
Collision with animal (from Worksheet 2F)	0.00	0.00	0.00					
Collision with fixed object (from Worksheet 2F)	0.01	0.05	0.06					
Collision with other object (from Worksheet 2F)	0.00	0.00	0.00					
Other single-vehicle collision (from Worksheet 2F)	0.00	0.00	0.00					
Single-vehicle noncollision (from Worksheet 2F)	0.00	0.00	0.00					
Collision with pedestrian (from Worksheet 2G or 2I)	0.13	0.00	0.13					
Collision with bicycle (from Worksheet 2J)	0.07	0.00	0.07					
Subtotal	0.21	0.06	0.28					
Total	0.65	1.19	1.84					

Worksheet 2L Summary Re	Worksheet 2L Summary Results for Urban and Suburban Arterial Intersections					
(1)	(2)					
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)					
	Total					
Total	1.84					
Fatal and injury (FI)	0.65					
Property damage only (PDO)	1.19					

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General Informat	ion	Location Information				
Analyst	Ben Erban	City	Milton			
Agency or Company	CTPS	Intersection	Route 138 at Blue HIII Terrace Street and Cheever Street			
Date Performed	5/1/2018	Jurisdiction	MassDOT Highwat District 6			
		Analysis Year	2014			
Input Data		Base Conditions	Site Conditions			
Intersection type (3ST, 3SG, 4ST, 4SG)			4SG			
AADT _{major} (veh/day)	$AADT_{MAX} = 67,700$ (veh/day)		10,150			
AADT _{minor} (veh/day)	$AADI_{MAX} = 33,400 \text{(veh/day)}$		1,850			
Intersection lighting (present/not present)		Not Present	Not Present			
Calibration factor, C _i		1.00	1.00			
Data for signalized intersections only:						
Number of approaches with left-turn lanes (0,1,2,3,4)	[for 3SG, use maximum value of 3]		0			
Number of approaches with right-turn lanes (0,1,2,3,4) [for 3SG, use maximum value of 3]		0			
Number of approaches with left-turn signal phasing [for	or 3SG, use maximum value of 3]		2			
Type of left-turn signal phasing for Leg #1		Permissive	Permissive			
Type of left-turn signal phasing for Leg #2			Permissive			
Type of left-turn signal phasing for Leg #3			Protected			
Type of left-turn signal phasing for Leg #4 (if applicab	le)		Protected			
Number of approaches with right-turn-on-red prohibite	ed [for 3SG, use maximum value of 3]	0	2			
Intersection red light cameras (present/not present)		Not Present	Not Present			
Sum of all pedestrian crossing volumes (PedVol) S	signalized intersections only		0			
Maximum number of lanes crossed by a pedestrian (r	N _{lanesx})		2			
Number of bus stops within 300 m (1,000 ft) of the int	ersection	0	2			
Schools within 300 m (1,000 ft) of the intersection (pro	esent/not present)	Not Present	Not Present			
Number of alcohol sales establishments within 300 m	(1,000 ft) of the intersection	0	0			

	SUMMARY OF INTERSECTION & CRASHES										
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF ₄i	CMF 5i	CMF _{1p}	CMF _{2p}	CMF _{3p}	N _{bi_total}	N _{pedi_total}	N _{bikei_total}	N predictedint
1.000											

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections									
(1) (2) (3) (4) (5) (6) (7)										
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF				
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}				
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)				
1.00	0.88	1.00	0.96	1.00	1.00	0.85				

	Worksheet 2C Multiple-Vehicle Collisions by Severity Level for Urban and Suburban Arterial Intersections									
(1)	(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)	
Crash Severity Level	SPF Coefficients		Overdispersion Parameter, k	Initial N _{bimv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bimv}	
	fro	om Table 12-10)	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	Hem radio 12 10	21		(')TOTAL (')	Worksheet 2B		(0) (1) (0)
Total	-11.79	0.92	0.52	0.10	1.82	1.00	1.82	0.85	1.00	1.55
Fatal and Injury (FI)					0.51		0.51	0.85	1.00	0.43
ratarana mjary (r 1)					0.51		0.51	0.65	1.00	0.43
Property Damage Only (PDO)					1.32		1.32	0.85	1.00	1.12

	Worksheet 2D Multiple-V	ehicle Collisions by Collis	ion Type for Urban and Suburba	an Arterial Intersections		
(1)	(2)	(3)	(4)	(5)	(6)	
Collision Type	Proportion of Collision Type _(FI)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)	
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C	
Total	1.00	0.43	1.00	1.12	1.55	
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)	
Rear-end collision	0.45	0.19	0.42	0.47	0.67	
Head-on collision	0.04	0.02	0.04	0.04	0.06	
Angle collision	0.49	0.21	0.43	0.48	0.69	
Sideswipe	0.02	0.01	0.11	0.13	0.13	
Other multiple-vehicle collision	0.00	0.00	0.00	0.00	0.00	

	Worksheet 2E Single-Vehicle Collisions by Severity Level for Urban and Suburban Arterial Intersections									
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
	SPF Coefficients		Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bisv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}	
Crash Severity Level	fro	om Table 12-12	2	ć -			(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	from Table 1A	(3) * (4) From Worksheet 2C			Worksheet 2B		.,,,,,
Total				0.05	0.09	1.00	0.09	0.85	1.00	0.08
Fatal and Injury (FI)				0.18	0.02		0.02	0.85	1.00	0.01
Property Damage Only (PDO)			1	0.82	0.07		0.07	0.85	1.00	0.06

	Worksheet 2F Single-Ve	ehicle Collisions by Collision	on Type for Urban and Suburba	n Arterial Intersections	
(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv} (FI) (crashes/year)	* * * * * * * * * * * * * * * * * * * *		Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.00	0.01	1.00	0.06	0.08
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.00	0.00	0.03	0.00	0.00
Collision with animal	0.00	0.00	0.03	0.00	0.00
Collision with fixed object	0.63	0.01	0.86	0.05	0.06
Collision with other object	0.00	0.00	0.06	0.00	0.00
Other single-vehicle collision	0.13	0.00	0.03	0.00	0.00
Single-vehicle noncollision	0.25	0.00	0.00	0.00	0.00

Worksheet 2G-1 Crash Mod	Worksheet 2G-1 Crash Modification Factors for Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Signalized Intersections								
(1)	(2)	(3)	(4)						
CMF for Bus Stops	CMF for Schools	CMF for Alcohol Sales Establishments	Combined CMF						
CMF _{1p}	CMF _{2p}	CMF _{3p}	Combined Civil						
from Table 12-28	from Table 12-29	from Table 12-30	(1)*(2)*(3)						
2.78	1.00	1.00	2.78						

	Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Intersections								
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)		
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	- Veh-Ped ω	N	Calibration	Predicted N _{pedi}		
	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	ven-rea ω	Npedbase	factor, C _i	(4) from Worksheet 2H*(6)*(7)		
Total	1.55	0.08	1.62	0.03	0.05	1.00	0.13		
Fatal and injury (FI)						1.00	0.13		

(1)	(2)	(2) (3)		(5)	(6)	(7)
Crash Severity Level	Predicted N _{bimv}		Predicted N _{bi}	Veh-Bike ω	Calibration factor, C _i	Predicted N _{bikei}
	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)			
Total	1.55	0.08	1.62	0.04	1.00	0.07
Fatal and injury (FI)					1.00	0.07

Worksh	Worksheet 2K Crash Severity Distribution for Urban and Suburban Arterial Intersections							
(1)	(2)	(3)	(4)					
	Fatal and injury (FI)	Property damage only (PDO)	Total					
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;					
	(8) from Worksheet 2G and (7) from 2J		(8) from Worksheet 2G and (7) from 2J					
	MULTIPLE-VEHICLE							
Rear-end collisions (from Worksheet 2D)	0.19	0.47	0.67					
Head-on collisions (from Worksheet 2D)	0.02	0.04	0.06					
Angle collisions (from Worksheet 2D)	0.21	0.48	0.69					
Sideswipe (from Worksheet 2D)	0.01	0.13	0.13					
Other multiple-vehicle collision (from Worksheet 2D)	0.00	0.00	0.00					
Subtotal	0.43	1.12	1.55					
	SINGLE-VEHICLE							
Collision with parked vehicle (from Worksheet 2F)	0.00	0.00	0.00					
Collision with animal (from Worksheet 2F)	0.00	0.00	0.00					
Collision with fixed object (from Worksheet 2F)	0.01	0.05	0.06					
Collision with other object (from Worksheet 2F)	0.00	0.00	0.00					
Other single-vehicle collision (from Worksheet 2F)	0.00	0.00	0.00					
Single-vehicle noncollision (from Worksheet 2F)	0.00	0.00	0.00					
Collision with pedestrian (from Worksheet 2G or 2I)	0.13	0.00	0.13					
Collision with bicycle (from Worksheet 2J)	0.07	0.00	0.07					
Subtotal	0.21	0.06	0.27					
Total	0.64	1.18	1.82					

Worksheet 2L Summary R	Worksheet 2L Summary Results for Urban and Suburban Arterial Intersections					
(1)	(2)					
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)					
	Total					
Total	1.82					
Fatal and injury (FI)	0.64					
Property damage only (PDO)	1.18					

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Worksh	eet 2A General Information and Input	Data for Urban and Suburban Arte	rial Intersections		
General Informat	ion	Location Information			
Analyst	Ben Erban	City	Milton		
Agency or Company	CTPS	Intersection	Route 138 at Blue HIII Terrace Street and Cheever Street		
Date Performed	5/1/2018	Jurisdiction	MassDOT Highwat District 6		
		Analysis Year	2013		
Input Data		Base Conditions	Site Conditions		
Intersection type (3ST, 3SG, 4ST, 4SG)			4SG		
AADT _{major} (veh/day)	$AADT_{MAX} = 67,700 (veh/day)$		10,050		
AADT _{minor} (veh/day)	$AAD1_{MAX} = 33,400 (veh/day)$		1,850		
Intersection lighting (present/not present)		Not Present	Not Present		
Calibration factor, C _i		1.00	1.00		
Data for signalized intersections only:					
Number of approaches with left-turn lanes (0,1,2,3,4)	[for 3SG, use maximum value of 3]		0		
Number of approaches with right-turn lanes (0,1,2,3,4) [for 3SG, use maximum value of 3]		0		
Number of approaches with left-turn signal phasing [for	or 3SG, use maximum value of 3]		2		
Type of left-turn signal phasing for Leg #1		Permissive	Permissive		
Type of left-turn signal phasing for Leg #2			Permissive		
Type of left-turn signal phasing for Leg #3			Protected		
Type of left-turn signal phasing for Leg #4 (if applicable	le)		Protected		
Number of approaches with right-turn-on-red prohibite	ed [for 3SG, use maximum value of 3]	0	2		
Intersection red light cameras (present/not present)		Not Present	Not Present		
Sum of all pedestrian crossing volumes (PedVol) S	signalized intersections only		0		
Maximum number of lanes crossed by a pedestrian (r	N _{lanesx})		2		
Number of bus stops within 300 m (1,000 ft) of the int	ersection	0	2		
Schools within 300 m (1,000 ft) of the intersection (pre	esent/not present)	Not Present	Not Present		
Number of alcohol sales establishments within 300 m	(1,000 ft) of the intersection	0	0		

	SUMMARY OF INTERSECTION & CRASHES										
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF ₄i	CMF 5i	CMF _{1p}	CMF _{2p}	CMF 3p	N _{bi_total}	N_{pedi_total}	N _{bikei_total}	N predictedint
1.000	17 27 OF THE LOCAL PERIOD MILITARY PERIOD MILI									1.805	

Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections									
(1) (2) (3) (4) (5) (6) (7)									
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF			
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}			
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)			
1.00	0.88	1.00	0.96	1.00	1.00	0.85			

Worksheet 2C Multiple-Vehicle Collisions by Severity Level for Urban and Suburban Arterial Intersections										
(1)	(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)	
Crash Severity Level	SPF Coefficients		Overdispersion Parameter, k	Initial N _{bimv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bimv}	
	fro	om Table 12-10)	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	Hem rable 12 16	21		() TOTAL ()	Worksheet 2B		(6) (1) (6)
Total	-11.79	0.92	0.52	0.10	1.81	1.00	1.81	0.85	1.00	1.53
Fatal and Injury (FI)					0.50		0.50	0.85	1.00	0.43
ratarana mjary (r 1)	y (11)		0.50	0.65	1.00	0.43				
Property Damage Only (PDO)					1.30		1.30	0.85	1.00	1.11

	Worksheet 2D Multiple-V	ehicle Collisions by Collis	ion Type for Urban and Suburba	an Arterial Intersections		
(1)	(2)	(3)	(4)	(5)	(6)	
Collision Type	Proportion of Collision Type _(FI)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)	
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C	
Total	1.00	0.43	1.00	1.11	1.53	
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)	
Rear-end collision	0.45	0.19	0.42	0.47	0.66	
Head-on collision	0.04	0.02	0.04	0.04	0.06	
Angle collision	0.49	0.21	0.43	0.47	0.68	
Sideswipe	0.02	0.01	0.11	0.12	0.13	
Other multiple-vehicle collision	0.00	0.00	0.00	0.00	0.00	

	Worksheet 2E Single-Vehicle Collisions by Severity Level for Urban and Suburban Arterial Intersections									
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
	SPF Coefficients		Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bisv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}	
Crash Severity Level	fro	from Table 12-12		ć -			(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	from Table 1A	(3) * (4) From Worksheet 2C			Worksheet 2B		.,,,,,
Total				0.05	0.09	1.00	0.09	0.85	1.00	0.08
Fatal and Injury (FI)				0.18	0.02		0.02	0.85	1.00	0.01
Property Damage Only (PDO)			1	0.82	0.07		0.07	0.85	1.00	0.06

	Worksheet 2F Single-Ve	ehicle Collisions by Collision	on Type for Urban and Suburba	n Arterial Intersections		
(1)	(2)	(3)	(4)	(5)	(6)	
Collision Type	Proportion of Collision Type _(FI)	Predicted N bisv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)	
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E	
Total	1.00	0.01	1.00	0.06	0.08	
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)	
Collision with parked vehicle	0.00	0.00	0.03	0.00	0.00	
Collision with animal	0.00	0.00	0.03	0.00	0.00	
Collision with fixed object	0.63	0.01	0.86	0.05	0.06	
Collision with other object	0.00	0.00	0.06	0.00	0.00	
Other single-vehicle collision	0.13	0.00	0.03	0.00	0.00	
Single-vehicle noncollision	0.25	0.00	0.00	0.00	0.00	

Worksheet 2G-1 Crash Modific	Worksheet 2G-1 Crash Modification Factors for Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Signalized Intersections							
(1)	(2)	(3)	(4)					
CMF for Bus Stops	CMF for Schools	CMF for Alcohol Sales Establishments	Combined CMF					
CMF _{1p}	CMF _{2p}	CMF _{3p}	Combined Civil					
from Table 12-28	from Table 12-29	from Table 12-30	(1)*(2)*(3)					
2.78	1.00	1.00	2.78					

	Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Intersections								
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)		
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	- Veh-Ped ω	N	Calibration	Predicted N _{pedi}		
	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	ven-rea ω	Npedbase	factor, C _i	(4) from Worksheet 2H*(6)*(7)		
Total	1.53	0.08	1.61	0.03	0.05	1.00	0.13		
Fatal and injury (FI)						1.00	0.13		

(1)	(2)	(3)	(4)	(5)	(6)	(7)
Crach Soverity Loyel	Predicted N _{bimv}		Predicted N _{bi}	Veh-Bike ω	Calibration factor, C _i	Predicted N _{bikei}
Crash Severity Level	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	Ven-bike ω	(4)*(5)*(6)	
Total	1.53	0.08	1.61	0.04	1.00	0.07
Fatal and injury (FI)					1.00	0.07

Workshe	Worksheet 2K Crash Severity Distribution for Urban and Suburban Arterial Intersections							
(1)	(2)	(3)	(4)					
	Fatal and injury (FI)	Property damage only (PDO)	Total					
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;					
	(8) from Worksheet 2G and (7) from 2J		(8) from Worksheet 2G and (7) from 2J					
	MULTIPLE-VEHICLE							
Rear-end collisions (from Worksheet 2D)	0.19	0.47	0.66					
Head-on collisions (from Worksheet 2D)	0.02	0.04	0.06					
Angle collisions (from Worksheet 2D)	0.21	0.47	0.68					
Sideswipe (from Worksheet 2D)	0.01	0.12	0.13					
Other multiple-vehicle collision (from Worksheet 2D)	0.00	0.00	0.00					
Subtotal	0.43	1.11	1.53					
	SINGLE-VEHICLE							
Collision with parked vehicle (from Worksheet 2F)	0.00	0.00	0.00					
Collision with animal (from Worksheet 2F)	0.00	0.00	0.00					
Collision with fixed object (from Worksheet 2F)	0.01	0.05	0.06					
Collision with other object (from Worksheet 2F)	0.00	0.00	0.00					
Other single-vehicle collision (from Worksheet 2F)	0.00	0.00	0.00					
Single-vehicle noncollision (from Worksheet 2F)	0.00	0.00	0.00					
Collision with pedestrian (from Worksheet 2G or 2I)	0.13	0.00	0.13					
Collision with bicycle (from Worksheet 2J)	0.07	0.00	0.07					
Subtotal	0.21	0.06	0.27					
Total	0.64	1.17	1.80					

Worksheet 2L Summary Re	esults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)
	Total
Total	1.80
Fatal and injury (FI)	0.64
Property damage only (PDO)	1.17

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W	orksheet 2A General Information and Input	Data for Urban and Suburban Arter	rial Intersections		
General Info	rmation	Location Information			
Analyst	Ben Erban	City	Milton		
Agency or Company	CTPS	Intersection	Route 138 at Blue HIII Terrace Street and Cheever Street		
Date Performed	5/1/2018	Jurisdiction	MassDOT Highwat District 6		
		Analysis Year	2012		
Input D	ata	Base Conditions	Site Conditions		
Intersection type (3ST, 3SG, 4ST, 4SG)			4SG		
AADT _{major} (veh/day)	$AADT_{MAX} = 67,700 (veh/day)$		10,000		
AADT _{minor} (veh/day)	$AADI_{MAX} = 33,400 (veh/day)$		1,800		
Intersection lighting (present/not present)		Not Present	Not Present		
Calibration factor, C _i		1.00	1.00		
Data for signalized intersections only:					
Number of approaches with left-turn lanes (0,1,2	2,3,4) [for 3SG, use maximum value of 3]		0		
Number of approaches with right-turn lanes (0,1	,2,3,4) [for 3SG, use maximum value of 3]		0		
Number of approaches with left-turn signal phas	ing [for 3SG, use maximum value of 3]		2		
Type of left-turn signal phasing for Leg #1		Permissive	Permissive		
Type of left-turn signal phasing for Leg #2			Permissive		
Type of left-turn signal phasing for Leg #3			Protected		
Type of left-turn signal phasing for Leg #4 (if app	plicable)		Protected		
Number of approaches with right-turn-on-red pro	phibited [for 3SG, use maximum value of 3]	0	2		
Intersection red light cameras (present/not present	ent)	Not Present	Not Present		
Sum of all pedestrian crossing volumes (PedVo	l) Signalized intersections only		0		
Maximum number of lanes crossed by a pedest	rian (n _{lanesx})		2		
Number of bus stops within 300 m (1,000 ft) of t	he intersection	0	2		
Schools within 300 m (1,000 ft) of the intersection	on (present/not present)	Not Present	Not Present		
Number of alcohol sales establishments within 3	300 m (1,000 ft) of the intersection	0	0		

	SUMMARY OF INTERSECTION & CRASHES										
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF 4i	CMF 5i	CMF _{1p}	CMF _{2p}	CMF 3p	N _{bi_total}	$N_{\text{pedi_total}}$	N _{bikei_total}	N predictedint
1.000										1.771	

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections									
(1) (2) (3) (4) (5) (6) (7)										
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF				
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}				
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)				
1.00	0.88	1.00	0.96	1.00	1.00	0.85				

	and Suburban Arterial In	itersections								
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
Crash Severity Level	SPF Coefficients		Overdispersion Parameter, k	Initial N _{bimv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bimv}	
	fro	m Table 12-10)	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С		21		()TOTAL (-)	Worksheet 2B		(5) (.) (5)
Total	-11.79	0.92	0.52	0.10	1.77	1.00	1.77	0.85	1.00	1.50
Fatal and Injury (FI)					0.49		0.49	0.85	1.00	0.42
r atai and injury (i i)		0.49		0.43	0.65	1.00	0.42			
Property Damage Only (PDO)					1.28		1.28	0.85	1.00	1.09

	Worksheet 2D Multiple-V	ehicle Collisions by Collis	ion Type for Urban and Suburb	an Arterial Intersections		
(1)	(2)	(3)	(4)	(5)	(6)	
Collision Type	Proportion of Collision Type _(FI)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)	
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C	
Total	1.00	0.42	1.00	1.09	1.50	
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)	
Rear-end collision	0.45	0.19	0.42	0.46	0.65	
Head-on collision	0.04	0.02	0.04	0.04	0.06	
Angle collision	0.49	0.21	0.43	0.46	0.67	
Sideswipe	0.02	0.01	0.11	0.12	0.13	
Other multiple-vehicle collision	0.00	0.00	0.00	0.00	0.00	

(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
	SF	PF Coefficients	S	Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bisv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}
Crash Severity Level	fro	om Table 12-12	2				(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	from Table 1A	(3) * (4) From Worksheet 2C			Worksheet 2B		
Total				0.05	0.09	1.00	0.09	0.85	1.00	0.07
Fatal and Injury (FI)				0.18	0.02		0.02	0.85	1.00	0.01
Property Damage Only (PDO)				0.82	0.07		0.07	0.85	1.00	0.06

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	Worksheet 2F Single-Ve	ehicle Collisions by Collision	on Type for Urban and Suburba	n Arterial Intersections	
(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv} (FI) (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.00	0.01	1.00	0.06	0.07
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.00	0.00	0.03	0.00	0.00
Collision with animal	0.00	0.00	0.03	0.00	0.00
Collision with fixed object	0.63	0.01	0.86	0.05	0.06
Collision with other object	0.00	0.00	0.06	0.00	0.00
Other single-vehicle collision	0.13	0.00	0.03	0.00	0.00
Single-vehicle noncollision	0.25	0.00	0.00	0.00	0.00

Worksheet 2G-1 Crash Modific	Worksheet 2G-1 Crash Modification Factors for Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Signalized Intersections							
(1)	(1) (2) (3)							
CMF for Bus Stops	CMF for Schools	CMF for Alcohol Sales Establishments	Combined CMF					
CMF _{1p}	CMF _{2p}	CMF _{3p}	Combined Civil					
from Table 12-28	from Table 12-29	from Table 12-30	(1)*(2)*(3)					
2.78	1.00	1.00	2.78					

	Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Intersections								
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)		
Crack Savarity Laval	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Ped ω	N	Calibration	Predicted N _{pedi}		
Crash Severity Level	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	ven-rea ω	Npedbase	factor, C _i	(4) from Worksheet 2H*(6)*(7)		
Total	1.50	0.07	1.58	0.03	0.05	1.00	0.13		
Fatal and injury (FI)						1.00	0.13		

(1)	(2)	(2) (3) (4)		(5)	(6)	(7)
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration factor, C _i	Predicted N _{bikei}
	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	Ven-bike ω	(4)*(5)*(6)	
Total	1.50	0.07	1.58	0.04	1.00	0.07
Fatal and injury (FI)					1.00	0.07

Worksheet 2K Crash Severity Distribution for Urban and Suburban Arterial Intersections								
(1)	(2)	(3)	(4)					
	Fatal and injury (FI)	Property damage only (PDO)	Total					
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;					
	(8) from Worksheet 2G and (7) from 2J		(8) from Worksheet 2G and (7) from 2J					
	MULTIPLE-VEHICLE	•	•					
Rear-end collisions (from Worksheet 2D)	0.19	0.46	0.65					
Head-on collisions (from Worksheet 2D)	0.02	0.04	0.06					
Angle collisions (from Worksheet 2D)	0.21	0.46	0.67					
Sideswipe (from Worksheet 2D)	0.01	0.12	0.13					
Other multiple-vehicle collision (from Worksheet 2D)	0.00	0.00	0.00					
Subtotal	0.42	1.09	1.50					
	SINGLE-VEHICLE							
Collision with parked vehicle (from Worksheet 2F)	0.00	0.00	0.00					
Collision with animal (from Worksheet 2F)	0.00	0.00	0.00					
Collision with fixed object (from Worksheet 2F)	0.01	0.05	0.06					
Collision with other object (from Worksheet 2F)	0.00	0.00	0.00					
Other single-vehicle collision (from Worksheet 2F)	0.00	0.00	0.00					
Single-vehicle noncollision (from Worksheet 2F)	0.00	0.00	0.00					
Collision with pedestrian (from Worksheet 2G or 2I)	0.13	0.00	0.13					
Collision with bicycle (from Worksheet 2J)	0.07	0.00	0.07					
Subtotal	0.21	0.06	0.27					
Total	0.62	1.15	1.77					

Worksheet 2L Summary R	esults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)
	Total
Total	1.77
Fatal and injury (FI)	0.62
Property damage only (PDO)	1.15

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Worksh	neet 2A General Information and Input	Data for Urban and Suburban Ar	terial Intersections			
General Informat	ion	Location Information				
Analyst	Ben Erban	City	Milton			
Agency or Company	CTPS	Intersection	Route 138 at Blue HIII Terrace Street and Cheever Street			
Date Performed	5/1/2018	Jurisdiction	MassDOT Highwat District 6			
		Analysis Year	2011			
Input Data		Base Conditions	Site Conditions			
Intersection type (3ST, 3SG, 4ST, 4SG)			4SG			
AADT _{major} (veh/day)	$AADT_{MAX} = 67,700 (veh/day)$		9,850			
AADT _{minor} (veh/day)	$AADI_{MAX} = 33,400 (veh/day)$		1,750			
Intersection lighting (present/not present)	-	Not Present	Not Present			
Calibration factor, C _i		1.00	1.00			
Data for signalized intersections only:						
Number of approaches with left-turn lanes (0,1,2,3,4)	[for 3SG, use maximum value of 3]		0			
Number of approaches with right-turn lanes (0,1,2,3,4) [for 3SG, use maximum value of 3]		0			
Number of approaches with left-turn signal phasing [for	or 3SG, use maximum value of 3]		2			
Type of left-turn signal phasing for Leg #1		Permissive	Permissive			
Type of left-turn signal phasing for Leg #2			Permissive			
Type of left-turn signal phasing for Leg #3			Protected			
Type of left-turn signal phasing for Leg #4 (if applicab	le)		Protected			
Number of approaches with right-turn-on-red prohibite	ed [for 3SG, use maximum value of 3]	0	2			
Intersection red light cameras (present/not present)		Not Present	Not Present			
Sum of all pedestrian crossing volumes (PedVol) S	Signalized intersections only		0			
Maximum number of lanes crossed by a pedestrian (n _{lanesx})		2			
Number of bus stops within 300 m (1,000 ft) of the int	ersection	0	2			
Schools within 300 m (1,000 ft) of the intersection (pr	esent/not present)	Not Present	Not Present			
Number of alcohol sales establishments within 300 m	(1.000 ft) of the intersection	0	0			

	SUMMARY OF INTERSECTION & CRASHES										
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF ₄i	CMF 5i	CMF _{1p}	CMF _{2p}	CMF 3p	N _{bi_total}	$N_{\text{pedi_total}}$	N _{bikei_total}	N predictedint
1.000											

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections									
(1) (2) (3) (4) (5) (6) (7)										
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF				
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}				
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)				
1.00	0.88	1.00	0.96	1.00	1.00	0.85				

Worksheet 2C Multiple-Vehicle Collisions by Severity Level for Urban and Suburban Arterial Intersections										
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
Crash Severity Level	SF	SPF Coefficients		Overdispersion		Proportion of Total	Adjusted	Combined	Calibration	Predicted
				Parameter, k	Initial N _{bimv}	Crashes	N_{bimv}	CMFs	Factor, C _i	N_{bimv}
	fro	om Table 12-1)	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	Hom Table 12-10	21		(I)TOTAL (O)	Worksheet 2B		(0) (1) (0)
Total	-11.79	0.92	0.52	0.10	1.72	1.00	1.72	0.85	1.00	1.46
Fatal and Injury (FI)					0.48		0.48	0.85	1.00	0.41
atar and injury (i i)	ind mjury (i i)			0.40		0.40	0.65	1.00	0.41	
Property Damage Only (PDO)					1.24		1.24	0.85	1.00	1.06

	Worksheet 2D Multiple-V	ehicle Collisions by Collis	ion Type for Urban and Suburba	an Arterial Intersections		
(1)	(2)	(3)	(4)	(5)	(6)	
Collision Type	Proportion of Collision Type _(FI)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)	
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C	
Total	1.00	0.41	1.00	1.06	1.46	
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)	
Rear-end collision	0.45	0.18	0.42	0.45	0.63	
Head-on collision	0.04	0.01	0.04	0.04	0.06	
Angle collision	0.49	0.20	0.43	0.45	0.65	
Sideswipe	0.02	0.01	0.11	0.12	0.13	
Other multiple-vehicle collision	0.00	0.00	0.00	0.00	0.00	

	Worksheet 2E Single-Vehicle Collisions by Severity Level for Urban and Suburban Arterial Intersections									
(1)	(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)	
	SPF Coefficients		Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bisv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}	
Crash Severity Level	fro	om Table 12-12	2				(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	from Table 1A	(3) * (4) From Worksheet 2C			Worksheet 2B		
Total				0.05	0.08	1.00	0.08	0.85	1.00	0.07
Fatal and Injury (FI)				0.18	0.02		0.02	0.85	1.00	0.01
Property Damage Only (PDO)				0.82	0.07		0.07	0.85	1.00	0.06

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	Worksheet 2F Single-Ve	ehicle Collisions by Collision	on Type for Urban and Suburba	n Arterial Intersections	
(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv (FI)} (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.00	0.01	1.00	0.06	0.07
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.00	0.00	0.03	0.00	0.00
Collision with animal	0.00	0.00	0.03	0.00	0.00
Collision with fixed object	0.63	0.01	0.86	0.05	0.06
Collision with other object	0.00	0.00	0.06	0.00	0.00
Other single-vehicle collision	0.13	0.00	0.03	0.00	0.00
Single-vehicle noncollision	0.25	0.00	0.00	0.00	0.00

Worksheet 2G-1 Crash Modif	Worksheet 2G-1 Crash Modification Factors for Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Signalized Intersections							
(1)	(2)	(3)	(4)					
CMF for Bus Stops	CMF for Schools	CMF for Alcohol Sales Establishments	Combined CMF					
CMF _{1p}	CMF _{2p}	CMF _{3p}	Combined Civil					
from Table 12-28	from Table 12-29	from Table 12-30	(1)*(2)*(3)					
2.78	1.00	1.00	2.78					

	Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Intersections								
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)		
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	- Veh-Ped ω	N	Calibration	Predicted N _{pedi}		
	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	ven-rea ω	Npedbase	factor, C _i	(4) from Worksheet 2H*(6)*(7)		
Total	1.46	0.07	1.53	0.03	0.04	1.00	0.12		
Fatal and injury (FI)		-				1.00	0.12		

(1)	(2)	(3)	(4)	(5)	(6)	(7)
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration factor, C	Predicted N _{bikei}
Crash Severity Level	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	Veii-bike ω	(4)*(5)*(6)	
Total	1.46	0.07	1.53	0.04	1.00	0.07
Fatal and injury (FI)					1.00	0.07

Worksh	eet 2K Crash Severity Distribution for Urban and	Suburban Arterial Intersections	
(1)	(2)	(3)	(4)
	Fatal and injury (FI)	Property damage only (PDO)	Total
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;
	(8) from Worksheet 2G and (7) from 2J		(8) from Worksheet 2G and (7) from 2J
	MULTIPLE-VEHICLE	•	•
Rear-end collisions (from Worksheet 2D)	0.18	0.45	0.63
Head-on collisions (from Worksheet 2D)	0.01	0.04	0.06
Angle collisions (from Worksheet 2D)	0.20	0.45	0.65
Sideswipe (from Worksheet 2D)	0.01	0.12	0.13
Other multiple-vehicle collision (from Worksheet 2D)	0.00	0.00	0.00
Subtotal	0.41	1.06	1.46
	SINGLE-VEHICLE		
Collision with parked vehicle (from Worksheet 2F)	0.00	0.00	0.00
Collision with animal (from Worksheet 2F)	0.00	0.00	0.00
Collision with fixed object (from Worksheet 2F)	0.01	0.05	0.06
Collision with other object (from Worksheet 2F)	0.00	0.00	0.00
Other single-vehicle collision (from Worksheet 2F)	0.00	0.00	0.00
Single-vehicle noncollision (from Worksheet 2F)	0.00	0.00	0.00
Collision with pedestrian (from Worksheet 2G or 2I)	0.12	0.00	0.12
Collision with bicycle (from Worksheet 2J)	0.07	0.00	0.07
Subtotal	0.20	0.06	0.26
Total	0.61	1.11	1.72

Worksheet 2L Summary Re	esults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)
	Total
Total	1.72
Fatal and injury (FI)	0.61
Property damage only (PDO)	1.11

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General Information	n	Locati	on Information	
Analyst	Analyst Ben Erban		Route 138 and Oak Street	
Agency or Company	CTPS	Intersection Type	4ST	
Date Performed	May-18	Jurisdiction	MassDOT Highwat District 6	
City	,		2018	

		Input Inf	ormation			
Year	Oserved MV crashes	Observed total crashes	Predicted MV crashes	Predicted total crashes	Combined CMF for veh-ped crashes	
2015	1	1	0.40	0.41	1.00	
2014	0	0	0.40	0.41	1.00	
2013	1	1	0.39	0.41	1.00	
2012	0	0	0.39	0.41	1.00	
2011	0	0	0.39	0.40	1.00	

I						Out	out Inform	ation				
	Observed MV crashes	Average observed total crashes	Total predicted MV crashes	Average predicted total crashes	Standard deviation of predicted total crashes	Weight	Total expected MV crashes	No of expected total crashes	Average expected total crashes	Intercection	Potential for Safety Improvement (PSI)	If avg observed total crashes > avg expected crashes
	2.00	0.40	1.97	0.41	0.00	0.58	1.98	0.44 0.44 0.44	0.44	Y	0.04	M
								0.44 0.44				

	Worksh	eet 2A General Informat	ion and Input	Data for Urban and Suburban A	rterial Interse	ections	
G	eneral Informati	on		Location Information			
Analyst		Ben Erban		City		Milton	
Agency or Company		CTPS		Intersection		Route 138 and Oak Street	
Date Performed		5/1/2018		Jurisdiction		MassDOT District 6	
				Analysis Year		2015	
			Base Conditions		Site Conditions		
Intersection type (3ST, 3SG, 4ST, 4SG)						4ST	
AADT _{major} (veh/day)		$AADT_{MAX} = 46800.00$	(veh/day)			10,250	
AADT _{minor} (veh/day)		$AADT_{MAX} = 5900.00$	(veh/day)			300	
Intersection lighting (present/not present)				Not Present		Not Present	
Calibration factor, C _i				1.00		1.04	
Data for unsignalized intersections only:							
Number of major-road approaches w	ith left-turn lanes	(0,1,2)				0	
Number of major-road approaches w	ith right-turn lane	s (0.1.2)				0	

	SUMMARY OF INTERSECTION & CRASHES										
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF 4i	CMF 5i	N _{bi_total}	N _{pedi_total}	N _{bikei_total}	N predictedint			
1.00	1.00	1.00	1.00	1.00	0.41	0.01	0.00	0.42			

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections										
(1)	(2)	(3)	(4)	(5)	(6)	(7)					
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF					
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}					
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)					
1.00	1.00	1.00	1.00	1.00	1.00	1.00					

		Worksheet 2	C Multiple-\	ehicle Collisions by Seve	rity Level for Urban	and Suburban Arterial In	tersections			
(1)	(1) (2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)	
Crash Severity Level SPF Coefficients		Overdispersion		Proportion of Total	Adjusted	Combined	Calibration	Predicted		
	from Table 12-10			Parameter, k from Table 12-10	Initial N _{bimv} from Equation 12-	Crashes	N_{bimv}	CMFs (7) from	Factor, C _i	N _{bimv} (6)*(7)*(8)
							(4) _{TOTAL} *(5)			
	а	b	С	Holli Table 12-10	21		(T)TOTAL (U)	Worksheet 2B	<i>i</i>	(0) (1) (0)
Total	-8.70	0.31	0.86	0.36	0.38	1.00	0.38	1.00	1.04	0.40
Fatal and Injury (FI)					0.14	0.14	0.14	1.00	1.04	0.15
r atai ana mjary (1 1)					0.11					
Property Damage Only (PDO)					0.24		0.24	1.00	1.04	0.25

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(Fi)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C
Total	1.00	0.15	1.00	0.25	0.40
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Rear-end collision	0.16	0.02	0.19	0.05	0.07
Head-on collision	0.01	0.00	0.02	0.00	0.00
Angle collision	0.82	0.12	0.72	0.18	0.30
Sideswipe	0.02	0.00	0.08	0.02	0.02
Other multiple-vehicle collision	0.00	0.00	0.00	0.00	0.00

(1) (2)			(3)	(4)	(5)	(6)	(7)	(8)	(9)	
, ,	SPF Coefficients			Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}
Crash Severity Level	from Table 12-12			(T.I. (A		(4) _{TOTAL} *(5)		(7) from		(6)*(7)*(8)
	а	b	С	from Table 1A	(3) * (4) From Worksheet 2C		. , , , , ,	Worksheet 2B		
Γotal				0.03	0.01	1.00	0.01	1.00	1.04	0.01
Fatal and Injury (FI)	1			0.30	0.00		0.00	1.00	1.04	0.00
Property Damage Only PDO)				0.70	0.01		0.01	1.00	1.04	0.01

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(Fi)	Predicted N _{bisv} (FI) (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.00	0.00	1.00	0.01	0.01
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.00	0.00	0.14	0.00	0.00
Collision with animal	0.00	0.00	0.29	0.00	0.00
Collision with fixed object	0.68	0.00	0.57	0.00	0.01
Collision with other object	0.09	0.00	0.00	0.00	0.00
Other single-vehicle collision	0.05	0.00	0.00	0.00	0.00
Single-vehicle noncollision	0.18	0.00	0.00	0.00	0.00

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V	/orksheet 2G Vehicle-Pedestr	ian Collisions for Urban and	d Suburban Arterial	Stop-Controlled Interse	ctions		
(1)	(2)	(3)	(4)	(5)	(6)	(7)	
ach Savarity Laval	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh- Ped ω	Calibration	Predicted N _{pedi}	
Crash Severity Level	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	from Table 1B	factor, C _i	(4)*(5)*(6)	
Total	0.40	0.01	0.41	0.02	1.04	0.01	
Fatal and injury (FI)					1.04	0.01	

	Worksheet 2J Vehicle	-Bicycle Collisions for Urb	an and Suburban <i>A</i>	Arterial Intersections					
(1)	(2)	(2) (3) (4) (5) (6)							
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration	Predicted N _{bikei}			
lorasii oeverity Lever	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	(4) * (5)	factor, C _i	(4)*(5)*(6)			
Total	0.40	0.01	0.41	0.01	1.04	0.00			
Fatal and injury (FI)					1.04	0.00			

Worksheet 2K Crash Severity Distribution for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)						
	Fatal and injury (FI)	Property damage only (PDO)	Total						
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;						
	(7) from Worksheet 2G and 2J	,	(7) from Worksheet 2G and 2J						
	MULTIPLE-VEHICLE	·							
Rear-end collisions (from Worksheet 2D)	0.02	0.05	0.07						
Head-on collisions (from Worksheet 2D)	0.00	0.00	0.00						
Angle collisions (from Worksheet 2D)	0.12	0.18	0.30						
Sideswipe (from Worksheet 2D)	0.00	0.02	0.02						
Other multiple-vehicle collision (from Worksheet 2D)	0.00	0.00	0.00						
Subtotal	0.15	0.25	0.40						
	SINGLE-VEHICLE								
Collision with parked vehicle (from Worksheet 2F)	0.00	0.00	0.00						
Collision with animal (from Worksheet 2F)	0.00	0.00	0.00						
Collision with fixed object (from Worksheet 2F)	0.00	0.00	0.01						
Collision with other object (from Worksheet 2F)	0.00	0.00	0.00						
Other single-vehicle collision (from Worksheet 2F)	0.00	0.00	0.00						
Single-vehicle noncollision (from Worksheet 2F)	0.00	0.00	0.00						
Collision with pedestrian (from Worksheet 2G or 2I)	0.01	0.00	0.01						
Collision with bicycle (from Worksheet 2J)	0.00	0.00	0.00						
Subtotal	0.01	0.01	0.02						
Total	0.16	0.26	0.42						

Worksheet 2L Summary R	lesults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)
	(Total) from Worksheet 2K
Total	0.42
Fatal and injury (FI)	0.16
Property damage only (PDO)	0.26

	Worksh	eet 2A General Informati	ion and Input	Data for Urban and Suburban A	rterial Interse	ections	
G	eneral Informati	on		Location Information			
Analyst		Ben Erban		City		Milton	
Agency or Company		CTPS		Intersection		Route 138 and Oak Street	
Date Performed		5/1/2018		Jurisdiction		MassDOT District 6	
				Analysis Year		2014	
		Base Conditions	Site Conditions				
Intersection type (3ST, 3SG, 4ST, 4SG)	_					4ST	
AADT _{major} (veh/day)		$AADT_{MAX} = 46800.00$	(veh/day)			10,150	
AADT _{minor} (veh/day)		$AADT_{MAX} = 5900.00$	(veh/day)			300	
ntersection lighting (present/not present)				Not Present		Not Present	
Calibration factor, C _i				1.00		1.04	
Data for unsignalized intersections only:							
Number of major-road approaches wi	th left-turn lanes	(0,1,2)	•			0	
Number of major-road approaches wi	s (0 1 2)		0				

	SUMMARY OF INTERSECTION & CRASHES									
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF 4i	CMF 5i	N _{bi_total}	N _{pedi_total}	N _{bikei_total}	N predictedint		
1.00	1.00	1.00	1.00	1.00	0.41	0.01	0.00	0.42		

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections										
(1)	(2)	(3)	(4)	(5)	(6)	(7)					
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF					
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}					
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)					
1.00	1.00	1.00	1.00	1.00	1.00	1.00					

		Worksheet 2	C Multiple-\	ehicle Collisions by Seve	rity Level for Urban	and Suburban Arterial In	tersections			
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
Crash Severity Level	SF	PF Coefficient	s	Overdispersion		Proportion of Total	Adjusted	Combined	Calibration	Predicted
				Parameter, k	Initial N _{bimv}	Crashes	N_{bimv}	CMFs	Factor, C _i	N _{bimv}
	fro	om Table 12-10)	from Table 12-10 from Equation 12-		(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)	
	а	b	С	Holli Table 12-10	21		(T)TOTAL (U)	Worksheet 2B		(0) (1) (0)
Total	-8.70	0.31	0.86	0.36	0.38	1.00	0.38	1.00	1.04	0.40
Fatal and Injury (FI)					0.14		0.14	1.00	1.04	0.15
r atai ana mjary (1 1)					0.11		0.11	1.00	1.01	0.10
Property Damage Only (PDO)					0.24		0.24	1.00	1.04	0.25

(1)	(2)	(3)	(4)	(5)	(6)	
Collision Type	Proportion of Collision Type _(Fi)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year	
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C	
Total	1.00	0.15	1.00	0.25	0.40	
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)	
Rear-end collision	0.16	0.02	0.19	0.05	0.07	
Head-on collision	0.01	0.00	0.02	0.00	0.00	
Angle collision	0.82	0.12	0.72	0.18	0.30	
Sideswipe	0.02	0.00	0.08	0.02	0.02	
Other multiple-vehicle collision	0.00	0.00	0.00	0.00	0.00	

(1)	(2)		(2)	(4)	(5)	(6)	(7)	(8)	(9)	
, ,	SPF Coefficients		Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C	Predicted N _{bisv}	
Crash Severity Level	fro	om Table 12-12	2	ć - T.I. 44		0.0000	(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	a b	С	c from Table 1A	(3) * (4) From Worksheet 2C		. , , , , ,	Worksheet 2B			
Γotal				0.03	0.01	1.00	0.01	1.00	1.04	0.01
Fatal and Injury (FI)	1			0.30	0.00		0.00	1.00	1.04	0.00
Property Damage Only PDO)				0.70	0.01		0.01	1.00	1.04	0.01

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N bisv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.00	0.00	1.00	0.01	0.01
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.00	0.00	0.14	0.00	0.00
Collision with animal	0.00	0.00	0.29	0.00	0.00
Collision with fixed object	0.68	0.00	0.57	0.00	0.01
Collision with other object	0.09	0.00	0.00	0.00	0.00
Other single-vehicle collision	0.05	0.00	0.00	0.00	0.00
Single-vehicle noncollision	0.18	0.00	0.00	0.00	0.00

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V	/orksheet 2G Vehicle-Pedestr	ian Collisions for Urban and	d Suburban Arterial	Stop-Controlled Interse	ctions		
(1)	(2)	(3)	(4)	(5)	(6)	(7)	
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi} Veh- Ped ω		Calibration	Predicted N _{pedi}	
	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	from Table 1B	factor, C _i	(4)*(5)*(6)	
Total	0.40	0.01	0.41	0.02	1.04	0.01	
Fatal and injury (FI)					1.04	0.01	

	Worksheet 2J Vehicle	e-Bicycle Collisions for Urba	an and Suburban A	Arterial Intersections		
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bisv} Predicted N _{bi}		Calibration	Predicted N _{bikei}
orasii Severity Lever	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	(4) * (5)	factor, C _i	(4)*(5)*(6)
Total	0.40	0.01	0.41	0.01	1.04	0.00
Fatal and injury (FI)					1.04	0.00

Worksheet 2K Crash Severity Distribution for Urban and Suburban Arterial Intersections								
(1)	(2)	(3)	(4)					
	Fatal and injury (FI)	Property damage only (PDO)	Total					
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;					
	(7) from Worksheet 2G and 2J	,	(7) from Worksheet 2G and 2J					
	MULTIPLE-VEHICLE	·						
Rear-end collisions (from Worksheet 2D)	0.02	0.05	0.07					
Head-on collisions (from Worksheet 2D)	0.00	0.00	0.00					
Angle collisions (from Worksheet 2D)	0.12	0.18	0.30					
Sideswipe (from Worksheet 2D)	0.00	0.02	0.02					
Other multiple-vehicle collision (from Worksheet 2D)	0.00	0.00	0.00					
Subtotal	0.15	0.25	0.40					
	SINGLE-VEHICLE							
Collision with parked vehicle (from Worksheet 2F)	0.00	0.00	0.00					
Collision with animal (from Worksheet 2F)	0.00	0.00	0.00					
Collision with fixed object (from Worksheet 2F)	0.00	0.00	0.01					
Collision with other object (from Worksheet 2F)	0.00	0.00	0.00					
Other single-vehicle collision (from Worksheet 2F)	0.00	0.00	0.00					
Single-vehicle noncollision (from Worksheet 2F)	0.00	0.00	0.00					
Collision with pedestrian (from Worksheet 2G or 2I)	0.01	0.00	0.01					
Collision with bicycle (from Worksheet 2J)	0.00	0.00	0.00					
Subtotal	0.01	0.01	0.02					
Total	0.16	0.26	0.42					

Worksheet 2L Summary Re	esults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)
	(Total) from Worksheet 2K
Total	0.42
Fatal and injury (FI)	0.16
Property damage only (PDO)	0.26

	Worksheet 2A General Informat	ion and Input	Data for Urban and Suburban A	Arterial Interse	ctions			
G	General Information				Location Information			
Analyst	Ben Erban		City		Milton			
Agency or Company	CTPS		Intersection		Route 138 and Oak Street			
Date Performed	5/1/2018		Jurisdiction		MassDOT District 6			
	Analysis Year		2013					
	Input Data	Base Conditions		Site Conditions				
ntersection type (3ST, 3SG, 4ST, 4SG)					4ST			
AADT _{major} (veh/day)	$AADT_{MAX} = 46800.00$	(veh/day)			10,050			
AADT _{minor} (veh/day)	$AADT_{MAX} = 5900.00$	(veh/day)			300			
ntersection lighting (present/not present)			Not Present		Not Present			
Calibration factor, C _i			1.00		1.04			
Data for unsignalized intersections only:								
Number of major-road approaches w	ith left-turn lanes (0,1,2)	•			0			
Number of major-road approaches w	ith right-turn lanes (0.1.2)				0			

SUMMARY OF INTERSECTION & CRASHES								
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF 4i	CMF 5i	N _{bi_total}	N _{pedi_total}	N _{bikei_total}	N predictedint
1.00	1.00	1.00	1.00	1.00	0.41	0.01	0.00	0.42

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections								
(1)	(2)	(3)	(4)	(5)	(6)	(7)			
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF			
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}			
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)			
1.00	1.00	1.00	1.00	1.00	1.00	1.00			

		Worksheet 2	C Multiple-	ehicle Collisions by Seve	rity Level for Urban	and Suburban Arterial In	tersections			
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
Crash Severity Level	SF	PF Coefficient	s	Overdispersion		Proportion of Total	Adjusted	Combined	Calibration	Predicted
				Parameter, k	Initial N _{bimv}	Crashes	N_{bimv}	CMFs	Factor, C _i	N _{bimv}
	fro	om Table 12-10)	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	Holli Table 12-10	21		(T)TOTAL (U)	Worksheet 2B		(0) (1) (0)
Total	-8.70	0.31	0.86	0.36	0.38	1.00	0.38	1.00	1.04	0.39
Fatal and Injury (FI)					0.14		0.14	1.00	1.04	0.15
r atai ana mjary (1 1)					0.11		0.11	1.00	1.01	0.10
Property Damage Only (PDO)					0.24		0.24	1.00	1.04	0.25

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(Fi)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C
Total	1.00	0.15	1.00	0.25	0.39
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Rear-end collision	0.16	0.02	0.19	0.05	0.07
Head-on collision	0.01	0.00	0.02	0.00	0.00
Angle collision	0.82	0.12	0.72	0.18	0.30
Sideswipe	0.02	0.00	0.08	0.02	0.02
Other multiple-vehicle collision	0.00	0.00	0.00	0.00	0.00

(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
, ,	SI	PF Coefficient	s	Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C	Predicted N _{bisv}
Crash Severity Level	fr	om Table 12-12	2				(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
а	а	b	С	from Table 1A	(3) * (4) From Worksheet 2C		()TOTAL (-)	Worksheet 2B		(0) (1) (0)
Total				0.03	0.01	1.00	0.01	1.00	1.04	0.01
atal and Injury (FI)				0.30	0.00		0.00	1.00	1.04	0.00
Property Damage Only PDO)				0.70	0.01		0.01	1.00	1.04	0.01

(1)	(2)	(3)	on Type for Urban and Suburba	(5)	(6)
Collision Type	Proportion of Collision Type _(F)	Predicted N _{bisv} (FI) (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.00	0.00	1.00	0.01	0.01
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.00	0.00	0.14	0.00	0.00
Collision with animal	0.00	0.00	0.29	0.00	0.00
Collision with fixed object	0.68	0.00	0.57	0.00	0.01
Collision with other object	0.09	0.00	0.00	0.00	0.00
Other single-vehicle collision	0.05	0.00	0.00	0.00	0.00
Single-vehicle noncollision	0.18	0.00	0.00	0.00	0.00

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	Worksheet 2G Vehicle-Pedestri	an Collisions for Urban and	l Suburban Arteria	Stop-Controlled Interse	ctions	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh- Ped ω	Calibration	Predicted N _{pedi}
	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	from Table 1B	factor, C _i	(4)*(5)*(6)
Total	0.39	0.01	0.41	0.02	1.04	0.01
Fatal and injury (FI)					1.04	0.01

	Worksheet 2J Vehicle-Bicycle Collisions for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)				
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration	Predicted N _{bikei}				
Crash Seventy Level	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	(4) * (5)	factor, C _i	(4)*(5)*(6)				
Total	0.39	0.01	0.41	0.01	1.04	0.00				
Fatal and injury (FI)					1.04	0.00				

Worksheet 2K Crash Severity Distribution for Urban and Suburban Arterial Intersections											
(1)	(2)	(3)	(4)								
	Fatal and injury (FI)	Property damage only (PDO)	Total								
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;								
	(7) from Worksheet 2G and 2J	,	(7) from Worksheet 2G and 2J								
MULTIPLE-VEHICLE											
Rear-end collisions (from Worksheet 2D)	0.02	0.05	0.07								
Head-on collisions (from Worksheet 2D)	0.00	0.00	0.00								
Angle collisions (from Worksheet 2D)	0.12	0.18	0.30								
Sideswipe (from Worksheet 2D)	0.00	0.02	0.02								
Other multiple-vehicle collision (from Worksheet 2D)	0.00	0.00	0.00								
Subtotal	0.15	0.25	0.39								
	SINGLE-VEHICLE										
Collision with parked vehicle (from Worksheet 2F)	0.00	0.00	0.00								
Collision with animal (from Worksheet 2F)	0.00	0.00	0.00								
Collision with fixed object (from Worksheet 2F)	0.00	0.00	0.01								
Collision with other object (from Worksheet 2F)	0.00	0.00	0.00								
Other single-vehicle collision (from Worksheet 2F)	0.00	0.00	0.00								
Single-vehicle noncollision (from Worksheet 2F)	0.00	0.00	0.00								
Collision with pedestrian (from Worksheet 2G or 2I)	0.01	0.00	0.01								
Collision with bicycle (from Worksheet 2J)	0.00	0.00	0.00								
Subtotal	0.01	0.01	0.02								
Total	0.16	0.25	0.42								

Worksheet 2L Summary R	lesults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)
	(Total) from Worksheet 2K
Total	0.42
Fatal and injury (FI)	0.16
Property damage only (PDO)	0.25

	Workshe	et 2A General Informa	tion and Input	Data for Urban and Suburban A	rterial Interse	ections	
Ge	eneral Information	on		Location Information			
Analyst		Ben Erbar	า	City	Milton		
Agency or Company		CTPS		Intersection		Route 138 and Oak Street	
Date Performed		5/1/2018		Jurisdiction		MassDOT District 6	
				Analysis Year		2012	
		Base Conditions		Site Conditions			
Intersection type (3ST, 3SG, 4ST, 4SG)	_					4ST	
AADT _{major} (veh/day)		$AADT_{MAX} = 46800.00$	(veh/day)			10,000	
AADT _{minor} (veh/day)		$AADT_{MAX} = 5900.00$	(veh/day)			300	
Intersection lighting (present/not present)				Not Present		Not Present	
Calibration factor, C _i				1.00		1.04	
Data for unsignalized intersections only:	ions only:						
Number of major-road approaches wi	th left-turn lanes ((0,1,2)	•		0		
Number of major-road approaches wi	th right-turn lanes	: (0 1 2)	·			0	

	SUMMARY OF INTERSECTION & CRASHES									
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF 4i	CMF 5i	N _{bi_total}	N _{pedi_total}	N _{bikei_total}	N predictedint		
1.00	1.00	1.00	1.00	1.00	0.41	0.01	0.00	0.42		

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)				
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF				
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}				
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)				
1.00	1.00	1.00	1.00	1.00	1.00	1.00				

		Worksheet 2	C Multiple-	ehicle Collisions by Seve	rity Level for Urban	and Suburban Arterial In	tersections			
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
Crash Severity Level	SF	PF Coefficient	s	Overdispersion		Proportion of Total	Adjusted	Combined	Calibration	Predicted
				Parameter, k	Initial N _{bimv}	Crashes	N_{bimv}	CMFs	Factor, C _i	N _{bimv}
	fro	om Table 12-10)	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	Holli Table 12-10	21		(T)TOTAL (O)	Worksheet 2B		(0) (1) (0)
Total	-8.70	0.31	0.86	0.36	0.38	1.00	0.38	1.00	1.04	0.39
Fatal and Injury (FI)					0.14		0.14	1.00	1.04	0.15
r atai ana mjary (1 1)					0.11		0.11	1.00	1.04	0.10
Property Damage Only (PDO)					0.24		0.24	1.00	1.04	0.25

(1)	(2)	(3)	(4)	(5)	(6)	
Collision Type	Proportion of Collision Type _(Fi)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)	
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C	
Total	1.00	0.15	1.00	0.25	0.39	
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)	
Rear-end collision	0.16	0.02	0.19	0.05	0.07	
Head-on collision	0.01	0.00	0.02	0.00	0.00	
Angle collision	0.82	0.12	0.72	0.18	0.30	
Sideswipe	0.02	0.00	0.08	0.02	0.02	
Other multiple-vehicle collision	0.00	0.00	0.00	0.00	0.00	

(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
, ,	SF	PF Coefficient	s	Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C	Predicted N _{bisv}
Crash Severity Level	fro	om Table 12-12	2	ć - T.I. 44		0.0000	(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	from Table 1A	(3) * (4) From Worksheet 2C		. , , , , ,	Worksheet 2B		
Γotal				0.03	0.01	1.00	0.01	1.00	1.04	0.01
Fatal and Injury (FI)	1			0.30	0.00		0.00	1.00	1.04	0.00
Property Damage Only PDO)				0.70	0.01		0.01	1.00	1.04	0.01

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N bisv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.00	0.00	1.00	0.01	0.01
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.00	0.00	0.14	0.00	0.00
Collision with animal	0.00	0.00	0.29	0.00	0.00
Collision with fixed object	0.68	0.00	0.57	0.00	0.01
Collision with other object	0.09	0.00	0.00	0.00	0.00
Other single-vehicle collision	0.05	0.00	0.00	0.00	0.00
Single-vehicle noncollision	0.18	0.00	0.00	0.00	0.00

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	Worksheet 2G Vehicle-Pedestri	an Collisions for Urban and	l Suburban Arteria	Stop-Controlled Interse	ctions	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh- Ped ω	Calibration	Predicted N _{pedi}
	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	from Table 1B	factor, C _i	(4)*(5)*(6)
Total	0.39	0.01	0.41	0.02	1.04	0.01
Fatal and injury (FI)					1.04	0.01

	Worksheet 2J Vehicle	e-Bicycle Collisions for Urba	Worksheet 2J Vehicle-Bicycle Collisions for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)						
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration	Predicted N _{bikei}						
Crash Severity Level	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	(4) * (5)	factor, C _i	(4)*(5)*(6)						
Total	0.39	0.01	0.41	0.01	1.04	0.00						
Fatal and injury (FI)					1.04	0.00						

(1)	eet 2K Crash Severity Distribution for Urban an (2)	(3)	(4)
(.)	Fatal and injury (FI)	Property damage only (PDO)	Total
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;
•	(7) from Worksheet 2G and 2J	, ,	(7) from Worksheet 2G and 2J
	MULTIPLE-VEHICLE		
Rear-end collisions (from Worksheet 2D)	0.02	0.05	0.07
lead-on collisions (from Worksheet 2D)	0.00	0.00	0.00
angle collisions (from Worksheet 2D)	0.12	0.18	0.30
Sideswipe (from Worksheet 2D)	0.00	0.02	0.02
Other multiple-vehicle collision (from Worksheet 2D)	0.00	0.00	0.00
Subtotal	0.15	0.25	0.39
	SINGLE-VEHICLE		
Collision with parked vehicle (from Worksheet 2F)	0.00	0.00	0.00
Collision with animal (from Worksheet 2F)	0.00	0.00	0.00
Collision with fixed object (from Worksheet 2F)	0.00	0.00	0.01
Collision with other object (from Worksheet 2F)	0.00	0.00	0.00
Other single-vehicle collision (from Worksheet 2F)	0.00	0.00	0.00
single-vehicle noncollision (from Worksheet 2F)	0.00	0.00	0.00
Collision with pedestrian (from Worksheet 2G or 2I)	0.01	0.00	0.01
Collision with bicycle (from Worksheet 2J)	0.00	0.00	0.00
Subtotal	0.01	0.01	0.02
Total	0.16	0.25	0.42

Worksheet 2L Summary Re	esults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)
	(Total) from Worksheet 2K
Total	0.42
Fatal and injury (FI)	0.16
Property damage only (PDO)	0.25

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	Workshe	eet 2A General Informati	ion and Input	Data for Urban and Suburban A	rterial Interse	ections	
Ge	neral Information	on		Location Information			
Analyst		Ben Erban		City		Milton	
Agency or Company		CTPS		Intersection		Route 138 and Oak Street	
Date Performed		5/1/2018		Jurisdiction		MassDOT District 6	
				Analysis Year		2011	
Input Data				Base Conditions		Site Conditions	
Intersection type (3ST, 3SG, 4ST, 4SG)	_					4ST	
AADT _{major} (veh/day)		$AADT_{MAX} = 46800.00$	(veh/day)			9,850	
AADT _{minor} (veh/day)		$AADT_{MAX} = 5900.00$	(veh/day)			300	
ntersection lighting (present/not present)				Not Present		Not Present	
Calibration factor, C _i				1.00		1.04	
Data for unsignalized intersections only:							
Number of major-road approaches wit	n left-turn lanes	(0,1,2)			0		
Number of major-road approaches wit	n right-turn lanes	(0.1.2)			0		

	SUMMARY OF INTERSECTION & CRASHES									
CMF _{1i}	CMF _{2i}	CMF _{3i}	CMF 4i	CMF 5i	N _{bi_total}	N _{pedi_total}	N _{bikei_total}	N predictedint		
1.00	1.00	1.00	1.00	1.00	0.40	0.01	0.00	0.41		

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)				
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF				
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}				
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)				
1.00	1.00	1.00	1.00	1.00	1.00	1.00				

		Worksheet 2	C Multiple-	ehicle Collisions by Seve	rity Level for Urban	and Suburban Arterial In	tersections			
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
Crash Severity Level	SF	PF Coefficient	s	Overdispersion		Proportion of Total	Adjusted	Combined	Calibration	Predicted
				Parameter, k	Initial N _{bimv}	Crashes	N_{bimv}	CMFs	Factor, C _i	N _{bimv}
	fro	om Table 12-1)	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	ITOTTI TABLE 12-10	21		(T)TOTAL (O)	Worksheet 2B		(0) (1) (0)
Total	-8.70	0.31	0.86	0.36	0.38	1.00	0.38	1.00	1.04	0.39
Fatal and Injury (FI)					0.14		0.14	1.00	1.04	0.15
ratarana nijary (11)					0.11		0.11	1.00	1.01	0.10
Property Damage Only (PDO)					0.23		0.23	1.00	1.04	0.24

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(Fi)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C
Total	1.00	0.15	1.00	0.24	0.39
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Rear-end collision	0.16	0.02	0.19	0.05	0.07
Head-on collision	0.01	0.00	0.02	0.00	0.00
Angle collision	0.82	0.12	0.72	0.18	0.30
Sideswipe	0.02	0.00	0.08	0.02	0.02
Other multiple-vehicle collision	0.00	0.00	0.00	0.00	0.00

(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
, ,	SF	PF Coefficient	s	Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bimv}	Combined CMFs	Calibration Factor, C	Predicted N _{bisv}
Crash Severity Level	fro	om Table 12-12	2	ć - T.I. 44		0.0000	(4) _{TOTAL} *(5)	(7) from		(6)*(7)*(8)
	а	b	С	from Table 1A	(3) * (4) From Worksheet 2C		. , , , , ,	Worksheet 2B		
Γotal				0.03	0.01	1.00	0.01	1.00	1.04	0.01
Fatal and Injury (FI)	1			0.30	0.00		0.00	1.00	1.04	0.00
Property Damage Only PDO)				0.70	0.01		0.01	1.00	1.04	0.01

(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N bisv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(9)FI from Worksheet 2E	from Table 12-13	(9)PDO from Worksheet 2E	(9)PDO from Worksheet 2E
Total	1.00	0.00	1.00	0.01	0.01
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.00	0.00	0.14	0.00	0.00
Collision with animal	0.00	0.00	0.29	0.00	0.00
Collision with fixed object	0.68	0.00	0.57	0.00	0.01
Collision with other object	0.09	0.00	0.00	0.00	0.00
Other single-vehicle collision	0.05	0.00	0.00	0.00	0.00
Single-vehicle noncollision	0.18	0.00	0.00	0.00	0.00

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Works	Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Stop-Controlled Intersections							
(1)	(2)	(3)	(4)	(5)	(6)	(7)		
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh- Ped ω	Calibration	Predicted N _{pedi}		
Crash Seventy Level	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	from Table 1B	factor, C _i	(4)*(5)*(6)		
Total	0.39	0.01	0.40	0.02	1.04	0.01		
Fatal and injury (FI)					1.04	0.01		

	Worksheet 2J Vehicle-Bicycle Collisions for Urban and Suburban Arterial Intersections							
(1)	(2)	(3)	(4)	(5)	(6)	(7)		
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration	Predicted N _{bikei}		
Chash Seventy Level	(9) from Worksheet 2C	(9) from Worksheet 2E	(2) + (3)	(4) * (5)	factor, C _i	(4)*(5)*(6)		
Total	0.39	0.01	0.40	0.01	1.04	0.00		
Fatal and injury (FI)					1.04	0.00		

Worksh	eet 2K Crash Severity Distribution for Urban an	d Suburban Arterial Intersections	_
(1)	(2)	(3)	(4)
	Fatal and injury (FI)	Property damage only (PDO)	Total
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;
	(7) from Worksheet 2G and 2J		(7) from Worksheet 2G and 2J
	MULTIPLE-VEHICLE		
Rear-end collisions (from Worksheet 2D)	0.02	0.05	0.07
Head-on collisions (from Worksheet 2D)	0.00	0.00	0.00
Angle collisions (from Worksheet 2D)	0.12	0.18	0.30
Sideswipe (from Worksheet 2D)	0.00	0.02	0.02
Other multiple-vehicle collision (from Worksheet 2D)	0.00	0.00	0.00
Subtotal	0.15	0.24	0.39
	SINGLE-VEHICLE		
Collision with parked vehicle (from Worksheet 2F)	0.00	0.00	0.00
Collision with animal (from Worksheet 2F)	0.00	0.00	0.00
Collision with fixed object (from Worksheet 2F)	0.00	0.00	0.01
Collision with other object (from Worksheet 2F)	0.00	0.00	0.00
Other single-vehicle collision (from Worksheet 2F)	0.00	0.00	0.00
Single-vehicle noncollision (from Worksheet 2F)	0.00	0.00	0.00
Collision with pedestrian (from Worksheet 2G or 2I)	0.01	0.00	0.01
Collision with bicycle (from Worksheet 2J)	0.00	0.00	0.00
Subtotal	0.01	0.01	0.02
Total	0.16	0.25	0.41

Worksheet 2L Summary R	lesults for Urban and Suburban Arterial Intersections					
(1)	(2)					
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)					
	(Total) from Worksheet 2K					
Total	0.41					
Fatal and injury (FI)	0.16					
Property damage only (PDO)	0.25					

General Information	n	Location Information			
Analyst	Ben Erban	Intersection	Route 138 and Brook Road		
Agency or Company	CTPS	Intersection Type	3SG		
Date Performed	May-18	Jurisdiction	MassDOT Highwat District 6		
City	Milton	Analysis Year	2018		

		Input Inf	ormation		
Year	Oserved MV crashes	Observed total crashes	Predicted MV crashes	Predicted total crashes	Combined CMF for veh-ped crashes
2015	3	4	1.41	1.70	5.60
2014	1	1	1.40	1.69	5.60
2013	3	3	1.39	1.67	5.60
2012	2	2	1.37	1.65	5.60
2011	1	2	1.35	1.63	5.60

	Output Information										
Observed MV crashes	Average observed total crashes	Total predicted MV crashes	Average predicted total crashes	Standard deviation of predicted total crashes	Weight	Total expected MV crashes	No of expected total crashes	Average expected total crashes	High-risk Intersection (Y/N)	Safety	If avg observed total crashes > avg expected crashes
10.00	2.40	6.92	1.67	0.03	0.26	9.19	2.28 2.26 2.24 2.21 2.18	2.23	Y	0.56	Y

Works	sheet 2A General Information and Input	Data for Urban and Suburban Ar	terial Intersections		
General Informa	ation		Location Information		
Analyst	Ben Erban	City	Milton		
Agency or Company	CTPS	Intersection	Route 138 and Brook Road		
Date Performed	5/1/2018	Jurisdiction	MassDOT District 6		
		Analysis Year	2015		
Input Data		Base Conditions	Site Conditions		
ntersection type (3ST, 3SG, 4ST, 4SG)			3SG		
AADT _{major} (veh/day)	$AADT_{MAX} = 58,100 (veh/day)$		6,400		
AADT _{minor} (veh/day)	$AADT_{MAX} = 16,400$ (veh/day)		2,250		
Intersection lighting (present/not present)		Not Present	Not Present		
Calibration factor, C _i		1.00	0.95		
Data for signalized intersections only:					
Number of approaches with left-turn lanes (0,1,2,3,4) [for 3SG, use maximum value of 3]		0		
Number of approaches with right-turn lanes (0,1,2,3	,4) [for 3SG, use maximum value of 3]		0		
Number of approaches with left-turn signal phasing	[for 3SG, use maximum value of 3]		0		
Type of left-turn signal phasing for Leg #1		Permissive	Permissive		
Type of left-turn signal phasing for Leg #2			Permissive		
Type of left-turn signal phasing for Leg #3			Protected		
Type of left-turn signal phasing for Leg #4 (if applica	ible)		Not Applicable		
Number of approaches with right-turn-on-red prohib	ted [for 3SG, use maximum value of 3]	0	0		
Intersection red light cameras (present/not present)		Not Present	Not Present		
Sum of all pedestrian crossing volumes (PedVol)	Signalized intersections only		0		
Maximum number of lanes crossed by a pedestrian	(n _{lanesx})		2		
Number of bus stops within 300 m (1,000 ft) of the i	ntersection	0	4		
Schools within 300 m (1,000 ft) of the intersection (p	present/not present)	Not Present	Present		
Number of alcohol sales establishments within 300	m (1,000 ft) of the intersection	0	0		

	SUMMARY OF INTERSECTION & CRASHES										
CMF ₁i	CMF _{2i}	CMF _{3i}	CMF 4i	CMF 5i	CMF _{1p}	CMF _{2p}	CMF 3p	N _{bi_total}	N_{pedi_total}	N _{bikei_total}	N predictedint
1.000	0.940	1.000	1.000	1.000	4.150	1.350	1.000	1.509	0.164	0.031	1.704

	Wo	rksheet 2B Crash Modifica	ation Factors for Urban and Sub	urban Arterial Intersectio	ns	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
CMF for Left-Turn Lanes	CMF for Left-Turn Signal	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF
	Phasing					
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)
1.000	0.940	1.000	1.000	1.000	1.000	0.940

		Worksheet 2	C Multiple-	ehicle Collisions by Seve	rity Level for Urban	and Suburban Arterial In	tersections			
(1)	(2)			(3)	(4)	(5)	(6)	(7)	(8)	(9)
Crash Severity Level	SPF Coefficients		Overdispersion		Proportion of Total	Adjusted	Combined	Calibration	Predicted	
,				Parameter, k	Initial N _{bimv}	Crashes	N_{bimv}	CMFs	Factor, C _i	N_{bimv}
	from Table 12-10)	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from	1	(6)*(7)*(8)
	а	b	С	Hom rable 12-10	21		(T)TOTAL (S)	Worksheet 2B		(0) (1) (0)
Total	-8.299	0.812	0.212	0.407	1.580	1.000	1.580	0.940	0.950	1.411
Fatal and Injury (FI)					0.471		0.471	0.940	0.950	0.420
r diar drid injury (i i)					0.471		0.471	0.040	0.000	0.420
Property Damage Only (PDO)					1.109		1.109	0.940	0.950	0.990

	Worksheet 2D Multiple-\	ehicle Collisions by Collis	ion Type for Urban and Suburba	an Arterial Intersections	
(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C
Total	1.000	0.420	1.000	0.990	1.411
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Rear-end collision	0.580	0.244	0.605	0.599	0.843
Head-on collision	0.034	0.014	0.014	0.014	0.029
Angle collision	0.341	0.144	0.265	0.262	0.406
Sideswipe	0.039	0.016	0.116	0.115	0.131
Other multiple-vehicle collision	0.005	0.002	0.000	0.000	0.002

		Worksheet	2E Single-V	ehicle Collisions by Sever	ity Level for Urban a	and Suburban Arterial Int	ersections			
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
	SPF Coefficients		Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bisv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}	
Crash Severity Level		om Table 12-12		from Table 1A	(3) * (4) From		(4) _{TOTAL} *(5)	(7) from Worksheet 2B		(6)*(7)*(8)
	а	D	С		Worksheet 2C					
Total				0.070	0.110	1.000	0.110	0.940	0.950	0.098
Fatal and Injury (FI)				0.208	0.023		0.023	0.940	0.950	0.021
Property Damage Only (PDO)				0.792	0.087		0.087	0.940	0.950	0.078

	Worksheet 2F Single-V	ehicle Collisions by Collision	on Type for Urban and Suburba	n Arterial Intersections	
(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv} (FI) (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(a) from Worksheet 2F	from Table 12-13	(a) ppo from Worksheet 2F	(a)ppo from Worksheet 2F

	HUIH TADIC 12-13	(a)FI HOHI WOIKSHEEL ZE	110111 Table 12-13	(a)PDO HOITI WOINSHEEL ZL	(3)PDO HOIH WOINSHEEL ZE
Total	1.000	0.021	1.000	0.078	0.098
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.000	0.000	0.053	0.004	0.004
Collision with animal	0.000	0.000	0.000	0.000	0.000
Collision with fixed object	0.900	0.018	0.895	0.070	0.088
Collision with other object	0.000	0.000	0.026	0.002	0.002
Other single-vehicle collision	0.000	0.000	0.026	0.002	0.002
Single-vehicle noncollision	0.100	0.002	0.000	0.000	0.002

Worksheet 2G-1 Crash Modif	Worksheet 2G-1 Crash Modification Factors for Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Signalized Intersections										
(1)	(2)	(3)	(4)								
CMF for Bus Stops	CMF for Schools	CMF for Alcohol Sales Establishments	Combined CME								
CMF _{1p}	CMF _{2p}	CMF _{3p}	Combined CMF								
from Table 12-28	from Table 12-29	from Table 12-30	(1)*(2)*(3)								
4.150	1.350	1.000	5.603								

	Worksheet 2G Ve	ehicle-Pedestrian Collisions	for Urban and Sul	burban Arterial Intersecti	ons		
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Ped ω	Npedbase	Calibration factor, C _i	Predicted N _{pedi}
	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)				(4) from Worksheet 2H*(6)*(7)
Total	1.411	0.098	1.509	0.020	0.031	0.950	0.164
Fatal and injury (FI)						0.950	0.164

(1)	(2)	(3)	(4)	(5)	(6)	(7)
Crack Soverity Lovel	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration factor, C	Predicted N _{bikei}
Crash Severity Level	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	Veii-biκe ω	Cambration factor, O	(4)*(5)*(6)
Total	1.411	0.098	1.509	0.022	0.950	0.031
Fatal and injury (FI)					0.950	0.031

Worksheet 2K Crash Severity Distribution for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)						
	Fatal and injury (FI)	Property damage only (PDO)	Total						
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;						
	(8) from Worksheet 2G and (7) from 2J		(8) from Worksheet 2G and (7) from 2J						
	MULTIPLE-VEHICLE								
Rear-end collisions (from Worksheet 2D)	0.244	0.599	0.843						
Head-on collisions (from Worksheet 2D)	0.014	0.014	0.029						
Angle collisions (from Worksheet 2D)	0.144	0.262	0.406						
Sideswipe (from Worksheet 2D)	0.016	0.115	0.131						
Other multiple-vehicle collision (from Worksheet 2D)	0.002	0.000	0.002						
Subtotal	0.420	0.990	1.411						
	SINGLE-VEHICLE								
Collision with parked vehicle (from Worksheet 2F)	0.000	0.004	0.004						
Collision with animal (from Worksheet 2F)	0.000	0.000	0.000						
Collision with fixed object (from Worksheet 2F)	0.018	0.070	0.088						
Collision with other object (from Worksheet 2F)	0.000	0.002	0.002						
Other single-vehicle collision (from Worksheet 2F)	0.000	0.002	0.002						
Single-vehicle noncollision (from Worksheet 2F)	0.002	0.000	0.002						
Collision with pedestrian (from Worksheet 2G or 2I)	0.164	0.000	0.164						
Collision with bicycle (from Worksheet 2J)	0.031	0.000	0.031						
Subtotal	0.215	0.078	0.293						
Total	0.636	1.068	1.704						

Worksheet 2L Summary Results for Urban and Suburban Arterial Intersections						
(1)	(2)					
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)					
	Total					
Total	1.704					
Fatal and injury (FI)	0.636					
Property damage only (PDO)	1.068					

Worksh	neet 2A General Information and Input	Data for Urban and Suburban A	Arterial Intersections			
General Informat	ion	Location Information				
Analyst	Ben Erban	City	Milton			
Agency or Company	CTPS	Intersection	Route 138 and Brook Road			
Date Performed	5/1/2018	Jurisdiction MassDOT District				
		Analysis Year	2014			
Input Data		Base Conditions	Site Conditions			
Intersection type (3ST, 3SG, 4ST, 4SG)			3SG			
AADT _{major} (veh/day)	$AADT_{MAX} = 58,100 (veh/day)$		6,350			
AADT _{minor} (veh/day)	$AADT_{MAX} = 16,400$ (veh/day)		2,250			
Intersection lighting (present/not present)	•	Not Present	Not Present			
Calibration factor, C _i		1.00	0.95			
Data for signalized intersections only:						
Number of approaches with left-turn lanes (0,1,2,3,4)	[for 3SG, use maximum value of 3]		0			
Number of approaches with right-turn lanes (0,1,2,3,4) [for 3SG, use maximum value of 3]		0			
Number of approaches with left-turn signal phasing [f	or 3SG, use maximum value of 3]		0			
Type of left-turn signal phasing for Leg #1		Permissive	Permissive			
Type of left-turn signal phasing for Leg #2			Permissive			
Type of left-turn signal phasing for Leg #3			Protected			
Type of left-turn signal phasing for Leg #4 (if applicab	ile)		Not Applicable			
Number of approaches with right-turn-on-red prohibite	ed [for 3SG, use maximum value of 3]	0	0			
Intersection red light cameras (present/not present)		Not Present	Not Present			
Sum of all pedestrian crossing volumes (PedVol) \$	Signalized intersections only		0			
Maximum number of lanes crossed by a pedestrian (n _{lanesx})		2			
Number of bus stops within 300 m (1,000 ft) of the int	ersection	0	4			
Schools within 300 m (1,000 ft) of the intersection (pr	esent/not present)	Not Present	Present			
Number of alcohol sales establishments within 300 m	(1,000 ft) of the intersection	0	0			

	SUMMARY OF INTERSECTION & CRASHES										
CMF ₁i	CMF 1i CMF 2i CMF 3i CMF 4i CMF 5i CMF 1p CMF 2p CMF 3p N bi total N pedi total N bikei total N predictedin									N predictedint	
1.000	11 21 01 1 01 1 0 1 1 0 0 0 0 0 0 0 0 0										

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections											
(1) (2) (3) (4) (5) (6) (7)												
CMF for Left-Turn Lanes	CMF for Left-Turn Signal	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF						
	Phasing											
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}						
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)						
1.000	0.940	1.000	1.000	1.000	1.000	0.940						

	Worksheet 2C Multiple-Vehicle Collisions by Severity Level for Urban and Suburban Arterial Intersections												
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)			
Crash Severity Level	SF	PF Coefficient	s	Overdispersion		Proportion of Total	Adjusted	Combined	Calibration	Predicted			
,		2 223000.00		Parameter, k	Initial N _{bimv}	Crashes	N_{bimv}	CMFs	Factor, C _i	N_{bimv}			
	fro	om Table 12-10)	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from	1	(6)*(7)*(8)			
	а	b	С	Hom rable 12-10	21		(T)TOTAL (O)	Worksheet 2B		(0) (1) (0)			
Total	-8.299	0.812	0.212	0.407	1.570	1.000	1.570	0.940	0.950	1.402			
Fatal and Injury (FI)					0.468		0.468	0.940	0.950	0.418			
r didi dila mjary (i i)					0.400		0.400	0.040	0.000	0.410			
Property Damage Only (PDO)					1.102		1.102	0.940	0.950	0.984			

	Worksheet 2D Multiple-\	ehicle Collisions by Collis	ion Type for Urban and Suburba	an Arterial Intersections	
(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C
Total	1.000	0.418	1.000	0.984	1.402
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Rear-end collision	0.580	0.242	0.605	0.595	0.837
Head-on collision	0.034	0.014	0.014	0.014	0.029
Angle collision	0.341	0.143	0.265	0.261	0.403
Sideswipe	0.039	0.016	0.116	0.114	0.130
Other multiple-vehicle collision	0.005	0.002	0.000	0.000	0.002

	Worksheet 2E Single-Vehicle Collisions by Severity Level for Urban and Suburban Arterial Intersections											
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)		
	SPF Coefficients		Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bisv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}			
Crash Severity Level		om Table 12-12		from Table 1A	(3) * (4) From		(4) _{TOTAL} *(5)	(7) from Worksheet 2B		(6)*(7)*(8)		
	а	D	С		Worksheet 2C							
Total				0.070	0.110	1.000	0.110	0.940	0.950	0.098		
Fatal and Injury (FI)				0.208	0.023		0.023	0.940	0.950	0.020		
Property Damage Only (PDO)				0.792	0.087		0.087	0.940	0.950	0.077		

	Worksheet 2F Single-Ve	ehicle Collisions by Collision	on Type for Urban and Suburba	n Arterial Intersections	
(1)	(2)	(3)	(4)	(5)	(6)
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv} (FI) (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)
	from Table 12-13	(a) from Morkeheat 2F	from Table 12-13	(a) ppo from Worksheet 2F	(9)000 from Worksheet 2F

	110111 10010 12-10	(3)FI HOIH WOIKSHEEL ZL	IIVIII I ADIG 12-13	(a)PDO HOIH WOINSHEEL ZL	(a) PDU HOH WOINSHEEL ZL
Total	1.000	0.020	1.000	0.077	0.098
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.000	0.000	0.053	0.004	0.004
Collision with animal	0.000	0.000	0.000	0.000	0.000
Collision with fixed object	0.900	0.018	0.895	0.069	0.088
Collision with other object	0.000	0.000	0.026	0.002	0.002
Other single-vehicle collision	0.000	0.000	0.026	0.002	0.002
Single-vehicle noncollision	0.100	0.002	0.000	0.000	0.002

Wo	orksheet 2G-1 Crash Modific	ation Factors for Vehicle-Pe	edestrian Collisions for Urban and Suburban	Arterial Signalized Intersections						
	(1) (2) (3) (4)									
	CMF for Bus Stops	CMF for Schools	CMF for Alcohol Sales Establishments	Combined CME						
	CMF _{1p}	CMF _{2p}	CMF _{3p}	Combined CMF						
	from Table 12-28	from Table 12-29	from Table 12-30	(1)*(2)*(3)						
	4.150	1.350	1.000	5.603						

	Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)			
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Ped ω	Npedbase	Calibration factor, C _i	Predicted N _{pedi}			
	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)				(4) from Worksheet 2H*(6)*(7)			
Total	1.402	0.098	1.500	0.020	0.031	0.950	0.163			
Fatal and injury (FI)						0.950	0.163			

(1)	(2)	(3)	(4)	(5)	(6)	(7)				
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration factor, C	Predicted N _{bikei}				
Crash Seventy Level	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	Vell-bike w	Canbration factor, of	(4)*(5)*(6)				
Total	1.402	0.098	1.500	0.022	0.950	0.031				
Fatal and injury (FI)					0.950	0.031				

Worksheet 2K Crash Severity Distribution for Urban and Suburban Arterial Intersections								
(1)	(2)	(3)	(4)					
	Fatal and injury (FI)	Property damage only (PDO)	Total					
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;					
	(8) from Worksheet 2G and (7) from 2J		(8) from Worksheet 2G and (7) from 2J					
	MULTIPLE-VEHICLE	·						
Rear-end collisions (from Worksheet 2D)	0.242	0.595	0.837					
Head-on collisions (from Worksheet 2D)	0.014	0.014	0.029					
Angle collisions (from Worksheet 2D)	0.143	0.261	0.403					
Sideswipe (from Worksheet 2D)	0.016	0.114	0.130					
Other multiple-vehicle collision (from Worksheet 2D)	0.002	0.000	0.002					
Subtotal	0.418	0.984	1.402					
	SINGLE-VEHICLE	·	·					
Collision with parked vehicle (from Worksheet 2F)	0.000	0.004	0.004					
Collision with animal (from Worksheet 2F)	0.000	0.000	0.000					
Collision with fixed object (from Worksheet 2F)	0.018	0.069	0.088					
Collision with other object (from Worksheet 2F)	0.000	0.002	0.002					
Other single-vehicle collision (from Worksheet 2F)	0.000	0.002	0.002					
Single-vehicle noncollision (from Worksheet 2F)	0.002	0.000	0.002					
Collision with pedestrian (from Worksheet 2G or 2I)	0.163	0.000	0.163					
Collision with bicycle (from Worksheet 2J)	0.031	0.000	0.031					
Subtotal	0.214	0.077	0.291					
Total	0.632	1.062	1.693					

Worksheet 2L Summary R	esults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)
	Total
Total	1.693
Fatal and injury (FI)	0.632
Property damage only (PDO)	1.062

Worksh	eet 2A General Information and Input	Data for Urban and Suburban A	rterial Intersections
General Informati	ion		Location Information
Analyst	Ben Erban	City	Milton
Agency or Company	CTPS	Intersection	Route 138 and Brook Road
Date Performed	5/1/2018	Jurisdiction	MassDOT District 6
		Analysis Year	2013
Input Data		Base Conditions	Site Conditions
Intersection type (3ST, 3SG, 4ST, 4SG)			3SG
AADT _{major} (veh/day)	$AADT_{MAX} = 58,100$ (veh/day)		6,300
AADT _{minor} (veh/day)	$AADT_{MAX} = 16,400$ (veh/day)		2,200
Intersection lighting (present/not present)		Not Present	Not Present
Calibration factor, C _i		1.00	0.95
Data for signalized intersections only:			
Number of approaches with left-turn lanes (0,1,2,3,4)	[for 3SG, use maximum value of 3]		0
Number of approaches with right-turn lanes (0,1,2,3,4) [for 3SG, use maximum value of 3]		0
Number of approaches with left-turn signal phasing [for	or 3SG, use maximum value of 3]		0
Type of left-turn signal phasing for Leg #1		Permissive	Permissive
Type of left-turn signal phasing for Leg #2			Permissive
Type of left-turn signal phasing for Leg #3			Protected
Type of left-turn signal phasing for Leg #4 (if applicable	le)		Not Applicable
Number of approaches with right-turn-on-red prohibite	ed [for 3SG, use maximum value of 3]	0	0
Intersection red light cameras (present/not present)		Not Present	Not Present
Sum of all pedestrian crossing volumes (PedVol) S	ignalized intersections only		0
Maximum number of lanes crossed by a pedestrian (r	n _{lanesx})		2
Number of bus stops within 300 m (1,000 ft) of the inter-	ersection	0	4
Schools within 300 m (1,000 ft) of the intersection (pre	esent/not present)	Not Present	Present
Number of alcohol sales establishments within 300 m	(1,000 ft) of the intersection	0	0

	SUMMARY OF INTERSECTION & CRASHES										
CMF 1i	CMF _{2i}	CMF _{3i}	CMF 4i	CMF 5i	CMF _{1p}	CMF _{2p}	CMF 3p	N _{bi_total}	N_{pedi_total}	N _{bikei_total}	N predictedint
1.000	0.940	1.000	1.000	1.000	4.150	1.350	1.000	1.483	0.161	0.031	1.675

Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)			
CMF for Left-Turn Lanes	CMF for Left-Turn Signal	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF			
	Phasing								
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}			
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)			
1.000	0.940	1.000	1.000	1.000	1.000	0.940			

		Worksheet 2	C Multiple-	ehicle Collisions by Seve	rity Level for Urban	and Suburban Arterial In	tersections			
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
Crash Severity Level	SPF Coefficients		Overdispersion		Proportion of Total	Adjusted	Combined	Calibration	Predicted	
,				Parameter, k	Initial N _{bimv}	Crashes	N_{bimv}	CMFs	Factor, Ci	N_{bimv}
	fro	om Table 12-10)	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from	1	(6)*(7)*(8)
	а	b	С	Hom rable 12-10	21		(T)TOTAL (S)	Worksheet 2B		(0) (1) (0)
Total	-8.299	0.812	0.212	0.407	1.552	1.000	1.552	0.940	0.950	1.386
Fatal and Injury (FI)					0.463		0.463	0.940	0.950	0.413
ratarana mjary (11)					0.400		0.400	0.040	0.000	0.410
Property Damage Only (PDO)					1.090		1.090	0.940	0.950	0.973

	Worksheet 2D Multiple-\	ehicle Collisions by Collis	ion Type for Urban and Suburb	an Arterial Intersections		
(1)	(2)	(3)	(4)	(5)	(6)	
Collision Type	Proportion of Collision Type _(FI)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)	
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C	
Total	1.000	0.413	1.000	0.973	1.386	
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)	
Rear-end collision	0.580	0.240	0.605	0.588	0.828	
Head-on collision	0.034	0.014	0.014	0.014	0.028	
Angle collision	0.341	0.141	0.265	0.258	0.399	
Sideswipe	0.039	0.016	0.116	0.113	0.129	
Other multiple-vehicle collision	0.005	0.002	0.000	0.000	0.002	

(4)	1	Worksheet	2E Single-V	ehicle Collisions by Seve	-f	and Suburban Arterial Int		/7\	(0)	(0)
(1)	SF	(2) (3) SPF Coefficients Historical D		Historical Data	(4) Initial N _{bisv}	Proportion of Total Crashes	(6) Adjusted N _{bisv}	Combined CMFs	(8) Calibration Factor, C _i	(9) Predicted N _{bisv}
Crash Severity Level		om Table 12-12	2	from Table 1A	(3) * (4) From		(4) _{TOTAL} *(5)	(7) from Worksheet 2B		(6)*(7)*(8)
	а	D	Ü		Worksheet 2C					
Total				0.070	0.108	1.000	0.108	0.940	0.950	0.097
Fatal and Injury (FI)				0.208	0.023		0.023	0.940	0.950	0.020
Property Damage Only (PDO)				0.792	0.086		0.086	0.940	0.950	0.077

Worksheet 2F Single-Vehicle Collisions by Collision Type for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)	(5)	(6)				
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv} (FI) (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)				
	from Table 12-13	(a) from Morkeheat 2F	from Table 12-13	(a) ppo from Worksheet 2F	(9)000 from Worksheet 2F				

	HOIH TADIE 12-13	(a)FITIOTH WOLKSHEEL ZL	110111 1 abic 12-13	(שומטוו אוטווו אוטווסוופפנ בב	(3) PUU IIUIII VVUINSIIGGI ZL
Total	1.000	0.020	1.000	0.077	0.097
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.000	0.000	0.053	0.004	0.004
Collision with animal	0.000	0.000	0.000	0.000	0.000
Collision with fixed object	0.900	0.018	0.895	0.069	0.087
Collision with other object	0.000	0.000	0.026	0.002	0.002
Other single-vehicle collision	0.000	0.000	0.026	0.002	0.002
Single-vehicle noncollision	0.100	0.002	0.000	0.000	0.002

Worksheet 2G-1 Crash Modif	Worksheet 2G-1 Crash Modification Factors for Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Signalized Intersections								
(1)	(1) (2) (3) (4)								
CMF for Bus Stops	CMF for Schools	CMF for Alcohol Sales Establishments	Combined CME						
CMF _{1p}	CMF _{2p}	CMF _{3p}	Combined CMF						
from Table 12-28	from Table 12-29	from Table 12-30	(1)*(2)*(3)						
4.150	1.350	1.000	5.603						

	Worksheet 2G Ve	ehicle-Pedestrian Collisions	for Urban and Sul	burban Arterial Intersecti	ons		
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	cted N _{bi} Veh-Ped ω N _P o		Calibration factor, C _i	Predicted $N_{ m pedi}$
	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)				(4) from Worksheet 2H*(6)*(7)
Total	1.386	0.097	1.483	0.020	0.030	0.950	0.161
Fatal and injury (FI)						0.950	0.161

(1)	(2)	(3)	(4)	(5)	(6)	(7)
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration factor, C	Predicted N _{bikei}
Crash Seventy Level	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	Vell-bike w	Canbration factor, of	(4)*(5)*(6)
Total	1.386	0.097	1.483	0.022	0.950	0.031
Fatal and injury (FI)					0.950	0.031

Worksheet 2K Crash Severity Distribution for Urban and Suburban Arterial Intersections								
(1)	(2)	(3)	(4)					
	Fatal and injury (FI)	Property damage only (PDO)	Total					
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;					
	(8) from Worksheet 2G and (7) from 2J		(8) from Worksheet 2G and (7) from 2J					
	MULTIPLE-VEHICLE	·						
Rear-end collisions (from Worksheet 2D)	0.240	0.588	0.828					
Head-on collisions (from Worksheet 2D)	0.014	0.014	0.028					
Angle collisions (from Worksheet 2D)	0.141	0.258	0.399					
Sideswipe (from Worksheet 2D)	0.016	0.113	0.129					
Other multiple-vehicle collision (from Worksheet 2D)	0.002	0.000	0.002					
Subtotal	0.413	0.973	1.386					
	SINGLE-VEHICLE	·	•					
Collision with parked vehicle (from Worksheet 2F)	0.000	0.004	0.004					
Collision with animal (from Worksheet 2F)	0.000	0.000	0.000					
Collision with fixed object (from Worksheet 2F)	0.018	0.069	0.087					
Collision with other object (from Worksheet 2F)	0.000	0.002	0.002					
Other single-vehicle collision (from Worksheet 2F)	0.000	0.002	0.002					
Single-vehicle noncollision (from Worksheet 2F)	0.002	0.000	0.002					
Collision with pedestrian (from Worksheet 2G or 2I)	0.161	0.000	0.161					
Collision with bicycle (from Worksheet 2J)	0.031	0.000	0.031					
Subtotal	0.212	0.077	0.288					
Total	0.625	1.050	1.675					

Worksheet 2L Summary R	esults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)
	Total
Total	1.675
Fatal and injury (FI)	0.625
Property damage only (PDO)	1.050

Intersection-Year 24-2 Page 106

Worksh	eet 2A General Information and Input	Data for Urban and Suburban Ar	rterial Intersections		
General Informati	on		Location Information		
Analyst	Ben Erban	City	Milton		
Agency or Company	CTPS	Intersection	Route 138 and Brook Road		
Date Performed	5/1/2018	Jurisdiction	MassDOT District 6		
		Analysis Year	2012		
Input Data		Base Conditions	Site Conditions		
Intersection type (3ST, 3SG, 4ST, 4SG)			3SG		
AADT _{major} (veh/day)	$AADT_{MAX} = 58,100$ (veh/day)		6,200		
AADT _{minor} (veh/day)	$AADT_{MAX} = 16,400$ (veh/day)		2,200		
Intersection lighting (present/not present)		Not Present	Not Present		
Calibration factor, C _i		1.00	0.95		
Data for signalized intersections only:					
Number of approaches with left-turn lanes (0,1,2,3,4)	[for 3SG, use maximum value of 3]		0		
Number of approaches with right-turn lanes (0,1,2,3,4			0		
Number of approaches with left-turn signal phasing [for	or 3SG, use maximum value of 3]		0		
Type of left-turn signal phasing for Leg #1		Permissive	Permissive		
Type of left-turn signal phasing for Leg #2			Permissive		
Type of left-turn signal phasing for Leg #3			Protected		
Type of left-turn signal phasing for Leg #4 (if applicable	e)		Not Applicable		
Number of approaches with right-turn-on-red prohibite	d [for 3SG, use maximum value of 3]	0	0		
Intersection red light cameras (present/not present)		Not Present	Not Present		
Sum of all pedestrian crossing volumes (PedVol) S	ignalized intersections only		0		
Maximum number of lanes crossed by a pedestrian (n	lanesx)		2		
Number of bus stops within 300 m (1,000 ft) of the inte	ersection	0	4		
Schools within 300 m (1,000 ft) of the intersection (pre	esent/not present)	Not Present	Present		
Number of alcohol sales establishments within 300 m	(1,000 ft) of the intersection	0	0		

	SUMMARY OF INTERSECTION & CRASHES										
CMF ₁i	CMF _{1i} CMF _{2i} CMF _{3i} CMF _{4i} CMF _{5i} CMF _{1p} CMF _{2p} CMF _{3p} N _{bi total} N _{pedi total} N _{bikei total} N _{predictedint}								N predictedint		
1.000	0.940	1.000	1.000	1.000	4.150	1.350	1.000	1.464	0.159	0.030	1.653

	Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections									
(1)	(1) (2) (3) (4) (5) (6) (7)									
CMF for Left-Turn Lanes	CMF for Left-Turn Signal	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF				
	Phasing									
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}				
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)				
1.000	0.940	1.000	1.000	1.000	1.000	0.940				

Worksheet 2C Multiple-Vehicle Collisions by Severity Level for Urban and Suburban Arterial Intersections										
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
Crash Severity Level	SF	PF Coefficient	s	Overdispersion		Proportion of Total	Adjusted	Combined	Calibration	Predicted
-				Parameter, k	Initial N _{bimv}	Crashes	N_{bimv}	CMFs	Factor, C _i	N_{bimv}
	fro	om Table 12-10)	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from	1	(6)*(7)*(8)
	а	b	С	ITOTTI TABLE 12-10	21		(4)101AL (3)	Worksheet 2B		(0) (1) (0)
Total	-8.299	0.812	0.212	0.407	1.532	1.000	1.532	0.940	0.950	1.368
Fatal and Injury (FI)					0.457		0.457	0.940	0.950	0.408
r didi dila mjary (i i)					0.407		0.407	0.040	0.000	0.400
Property Damage Only (PDO)					1.076		1.076	0.940	0.950	0.961

	Worksheet 2D Multiple-\	ehicle Collisions by Collis	ion Type for Urban and Suburba	an Arterial Intersections	
(1)	(2)	(3)		(5)	(6)
Collision Type	Proportion of Collision Type(FI)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C
Total	1.000	0.408	1.000	0.961	1.368
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Rear-end collision	0.580	0.237	0.605	0.581	0.817
Head-on collision	0.034	0.014	0.014	0.014	0.028
Angle collision	0.341	0.139	0.265	0.255	0.394
Sideswipe	0.039	0.016	0.116	0.111	0.127
Other multiple-vehicle collision	0.005	0.002	0.000	0.000	0.002

		Worksheet	2E Single-V	ehicle Collisions by Sever	ity Level for Urban a	and Suburban Arterial Int	ersections	•	•	
(1)	(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)	
	SI	PF Coefficients	S	Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bisv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}
Crash Severity Level		om Table 12-12		from Table 1A	(3) * (4) From		(4) _{TOTAL} *(5)	(7) from Worksheet 2B		(6)*(7)*(8)
	а	D	С		Worksheet 2C					
Total				0.070	0.107	1.000	0.107	0.940	0.950	0.095
Fatal and Injury (FI)				0.208	0.022		0.022	0.940	0.950	0.020
Property Damage Only (PDO)				0.792	0.085		0.085	0.940	0.950	0.076

	Worksheet 2F Single-Vehicle Collisions by Collision Type for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)	(5)	(6)					
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv} (FI) (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)					
	from Table 12-13	(a) from Morkeheat 2F	from Table 12-13	(a) ppo from Worksheet 2F	(9)000 from Worksheet 2F					

	IIUIII I ADIG 12-10	(a)FI HOIH WOIKSHEEL ZL	IIUIII TADIG 12-10	(a)PDO HOHI WORKSHEEL ZL	(a)PDO HOIH VVOIRSHEEL ZL
Total	1.000	0.020	1.000	0.076	0.095
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.000	0.000	0.053	0.004	0.004
Collision with animal	0.000	0.000	0.000	0.000	0.000
Collision with fixed object	0.900	0.018	0.895	0.068	0.086
Collision with other object	0.000	0.000	0.026	0.002	0.002
Other single-vehicle collision	0.000	0.000	0.026	0.002	0.002
Single-vehicle noncollision	0.100	0.002	0.000	0.000	0.002

Worksheet 2G-1 Crash Modif	Worksheet 2G-1 Crash Modification Factors for Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Signalized Intersections								
(1)	(1) (2) (3) (4)								
CMF for Bus Stops	CMF for Schools	CMF for Alcohol Sales Establishments	Combined CME						
CMF _{1p}	CMF _{2p}	CMF _{3p}	Combined CMF						
from Table 12-28	from Table 12-29	from Table 12-30	(1)*(2)*(3)						
4.150	1.350	1.000	5.603						

	Worksheet 2G Ve	ehicle-Pedestrian Collisions	for Urban and Sul	burban Arterial Intersecti	ons		
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Ped ω	Npedbase	Calibration factor, C _i	Predicted $N_{ m pedi}$
	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)				(4) from Worksheet 2H*(6)*(7)
Total	1.368	0.095	1.464	0.020	0.030	0.950	0.159
Fatal and injury (FI)						0.950	0.159

(1)	(2)	(3)	(4)	(5)	(6)	(7)
Crash Severity Level	Predicted N _{bimv}		Predicted N _{bi}	Veh-Bike ω Calibration factor, C _i		Predicted N _{bikei}
Crash Severity Level	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	Vell-blke w	Cambration factor, O	(4)*(5)*(6)
Total	1.368	0.095	1.464	0.022	0.950	0.030
Fatal and injury (FI)					0.950	0.030

Worksheet 2K Crash Severity Distribution for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)						
	Fatal and injury (FI)	Property damage only (PDO)	Total						
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;						
	(8) from Worksheet 2G and (7) from 2J		(8) from Worksheet 2G and (7) from 2J						
	MULTIPLE-VEHICLE	·							
Rear-end collisions (from Worksheet 2D)	0.237	0.581	0.817						
Head-on collisions (from Worksheet 2D)	0.014	0.014	0.028						
Angle collisions (from Worksheet 2D)	0.139	0.255	0.394						
Sideswipe (from Worksheet 2D)	0.016	0.111	0.127						
Other multiple-vehicle collision (from Worksheet 2D)	0.002	0.000	0.002						
Subtotal	0.408	0.961	1.368						
	SINGLE-VEHICLE								
Collision with parked vehicle (from Worksheet 2F)	0.000	0.004	0.004						
Collision with animal (from Worksheet 2F)	0.000	0.000	0.000						
Collision with fixed object (from Worksheet 2F)	0.018	0.068	0.086						
Collision with other object (from Worksheet 2F)	0.000	0.002	0.002						
Other single-vehicle collision (from Worksheet 2F)	0.000	0.002	0.002						
Single-vehicle noncollision (from Worksheet 2F)	0.002	0.000	0.002						
Collision with pedestrian (from Worksheet 2G or 2I)	0.159	0.000	0.159						
Collision with bicycle (from Worksheet 2J)	0.030	0.000	0.030						
Subtotal	0.209	0.076	0.284						
「otal	0.617	1.036	1.653						

Worksheet 2L Summary R	esults for Urban and Suburban Arterial Intersections
(1)	(2)
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)
	Total
Total	1.653
Fatal and injury (FI)	0.617
Property damage only (PDO)	1.036

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Worksh	eet 2A General Information and Input	Data for Urban and Suburban A	rterial Intersections		
General Informati	ion		Location Information		
Analyst	Ben Erban	City	Milton		
Agency or Company	CTPS	Intersection	Route 138 and Brook Road		
Date Performed	5/1/2018	Jurisdiction	MassDOT District 6		
		Analysis Year 2011			
Input Data		Base Conditions	Site Conditions		
Intersection type (3ST, 3SG, 4ST, 4SG)			3SG		
AADT _{major} (veh/day)	$AADT_{MAX} = 58,100 \text{(veh/day)}$		6,150		
AADT _{minor} (veh/day)	$AADT_{MAX} = 16,400$ (veh/day)		2,150		
Intersection lighting (present/not present)		Not Present	Not Present		
Calibration factor, C _i		1.00	0.95		
Data for signalized intersections only:					
Number of approaches with left-turn lanes (0,1,2,3,4)	[for 3SG, use maximum value of 3]		0		
Number of approaches with right-turn lanes (0,1,2,3,4) [for 3SG, use maximum value of 3]		0		
Number of approaches with left-turn signal phasing [for	or 3SG, use maximum value of 3]		0		
Type of left-turn signal phasing for Leg #1		Permissive	Permissive		
Type of left-turn signal phasing for Leg #2			Permissive		
Type of left-turn signal phasing for Leg #3			Protected		
Type of left-turn signal phasing for Leg #4 (if applicable	le)		Not Applicable		
Number of approaches with right-turn-on-red prohibite	ed [for 3SG, use maximum value of 3]	0	0		
Intersection red light cameras (present/not present)		Not Present	Not Present		
Sum of all pedestrian crossing volumes (PedVol) S	ignalized intersections only		0		
Maximum number of lanes crossed by a pedestrian (r	n _{lanesx})		2		
Number of bus stops within 300 m (1,000 ft) of the interest of	ersection	0	4		
Schools within 300 m (1,000 ft) of the intersection (pre	esent/not present)	Not Present	Present		
Number of alcohol sales establishments within 300 m	(1,000 ft) of the intersection	0	0		

SUMMARY OF INTERSECTION & CRASHES											
CMF 1i	CMF _{2i}	CMF _{3i}	CMF ₄i	CMF 5i	CMF _{1p}	CMF _{2p}	CMF 3p	N _{bi_total}	N_{pedi_total}	N _{bikei_total}	N predictedint
1.000	0.940	1.000	1.000	1.000	4.150	1.350	1.000	1.447	0.157	0.030	1.634

	Wo	rkshoot 2B Crash Modific	ation Factors for Urban and Sub	urhan Arterial Intersectio	ne			
Worksheet 2B Crash Modification Factors for Urban and Suburban Arterial Intersections (1) (2) (3) (4) (5) (6) (7)								
CMF for Left-Turn Lanes	CMF for Left-Turn Signal Phasing	CMF for Right-Turn Lanes	CMF for Right Turn on Red	CMF for Lighting	CMF for Red Light Cameras	Combined CMF		
CMF 1i	CMF 2i	CMF 3i	CMF 4i	CMF 5i	CMF 6i	CMF _{COMB}		
from Table 12-24	from Table 12-25	from Table 12-26	from Equation 12-35	from Equation 12-36	from Equation 12-37	(1)*(2)*(3)*(4)*(5)*(6)		
1.000	0.940	1.000	1.000	1.000	1.000	0.940		

		Worksheet 2	C Multiple-	ehicle Collisions by Seve	rity Level for Urban	and Suburban Arterial In	tersections			
(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)
Crash Severity Level	SF	SPF Coefficients		Overdispersion		Proportion of Total	Adjusted	Combined	Calibration	Predicted
-				Parameter, k	Initial N _{bimv}	Crashes	N_{bimv}	CMFs	Factor, Ci	N_{bimv}
	fro	om Table 12-10)	from Table 12-10	from Equation 12-		(4) _{TOTAL} *(5)	(7) from	1	(6)*(7)*(8)
	а	b	С	Hom rable 12-10	21		(T)TOTAL (O)	Worksheet 2B		(0) (1) (0)
Total	-8.299	0.812	0.212	0.407	1.515	1.000	1.515	0.940	0.950	1.353
Fatal and Injury (FI)					0.451		0.451	0.940	0.950	0.403
ratarana mjary (11)					0.401		0.401	0.040	0.000	0.400
Property Damage Only (PDO)					1.064		1.064	0.940	0.950	0.950

Worksheet 2D Multiple-Vehicle Collisions by Collision Type for Urban and Suburban Arterial Intersections											
(1)	(2)	(3)	(4)	(5)	(6)						
Collision Type	Proportion of Collision Type _(FI)	Adjusted N bimv (FI) (crashes/year)	Proportion of Collision Type (PDO)	Adjusted N bimv (PDO) (crashes/year)	Adjusted N _{bimv (TOTAL)} (crashes/year)						
	from Table 12-11	(9)FI from Worksheet 2C	from Table 12-11	(9)PDO from Worksheet 2C	(9) _{Nbimv} from Worksheet 2C						
Total	1.000	0.403	1.000	0.950	1.353						
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)						
Rear-end collision	0.580	0.234	0.605	0.574	0.808						
Head-on collision	0.034	0.014	0.014	0.014	0.028						
Angle collision	0.341	0.138	0.265	0.252	0.389						
Sideswipe	0.039	0.016	0.116	0.110	0.126						
Other multiple-vehicle collision	0.005	0.002	0.000	0.000	0.002						

(1)	Worksheet 2E Single-Ve		(3)	(4)	(5)	(6)	(7)	(8)	(9)	
	SF	PF Coefficients	S	Historical Data	Initial N _{bisv}	Proportion of Total Crashes	Adjusted N _{bisv}	Combined CMFs	Calibration Factor, C _i	Predicted N _{bisv}
Crash Severity Level		om Table 12-12	_	from Table 1A	(3) * (4) From		(4) _{TOTAL} *(5)	(7) from Worksheet 2B		(6)*(7)*(8)
	а	D	С		Worksheet 2C					
Total				0.070	0.106	1.000	0.106	0.940	0.950	0.094
Fatal and Injury (FI)				0.208	0.022		0.022	0.940	0.950	0.020
Property Damage Only (PDO)				0.792	0.084		0.084	0.940	0.950	0.075

Worksheet 2F Single-Vehicle Collisions by Collision Type for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)	(5)	(6)				
Collision Type	Proportion of Collision Type _(FI)	Predicted N _{bisv} (FI) (crashes/year)	Proportion of Collision Type (PDO)	Predicted N bisv (PDO) (crashes/year)	Predicted N _{bisv (TOTAL)} (crashes/year)				
	from Table 12-13	(a) from Morkeheat 2F	from Table 12-13	(a) ppo from Worksheet 2F	(9)000 from Worksheet 2F				

	HOIH TADIE 12-13	(3)FITIOHI WOINSHEEL ZL	110111 1 abic 12-13	(a)PDO HOIH WORKSHEEL ZE	(a)PDO HOTH WOINSHEEL ZL
Total	1.000	0.020	1.000	0.075	0.094
		(2)*(3) _{FI}		(4)*(5) _{PDO}	(3)+(5)
Collision with parked vehicle	0.000	0.000	0.053	0.004	0.004
Collision with animal	0.000	0.000	0.000	0.000	0.000
Collision with fixed object	0.900	0.018	0.895	0.067	0.085
Collision with other object	0.000	0.000	0.026	0.002	0.002
Other single-vehicle collision	0.000	0.000	0.026	0.002	0.002
Single-vehicle noncollision	0.100	0.002	0.000	0.000	0.002

Worksheet 2G-1 Crash Modification Factors for Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Signalized Intersections										
(1)	(2)	(3)	(4)							
CMF for Bus Stops	CMF for Schools	CMF for Alcohol Sales Establishments	Combined CME							
CMF _{1p}	CMF _{2p}	CMF _{3p}	Combined CMF							
from Table 12-28	from Table 12-29	from Table 12-30	(1)*(2)*(3)							
4.150	1.350	1.000	5.603							

	Worksheet 2G Vehicle-Pedestrian Collisions for Urban and Suburban Arterial Intersections											
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)					
Crash Severity Level	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Ped ω	Npedbase	Calibration factor, C _i	Predicted N _{pedi}					
	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)				(4) from Worksheet 2H*(6)*(7)					
Total	1.353	0.094	1.447	0.020	0.029	0.950	0.157					
Fatal and injury (FI)						0.950	0.157					

Translated 20 Taillate Biogram Collection of State and Cabandan Factorial Interceptions										
(1)	(2)	(3)	(4)	(5)	(6)	(7)				
Crash Savarity Laval	Predicted N _{bimv}	Predicted N _{bisv}	Predicted N _{bi}	Veh-Bike ω	Calibration factor, C	Predicted N _{bikei}				
Crash Severity Level	(9) from Worksheet 2C	(4) from Worksheet 2E	(2) + (3)	Vell-bike w	Cambration factor, of	(4)*(5)*(6)				
Total	1.353	0.094	1.447	0.022	0.950	0.030				
Fatal and injury (FI)					0.950	0.030				

Worksheet 2K Crash Severity Distribution for Urban and Suburban Arterial Intersections									
(1)	(2)	(3)	(4)						
	Fatal and injury (FI)	Property damage only (PDO)	Total						
Collision type	(3) from Worksheet 2D and 2F;	(5) from Worksheet 2D and 2F	(6) from Worksheet 2D and 2F;						
	(8) from Worksheet 2G and (7) from 2J		(8) from Worksheet 2G and (7) from 2J						
	MULTIPLE-VEHICLE	·							
Rear-end collisions (from Worksheet 2D)	0.234	0.574	0.808						
Head-on collisions (from Worksheet 2D)	0.014	0.014	0.028						
Angle collisions (from Worksheet 2D)	0.138	0.252	0.389						
Sideswipe (from Worksheet 2D)	0.016	0.110	0.126						
Other multiple-vehicle collision (from Worksheet 2D)	0.002	0.000	0.002						
Subtotal	0.403	0.950	1.353						
	SINGLE-VEHICLE	·	·						
Collision with parked vehicle (from Worksheet 2F)	0.000	0.004	0.004						
Collision with animal (from Worksheet 2F)	0.000	0.000	0.000						
Collision with fixed object (from Worksheet 2F)	0.018	0.067	0.085						
Collision with other object (from Worksheet 2F)	0.000	0.002	0.002						
Other single-vehicle collision (from Worksheet 2F)	0.000	0.002	0.002						
Single-vehicle noncollision (from Worksheet 2F)	0.002	0.000	0.002						
Collision with pedestrian (from Worksheet 2G or 2I)	0.157	0.000	0.157						
Collision with bicycle (from Worksheet 2J)	0.030	0.000	0.030						
Subtotal	0.207	0.075	0.281						
Total	0.610	1.024	1.634						

Worksheet 2L Summary Results for Urban and Suburban Arterial Intersections						
(1)	(2)					
Crash severity level	Predicted average crash frequency, N _{predicted int} (crashes/year)					
	Total					
Total	1.634					
Fatal and injury (FI)	0.610					
Property damage only (PDO)	1.024					

Hig	hway	y Safety Software	e Urban Seg	gment Repor	t	
Project Information						
Analyst	Ben	Erban	Date		5/21,	/2018
Jurisdiction	Mass	DOT Hwy District 6	Analysis Year		2018	
Project Description	Prior	ity Corridors - Route 138 ir	n Milton			
Input Data						
Segment Type		Two-Lane Undivided Seg	ment (2U)			
Length of Segment (mi)		0.13	AADT (veh/day)			33950
Median Width (ft)		2	Lighting			No
Type of On-street Parking		None	Proportion w/On-	-street Parking		0.00
Automated Speed Enforcement		No	Posted Speed (m	ph)		45
Roadside Fixed Object Density		30	Offset to Roadsid	e Fixed Obj. (ft)		15
# Major Commercial Driveways		0	# Minor Commer	cial Driveways		1
# Major Industrial/Insti. Driveways		0	# Minor Industria	l/Insti. Driveways		0
# Major Residential Driveways		0	# Minor Resident	ial Driveways		0
# Other Driveways		0	Calibration Factor			1.00
Crash Modification Facto	rs					
On-Street Parking - CMF1		1.000	Lighting - CMF4			1.000
Roadside Fixed Objects - CMF2		1.061	Automated Speed Enforcement - CMF5		5	1.000
Median Width - CMF3		1.000	Combined CMF			1.061
Predicted Roadway Section	on Cra	shes				
Crash Severity		Predicted Crash Freq	uency	Crash Rate (crashes/mi/year)		
Fatal and Injury (FI)	0.481			3.702		
Property Damage Only (PDO)	1.240			9.539		
Total	1.721			13.241		
Expected Roadway Section	n Cra	shes				
Crash Severity		Expected Crash Freq	uency	Expected Cra	ısh Rat	te (crashes/mi/year)
Fatal and Injury (FI)	0.392			3.017		
Property Damage Only (PDO)	1.011			7.775		
Total	1.403 10.792					
Economic Analysis (Expe	ted C	rashes)				
Crash Severity	Per	Crash Societal Crash Cost	Expected A	nnual Crashes	To	otal Societal Crash Cost
Fatal and Injury (FI)	_	3,200.00	0.392			048.26
Property Damage Only (PDO)		00.00	1.011			79.82
Total	-		1.403			528.07
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Hig	hway	y Safety Software	e Urban Seg	gment Repor	t	
Project Information						
Analyst	Ben	Erban	Date		5/21,	/2018
Jurisdiction	Mass	DOT Hwy District 6	Analysis Year		2018	
Project Description	Prior	ity Corridors - Route 138 ir	n Milton			
Input Data						
Segment Type		Two-Lane Undivided Seg	ment (2U)			
Length of Segment (mi)		0.20	AADT (veh/day)			33950
Median Width (ft)		2	Lighting			No
Type of On-street Parking		None	Proportion w/On-	-street Parking		0.00
Automated Speed Enforcement		No	Posted Speed (m	ph)		45
Roadside Fixed Object Density		30	Offset to Roadsid	e Fixed Obj. (ft)		15
# Major Commercial Driveways		0	# Minor Commer	cial Driveways		1
# Major Industrial/Insti. Driveways		0	# Minor Industria	l/Insti. Driveways		1
# Major Residential Driveways		1	# Minor Residential Driveways			5
# Other Driveways (0	Calibration Factor			1.00
Crash Modification Facto	rs					
On-Street Parking - CMF1		1.000	Lighting - CMF4			1.000
Roadside Fixed Objects - CMF2		1.061	Automated Speed Enforcement - CMF5		5	1.000
Median Width - CMF3		1.000	Combined CMF		1.061	
Predicted Roadway Section	on Cra	shes				
Crash Severity		Predicted Crash Freq	uency	Crash Rate (crashes/mi/year)		
Fatal and Injury (FI)	0.867			4.336		
Property Damage Only (PDO)	2.167			10.833		
Total	3.034			15.169		
Expected Roadway Section	n Cra	shes				
Crash Severity		Expected Crash Freq	uency	Expected Cra	ısh Rat	te (crashes/mi/year)
Fatal and Injury (FI)	1.421			7.106		
Property Damage Only (PDO)	3.551			17.754		
Total	4.972			24.860		
Economic Analysis (Expe	ted C	rashes)				
				otal Societal Crash Cost		
Fatal and Injury (FI)	\$158	3,200.00	1.421		\$224	l,835.70
Property Damage Only (PDO)		00.00	3.551			275.83
Total	-		4.972			,111.53
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Hig	hway	y Safety Software	e Urban Seg	gment Repor	t		
Project Information							
Analyst	Ben	Erban	Date		5/21/2018		
Jurisdiction	Mass	DOT Hwy District 6	Analysis Year		2018	3	
Project Description	Prior	ity Corridors - Route 138 ir	Milton				
Input Data							
Segment Type		Two-Lane Undivided Seg	ment (2U)				
Length of Segment (mi)		0.44	AADT (veh/day)			21000	
Median Width (ft)		2	Lighting			No	
Type of On-street Parking		None	Proportion w/On-	-street Parking		0.00	
Automated Speed Enforcement		No	Posted Speed (m	ph)		45	
Roadside Fixed Object Density		30	Offset to Roadsid	e Fixed Obj. (ft)		15	
# Major Commercial Driveways		0	# Minor Commer	cial Driveways		0	
# Major Industrial/Insti. Driveways		0	# Minor Industria	l/Insti. Driveways		0	
# Major Residential Driveways		1	# Minor Resident	ial Driveways		10	
# Other Driveways		0	Calibration Factor			1.00	
Crash Modification Facto	rs						
On-Street Parking - CMF1		1.000	Lighting - CMF4			1.000	
Roadside Fixed Objects - CMF2		1.061	Automated Speed Enforcement - CMF5		5	1.000	
Median Width - CMF3		1.000	Combined CMF			1.061	
Predicted Roadway Section	on Cra	shes					
Crash Severity		Predicted Crash Freq	uency	Crash Rate (crashes/mi/year)			
Fatal and Injury (FI)	0.841			1.911			
Property Damage Only (PDO)	2.157			4.903			
Total	2.998			6.814			
Expected Roadway Section	n Cra	shes					
Crash Severity		Expected Crash Freq	uency	Expected Cra	sh Rat	te (crashes/mi/year)	
Fatal and Injury (FI)	0.627			1.425			
Property Damage Only (PDO)	1.608			3.655			
Total	2.235		5.080				
Economic Analysis (Expe	ted C	rashes)					
Crash Severity	Per	Crash Societal Crash Cost	Expected A	nnual Crashes	To	otal Societal Crash Cost	
Fatal and Injury (FI)	_	3,200.00	0.627			176.52	
Property Damage Only (PDO)		00.00	1.608	\$11,899.90			
Total	-		2.235			,076.42	
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Hi	ghwa	y Safety Softwa	re Urban Seg	gment Repo	rt
Project Information					
Analyst	Ben	Erban	Date		5/21/2018
Jurisdiction	Mas	sDOT Hwy District 6	Analysis Year		2018
Project Description	Prio	rity Corridors - Route 138	in Milton		-
Input Data					
Segment Type		Two-Lane Undivided Se	gment (2U)		
Length of Segment (mi)		0.23	AADT (veh/day)		14300
Median Width (ft)		2	Lighting		No
Type of On-street Parking		None	Proportion w/On	-street Parking	0.00
Automated Speed Enforcement		No	Posted Speed (m	ph)	35
Roadside Fixed Object Density		30	Offset to Roadsid	le Fixed Obj. (ft)	15
# Major Commercial Driveways		0	# Minor Commer	cial Driveways	0
# Major Industrial/Insti. Driveways		0	# Minor Industria	ıl/Insti. Driveways	0
# Major Residential Driveways		0	# Minor Resident	# Minor Residential Driveways	
# Other Driveways		0	Calibration Facto	r	1.00
Crash Modification Factor	ors				
On-Street Parking - CMF1		1.000 Lighting - CMF4			1.000
Roadside Fixed Objects - CMF2		1.061	Automated Speed Enforcement - CMF5		F5 1.000
Median Width - CMF3		1.000	Combined CMF		1.061
Predicted Roadway Sect	ion Cra	ashes			
Crash Severity	Τ	Predicted Crash Fre	equency	Crash R	ate (crashes/mi/year)
Fatal and Injury (FI)	0.219			0.950	
Property Damage Only (PDO)	0.580			2.523	
Total	0.799			3.474	
Expected Roadway Secti	on Cra	shes		<u>'</u>	
Crash Severity	Т	Expected Crash Fre	quency	Expected Cra	ash Rate (crashes/mi/year)
Fatal and Injury (FI)	0.310	· · · · · · · · · · · · · · · · · · ·	· · · ·	1.346	
Property Damage Only (PDO)	0.822			3.575	
Total	1.132			4.922	
Economic Analysis (Expe	cted C	Crashes)		<u>'</u>	
Crash Severity	Per	Crash Societal Crash Cos	Expected A	nnual Crashes	Total Societal Crash Cost
Fatal and Injury (FI)		3,200.00	0.310		\$48,991.62
Property Damage Only (PDO)		00.00	0.822		\$6,085.16
Property Damage Only (PDO)	Ψ1,1				1 - 7

Hig	hway	y Safety Software	e Urban Seg	gment Repor	t	
Project Information						
Analyst	Ben	Erban	Date		5/21,	/2018
Jurisdiction	Mass	DOT Hwy District 6	Analysis Year		2018	3
Project Description	Prior	ity Corridors - Route 138 ir	n Milton			
Input Data						
Segment Type		Two-Lane Undivided Seg	ment (2U)			
Length of Segment (mi)		0.28	AADT (veh/day)			16000
Median Width (ft)		2	Lighting			No
Type of On-street Parking		None	Proportion w/On-	-street Parking		0.00
Automated Speed Enforcement		No	Posted Speed (m	ph)		45
Roadside Fixed Object Density		30	Offset to Roadsid	e Fixed Obj. (ft)		15
# Major Commercial Driveways		0	# Minor Commer	cial Driveways		1
# Major Industrial/Insti. Driveways		0	# Minor Industria	l/Insti. Driveways		0
# Major Residential Driveways		0	# Minor Resident	ial Driveways		4
# Other Driveways		0	Calibration Factor	r		1.00
Crash Modification Facto	rs					
On-Street Parking - CMF1	1.000 Lighting - CMF4 1.000				1.000	
Roadside Fixed Objects - CMF2		1.061	Automated Speed	d Enforcement - CMF	5	1.000
Median Width - CMF3		1.000	Combined CMF			1.061
Predicted Roadway Section	on Cra	shes				
Crash Severity		Predicted Crash Freq	uency	Crash R	ate (cr	rashes/mi/year)
Fatal and Injury (FI)	0.354			1.263		
Property Damage Only (PDO)	0.913			3.261		
Total	1.267			4.524		
Expected Roadway Section	n Cra	shes				
Crash Severity		Expected Crash Freq	uency	Expected Cra	ısh Rat	te (crashes/mi/year)
Fatal and Injury (FI)	0.349			1.247		·
Property Damage Only (PDO)	0.902			3.221		
Total	1.251			4.468		
Economic Analysis (Expe	ted C	rashes)				
Crash Severity	_	Crash Societal Crash Cost	Expected A	nnual Crashes	To	otal Societal Crash Cost
Fatal and Injury (FI)		3,200.00	0.349			239.39
Property Damage Only (PDO)		00.00	0.902		\$6,67	
Total	-		1.251			912.90
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Hig	hway	y Safety Software	e Urban Seg	gment Repor	t		
Project Information							
Analyst	Ben	Erban	Date		5/21,	5/21/2018	
Jurisdiction	Mass	sDOT Hwy District 6	Analysis Year		2018	3	
Project Description	Prior	ity Corridors - Route 138 ir	n Milton				
Input Data							
Segment Type		Two-Lane Undivided Seg	ment (2U)				
Length of Segment (mi)		0.27	AADT (veh/day)			16000	
Median Width (ft)		2	Lighting			No	
Type of On-street Parking		None	Proportion w/On-	-street Parking		0.00	
Automated Speed Enforcement		No	Posted Speed (m	ph)		45	
Roadside Fixed Object Density		30	Offset to Roadsid	e Fixed Obj. (ft)		15	
# Major Commercial Driveways		0	# Minor Commer	cial Driveways		0	
# Major Industrial/Insti. Driveways		0	# Minor Industria	l/Insti. Driveways		1	
# Major Residential Driveways		0	# Minor Resident	ial Driveways		7	
# Other Driveways		0	Calibration Factor	r		1.00	
Crash Modification Facto	rs						
On-Street Parking - CMF1	1.000 Lighting - CMF4 1.000				1.000		
Roadside Fixed Objects - CMF2		1.061	Automated Speed	d Enforcement - CMF	5	1.000	
Median Width - CMF3		1.000	Combined CMF			1.061	
Predicted Roadway Section	on Cra	shes					
Crash Severity		Predicted Crash Freq	uency	Crash R	ate (cr	rashes/mi/year)	
Fatal and Injury (FI)	0.350			1.298			
Property Damage Only (PDO)	0.900			3.332			
Total	1.250			4.630			
Expected Roadway Section	n Cra	shes					
Crash Severity		Expected Crash Freq	uency	Expected Cra	sh Rat	te (crashes/mi/year)	
Fatal and Injury (FI)	0.260			0.962		·	
Property Damage Only (PDO)	0.667			2.471			
Total	0.927			3.433			
Economic Analysis (Expe	ted C	rashes)					
Crash Severity	_	Crash Societal Crash Cost	Expected A	nnual Crashes	To	otal Societal Crash Cost	
Fatal and Injury (FI)	_	3,200.00	0.260			100.52	
Property Damage Only (PDO)		00.00	0.667		\$4,93		
Total	-		0.927			037.79	
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Hig	hway	y Safety Software	e Urban Seg	gment Repor	t	
Project Information						
Analyst	Ben	Erban	Date		5/21,	/2018
Jurisdiction	Mass	DOT Hwy District 6	Analysis Year		2018	1
Project Description	Prior	ity Corridors - Route 138 ir	n Milton			
Input Data						
Segment Type		Two-Lane Undivided Seg	ment (2U)			
Length of Segment (mi)		0.67	AADT (veh/day)			12300
Median Width (ft)		2	Lighting			No
Type of On-street Parking		None	Proportion w/On-	-street Parking		0.00
Automated Speed Enforcement		No	Posted Speed (m	ph)		40
Roadside Fixed Object Density		30	Offset to Roadsid	e Fixed Obj. (ft)		10
# Major Commercial Driveways		0	# Minor Commer	cial Driveways		0
# Major Industrial/Insti. Driveways		0	# Minor Industria	l/Insti. Driveways		0
# Major Residential Driveways		0	# Minor Residential Driveways			19
# Other Driveways		0	Calibration Factor	•		1.00
Crash Modification Factor	's					
On-Street Parking - CMF1		1.000	Lighting - CMF4			1.000
Roadside Fixed Objects - CMF2		1.095	Automated Speed Enforcement - CMF5 1.000			1.000
Median Width - CMF3		1.000	Combined CMF			1.095
Predicted Roadway Section	n Cra	shes				
Crash Severity		Predicted Crash Freq	uency	Crash Ra	ate (cr	ashes/mi/year)
Fatal and Injury (FI)	0.628			0.937		
Property Damage Only (PDO)	1.606			2.398		
Total	2.234			3.334		
Expected Roadway Sectio	n Cra	shes				
Crash Severity		Expected Crash Frequ	uency	Expected Cra	ısh Rat	te (crashes/mi/year)
Fatal and Injury (FI)	0.708			1.056		
Property Damage Only (PDO)	1.811			2.703		
Total	2.519			3.760		
Economic Analysis (Expec	ted C	rashes)				
Crash Severity	Per	Crash Societal Crash Cost	Expected A	nnual Crashes	To	otal Societal Crash Cost
Fatal and Injury (FI)	\$158	3,200.00	0.708		\$111	,975.28
Property Damage Only (PDO)	\$7,40	00.00	1.811		\$13,4	402.82
Total	-		2.519		\$125	,378.10
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Hig	hway	/ Safety Software	e Urban Seg	gment Repor	t	
Project Information						
Analyst	Ben	Erban	Date		5/21,	/2018
Jurisdiction	Mass	DOT Hwy District 6	Analysis Year		2018	3
Project Description	Prior	ity Corridors - Route 138 ir	n Milton			
Input Data						
Segment Type		Two-Lane Undivided Seg	ment (2U)			
Length of Segment (mi)		0.68	AADT (veh/day)			9800
Median Width (ft)		2	Lighting			No
Type of On-street Parking		Parallel Parking	Proportion w/On-	-street Parking		0.20
Automated Speed Enforcement		No	Posted Speed (m	ph)		45
Roadside Fixed Object Density		30	Offset to Roadsid	e Fixed Obj. (ft)		10
# Major Commercial Driveways		0	# Minor Commer	cial Driveways		0
# Major Industrial/Insti. Driveways		0	# Minor Industria	l/Insti. Driveways		0
# Major Residential Driveways		0	# Minor Resident	ial Driveways		45
# Other Driveways		0	Calibration Factor	r		1.00
Crash Modification Facto	rs					
On-Street Parking - CMF1		1.068 Lighting - CMF4 1.000				1.000
Roadside Fixed Objects - CMF2		1.095	Automated Speed	d Enforcement - CMF	5	1.000
Median Width - CMF3		1.000	Combined CMF			1.170
Predicted Roadway Section	on Cra	shes				
Crash Severity		Predicted Crash Freq	uency	Crash R	ate (cr	rashes/mi/year)
Fatal and Injury (FI)	0.620			0.911		
Property Damage Only (PDO)	1.516			2.230		
Total	2.136			3.141		
Expected Roadway Section	n Cra	shes				
Crash Severity		Expected Crash Freq	uency	Expected Cra	ish Rat	te (crashes/mi/year)
Fatal and Injury (FI)	0.691			1.017		
Property Damage Only (PDO)	1.692			2.488		
Total	2.383			3.504		
Economic Analysis (Exped	ted C	rashes)				
Crash Severity	Per	Crash Societal Crash Cost	Expected A	nnual Crashes	To	otal Societal Crash Cost
Fatal and Injury (FI)		5,200.00	0.691),362.22
Property Damage Only (PDO)	_	00.00	1.692			518.65
Total	-		2.383			,880.87
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Highway Safety Software Text Report

File Name:	9.xhz	
Analyst:	Ben Erban	
Agency:	CTPS	
Jurisdiction:	MassDOT Hwy Distri	ct 6
Date:	5/21/2018	
Analysis Year:	2018	
Project Description:		- Route 138 in Milton
Units:	U.S. Customary	
U	0.0. cascoa. ,	
The total number of predicted crashes is:	0.981	crashes/year
	Section 01:	
Facility Type	Urban	
Section Type	Segment	
Model Type	Two-Lane Undivided	Segment (2U)
	== Input =======	
AADT	9800	veh/day
Length	0.24	mi
Number of Major Commercial Driveways	0	
Number of Minor Commercial Driveways	0	
Number of Major Industrial/Institutional Driveways	1	
Number of Minor Industrial/Institutional Driveways	0	
Number of Major Residential Driveways	1	
Number of Minor Residential Driveways	0	
Number of Other Driveways	0	
Roadside Fixed Objects	Yes	
Fixed-Object Offset	10	ft
Fixed-Object Density	40	fixed objects/mile
Median Berries	No	
Lighting	No	
Automated Speed Enforcement	No	
Posted Speed	35	mi/h
Obse	erved Crashes	
Number of Observed Crashes	0	
	- Output =======	
	SPF	
N/CDE 4:)	0.240	
N(SPF, fi)	0.249 0.599	
N(SPF, pdo)	0.005	
N(pedi) N(bikei)	0.004	
N(DIKEI)	0.004	
	CMFs	
CMF1 (On-Street Parking)	1.000	
CMF2 (Roadside Fixed Objects)	1.146	
CMF3 (Median Width)	1.000	
CMF4 (Lighting)	1.000	
CMF5 (Automated Speed Enforcement)	1.000	
CMF Combined	1.146	
Pred	dicted Crashes	
Predicted Crashes (pdo)	0.687	crashes/year
Predicted Crashes (pub) Predicted Crashes (fi)	0.294	crashes/year
	· · ·	c. acrico, y car

Total Predicted Crashes	0.981	crashes/year
	- Expected Crashes	
<pre>Expected Crashes(mv, Driveway) Expected Crashes(mv, Non-Driveway) Expected Crashes(sv)</pre>	1.174 0.612 0.467	crashes/year crashes/year crashes/year
Total Expected Crashes	2.253	crashes/year
	- Economic Analysis	
*Based on Expected Crash Frequency Annual Societal Crash Cost (FI) Annual Societal Crash Cost (PDO) Total Annual Societal Crash Cost	\$106,827.04 \$11,675.23 \$118,502.27	
TOTAL MINICAL SOCIETAL CLASH COST	Ψ110, 302.27	

This Highway Safety Software text report was created in HSS™ Version 7.5 on 5/23/2018 11:59:14

Hig	jhway	/ Safety Software	e Urban Seg	gment Repor	t		
Project Information							
Analyst	Ben	Erban	Date		5/21	5/21/2018	
Jurisdiction	Mass	DOT Hwy District 6	Analysis Year		2018	3	
Project Description	Prior	ity Corridors - Route 138 ir	n Milton				
Input Data							
Segment Type		Two-Lane Undivided Seg	ment (2U)				
Length of Segment (mi)		0.11	AADT (veh/day)			9800	
Median Width (ft)		2	Lighting			No	
Type of On-street Parking		Parallel Parking	Proportion w/On-	-street Parking		0.08	
Automated Speed Enforcement		No	Posted Speed (m	ph)		35	
Roadside Fixed Object Density		40	Offset to Roadsid	e Fixed Obj. (ft)		10	
# Major Commercial Driveways		0	# Minor Commer	cial Driveways		0	
# Major Industrial/Insti. Driveways		0	# Minor Industria	l/Insti. Driveways		0	
# Major Residential Driveways		0	# Minor Resident	ial Driveways		12	
# Other Driveways		0	Calibration Factor	r		1.00	
Crash Modification Facto	rs						
On-Street Parking - CMF1		1.169	Lighting - CMF4			1.000	
Roadside Fixed Objects - CMF2		1.146	Automated Speed	d Enforcement - CMF	5	1.000	
Median Width - CMF3		1.000	Combined CMF			1.340	
Predicted Roadway Section	on Cra	shes					
Crash Severity		Predicted Crash Freq	uency	Crash R	ate (cr	rashes/mi/year)	
Fatal and Injury (FI)	0.137			1.243			
Property Damage Only (PDO)	0.326			2.961			
Total	0.462			4.204			
Expected Roadway Section	n Cra	shes					
Crash Severity	П	Expected Crash Frequency	uency	Expected Cra	ish Ra	te (crashes/mi/year)	
Fatal and Injury (FI)	0.159	<u> </u>		1.449			
Property Damage Only (PDO)	0.380			3.451			
Total	0.539			4.900			
Economic Analysis (Expe		rashes)					
Crash Severity		Crash Societal Crash Cost	Expected A	nnual Crashes	To	otal Societal Crash Cost	
Fatal and Injury (FI)		,200.00	0.159			215.27	
Property Damage Only (PDO)		00.00	0.380			09.12	
Total	-		0.539			024.40	
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Hig	hway	/ Safety Software	e Urban Seg	gment Repor	t		
Project Information							
Analyst	Ben	Erban	Date		5/21	5/21/2018	
Jurisdiction	Mass	DOT Hwy District 6	Analysis Year		2018	3	
Project Description	Prior	ity Corridors - Route 138 ir	n Milton				
Input Data							
Segment Type		Two-Lane Undivided Seg	ment (2U)				
Length of Segment (mi)		0.12	AADT (veh/day)			9800	
Median Width (ft)		2	Lighting			No	
Type of On-street Parking		Parallel Parking	Proportion w/On-	-street Parking		0.12	
Automated Speed Enforcement		No	Posted Speed (m	ph)		30	
Roadside Fixed Object Density		40	Offset to Roadsid	e Fixed Obj. (ft)		10	
# Major Commercial Driveways		0	# Minor Commer	cial Driveways		0	
# Major Industrial/Insti. Driveways		0	# Minor Industria	l/Insti. Driveways		0	
# Major Residential Driveways		0	# Minor Resident	ial Driveways		13	
# Other Driveways		0	Calibration Factor	r		1.00	
Crash Modification Facto	rs						
On-Street Parking - CMF1		1.233	Lighting - CMF4			1.000	
Roadside Fixed Objects - CMF2		1.146	Automated Speed	d Enforcement - CMF	5	1.000	
Median Width - CMF3		1.000	Combined CMF			1.413	
Predicted Roadway Section	on Cra	shes					
Crash Severity		Predicted Crash Freq	uency	Crash R	ate (cr	rashes/mi/year)	
Fatal and Injury (FI)	0.181			1.504			
Property Damage Only (PDO)	0.374			3.114			
Total	0.554			4.618			
Expected Roadway Section	n Cra	shes					
Crash Severity		Expected Crash Freq	uency	Expected Cra	sh Ra	te (crashes/mi/year)	
Fatal and Injury (FI)	0.463		·	3.862		·	
Property Damage Only (PDO)	0.960			7.996			
Total	1.423		11.858				
Economic Analysis (Exped	ted C	rashes)					
Crash Severity	Per	Crash Societal Crash Cost	Expected A	nnual Crashes	To	otal Societal Crash Cost	
Fatal and Injury (FI)		5,200.00	0.463			321.00	
Property Damage Only (PDO)	_	00.00	0.960			00.52	
Total	-		1.423			421.52	
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Appendix F: Level of Service Analysis

Part 1: Existing and Future Pedestrian Report Card Assessment

Part 2: Existing Intersection Levels of Service Part 3: Future Intersection Levels of Service

Part 1: Existing and Future Pedestrian Report Card Assessment





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Pedestrian Report Card Assessment (PRCA):

Roadway Segment

Roadway Segment Location

Route 138 - Milton, MA

Grading Categories	Score	Rating
Safety	2.8	Good
System Preservation	N/A	Fair
Capacity Management and Mobility	1.5	Poor
Economic Vitality	1.5	Poor

Transportation Equity						
High Priority Area	✓					
Moderate Priority Area						
Not a Priority Area						

Category Ratings

Good: Score of 2.3 or more (maximum 3.0)

Fair: Score is between 1.7 and 2.3 Poor: Score is 1.7 or less (minimum 0)

Grading Categories: Scoring Breakdown Roadway Segment

Capacity Management and Mobility								
Performance Measure	Weight	Rating	Weighted Score					
Sidewalk Presence	3	Fair	6					
Crossing Opportunities	2	Poor	2					
Walkway Width	1	Poor	1					
Total	6		9					

Economic Vitality			
Performance Measure	Weight	Rating	Weighted Score
Pedestrian Volumes	1	Fair	2
Adjacent Bicycle Accommodations	1	Poor	1
Total	2		3

Category rating = total rating/total weight Rating Score: Good = 3 Fair = 2 Poor = 1

Safety			
Performance Measure	Weight	Rating	Weighted Score
Pedestrian Crashes	3	Good	9
Pedestrian-Vehicle Buffer	1	Good	3
Vehicle Travel Speed	1	Fair	2
Total	5		14

System Preservation		
Performance Measure	Rating	
Sidewalk Condition	Fair	

Transportation Equity Priority				
Area Condition	Yes/No			
Environmental Justice zone?	✓			
School or college within one-quarter mile?	✓			
More than 8.9% of population older than 75 years?	√			
More than 27.5% of households do not own a vehicle?				

Category Ratings

Good: Score of 2.3 or more (maximum 3.0)

Fair: Score is between 1.7 and 2.3 Poor: Score is 1.7 or less (minimum 0)

Detailed Performance Measure Information: Roadway Segment

Goal	Performance Measure	Features of Analyzed Locations	
	Sidewalk Presence	Sidewalks are present on one side of the street	
Mobility	Crossing Opportunities	9 crosswalks/ 3.6 miles = 2.5 crosswalks per mile	
	Walkway Width	4 foot sidewalks	
Economic Vitality	Pedestrian Volumes	Estimated 5 to 60 pedestrians	
	Adjacent Bicycle Accommodations	Some bike lanes are present at the southern portion of the corridor but the bike lanes are inconsistent	
	Pedestrian Crashes	Not in HSIP cluster	
Safety	Pedestrian-Vehicle Buffer	13 feet	
	Vehicle Travel Speed	32 MPH	
System Preservation	Sidewalk Condition	Fair	

Pedestrian Report Card Assessment with Improvements





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Pedestrian Report Card Assessment (PRCA):

Roadway Segment

Roadway Segment Location

Route 138 - Milton, MA

Grading Categories	Score	Rating
Safety	2.8	Good
System Preservation	3.0	Good
Capacity Management and Mobility	2.7	Good
Economic Vitality	2.5	Good

Transportation Equity		
High Priority Area	✓	
Moderate Priority Area		
Not a Priority Area		

Category Ratings

Good: Score of 2.3 or more (maximum 3.0)

Fair: Score is between 1.7 and 2.3 Poor: Score is 1.7 or less (minimum 0)

Grading Categories: Scoring Breakdown Roadway Segment

Capacity Management and Mobility			
Performance Measure	Weight	Rating	Weighted Score
Sidewalk Presence	3	Good	9
Crossing Opportunities	2	Fair	4
Walkway Width	1	Good	3
Total	6		16

Economic Vitality			
Performance Measure	Weight	Rating	Weighted Score
Pedestrian Volumes	1	Fair	2
Adjacent Bicycle Accommodations	1	Good	3
Total	2		5

Category rating = total rating/total weight Rating Score: Good = 3 Fair = 2 Poor = 1

Safety			
Performance Measure	Weight	Rating	Weighted Score
Pedestrian Crashes	3	Good	9
Pedestrian-Vehicle Buffer	1	Good	3
Vehicle Travel Speed	1	Fair	2
Total	5		14

System Preservation		
Performance Measure Rating		
Sidewalk Condition	Good	

Transportation Equity Prior	rity
Area Condition	Yes/No
Environmental Justice zone?	✓
School or college within one-quarter mile?	✓
More than 8.9% of population older than 75 years?	√
More than 27.5% of households do not own a vehicle?	

Category Ratings

Good: Score of 2.3 or more (maximum 3.0)

Fair: Score is between 1.7 and 2.3 Poor: Score is 1.7 or less (minimum 0)

Detailed Performance Measure Information: Roadway Segment

Goal	Performance Measure	Features of Analyzed Locations
	Sidewalk Presence	Sidewalks are present at least on one side of the street
Mobility	Crossing Opportunities	16 crosswalks/ 3.5 miles = 5.1 crosswalks per mile
	Walkway Width	5.5 foot sidewalks
Economic Vitality	Pedestrian Volumes	Estimated 5 to 60 pedestrians
	Adjacent Bicycle Accommodations	Bike lanes or multi-use path are present in the corridor
	Pedestrian Crashes	Not in HSIP cluster
Safety	Pedestrian-Vehicle Buffer	10 feet
	Vehicle Travel Speed	32 MPH
System Preservation	Sidewalk Condition	Good

Part 2: Existing Intersection Levels of Service

Table F-1
Summary of Intersection Capacity Analysis

Analysis Period				AM Pea	k Hour					PM Pea	ak Hour			Weekend Peak Hour					
Scenario		Existi	ng Cond	litions	With	Improve	ments	Existi	ng Cond	ditions	With I	mprove	ements	Exist	ing Con	ditions	With	Improve	ements
Intersection	Move- ment	LOS	Delay	95th Queue	LOS	Delay	95th Queue	LOS	Delay	95th Queue	LOS	Delay	95th Queue	LOS	Delay	95th Queue	LOS	Delay	95th Queue
Route 138 at Green Street																			
Route 138 Northbound	LTR	Α	0.0	0	Α	0.0	0	Α	0.1	3	Α	0.1	3	Α	0.0	0	Α	0.0	0
Route 138 Southbound	TR	Α	0.0	0	Α	0.0	0	Α	0.0	0	Α	0.0	0	Α	0.0	0	Α	0.0	0
Green Street	LR	F	272.8	45	F	272.8	45	F	69.4	13	F	69.4	13	F	138.5	33	F	138.5	33
Parking Lot Exit	LTR	D	34.4	8	D	34.4	8	С	21.3	8	С	21.3	8	F	65.9	75	F	65.9	75
Intersection Average		Α	1.6		Α	1.6		Α	0.5		Α	0.5		Α	2.7		Α	2.7	
Route 138 at Brush Hill Road																			
Route 138 Northbound	TR	F	116.1	#779	F	118.3	#779	Е	62.3	#606	E	62.4	#606	D	41.6	#652	D	41.6	#652
Route 138 Southbound	LTR	С	21.1	378	С	21.3	378	F	181.9	#589	F	182.0	#589	D	51.7	#573	D	51.7	#570
Brush Hill Road Eastbound	LR	Е	59.7	#259	D	48.6	235	F	290.7	#323	F	290.9	#323	F	207.7	166	F	207.7	#297
Brush Hill Road Westbound	L	F	127.2	#181	F	135.2	#186	F	119.8	#151	F	121.5	#151	F	179.5	#205	F	180.3	#205
	TR	С	25.7	92	С	25.7	92	С	21.3	57	С	21.2	57	С	21.7	87	С	21.7	87
Intersection Average		E	75.5		E	75.6		F	147.7		F	147.9		E	79.1		E	79.2	
Route 138 at Neponset Valley Parkway																			
Route 138 Northbound	L	D	32.3	193	В	16.8	#299	С	19.7	90	D	54.1	#203	В	11.2	50	E	74.1	#319
	Т	Α	0.0	0	Α	6.4	199	Α	0.0	0	В	10.1	181	Α	0.0	0	Α	8.9	172
Route 138 Southbound	TR	Α	0.0	0	С	24.4	#390	Α	0.0	0	D	51.2	#438	Α	0.0	0	E	58.9	#520
Neponset Valley Parkway	LR	Е	44.2	113	С	30.2	145	F	293.0	390	D	38.3	#235	С	23.8	75	D	42.3	#232
Intersection Average		В	13.2		В	17.0		Е	48.1		D	37.0		Α	5.6		D	45.6	
Route 138 at Milton St. and Dollar Ln.																			
Route 138 Northbound	L	В	13.3	263	Α	5.5	7	В	11.2	227	Α	5.4	11	Α	7.9	199	Α	5.4	10
	TR	В	13.3	263	В	13.6	237	В	11.2	227	В	11.3	214	Α	7.9	199	В	10.9	195
Route 138 Southbound	LTR	Α	9.9	222	Α	9.7	207	D	40.3	262	D	52.7	263	В	14.4	250	С	30.9	261
Milton Street	LTR	С	23.5	77	С	23.9	78	Е	71.8	114	E	78.5	123	Е	58.8	98	E	72.8	100
Dollar Lane	LTR	D	35.6	134	D	37.0	136	D	41.7	101	D	43.6	109	С	29.8	76	D	39.1	78
Intersection Average		В	16.9		В	17.2		С	33.6		D	39.6		В	17.9		С	28.5	
Route 138 at Blue Jay Way																			
Route 138 Northbound	LT	Α	3.3	30	Α	3.3	30	Α	2.1	15	Α	2.1	15						
Route 138 Southbound	TR	Α	0.0	0	Α	0.0	0	Α	0.0	0	Α	0.0	0						
Blue Jay Way	LR	D	32.9	15	D	32.9	15	Е	43.4	102	Е	43.4	102						
Intersection Average		Α	2.4		Α	2.4		Α	6.6		Α	6.6							
Route 138 at Atherton St./Bradlee Rd.																			
Route 138 Northbound	LTR	В	15.8	#429	В	16.5	#456	В	10.8	#299	Α	9.3	255						
Route 138 Southbound	LTR	В	14.3	354	В	15.0	#382	Α	8.9	217	Α	7.9	188						
Atherton Street Eastbound	LTR	С	30.6	18	С	31.4	18	С	25.2	10	С	27.0	11						
Atherton Street Westbound	LTR	С	25.8	127	С	24.5	126	С	21.0	71	С	21.6	78						
Bradlee Road	LTR	С	22.8	63	С	22.1	63	В	19.5	48	С	20.6	53						
Intersection Average		В	16.7		В	17.1		В	11.3		В	10.3							

Table F-1
Summary of Intersection Capacity Analysis

Analysis Period				AM Pea	k Hour	,				PM Pea	ak Hour				Weekend Peak Hour					
Scenario		Existi	ng Cond	ditions	With	Improve	ments	Existi	ng Con	ditions	With	Improve	ments	Existi	ng Con	ditions	With	Improve	ments	
Intersection	Move- ment	LOS	Delay	95th Queue	LOS	Delay	95th Queue	LOS	Delay	95th Queue	LOS	Delay	95th Queue	LOS	Delay	95th Queue	LOS	Delay	95th Queue	
Route 138 at Robbins St.																				
Route 138 Northbound	LTR	Α	8.4	163	Α	8.6	221	Α	8.4	298	Α	7.5	267							
Route 138 Southbound	LTR	Α	8.1	149	Α	8.0	201	Α	8.0	259	Α	7.1	233							
Robbins Street Eastbound	LTR	В	12.8	24	В	13.1	25	С	21.8	52	С	21.7	53							
Robbins Street Westbound	LTR	В	13.5	50	В	13.9	52	С	22.1	56	С	21.7	57							
Intersection Average		Α	8.7		Α	8.8		Α	9.5		Α	8.6								
Route 138 at Blue Hill Terrace/Cheever St.																				
Route 138 Northbound	LTR	D	42.9	#402	В	19.7	#626	В	15.4	#324	В	11.1	287							
Route 138 Southbound	LTR	D	42.9	#402	В	19.7	#626	В	15.4	#324	В	11.1	287							
Blue Hill Terrace	LTR	D	43.6	#136	С	34.4	156	D	39.3	#47	D	47.6	61							
Cheever Street	LTR	D	35.1	24	D	43.2	35	D	51.4	15	Е	55.7	21							
Intersection Average		С	32.7		В	19.2		В	16.2		В	13.4								
Route 138 at Aberdeen Road																				
Route 138 Northbound	LT	Α	0.1	0	Α	0.1	0	Α	0.2	3	Α	0.2	3	Α	0.1	0	Α	0.1	0	
Route 138 Southbound	TR	Α	0.0	0	Α	0.0	0	Α	0.0	0	Α	0.0	0	Α	0.0	0	Α	0.0	0	
Aberdeen Road	LTR	С	20.3	5	С	20.3	5	D	26.1	5	D	26.1	5	В	11.6	0	В	11.6	0	
Intersection Average		Α	0.3		Α	0.3		Α	0.4		Α	0.4		Α	0.1		Α	0.1		
Route 138 at Oak Street																				
Route 138 Northbound	LTR	Α	0.0	0	Α	0.0	0	Α	0.4	3	Α	0.4	3							
Route 138 Southbound	LTR	Α	0.2	0	Α	0.2	0	Α	0.3	3	Α	0.3	3							
Oak Street Eastbound	LTR	С	23.5	5	С	23.5	5	D	34.1	10	D	34.1	10							
Oak Street Westbound	LTR	Α	0.0	0	Α	0.0	0	Α	0.4	3	Α	0.4	3							
Intersection Average		Α	0.7		Α	0.7		Α	1.2		Α	1.2								
Route 138 at Brook Road																				
Route 138 Northbound	LTR	В	11.3	281	В	10.8	272	В	10.2	177	Α	9.9	188							
Route 138 Southbound	LTR	Α	9.4	194	Α	9.2	189	В	11.1	197	В	10.6	210							
Brook Road	LTR	D	37.8	124	D	40.6	144	D	46.2	96	D	44.4	99							
Intersection Average		В	15.2		В	15.3		В	16.2		В	15.6								

= 95th percentile volume exceeds capacity, queue shown is after two cycles but may be longer. 95th Queue = 95th percentile queue length (feet). Delay = Average vehicle delay (seconds per vehicle). Err = Value exceeds constraints of HCM 2000 formulas. L = Left turning traffic. LOS = Level of Service. LT = Left and through traffic. TR = Left, through, and right traffic. m = Volume for 95th percentile queue is metered by upstream signal. R = Right turning traffic. TR = Through and right traffic.

Note:

- Shaded cells indicate intersections with no available count data.
- Analysis uses Highway Capacity Manual (HCM) 2010 formulas.

Table F–2
Intersection Capacity Analysis with Left Turn Bay at Brush Hill Road

Analysis Period				AM Pe	ak Hou	ır			PM Peak Hour Weeke						Veekend	d Peak Hour			
Scenario		With	Improve	ments	Witl	ո Left Tu	rn Bay	With Improvements With Left Turn Bay					With	Improve	ements	With Left Turn Bay			
Intersection	Move- ment	LOS	Delay	95th Queue	LOS	Delay	95th Queue	LOS	Delay	95th Queue	LOS	Delay	95th Queue	LOS	Delay	95th Queue	LOS	Delay	95th Queue
Route 138 at Brush Hill Road																			
Route 138 Northbound	L				С	22.8	60				С	25.1	41				F	191.8	#191
	T	F	118.3	#779	D	49.3	#787	Е	62.4	#606	С	28.4	#639	D	41.6	#652	С	28.2	#719
Route 138 Southbound	TR	С	21.3	378	С	35.0	#492	F	182.0	#589	F	222.5	#717	D	51.7	#570	F	141.8	#727
Brush Hill Road Eastbound	LR	D	48.6	235	F	188.5	#321	F	290.9	#323	F	407.1	#406	F	207.7	#297	F	233.1	#352
Brush Hill Road Westbound	L	F	135.2	#186	Е	70.7	#178	F	121.5	#151	F	280.2	#199	F	180.3	#205	F	261.9	#250
	TR	С	25.7	92	С	29.5	17	С	21.2	57	С	27.4	16	С	21.7	87	С	25.2	22
Intersection Average		Е	75.6		Е	63.8		F	147.9		F	179.4		Е	79.2		F	125.0	

^{# = 95}th percentile volume exceeds capacity, queue shown is after two cycles but may be longer. 95th Queue = 95th percentile queue length (feet). Delay = Average vehicle delay (seconds per vehicle). Err = Value exceeds constraints of HCM 2000 formulas. L = Left turning traffic. LOS = Level of Service. LT = Left and through traffic. LTR = Left, through, and right traffic. m = Volume for 95th percentile queue is metered by upstream signal. R = Right turning traffic. TR = Through and right traffic.

Note:

- Shaded cells indicate intersections with no available count data.
- Analysis uses Highway Capacity Manual (HCM) 2010 formulas.

8: Route 138 & South Parking Lot Entrance

	•	•	†	~	>	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations			ĵ»			ર્ન
Traffic Volume (vph)	0	0	1490	1	1	1070
Future Volume (vph)	0	0	1490	1	1	1070
Satd. Flow (prot)	0	0	1749	0	0	1733
Flt Permitted						
Satd. Flow (perm)	0	0	1749	0	0	1733
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	2%	2%	5%	0%	0%	6%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	1569	0	0	1127
Sign Control	Stop		Free			Free
Intersection Summary						
Control Tuno. Unaignolized			<u> </u>		<u> </u>	

Control Type: Unsignalized Intersection Capacity Utilization 81.8%

ICU Level of Service D

9: Route 138 & South Parking Lot Exit

	•	•	†	~	\	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		†			†
Traffic Volume (vph)	0	1	1490	0	0	1080
Future Volume (vph)	0	1	1490	0	0	1080
Satd. Flow (prot)	1589	0	1749	0	0	1733
Flt Permitted						
Satd. Flow (perm)	1589	0	1749	0	0	1733
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	0%	0%	5%	2%	2%	6%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	1	0	1568	0	0	1137
Sign Control	Stop		Free			Free
Intersection Summary						

Control Type: Unsignalized Intersection Capacity Utilization 88.4%

ICU Level of Service E

Intercection						
Intersection	0					
Int Delay, s/veh						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	N/					
Traffic Vol, veh/h	0	1	1490	0	0	1080
Future Vol, veh/h	0	1	1490	0	0	1080
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	0	5	2	2	6
Mvmt Flow	0	1	1568	0	0	1137
	_	-			-	
	Minor1		/lajor1	I\	/lajor2	
Conflicting Flow All	2705	1568	0	-	-	-
Stage 1	1568	-	-	-	-	-
Stage 2	1137	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	-	-
Pot Cap-1 Maneuver	24	138	-	0	0	-
Stage 1	191	-	-	0	0	-
Stage 2	309	-	-	0	0	-
Platoon blocked, %			-			-
Mov Cap-1 Maneuver	24	138	_	-	-	-
Mov Cap-2 Maneuver	24	-	_		_	
Stage 1	191	-	_	-	_	_
Stage 2	309	_	_	_	_	_
Jugo 2	307					
Approach	WB		NB		SB	
HCM Control Delay, s	31.3		0		0	
HCM LOS	D					
Minor Lane/Major Mvm	nt	NBTV	/RI n1	SBT		
	IL					
Capacity (veh/h) HCM Lane V/C Ratio		-	138	-		
			0.008	-		
HCM Long LOS		-	31.3	-		
HCM Lane LOS	\	-	D	-		
HCM 95th %tile Q(veh)	-	0	-		

10: Route 138 & North Parking Lot Entrance

	•	•	†	~	\	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations			₽			ની
Traffic Volume (vph)	0	0	1490	7	6	1090
Future Volume (vph)	0	0	1490	7	6	1090
Satd. Flow (prot)	0	0	1746	0	0	1733
Flt Permitted						
Satd. Flow (perm)	0	0	1746	0	0	1733
Peak Hour Factor	0.92	0.92	0.96	0.58	0.75	0.94
Heavy Vehicles (%)	2%	2%	5%	14%	0%	6%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	1564	0	0	1168
Sign Control	Stop		Free			Free
Intersection Summary						

Control Type: Unsignalized Intersection Capacity Utilization 82.2%

ICU Level of Service E

	۶	→	•	•	←	•	•	†	<i>></i>	>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			ĵ.	
Traffic Volume (vph)	7	0	7	0	0	10	1	1480	0	0	1090	7
Future Volume (vph)	7	0	7	0	0	10	1	1480	0	0	1090	7
Satd. Flow (prot)	0	1671	0	0	1444	0	0	1766	0	0	1732	0
Flt Permitted		0.976										
Satd. Flow (perm)	0	1671	0	0	1444	0	0	1766	0	0	1732	0
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	0%	2%	0%	2%	2%	10%	0%	4%	0%	2%	6%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	14	0	0	11	0	0	1559	0	0	1154	0
Sign Control		Stop			Stop			Free			Free	

Intersection Summary

Control Type: Unsignalized

Intersection Capacity Utilization 92.5%

ICU Level of Service F

Intersection												
Int Delay, s/veh	1.6											
					==					05:		0.5.5
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			सी			₽	
Traffic Vol, veh/h	7	0	7	0	0	10	1	1480	0	0	1090	7
Future Vol, veh/h	7	0	7	0	0	10	1	1480	0	0	1090	7
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	0	2	0	2	2	10	0	4	0	2	6	0
Mvmt Flow	7	0	7	0	0	11	1	1558	0	0	1147	7
Major/Minor	Minor2			Minor1		Λ	/lajor1		Λ	/lajor2		
		2711			2715			0	1	najulZ		0
Conflicting Flow All	2716	2711	1151	2715	2715	1558	1155	0	-	-	-	0
Stage 1	1151	1151	-	1560	1560	-	-	-	-	-	-	-
Stage 2	1565	1560	- ()	1155	1155	- ()	- 11	-	-	-	-	-
Critical Hdwy	7.1	6.52	6.2	7.12	6.52	6.3	4.1	-	-	-	-	-
Critical Hdwy Stg 1	6.1	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4.018	3.3		4.018	3.39	2.2	-	-	-	-	-
Pot Cap-1 Maneuver	14	21	243	14	21	133	612	-	0	0	-	-
Stage 1	243	272	-	141	173	-	-	-	0	0	-	-
Stage 2	141	173	-	240	271	-	-	-	0	0	-	-
Platoon blocked, %								-			-	-
Mov Cap-1 Maneuver		21	243	13	21	133	612	-	-	-	-	-
Mov Cap-2 Maneuver		21	-	13	21	-	-	-	-	-	-	-
Stage 1	240	272	-	139	171	-	-	-	-	-	-	-
Stage 2	128	171	-	233	271	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s				34.4			0			0		
HCM LOS	F F			D								
	,											
Minor Long/Maior M.	w.ł	NDI	NDT	FDL 41	VDI 1	CDT	CDD					
Minor Lane/Major Mvr	nt	NBL	MRT	EBLn1V		SBT	SBR					
Capacity (veh/h)		612	-	25	133	-	-					
HCM Lane V/C Ratio		0.002		0.589		-	-					
HCM Control Delay (s	5)	10.9	0	272.8	34.4	-	-					
HCM Lane LOS		В	Α	F	D	-	-					
HCM 95th %tile Q(veh	1)	0	-	1.8	0.3	-	-					

12: Route 138 & Summit Rd.

	•	•	†	~	-	↓
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	, A		^			ર્ન
Traffic Volume (vph)	0	1	970	550	7	1090
Future Volume (vph)	0	1	970	550	7	1090
Satd. Flow (prot)	1558	0	1672	0	0	1717
Flt Permitted						
Satd. Flow (perm)	1558	0	1672	0	0	1717
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	0%	2%	7%	0%	2%	7%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	1	0	1600	0	0	1154
Sign Control	Stop		Free			Free
Intersection Summary						

Control Type: Unsignalized

Intersection Capacity Utilization 94.6%

ICU Level of Service F

Intersection						
Int Delay, s/veh	0.1					
		WIDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		↑	550	-	4
Traffic Vol, veh/h	0	1	970	550	7	1090
Future Vol, veh/h	0	1	970	550	7	1090
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	2	7	0	2	7
Mvmt Flow	0	1	1021	579	7	1147
N A - ' /N A'	N 4' 1		1-11		M-!	
	Minor1		Major1		Major2	
Conflicting Flow All	2473	1311	0	0	1600	0
Stage 1	1311	-	-	-	-	-
Stage 2	1162	-	-	-	-	-
Critical Hdwy	6.4	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	33	194	-	-	409	-
Stage 1	255	-	-	-	-	-
Stage 2	300	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	31	194	-	-	409	-
Mov Cap-2 Maneuver	31	-		_	-	_
Stage 1	255	_	_	_	_	_
Stage 2	286	-				_
Juge 2	200		-			_
Approach	WB		NB		SB	
HCM Control Delay, s	23.7		0		0.1	
HCM LOS	С					
Minor Long/Major Mum	. +	NDT	MDDW	MDI n1	CDI	CDT
Minor Lane/Major Mvm	Il	NBT		VBLn1	SBL	SBT
Capacity (veh/h)		-	-	. , ,	409	-
HCM Lane V/C Ratio		-	-	0.005		-
HCM Control Delay (s)		-	-	_0.,	14	0
HCM Lane LOS		-	-	С	В	Α
HCM 95th %tile Q(veh))	-	-	0	0.1	-

13: Route 138 & Thacher Montessori School

	•	•	4	†	↓	1
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	M.			4	ĵ.	
Traffic Volume (vph)	0	9	3	960	1080	45
Future Volume (vph)	0	9	3	960	1080	45
Satd. Flow (prot)	1589	0	0	1733	1728	0
Flt Permitted						
Satd. Flow (perm)	1589	0	0	1733	1728	0
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	2%	0%	0%	6%	6%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	10	0	0	1024	1197	0
Sign Control	Stop			Free	Free	
Intersection Summary						

Control Type: Unsignalized Intersection Capacity Utilization 69.6%

ICU Level of Service C

ntersection								
nt Delay, s/veh	0.2							
	EBL	EBR	NBL	NBT	SBT	CDD		
Movement and Configurations	EBL	EDK	NDL	₩.		SBR		
ane Configurations raffic Vol, veh/h	T	9	3	€ 1	1 080	45		
Future Vol, veh/h	0	9	3	960	1080	45		
Conflicting Peds, #/hr		0	0	0	0	0		
Sign Control	Stop	Stop	Free	Free	Free	Free		
RT Channelized	310p -	None	-	None	-	None		
Storage Length	0	-	_	-		TNOTIC		
/eh in Median Storag		_	_	0	0	_		
Grade, %	0	_	_	0	0	_		
Peak Hour Factor	94	94	94	94	94	94		
Heavy Vehicles, %	2	0	0	6	6	0		
Nymt Flow	0	10	3	1021	1149	48		
VIVIII I IOVV	U	10	3	1021	1177	70		
	Minor2		/lajor1		Najor2			
Conflicting Flow All	2201	1173	1197	0	-	0		
Stage 1	1173	-	-	-	-	-		
Stage 2	1028	-	-	-	-	-		
Critical Hdwy	6.42	6.2	4.1	-	-	-		
Critical Hdwy Stg 1	5.42	-	-	-	-	-		
Critical Hdwy Stg 2	5.42	-	-	-	-	-		
Follow-up Hdwy	3.518	3.3	2.2	-	-	-		
Pot Cap-1 Maneuver	49	*109	*327	-	-	-		
Stage 1	294	-	-	-	-	-		
Stage 2	345	-	-	-	-	-		
Platoon blocked, %		1	1	-	-	-		
Nov Cap-1 Maneuver		*109	*327	-	-	-		
Nov Cap-2 Maneuver	48	-	-	-	-	-		
Stage 1	294	-	-	-	-	-		
Stage 2	338	-	-	-	-	-		
Approach	EB		NB		SB			
			0.1					
HCM Control Delay, s			0.1		0			
HCM LOS	E							
/linor Lane/Major Mvr	mt	NBL	NBT I	EBLn1	SBT	SBR		
Capacity (veh/h)		* 327	-		-	-		
HCM Lane V/C Ratio		0.01	-	0.088	-	-		
HCM Control Delay (s	s)	16.1	0	41.2	-	-		
HCM Lane LOS	,	С	A	E	-	_		
HCM 95th %tile Q(veh	n)	0	-	0.3	-	-		
·	,							
Votes								
-: Volume exceeds ca	apacity	\$: De	lay exc	eeds 30	J0s	+: Com	outation Not Defined	*: All m

	۶	→	•	•	+	•	•	†	<i>></i>	/	+	✓
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		ሻ	ĥ			†			ĵ.	
Traffic Volume (vph)	6	0	310	160	110	15	0	970	0	0	660	7
Future Volume (vph)	6	0	310	160	110	15	0	970	0	0	660	7
Satd. Flow (prot)	0	1545	0	1728	1760	0	0	1733	0	0	1683	0
Flt Permitted		0.995		0.396								
Satd. Flow (perm) Satd. Flow (RTOR)	0	1539	0	720	1760	0	0	1733	0	0	1683	0
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	0%	2%	3%	1%	1%	13%	0%	6%	2%	2%	9%	14%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	329	0	167	131	0	0	1010	0	0	695	0
Turn Type	Perm	NA		Perm	NA			NA			NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8								
Detector Phase	4	4		8	8			2			6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0			10.0			10.0	
Minimum Split (s)	16.0	16.0		20.0	20.0			45.0			45.0	
Total Split (s)	30.0	30.0		30.0	30.0			50.0			50.0	
Total Split (%)	37.5%	37.5%		37.5%	37.5%			62.5%			62.5%	
Yellow Time (s)	5.0	5.0		5.0	5.0			4.0			4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0			1.0			1.0	
Lost Time Adjust (s)		0.0		0.0	0.0			0.0			0.0	
Total Lost Time (s)		6.0		6.0	6.0			5.0			5.0	
Lead/Lag												
Lead-Lag Optimize?	Nissa	Mana		Maria	Maria			Maria			Niero	
Recall Mode	None	None		None	None			None			None	
Act Effet Green (s)		20.2		20.2	20.2			45.1 0.59			45.1 0.59	
Actuated g/C Ratio v/c Ratio		0.26 0.81		0.26 0.88	0.26 0.28			0.59			0.59	
		42.6		68.7	23.6			44.2			16.7	
Control Delay Queue Delay		0.0		0.0	0.0			0.0			0.0	
Total Delay		42.6		68.7	23.6			44.2			16.7	
LOS		42.0 D		66.7 E	23.0 C			44.2 D			В	
Approach Delay		42.6		L	48.9			44.2			16.7	
Approach LOS		42.0 D			40.7 D			D			В	
Queue Length 50th (ft)		145		76	49			~495			230	
Queue Length 95th (ft)		#259		#181	92			#779			378	
Internal Link Dist (ft)		906		# 101	198			187			2213	
Turn Bay Length (ft)		700			170			107			2210	
Base Capacity (vph)		484		227	554			1023			994	
Starvation Cap Reductn		0		0	0			0			0	
Spillback Cap Reductn		0		0	0			0			0	
Storage Cap Reductn		0		0	0			0			0	
Reduced v/c Ratio		0.68		0.74	0.24			0.99			0.70	
Intersection Summary					-						-	

Intersection Summary

Cycle Length: 80

Actuated Cycle Length: 76.4

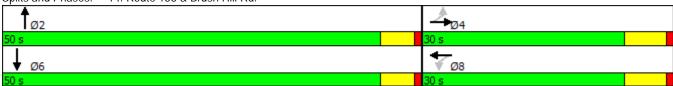
14: Route 138 & Brush Hill Rd.

Natural Cycle: 90
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.99
Intersection Signal Delay: 36.4 Intersection LOS: D
Intersection Capacity Utilization 93.6% ICU Level of Service F
Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Splits and Phases: 14: Route 138 & Brush Hill Rd.



	•	→	•	•	—	•	1	†	<i>></i>	/	+	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		7	f)			†			ĵ.	
Traffic Volume (veh/h)	6	0	310	160	110	15	0	970	0	0	660	7
Future Volume (veh/h)	6	0	310	160	110	15	0	970	0	0	660	7
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	10	0	5	10	0	0	20	0	0	15	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1900	1846	1900	1881	1854	1900	0	1792	0	0	1742	1900
Adj Flow Rate, veh/h	6	0	323	167	115	16	0	1010	0	0	688	7
Adj No. of Lanes	0	1	0	1	1	0	0	1	0	0	1	0
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	2	2	2	1	1	1	0	6	0	0	9	9
Cap, veh/h	46	6	370	179	518	59	0	1008	0	0	978	9
Arrive On Green	0.29	0.00	0.29	0.29	0.29	0.29	0.00	0.57	0.00	0.00	0.57	0.57
Sat Flow, veh/h	9	20	1541	1063	1593	222	0	1792	0	0	1722	18
Grp Volume(v), veh/h	329	0	0	167	0	131	0	1010	0	0	0	695
Grp Sat Flow(s),veh/h/ln	1570	0	0	1063	0	1815	0	1792	0	0	0	1739
Q Serve(g_s), s	0.0	0.0	0.0	5.4	0.0	4.4	0.0	43.7	0.0	0.0	0.0	22.6
Cycle Q Clear(g_c), s	14.8	0.0	0.0	20.2	0.0	4.4	0.0	43.7	0.0	0.0	0.0	22.6
Prop In Lane	0.02		0.98	1.00		0.12	0.00		0.00	0.00		0.01
Lane Grp Cap(c), veh/h	398	0	0	179	0	529	0	1008	0	0	0	989
V/C Ratio(X)	0.83	0.00	0.00	0.93	0.00	0.25	0.00	1.00	0.00	0.00	0.00	0.70
Avail Cap(c_a), veh/h	524	0	0	303	0	552	0	1022	0	0	0	992
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00	0.00	1.00
Uniform Delay (d), s/veh	25.1	0.0	0.0	34.0	0.0	22.0	0.0	17.5	0.0	0.0	0.0	13.3
Incr Delay (d2), s/veh	8.2	0.0	0.0	23.2	0.0	0.2	0.0	28.5	0.0	0.0	0.0	2.2
Initial Q Delay(d3),s/veh	26.4	0.0	0.0	70.0	0.0	3.4	0.0	70.1	0.0	0.0	0.0	5.6
%ile BackOfQ(50%),veh/ln	10.2 59.7	0.0	0.0	8.6 127.2	0.0	3.7 25.7	0.0	49.8 116.1	0.0	0.0	0.0	15.0
LnGrp Delay(d),s/veh		0.0	0.0	121.2 F	0.0		0.0		0.0	0.0	0.0	21.1
LnGrp LOS	<u>E</u>	220		Г	200	С		F 1010			/05	<u>C</u>
Approach Vol, veh/h		329			298			1010			695	
Approach LOS		59.7			82.6			116.1			21.1	
Approach LOS		E			F			F			С	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		50.0		28.9		50.0		28.9				
Change Period (Y+Rc), s		5.0		6.0		5.0		6.0				
Max Green Setting (Gmax), s		45.0		24.0		45.0		24.0				
Max Q Clear Time (g_c+l1), s		45.7		16.8		24.6		22.2				
Green Ext Time (p_c), s		0.0		2.2		13.4		0.7				
Intersection Summary												
HCM 2010 Ctrl Delay			75.5									
HCM 2010 LOS			Ε									

	۶	•	4	†	ļ	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations			7	†		7
Traffic Volume (vph)	0	0	160	390	0	130
Future Volume (vph)	0	0	160	390	0	130
Satd. Flow (prot)	0	0	1711	1801	0	1573
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1711	1801	0	1573
Peak Hour Factor	0.92	0.92	0.84	0.86	0.92	0.85
Heavy Vehicles (%)	2%	2%	2%	2%	2%	1%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	190	453	0	153
Sign Control	Free			Free	Free	
Intersection Summary						
Control Type: Unsignalized						

ICU Level of Service A

Intersection Capacity Utilization 23.9% Analysis Period (min) 15

16: Route 138 & Neponset Valley Pkwy.

	۶	•	4	†	ļ	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			4	f.	
Traffic Volume (vph)	4	170	440	550	490	3
Future Volume (vph)	4	170	440	550	490	3
Satd. Flow (prot)	1348	0	0	1693	1745	0
Flt Permitted	0.999			0.978		
Satd. Flow (perm)	1348	0	0	1693	1745	0
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98
Heavy Vehicles (%)	25%	18%	5%	7%	5%	33%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	177	0	0	1010	503	0
Sign Control	Stop			Free	Free	
Intersection Summary						

Control Type: Unsignalized Intersection Capacity Utilization 100.0%

ICU Level of Service G

Intersection							
Int Delay, s/veh	13.2						
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	¥			4	ĵ.		
Traffic Vol, veh/h	4	170	440	550	490	3	
Future Vol, veh/h	4	170	440	550	490	3	
Conflicting Peds, #/hr		0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-	None	-	None	-	None	
Storage Length	0	-	-	-	-	-	
Veh in Median Storag		-	-	0	0	-	
Grade, %	0	-	-	0	0	-	
Peak Hour Factor	98	98	98	98	98	98	
Heavy Vehicles, %	25	18	5	7	5	33	
Mvmt Flow	4	173	449	561	500	3	
Major/Minor	Minor2	ı	Major1	Λ	/lajor2		
Conflicting Flow All	1961	502	503	0	-	0	
Stage 1	502	-	-	-	-	-	
Stage 2	1459	_	-		_	_	
Critical Hdwy	6.65	6.38	4.15	-	-	-	
Critical Hdwy Stg 1	5.65	-	-	-	_	-	
Critical Hdwy Stg 2	5.65	-	-	-	-	-	
Follow-up Hdwy	3.725	3.462	2.245	_	_	_	
Pot Cap-1 Maneuver	60	*260	*561	-	-	-	
Stage 1	564	-	-	-	-	-	
Stage 2	190	-	-	-	-	-	
Platoon blocked, %		1	1	-	-	-	
Mov Cap-1 Maneuver	r 0	*260	*561	-	-	-	
Mov Cap-2 Maneuver		-	-	-	-	-	
Stage 1	564	-	-	-	-	-	
Stage 2	0	-	-	-	-	-	
<u> </u>							
Approach	EB		NB		SB		
HCM Control Delay, s			14.4		0		
HCM LOS	5 44.Z E		14.4		U		
TIOW LOO							
Minor Lanc/Major My	mt	NDI	NDT	EDI n1	CDT	CDD	
Minor Lane/Major My	IIIL	NBL * F/1	INDII	EBLn1	SBT	SBR	
Capacity (veh/h)		* 561	-	260	-	-	
HCM Cantral Dalay (0.8		0.683	-	-	
HCM Control Delay (s	5)	32.3	0	44.2	-	-	
HCM Lane LOS	b \	D	А	E	-	-	
HCM 95th %tile Q(vel	n)	7.7	-	4.5	-	-	
Notes							

	۶	→	•	•	←	•	1	†	<i>></i>	/	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	30	90	15	30	180	35	9	540	5	1	460	35
Future Volume (vph)	30	90	15	30	180	35	9	540	5	1	460	35
Satd. Flow (prot)	0	1731	0	0	1758	0	0	1716	0	0	1719	0
Flt Permitted		0.907			0.949			0.991			0.999	
Satd. Flow (perm)	0	1588	0	0	1678	0	0	1702	0	0	1717	0
Satd. Flow (RTOR)												
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	3%	4%	0%	0%	2%	3%	0%	7%	0%	0%	6%	3%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	139	0	0	253	0	0	571	0	0	511	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	7.0	7.0		7.0	7.0		30.0	30.0		30.0	30.0	
Minimum Split (s)	12.0	12.0		12.0	12.0		37.0	37.0		37.0	37.0	
Total Split (s)	25.0	25.0		25.0	25.0		37.0	37.0		37.0	37.0	
Total Split (%)	40.3%	40.3%		40.3%	40.3%		59.7%	59.7%		59.7%	59.7%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.0			5.0			7.0			7.0	
Lead/Lag												
Lead-Lag Optimize? Recall Mode	None	None		None	None		Min	Min		Min	Min	
Act Effct Green (s)	None	14.7		None	15.1		IVIII I	30.1		IVIIII	30.1	
Actuated g/C Ratio		0.26			0.26			0.53			0.53	
v/c Ratio		0.20			0.20			0.53			0.53	
Control Delay		19.2			23.6			14.7			13.2	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		19.2			23.6			14.7			13.2	
LOS		17.2 B			23.0 C			В			В	
Approach Delay		19.2			23.6			14.7			13.2	
Approach LOS		В			23.0 C			В			В	
Queue Length 50th (ft)		38			75			127			108	
Queue Length 95th (ft)		77			134			263			222	
Internal Link Dist (ft)		1017			791			1154			1181	
Turn Bay Length (ft)		1017			,,,,			1101			1101	
Base Capacity (vph)		556			588			894			902	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.25			0.43			0.64			0.57	

Addressing Priority Corridors from the LRTP Needs Assessment: Route 138 in Milton 2018 Existing Conditions - AM Peak Hour

Intersection Summary
Cycle Length: 62

Actuated Cycle Length: 57.3

17: Route 138 & Milton St./Dollar Ln.

Natural Cycle: 55
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.64
Intersection Signal Delay: 16.1 Intersection LOS: B
Intersection Capacity Utilization 60.9% ICU Level of Service B
Analysis Period (min) 15

Splits and Phases: 17: Route 138 & Milton St./Dollar Ln.



	۶	→	•	•	←	•	1	†	<i>></i>	/	+	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	30	90	15	30	180	35	9	540	5	1	460	35
Future Volume (veh/h)	30	90	15	30	180	35	9	540	5	1	460	35
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	10	0	0	15	0	0	15	0	0	10	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1900	1838	1900	1900	1865	1900	1900	1779	1900	1900	1796	1900
Adj Flow Rate, veh/h	31	93	15	31	186	36	9	557	5	1	474	36
Adj No. of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	4	4	4	2	2	2	7	7	7	6	6	6
Cap, veh/h	124	321	36	96	335	49	70	961	8	67	913	63
Arrive On Green	0.22	0.22	0.22	0.22	0.22	0.22	0.56	0.56	0.56	0.56	0.56	0.56
Sat Flow, veh/h	216	1319	186	123	1390	251	8	1744	15	0	1648	125
Grp Volume(v), veh/h	139	0	0	253	0	0	571	0	0	511	0	0
Grp Sat Flow(s),veh/h/ln	1720	0	0	1764	0	0	1767	0	0	1774	0	0
Q Serve(g_s), s	0.0	0.0	0.0	2.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	3.5	0.0	0.0	6.9	0.0	0.0	11.2	0.0	0.0	9.5	0.0	0.0
Prop In Lane	0.22		0.11	0.12		0.14	0.02		0.01	0.00		0.07
Lane Grp Cap(c), veh/h	469	0	0	486	0	0	1033	0	0	1047	0	0
V/C Ratio(X)	0.30	0.00	0.00	0.52	0.00	0.00	0.55	0.00	0.00	0.49	0.00	0.00
Avail Cap(c_a), veh/h	704	0	0	727	0	0	1057	0	0	1060	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	18.1	0.0	0.0	19.5	0.0	0.0	9.3	0.0	0.0	8.2	0.0	0.0
Incr Delay (d2), s/veh	0.7	0.0	0.0	1.8	0.0	0.0	0.6	0.0	0.0	0.4	0.0	0.0
Initial Q Delay(d3),s/veh	4.6	0.0	0.0	14.3	0.0	0.0	3.4	0.0	0.0	1.3	0.0	0.0
%ile BackOfQ(50%),veh/ln	3.2	0.0	0.0	6.7	0.0	0.0	8.3	0.0	0.0	6.1	0.0	0.0
LnGrp Delay(d),s/veh	23.5	0.0	0.0	35.6	0.0	0.0	13.3	0.0	0.0	9.9	0.0	0.0
LnGrp LOS	С			D			В			Α		
Approach Vol, veh/h		139			253			571			511	
Approach Delay, s/veh		23.5			35.6			13.3			9.9	
Approach LOS		С			D			В			А	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		37.0		16.6		37.0		16.6				
Change Period (Y+Rc), s		7.0		5.0		7.0		5.0				
Max Green Setting (Gmax), s		30.0		20.0		30.0		20.0				
Max Q Clear Time (g_c+I1), s		13.2		5.5		11.5		8.9				
Green Ext Time (p_c), s		6.8		3.5		7.2		3.0				
Intersection Summary												
HCM 2010 Ctrl Delay			16.9									
HCM 2010 LOS			В									

18: Route 138 & Blue Jay Way (Curry College)

	•	•	4	†	↓	1
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			ર્ન	f)	
Traffic Volume (vph)	5	15	130	480	480	60
Future Volume (vph)	5	15	130	480	480	60
Satd. Flow (prot)	1596	0	0	1705	1717	0
Flt Permitted	0.979			0.989		
Satd. Flow (perm)	1596	0	0	1705	1717	0
Confl. Peds. (#/hr)	1		16			16
Peak Hour Factor	0.42	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	7%	1%	8%	6%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	28	0	0	663	587	0
Sign Control	Stop			Free	Free	
Intersection Cummery						
Intersection Summary						
Control Type: Unsignalized				10		(C
Intersection Capacity Utiliza	ition /4.9%			IC	U Level c	of Service [
Analysis Period (min) 15						

ntersection								
nt Delay, s/veh	2.4							
Movement	EBL	EBR	NBL	NBT	SBT	SBR		
_ane Configurations	¥	LDIN	INDL	4	<u>381</u>	JUIN		
raffic Vol, veh/h	5	15	130	480	480	60		
iture Vol, veh/h	5	15	130	480	480	60		
onflicting Peds, #/hr		0	16	0	0	16		
gn Control	Stop	Stop	Free	Free	Free	Free		
Channelized	-	None		None	-			
orage Length	0	-	_	-		-		
eh in Median Storag		-	-	0	0	-		
rade, %	0	-	-	0	0	-		
eak Hour Factor	42	92	92	92	92	92		
eavy Vehicles, %	0	7	1	8	6	0		
vmt Flow	12	16	141	522	522	65		
jor/Minor	Minor2	ı	Major1	N	/lajor2			
onflicting Flow All	1375	570	603	0	-	0		
Stage 1	570	-	-	-	-	-		
Stage 2	805	-	_	-	-	-		
tical Hdwy	6.4	6.27	4.11	-	-	-		
tical Hdwy Stg 1	5.4	-	-	-	-	-		
tical Hdwy Stg 2	5.4	-	-	-	-	-		
llow-up Hdwy	3.5	3.363	2.209	-	-	-		
t Cap-1 Maneuver	162	*321	*488	-	-	-		
Stage 1	570	-	-	-	-	-		
Stage 2	443	-	-	-	-	-		
atoon blocked, %		1	1	-	-	-		
lov Cap-1 Maneuver		*316	*488		-	-		
ov Cap-2 Maneuver		-	-	-	-	-		
Stage 1	562	-	-	-	-	-		
Stage 2	259	-	-	-	-	-		
proach	EB		NB		SB			
CM Control Delay, s	32.9		3.3		0			
CM LOS	D							
nor Lane/Major Mvi	mt	NBL	NBT I	EBLn1	SBT	SBR		
pacity (veh/h)		* 488	-	157	-	-		
M Lane V/C Ratio		0.29	-	0.18	-	-		
CM Control Delay (s	s)	15.4	0	32.9	-	-		
M Lane LOS		С	Α	D	-	-		
CM 95th %tile Q(vel	n)	1.2	-	0.6	-	-		
otes								
olume exceeds ca	anacity	\$. D.	alay ovo	eeds 30	ηρς	T. Com	outation Not Defined	*: All major volume in platoon
nume exceeds Ca	apacity	φ. De	ciay ext	ccus 30	103	T. CUIII	butation Not Delined	. Ali major volume in piat0011

19: Route 138 & Atherton St. & Bradlee Rd.

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Lane Group	EBL	EBR	EBR2	NBL2	NBL	NBT	NBR	SBL	SBT	SBR	SBR2	SEL
Lane Configurations	M					4			4			
Traffic Volume (vph)	2	2	3	2	25	450	9	2	460	1	1	5
Future Volume (vph)	2	2	3	2	25	450	9	2	460	1	1	5
Satd. Flow (prot)	1432	0	0	0	0	1689	0	0	1732	0	0	0
Flt Permitted	0.986					0.960			0.998			
Satd. Flow (perm)	1432	0	0	0	0	1627	0	0	1728	0	0	0
Satd. Flow (RTOR)												
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	50%	0%	0%	50%	7%	8%	11%	0%	6%	0%	0%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	7	0	0	0	0	506	0	0	483	0	0	0
Turn Type	Prot			Perm	Perm	NA		Perm	NA			Perm
Protected Phases	10					2			6			
Permitted Phases				2	2			6				4
Detector Phase	10			2	2	2		6	6			4
Switch Phase				_	_	_						
Minimum Initial (s)	4.0			22.0	22.0	22.0		22.0	22.0			10.0
Minimum Split (s)	9.0			28.0	28.0	28.0		28.0	28.0			15.0
Total Split (s)	10.0			40.0	40.0	40.0		40.0	40.0			20.0
Total Split (%)	11.1%			44.4%	44.4%	44.4%		44.4%	44.4%			22.2%
Yellow Time (s)	4.0			5.0	5.0	5.0		5.0	5.0			4.0
All-Red Time (s)	1.0			1.0	1.0	1.0		1.0	1.0			1.0
Lost Time Adjust (s)	0.0			1.0	1.0	0.0		1.0	0.0			1.0
Total Lost Time (s)	5.0					6.0			6.0			
Lead/Lag	Lead					0.0			0.0			
Lead-Lag Optimize?	Loud											
Recall Mode	None			Min	Min	Min		Min	Min			None
Act Effct Green (s)	5.4			171111	IVIIII	31.0		171111	31.0			TTOTIC
Actuated g/C Ratio	0.10					0.58			0.58			
v/c Ratio	0.05					0.53			0.48			
Control Delay	30.6					15.8			14.3			
Queue Delay	0.0					0.0			0.0			
Total Delay	30.6					15.8			14.3			
LOS	00.0 C					В			В			
Approach Delay	30.6					15.8			14.3			
Approach LOS	00.0 C					В			В			
Queue Length 50th (ft)	2					76			70			
Queue Length 95th (ft)	18					#429			354			
Internal Link Dist (ft)	1228					1586			2667			
Turn Bay Length (ft)	1220					1300			2007			
Base Capacity (vph)	146					1201			1276			
Starvation Cap Reductn	0					0			0			
Spillback Cap Reductn	0					0			0			
Storage Cap Reductn	0					0			0			
Reduced v/c Ratio	0.05					0.42			0.38			
	0.03					0.42			0.30			
Intersection Summary												
Cycle Length: 90	1											
Actuated Cycle Length: 53.	.1											

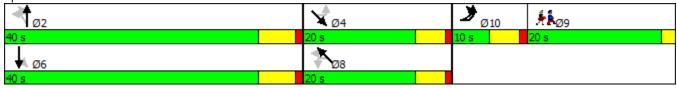
19: Route 138 & Atherton St. & Bradlee Rd.

	\mathbf{x}	>	€	*	×	*			
Lane Group	SET	SER	NWL2	NWL	NWT	NWR	Ø9		
Lane Configurations	4				4				
Traffic Volume (vph)	20	30	45	1	50	30			
Future Volume (vph)	20	30	45	1	50	30			
Satd. Flow (prot)	1639	0	0	0	1734	0			
Flt Permitted	0.966	· ·		J	0.855	· ·			
Satd. Flow (perm)	1589	0	0	0	1509	0			
Satd. Flow (RTOR)	1307	U	U	U	1007	U			
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96			
Heavy Vehicles (%)	5%	3%	0.70	0.70	0%	3%			
Shared Lane Traffic (%)	370	370	070	0 70	070	370			
Lane Group Flow (vph)	57	0	0	0	131	0			
		U				U			
Turn Type	NA		Perm	Perm	NA		0		
Protected Phases	4		0	0	8		9		
Permitted Phases	4		8	8	0				
Detector Phase	4		8	8	8				
Switch Phase	100		40.0	40.0	400		1.0		
Minimum Initial (s)	10.0		10.0	10.0	10.0		1.0		
Minimum Split (s)	15.0		15.0	15.0	15.0		20.0		
Total Split (s)	20.0		20.0	20.0	20.0		20.0		
Total Split (%)	22.2%		22.2%	22.2%	22.2%		22%		
Yellow Time (s)	4.0		4.0	4.0	4.0		2.0		
All-Red Time (s)	1.0		1.0	1.0	1.0		0.0		
Lost Time Adjust (s)	0.0				0.0				
Total Lost Time (s)	5.0				5.0				
Lead/Lag							Lag		
Lead-Lag Optimize?									
Recall Mode	None		None	None	None		None		
Act Effct Green (s)	11.8				11.8				
Actuated g/C Ratio	0.22				0.22				
v/c Ratio	0.16				0.39				
Control Delay	22.8				25.8				
Queue Delay	0.0				0.0				
Total Delay	22.8				25.8				
LOS	С				С				
Approach Delay	22.8				25.8				
Approach LOS	C				C				
Queue Length 50th (ft)	11				27				
Queue Length 95th (ft)	63				127				
Internal Link Dist (ft)	689				1410				
Turn Bay Length (ft)	007				1-710				
Base Capacity (vph)	488				463				
Starvation Cap Reductn	0				403				
Spillback Cap Reductn	0				0				
Storage Cap Reductin	0				0				
Reduced v/c Ratio									
	0.12				0.28				
Intersection Summary									

19: Route 138 & Atherton St. & Bradlee Rd.

Natural Cycle: 75
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.53
Intersection Signal Delay: 16.7 Intersection LOS: B
Intersection Capacity Utilization 74.3% ICU Level of Service D
Analysis Period (min) 15
95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Splits and Phases: 19: Route 138 & Atherton St. & Bradlee Rd.



Bane Group		•	→	•	•	←	•	4	†	~	>	ļ	4
Traffic Volume (vph)	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph)	Lane Configurations		4			4			4			4	
Satist Flow (pront)	Traffic Volume (vph)	10		3	30		15	4		10	0		7
Fit Permitted 0.845 0.855 0.997 Satd. Flow (perm) 0 1530 0 0 1474 0 0 1691 0 0 1715 0 Satd. Flow (RTOR) Flow (RTOR) Flow (PTOR) Flow	Future Volume (vph)	10	15	3	30	30	15	4	490	10	0	470	7
Satd, Flow (perm) 0 1530 0 0 1474 0 0 1691 0 1715 0 Satd, Flow (RTOR) Peak Hour Factor 0.92 0.90 <td>Satd. Flow (prot)</td> <td>0</td> <td>1778</td> <td>0</td> <td>0</td> <td>1690</td> <td>0</td> <td>0</td> <td>1696</td> <td>0</td> <td>0</td> <td>1715</td> <td>0</td>	Satd. Flow (prot)	0	1778	0	0	1690	0	0	1696	0	0	1715	0
Peak Hour Factor	Flt Permitted		0.845			0.855			0.997				
Peak Hour Factor	Satd. Flow (perm)	0	1530	0	0	1474	0	0	1691	0	0	1715	0
Heavy Vehicles (%)	Satd. Flow (RTOR)												
Shared Lane Traffic (%) Lane Group Flow (ph) 0 30 0 0 82 0 0 548 0 0 519 0 1 1 1 1 1 1 1 1 1	Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Lane Group Flow (vph)	Heavy Vehicles (%)	0%	0%	0%	3%	0%	13%	0%	8%	8%	0%	7%	0%
Turn Type	Shared Lane Traffic (%)												
Protected Phases	Lane Group Flow (vph)	0	30	0	0	82	0	0	548	0	0	519	0
Permitted Phases	Turn Type	Perm	NA		Perm	NA		Perm	NA			NA	
Detector Phase 4	Protected Phases		4			8			2			6	
Switch Phase Minimum Initial (s) 7.0 7.0 7.0 10.0 10.0 10.0 10.0 Minimum Spitt (s) 12.0 12.0 12.0 20.0 45.0 22.0 22.0 27.8 45.2 45.0 45.0 45.0 45.0 45.0 45.0 45.0 45.0 42.0 45.0	Permitted Phases	4			8			2			6		
Minimum Initial (s) 7.0 7.0 7.0 7.0 10.0 10.0 10.0 Minimum Spit (s) 12.0 12.0 12.0 20.0 45.0 20.2 20.2 69.2% 69.2% 69.2% 69.2% 69.2% 69.2% 69.2% 69.2% 69.2% 69.2% 69.2% 69.2% 69.2% 69.2% 69.2% 69.2% 69.2% 69.2% 69.2% 69.2	Detector Phase	4	4		8	8		2	2		6	6	
Minimum Split (s) 12.0 12.0 12.0 20.0 20.0 20.0 20.0 20.0 20.0 20.0 20.0 20.0 45.0 <td>Switch Phase</td> <td></td>	Switch Phase												
Total Split (s) 20.0 20.0 20.0 20.0 45.0 45.0 45.0 45.0 Total Split (%) 30.8% 30.8% 30.8% 30.8% 69.2% 69.2% 69.2% Yellow Time (s) 4.0 4.0 4.0 4.0 5.0 5.0 5.0 5.0 All-Red Time (s) 1.0 1.0 1.0 1.0 1.0 1.0 1.0 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 0.0 Total Lost Time (s) 5.0 5.0 6.0 6.0 6.0 Lead/Lag 1.0 1.0 1.0 1.0 1.0 1.0 Lead/Lag Optimize? 1.0 1.	Minimum Initial (s)	7.0	7.0		7.0	7.0		10.0	10.0		10.0	10.0	
Total Split (%) 30.8% 30.8% 30.8% 69.2% 69.2% 69.2% Yellow Time (s) 4.0 4.0 4.0 5.0 5.0 5.0 5.0 All-Red Time (s) 1.0<	Minimum Split (s)	12.0	12.0		12.0	12.0		20.0	20.0		20.0	20.0	
Yellow Time (s) 4.0 4.0 4.0 5.0 5.0 5.0 5.0 All-Red Time (s) 1.0 </td <td>Total Split (s)</td> <td>20.0</td> <td>20.0</td> <td></td> <td>20.0</td> <td>20.0</td> <td></td> <td>45.0</td> <td>45.0</td> <td></td> <td>45.0</td> <td>45.0</td> <td></td>	Total Split (s)	20.0	20.0		20.0	20.0		45.0	45.0		45.0	45.0	
All-Red Time (s) 1.0	Total Split (%)	30.8%	30.8%		30.8%	30.8%		69.2%	69.2%		69.2%	69.2%	
Lost Time Adjust (s) 0.0 0.0 0.0 0.0 Total Lost Time (s) 5.0 5.0 6.0 6.0 Lead-Lag Optimize? Recall Mode None None None Min Min Min Min Recall Mode None None None None Min Min Min Min Recall Mode None None None None Min Min Min Min Recall Mode None None None None Min	Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
Total Lost Time (s) 5.0 5.0 6.0 6.0 Lead/Lag Lead-Lag Optimize? Recall Mode None None None None None Min Min Min Min Act Effet Green (s) 8.3 8.3 27.9 27.9 Actuated g/C Ratio 0.22 0.22 0.72 0.72 v/c Ratio 0.09 0.26 0.45 0.42 Control Delay 15.3 17.1 7.1 6.7 Queue Delay 0.0 0.0 0.0 0.0 Total Delay 15.3 17.1 7.1 6.7 LOS B B B A A A Approach Delay 15.3 17.1 7.1 6.7 Approach LOS B B B A A A Queue Length 50th (ft) 6 16 71 65 Queue Length 95th (ft) 24 50 163 149 Internal Link Dist (ft) 939 647 711 3441 Turn Bay Length (ft) Base Capacity (vph) 615 592 1592 1615 Starvation Cap Reductn 0 0 0 0 0 Reduced v/c Ratio 0.05 0.14 0.34 0.32	All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lead/Lag Lead-Lag Optimize? Recall Mode None None None Min A A C C 0	Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Lead-Lag Optimize? Recall Mode None None None None Min Min Min Min Min Act Effect Green (s) 8.3 8.3 27.9 27.9 27.9 Actualed g/C Ratio 0.22 0.22 0.72 0.72 0.72 V/c Ratio 0.09 0.26 0.45 0.42 Control Delay 15.3 17.1 7.1 6.7 Cueue Delay 0.0 0.0 0.0 0.0 0.0 0.0 Total Delay 15.3 17.1 7.1 6.7 Cueue Length Delay 15.3 17.1 7.1 6.7 Cueue Length Soth (ft) 6 16 71 6.5 Cueue Length Soth (ft) 6 16 71 65 Cueue Length Soth (ft) 24 50 163 149 Internal Link Dist (ft) 939 647 711 3441 Turn Bay Length (ft) Sase Capacity (vph) 615 592 1592 1615 Starvation Cap Reducth 0 0 0 0 0 Cueue Capacity Capacity 0.05 0.14 0.34 0.32 Cueued Capacity (Capacity Capacity Capacity Capacity Capacity 0.05 0.14 0.34 0.32 Cueued Capacity (Capacity Capacity	Total Lost Time (s)		5.0			5.0			6.0			6.0	
Recall Mode None None None Min	Lead/Lag												
Act Effct Green (s) 8.3 8.3 27.9 27.9 Actuated g/C Ratio 0.22 0.22 0.72 0.72 v/c Ratio 0.09 0.26 0.45 0.42 Control Delay 15.3 17.1 7.1 6.7 Queue Delay 0.0 0.0 0.0 0.0 Total Delay 15.3 17.1 7.1 6.7 LOS B B B A A Approach Delay 15.3 17.1 7.1 6.7 Approach LOS B B B A A Queue Length 50th (ft) 6 16 71 65 Queue Length 95th (ft) 24 50 163 149 Internal Link Dist (ft) 939 647 711 3441 Turn Bay Length (ft) 8 592 1592 1615 Starvation Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0 <td>Lead-Lag Optimize?</td> <td></td>	Lead-Lag Optimize?												
Actuated g/C Ratio 0.22 0.22 0.72 0.72 v/c Ratio 0.09 0.26 0.45 0.42 Control Delay 15.3 17.1 7.1 6.7 Queue Delay 0.0 0.0 0.0 0.0 Total Delay 15.3 17.1 7.1 6.7 LOS B B A A Approach Delay 15.3 17.1 7.1 6.7 Approach LOS B B B A A Queue Length 50th (ft) 6 16 71 65 Queue Length 95th (ft) 24 50 163 149 Internal Link Dist (ft) 939 647 711 3441 Turn Bay Length (ft) 8 592 1592 1615 Starvation Cap Reductn 0 0 0 0 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0 Reduced v/c Ratio 0.05 0.14 0.34 0.32 <	Recall Mode	None	None		None	None		Min	Min		Min	Min	
v/c Ratio 0.09 0.26 0.45 0.42 Control Delay 15.3 17.1 7.1 6.7 Queue Delay 0.0 0.0 0.0 0.0 Total Delay 15.3 17.1 7.1 6.7 LOS B B A A Approach Delay 15.3 17.1 7.1 6.7 Approach LOS B B B A A Queue Length 50th (ft) 6 16 71 65 Queue Length 95th (ft) 24 50 163 149 Internal Link Dist (ft) 939 647 711 3441 Turn Bay Length (ft) Base Capacity (vph) 615 592 1592 1615 Starvation Cap Reductn 0 0 0 0 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0 Reduced v/c Ratio 0.05 0.14 0.34 </td <td>Act Effct Green (s)</td> <td></td> <td>8.3</td> <td></td> <td></td> <td>8.3</td> <td></td> <td></td> <td>27.9</td> <td></td> <td></td> <td>27.9</td> <td></td>	Act Effct Green (s)		8.3			8.3			27.9			27.9	
Control Delay 15.3 17.1 7.1 6.7 Queue Delay 0.0 0.0 0.0 0.0 Total Delay 15.3 17.1 7.1 6.7 LOS B B B A A Approach Delay 15.3 17.1 7.1 6.7 Approach LOS B B B A A Queue Length 50th (ft) 6 16 71 65 Queue Length 95th (ft) 24 50 163 149 Internal Link Dist (ft) 939 647 711 3441 Turn Bay Length (ft) 939 647 711 3441 Base Capacity (vph) 615 592 1592 1615 Starvation Cap Reductn 0 0 0 0 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0 Reduced v/c Ratio 0.05 0.14 0.34	Actuated g/C Ratio		0.22			0.22			0.72			0.72	
Queue Delay 0.0 0.0 0.0 Total Delay 15.3 17.1 7.1 6.7 LOS B B B A A Approach Delay 15.3 17.1 7.1 6.7 Approach LOS B B B A A Queue Length 50th (ft) 6 16 71 65 Queue Length 95th (ft) 24 50 163 149 Internal Link Dist (ft) 939 647 711 3441 Turn Bay Length (ft) 8 592 1592 1615 Starvation Cap Reductn 0 0 0 0 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0 Reduced v/c Ratio 0.05 0.14 0.34 0.32	v/c Ratio		0.09			0.26			0.45			0.42	
Total Delay 15.3 17.1 7.1 6.7 LOS B B A A Approach Delay 15.3 17.1 7.1 6.7 Approach LOS B B B A A Queue Length 50th (ft) 6 16 71 65 Queue Length 95th (ft) 24 50 163 149 Internal Link Dist (ft) 939 647 711 3441 Turn Bay Length (ft) 8 8 8 8 1592 1615 Starvation Cap Reductn 0 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 0 0 Storage Cap Reductn 0	Control Delay		15.3			17.1			7.1			6.7	
LOS B B A A Approach Delay 15.3 17.1 7.1 6.7 Approach LOS B B A A Queue Length 50th (ft) 6 16 71 65 Queue Length 95th (ft) 24 50 163 149 Internal Link Dist (ft) 939 647 711 3441 Turn Bay Length (ft) Base Capacity (vph) 615 592 1592 1615 Starvation Cap Reductn 0 0 0 0 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0 Reduced v/c Ratio 0.05 0.14 0.34 0.32	Queue Delay		0.0			0.0			0.0			0.0	
Approach Delay 15.3 17.1 7.1 6.7 Approach LOS B B A A Queue Length 50th (ft) 6 16 71 65 Queue Length 95th (ft) 24 50 163 149 Internal Link Dist (ft) 939 647 711 3441 Turn Bay Length (ft) 8 8 1592 1615 Starvation Cap Reductn 0 0 0 0 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0 Reduced v/c Ratio 0.05 0.14 0.34 0.32	Total Delay		15.3			17.1			7.1			6.7	
Approach LOS B B A A Queue Length 50th (ft) 6 16 71 65 Queue Length 95th (ft) 24 50 163 149 Internal Link Dist (ft) 939 647 711 3441 Turn Bay Length (ft) 8 592 1592 1615 Starvation Cap Reductn 0 0 0 0 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0 Reduced v/c Ratio 0.05 0.14 0.34 0.32	LOS		В			В			Α			Α	
Queue Length 50th (ft) 6 16 71 65 Queue Length 95th (ft) 24 50 163 149 Internal Link Dist (ft) 939 647 711 3441 Turn Bay Length (ft) Base Capacity (vph) 615 592 1592 1615 Starvation Cap Reductn 0 0 0 0 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0 Reduced v/c Ratio 0.05 0.14 0.34 0.32	Approach Delay		15.3			17.1			7.1			6.7	
Queue Length 95th (ft) 24 50 163 149 Internal Link Dist (ft) 939 647 711 3441 Turn Bay Length (ft) Turn Bay Length (ft) Base Capacity (vph) 615 592 1592 1615 Starvation Cap Reductn 0 0 0 0 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0 Reduced v/c Ratio 0.05 0.14 0.34 0.32	Approach LOS		В			В			Α			Α	
Internal Link Dist (ft) 939 647 711 3441 Turn Bay Length (ft) Base Capacity (vph) 615 592 1592 1615 Starvation Cap Reductn 0 0 0 0 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0 Reduced v/c Ratio 0.05 0.14 0.34 0.32	Queue Length 50th (ft)		6			16			71			65	
Turn Bay Length (ft) Base Capacity (vph) 615 592 1592 1615 Starvation Cap Reductn 0 0 0 0 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0 Reduced v/c Ratio 0.05 0.14 0.34 0.32	Queue Length 95th (ft)		24			50			163			149	
Base Capacity (vph) 615 592 1592 1615 Starvation Cap Reductn 0 0 0 0 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0 Reduced v/c Ratio 0.05 0.14 0.34 0.32	Internal Link Dist (ft)		939			647			711			3441	
Starvation Cap Reductn 0 0 0 Spillback Cap Reductn 0 0 0 Storage Cap Reductn 0 0 0 Reduced v/c Ratio 0.05 0.14 0.34 0.32	Turn Bay Length (ft)												
Starvation Cap Reductn 0 0 0 Spillback Cap Reductn 0 0 0 Storage Cap Reductn 0 0 0 Reduced v/c Ratio 0.05 0.14 0.34 0.32			615			592			1592			1615	
Storage Cap Reductn 0 0 0 0 Reduced v/c Ratio 0.05 0.14 0.34 0.32			0			0			0			0	
Storage Cap Reductn 0 0 0 0 Reduced v/c Ratio 0.05 0.14 0.34 0.32	•		0										
Reduced v/c Ratio 0.05 0.14 0.34 0.32			0			0						0	
Intersection Summary			0.05										
Ovale Longth, /F	Intersection Summary												

Cycle Length: 65

Actuated Cycle Length: 38.6

20: Route 138 & Robbins St.

Natural Cycle: 40	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.45	
Intersection Signal Delay: 7.8	Intersection LOS: A
Intersection Capacity Utilization 44.6%	ICU Level of Service A
Analysis Period (min) 15	

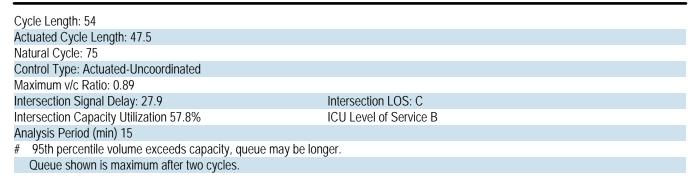
Splits and Phases: 20: Route 138 & Robbins St.



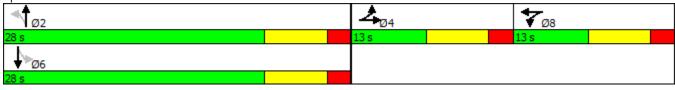
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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		₩.			4			4			4	
Traffic Volume (veh/h)	10	15	3	30	30	15	4	490	10	0	470	7
Future Volume (veh/h)	10	15	3	30	30	15	4	490	10	0	470	7
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	2	0	0	2	0	0	10	0	0	10	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1900	1900	1900	1900	1831	1900	1900	1760	1900	1900	1777	1900
Adj Flow Rate, veh/h	11	16	3	33	33	16	4	533	11	0	511	8
Adj No. of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	0	0	0	0	0	0	8	8	8	7	7	7
Cap, veh/h	201	176	22	219	127	41	107	894	18	0	923	14
Arrive On Green	0.14	0.14	0.14	0.14	0.14	0.14	0.52	0.52	0.52	0.00	0.52	0.52
Sat Flow, veh/h	380	1172	173	490	804	314	3	1713	35	0	1745	27
Grp Volume(v), veh/h	30	0	0	82	0	0	548	0	0	0	0	519
Grp Sat Flow(s),veh/h/ln	1725	0	0	1608	0	0	1751	0	0	0	0	1773
Q Serve(g_s), s	0.0	0.0	0.0	0.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0	6.4
Cycle Q Clear(g_c), s	0.5	0.0	0.0	1.4	0.0	0.0	7.0	0.0	0.0	0.0	0.0	6.4
Prop In Lane	0.37		0.10	0.40		0.20	0.01		0.02	0.00		0.02
Lane Grp Cap(c), veh/h	390	0	0	378	0	0	1014	0	0	0	0	939
V/C Ratio(X)	0.08	0.00	0.00	0.22	0.00	0.00	0.54	0.00	0.00	0.00	0.00	0.55
Avail Cap(c_a), veh/h	922	0	0	884	0	0	2208	0	0	0	0	2130
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	1.00
Uniform Delay (d), s/veh	12.5	0.0	0.0	12.9	0.0	0.0	6.4	0.0	0.0	0.0	0.0	5.8
Incr Delay (d2), s/veh	0.1	0.0	0.0	0.3	0.0	0.0	0.5	0.0	0.0	0.0	0.0	0.5
Initial Q Delay(d3),s/veh	0.2	0.0	0.0	0.3	0.0	0.0	1.5	0.0	0.0	0.0	0.0	1.8
%ile BackOfQ(50%),veh/ln	0.4	0.0	0.0	0.9	0.0	0.0	5.2	0.0	0.0	0.0	0.0	4.8
LnGrp Delay(d),s/veh	12.8	0.0	0.0	13.5	0.0	0.0	8.4	0.0	0.0	0.0	0.0	8.1
LnGrp LOS	В			В			А					Α
Approach Vol, veh/h		30			82			548			519	
Approach Delay, s/veh		12.8			13.5			8.4			8.1	
Approach LOS		В			В			А			А	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2	J	4	J	6	1	8				
Phs Duration (G+Y+Rc), s		23.0		9.4		23.0		9.4				
Change Period (Y+Rc), s		6.0		5.0		6.0		5.0				
Max Green Setting (Gmax), s		39.0		15.0		39.0		15.0				
Max Q Clear Time (g_c+l1), s		9.0		2.5		8.4		3.4				
Green Ext Time (p_c), s		8.0		0.4		8.0		0.4				
		0.0		U.4		0.0		U.4				
Intersection Summary			<u> </u>									
HCM 2010 Ctrl Delay			8.7									
HCM 2010 LOS			Α									

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	4	15	3	100	30	15	1	530	50	8	410	8
Future Volume (vph)	4	15	3	100	30	15	1	530	50	8	410	8
Satd. Flow (prot)	0	1777	0	0	1735	0	0	1519	0	0	1525	0
Flt Permitted		0.991			0.967			0.999			0.983	
Satd. Flow (perm)	0	1776	0	0	1730	0	0	1518	0	0	1501	0
Satd. Flow (RTOR)												
Confl. Peds. (#/hr)	1		1	1		1			2	2		
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82
Heavy Vehicles (%)	0%	0%	0%	1%	0%	0%	0%	8%	0%	13%	8%	0%
Parking (#/hr)							0	0	0	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	27	0	0	177	0	0	708	0	0	520	0
Turn Type	Split	NA		Split	NA		Perm	NA		Perm	NA	
Protected Phases	4	4		8	8			2			6	
Permitted Phases							2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	4.0	4.0		4.0	4.0		6.0	6.0		6.0	6.0	
Minimum Split (s)	11.0	11.0		11.0	11.0		13.0	13.0		13.0	13.0	
Total Split (s)	13.0	13.0		13.0	13.0		28.0	28.0		28.0	28.0	
Total Split (%)	24.1%	24.1%		24.1%	24.1%		51.9%	51.9%		51.9%	51.9%	
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		7.0			7.0			7.0			7.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		Min	Min		Min	Min	
Act Effct Green (s)		5.9			6.1			24.8			24.8	
Actuated g/C Ratio		0.12			0.13			0.52			0.52	
v/c Ratio		0.12			0.80			0.89			0.66	
Control Delay		19.9			49.8			30.8			17.1	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		19.9			49.8			30.8			17.1	
LOS		В			D			С			В	
Approach Delay		19.9			49.8			30.8			17.1	
Approach LOS		В			D			С			В	
Queue Length 50th (ft)		6			40			129			78	
Queue Length 95th (ft)		24			#136			#402			#268	
Internal Link Dist (ft)		838			877			3441			407	
Turn Bay Length (ft)												
Base Capacity (vph)		228			222			793			784	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.12			0.80			0.89			0.66	
Intersection Summary												

21: Route 138 & Cheever St./Blue Hill Terrace St.



Splits and Phases: 21: Route 138 & Cheever St./Blue Hill Terrace St.



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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	4	15	3	100	30	15	1	530	50	8	410	8
Future Volume (veh/h)	4	15	3	100	30	15	1	530	50	8	410	8
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	0	0	0	2	0	0	5	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.99	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.90	1.00	1.00	0.90
Adj Sat Flow, veh/h/ln	1900	1900	1900	1900	1887	1900	1900	1771	1900	1900	1760	1900
Adj Flow Rate, veh/h	5	18	4	122	37	18	1	646	61	10	500	10
Adj No. of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82
Percent Heavy Veh, %	0	0	0	0	0	0	8	8	8	8	8	8
Cap, veh/h	9	31	7	130	99	16	73	625	46	79	654	13
Arrive On Green	0.03	0.03	0.03	0.12	0.12	0.12	0.43	0.43	0.43	0.43	0.43	0.43
Sat Flow, veh/h	339	1222	272	1235	375	182	0	1434	135	9	1533	30
Grp Volume(v), veh/h	27	0	0	177	0	0	708	0	0	520	0	0
Grp Sat Flow(s),veh/h/ln	1833	0	0	1792	0	0	1569	0	0	1573	0	0
Q Serve(g_s), s	0.7	0.0	0.0	4.7	0.0	0.0	3.2	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	0.7	0.0	0.0	4.7	0.0	0.0	21.0	0.0	0.0	13.9	0.0	0.0
Prop In Lane	0.19	_	0.15	0.69	_	0.10	0.00	_	0.09	0.02	_	0.02
Lane Grp Cap(c), veh/h	46	0	0	220	0	0	746	0	0	745	0	0
V/C Ratio(X)	0.59	0.00	0.00	0.80	0.00	0.00	0.95	0.00	0.00	0.70	0.00	0.00
Avail Cap(c_a), veh/h	223	0	0	218	0	0	742	0	0	745	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	23.7	0.0	0.0	21.1	0.0	0.0	14.9	0.0	0.0	12.1	0.0	0.0
Incr Delay (d2), s/veh	11.3	0.0	0.0	19.4	0.0	0.0	21.6	0.0	0.0	2.9	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	3.0	0.0	0.0	6.4	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.5	0.0	0.0	3.8	0.0	0.0	15.5	0.0	0.0	6.5	0.0	0.0
LnGrp Delay(d),s/veh	35.1	0.0	0.0	43.6 D	0.0	0.0	42.9 D	0.0	0.0	14.9	0.0	0.0
LnGrp LOS	D	27		U	177		D	700		В	F20	
Approach Vol, veh/h					177			708			520	
Approach Delay, s/veh Approach LOS		35.1 D			43.6			42.9 D			14.9	
					D						В	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		28.0		8.2		28.0		13.0				
Change Period (Y+Rc), s		7.0		7.0		7.0		7.0				
Max Green Setting (Gmax), s		21.0		6.0		21.0		6.0				
Max Q Clear Time (g_c+I1), s		23.0		2.7		15.9		6.7				
Green Ext Time (p_c), s		0.0		0.0		3.3		0.0				
Intersection Summary			06 =									
HCM 2010 Ctrl Delay			32.7									
HCM 2010 LOS			С									

22: Route 138 & Aberdeen Rd.

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			4	f)	
Traffic Volume (vph)	10	4	4	540	410	30
Future Volume (vph)	10	4	4	540	410	30
Satd. Flow (prot)	1701	0	0	1546	1638	0
Flt Permitted	0.967					
Satd. Flow (perm)	1701	0	0	1546	1638	0
Confl. Peds. (#/hr)	2					
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88
Heavy Vehicles (%)	0%	0%	0%	7%	0%	0%
Parking (#/hr)			0	0	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	16	0	0	619	500	0
Sign Control	Stop			Free	Free	
Intersection Summary						

Control Type: Unsignalized Intersection Capacity Utilization 41.6%

ICU Level of Service A

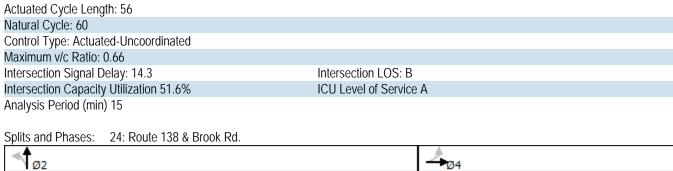
Intersection								
Int Delay, s/veh	0.3							
Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations	W			4	(
Traffic Vol, veh/h	10	4	4	540	410	30		
Future Vol, veh/h	10	4	4	540	410	30		
Conflicting Peds, #/hr	2	0	0	0	0	0		
Sign Control	Stop	Stop	Free	Free	Free	Free		
RT Channelized	-	None	-	None	-	None		
Storage Length	0	-	-	-	-	-		
Veh in Median Storage	e, # 0	-	-	0	0	-		
Grade, %	0	-	-	0	0	-		
Peak Hour Factor	88	88	88	88	88	88		
Heavy Vehicles, %	0	0	0	7	0	0		
Mvmt Flow	11	5	5	614	466	34		
Major/Minor	Minor2	N	/lajor1	٨	/lajor2			
Conflicting Flow All	1108	483	500	0	-	0		
Stage 1	483	-	-	-	-	-		
Stage 2	625	_	_	_	_	_		
Critical Hdwy	6.4	6.2	4.1	-	_	-		
Critical Hdwy Stg 1	5.4	-	-	_	_	_		
Critical Hdwy Stg 2	5.4	_	_	_	_	_		
Follow-up Hdwy	3.5	3.3	2.2	_	_	_		
Pot Cap-1 Maneuver	234	*327	*491	_	_	_		
Stage 1	625	527		_	_	_		
Stage 2	537	_	_	_	_	_		
Platoon blocked, %	331	1	1	_	_	_		
Mov Cap-1 Maneuver	230	*327	*491	-	-	-		
Mov Cap-1 Maneuver	230	JZ 1 -	471	_	_	_		
Stage 1	625		_	-	-	_		
Stage 2	529	_	_	_	_	_		
Jiago Z	J2 7							
Approach	EB		NB		SB			
HCM Control Delay, s	20.3		0.1		0			
HCM LOS	20.5 C		0.1		U			
TIOW EOO	U							
Minor Lane/Major Mvn	nt	NBL	NBT	EBLn1	SBT	SBR		
Capacity (veh/h)		* 491	-	251				
HCM Lane V/C Ratio		0.009		0.063	_	_		
HCM Control Delay (s)	12.4	0	20.3	-	-		
HCM Lane LOS	1	В	A	20.5 C	_	_		
HCM 95th %tile Q(veh	1)	0	-	0.2	_	_		
	7	U		0.2				
Notes		4 -		, .		-		* 411
~: Volume exceeds ca	pacity	\$: De	lay exc	eeds 30)0s	+: Com _l	outation Not Defined	*: All major volume in platoo

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	2	6	2	4	6	7	3	540	35	10	400	1
Future Volume (vph)	2	6	2	4	6	7	3	540	35	10	400	1
Satd. Flow (prot)	0	1775	0	0	1616	0	0	1710	0	0	1702	0
Flt Permitted		0.991			0.988						0.999	
Satd. Flow (perm)	0	1775	0	0	1616	0	0	1710	0	0	1702	0
Confl. Peds. (#/hr)	7					7	1		16	16		1
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Heavy Vehicles (%)	0%	0%	0%	25%	0%	0%	0%	7%	0%	0%	8%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	11	0	0	20	0	0	664	0	0	472	0
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilizat	ion 44.1%			IC	U Level	of Service	Α					
Analysis Period (min) 15												

Intersection													
Int Delay, s/veh	0.7												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4			4			4			4		
Traffic Vol, veh/h	2	6	2	4	6	7	3	540	35	10	400	1	
Future Vol, veh/h	2	6	2	4	6	7	3	540	35	10	400	1	
Conflicting Peds, #/hr	7	0	0	0	0	7	1	0	16	16	0	1	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	-	_	-	_		-	_		-	-		-	
Veh in Median Storage,	# -	0	-	_	0	-	-	0	-	_	0	_	
Grade, %	_	0	_	_	0	_	_	0	_	_	0	_	
Peak Hour Factor	87	87	87	87	87	87	87	87	87	87	87	87	
Heavy Vehicles, %	0	0	0	25	0	0	0	7	0	0	8	0	
Mymt Flow	2	7	2	5	7	8	3	621	40	11	460	1	
IVIVIIIL I IOW		,	2	J	1	0	J	021	40	- 11	400		
Major/Minor M	linor2			Minor1			Major1		, N	/laior?			
		11/0			1140		Major1	0		Major2	0	0	
Conflicting Flow All	1146	1168	461	1152	1149	664	462	0	0	677	0	0	
Stage 1	484	484	-	664	664	-	-	-	-	-	-	-	
Stage 2	662	684	-	488	485	-	-	-	-	-	-	-	
Critical Hdwy	7.1	6.5	6.2	7.35	6.5	6.2	4.1	-	-	4.1	-	-	
Critical Hdwy Stg 1	6.1	5.5	-	6.35	5.5	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.1	5.5	-	6.35	5.5	-	-	-	-	-	-	-	
Follow-up Hdwy	3.5	4	3.3	3.725	4	3.3	2.2	-	-	2.2	-	-	
Pot Cap-1 Maneuver	178	195	*436	157	200	*436	1110	-	-	924	-	-	
Stage 1	568	555	-	414	461	-	-	-	-	-	-	-	
Stage 2	454	452	-	520	555	-	-	-	-	-	-	-	
Platoon blocked, %			1			1		-	-		-	-	
Mov Cap-1 Maneuver	166	188	*436	147	193	*427	1110	-	-	918	-	-	
Mov Cap-2 Maneuver	166	188	-	147	193	-	-	-	-	-	-	-	
Stage 1	565	546	-	407	453	-	-	-	-	-	-	-	
Stage 2	434	444	-	503	546	_	-	-	-	-	-	-	
g- -					2.3								
Approach	EB			WB			NB			SB			
HCM Control Delay, s	23.5			22.3			0			0.2			
HCM LOS	23.3 C			C			U			0.2			
TIGW E03	C			C									
N 4' I /N 4		NDI	NET	NDD		NDI 4	CDI	CDT	CDD				
Minor Lane/Major Mvmt		NBL	NBT	MRK	EBLn1V		SBL	SBT	SBR				
Capacity (veh/h)		1110	-	-	206	228	918	-	-				
HCM Lane V/C Ratio		0.003	-	-		0.086	0.013	-	-				
HCM Control Delay (s)		8.3	0	-	23.5	22.3	9	0	-				
HCM Lane LOS		Α	Α	-	С	С	Α	Α	-				
HCM 95th %tile Q(veh)		0	-	-	0.2	0.3	0	-	-				
Notes													
~: Volume exceeds cap	acity	\$· Do	lav evo	eeds 3	00s	+. Com	putation	Not D	efined	*· ΔII	majory	nluma i	in platoon
. volume exceeds cap	acity	ψ. De	iay cxt	ccus s	003	r. Cuili	pulation	ו ואטנ טי	Cillicu	. 7411	major v	rolullic I	η ριαισση

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4						4			4	
Traffic Volume (vph)	8	190	4	0	0	0	5	500	45	9	400	20
Future Volume (vph)	8	190	4	0	0	0	5	500	45	9	400	20
Satd. Flow (prot)	0	1802	0	0	0	0	0	1689	0	0	1695	0
Flt Permitted		0.998						0.996			0.986	
Satd. Flow (perm)	0	1802	0	0	0	0	0	1682	0	0	1673	0
Satd. Flow (RTOR)												
Confl. Peds. (#/hr)	1		1				1		7	7		1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	13%	1%	0%	2%	2%	2%	20%	7%	9%	0%	8%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	224	0	0	0	0	0	612	0	0	476	0
Turn Type	Perm	NA					Perm	NA		Perm	NA	
Protected Phases		4						2			6	
Permitted Phases	4						2			6		
Detector Phase	4	4					2	2		6	6	
Switch Phase												
Minimum Initial (s)	7.0	7.0					30.0	30.0		30.0	30.0	
Minimum Split (s)	23.0	23.0					36.0	36.0		36.0	36.0	
Total Split (s)	25.0	25.0					40.0	40.0		40.0	40.0	
Total Split (%)	38.5%	38.5%					61.5%	61.5%		61.5%	61.5%	
Yellow Time (s)	4.0	4.0					5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0					1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0						0.0			0.0	
Total Lost Time (s)		5.0						6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None					Min	Min		Min	Min	
Act Effct Green (s)		14.0						30.9			30.9	
Actuated g/C Ratio		0.25						0.55			0.55	
v/c Ratio		0.50						0.66			0.52	
Control Delay		22.1						14.0			11.1	
Queue Delay		0.0						0.0			0.0	
Total Delay		22.1						14.0			11.1	
LOS		С						В			В	
Approach Delay		22.1						14.0			11.1	
Approach LOS		С						В			В	
Queue Length 50th (ft)		62						124			86	
Queue Length 95th (ft)		124						281			194	
Internal Link Dist (ft)		967			1001			519			1021	
Turn Bay Length (ft)												
Base Capacity (vph)		648						1029			1023	
Starvation Cap Reductn		0						0			0	
Spillback Cap Reductn		0						0			0	
Storage Cap Reductn		0						0			0	
Reduced v/c Ratio		0.35						0.59			0.47	
Intersection Summary												
Cycle Length: 65												

24: Route 138 & Brook Rd.



1 Ø2
40 s
25 s

40 s
40 s

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4						4			4	
Traffic Volume (veh/h)	8	190	4	0	0	0	5	500	45	9	400	20
Future Volume (veh/h)	8	190	4	0	0	0	5	500	45	9	400	20
Number	7	4	14				5	2	12	1	6	16
Initial Q (Qb), veh	0	10	0				0	15	0	0	15	0
Ped-Bike Adj(A_pbT)	1.00		1.00				1.00		0.99	1.00		0.99
Parking Bus, Adj	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1900	1873	1900				1900	1771	1900	1900	1768	1900
Adj Flow Rate, veh/h	9	211	4				6	556	50	10	444	22
Adj No. of Lanes	0	1	0				0	1	0	0	1	0
Peak Hour Factor	0.90	0.90	0.90				0.90	0.90	0.90	0.90	0.90	0.90
Percent Heavy Veh, %	0	1	0				7	7	7	8	8	8
Cap, veh/h	12	348	5				72	950	77	77	980	43
Arrive On Green	0.18	0.18	0.18				0.60	0.60	0.60	0.60	0.60	0.60
Sat Flow, veh/h	75	1755	33				4	1595	142	10	1649	80
Grp Volume(v), veh/h	224	0	0				612	0	0	476	0	0
Grp Sat Flow(s),veh/h/ln	1863	0	0				1741	0	0	1740	0	0
Q Serve(g_s), s	5.6	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	5.6	0.0	0.0				10.8	0.0	0.0	7.4	0.0	0.0
Prop In Lane	0.04		0.02				0.01		0.08	0.02		0.05
Lane Grp Cap(c), veh/h	361	0	0				1096	0	0	1102	0	0
V/C Ratio(X)	0.62	0.00	0.00				0.56	0.00	0.00	0.43	0.00	0.00
Avail Cap(c_a), veh/h	746	0	0				1257	0	0	1256	0	0
HCM Platoon Ratio	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	0.00				1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	19.5	0.0	0.0				7.8	0.0	0.0	6.8	0.0	0.0
Incr Delay (d2), s/veh	3.7	0.0	0.0				0.4	0.0	0.0	0.3	0.0	0.0
Initial Q Delay(d3),s/veh	14.6	0.0	0.0				3.1	0.0	0.0	2.3	0.0	0.0
%ile BackOfQ(50%),veh/ln	5.5	0.0	0.0				8.2	0.0	0.0	6.0	0.0	0.0
LnGrp Delay(d),s/veh	37.8 D	0.0	0.0				11.3 B	0.0	0.0	9.4	0.0	0.0
LnGrp LOS	D	224					D	/10		A	17/	
Approach Vol, veh/h		224						612			476	
Approach LOS		37.8						11.3 B			9.4	
Approach LOS		D									А	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6						
Phs Duration (G+Y+Rc), s		36.0		13.9		36.0						
Change Period (Y+Rc), s		6.0		5.0		6.0						
Max Green Setting (Gmax), s		34.0		20.0		34.0						
Max Q Clear Time (g_c+l1), s		12.8		7.6		9.4						
Green Ext Time (p_c), s		7.4		1.7		7.9						
Intersection Summary												
HCM 2010 Ctrl Delay			15.2									
HCM 2010 LOS			В									

102: Route 138 & Concord Baptist Church

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	*		f)			ર્ન
Traffic Volume (vph)	0	0	550	3	3	410
Future Volume (vph)	0	0	550	3	3	410
Satd. Flow (prot)	1837	0	1831	0	0	1835
Flt Permitted						0.999
Satd. Flow (perm)	1837	0	1831	0	0	1835
Confl. Peds. (#/hr)				4	4	
Peak Hour Factor	0.92	0.92	0.86	0.75	0.38	0.89
Heavy Vehicles (%)	0%	0%	0%	33%	0%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	644	0	0	469
Sign Control	Stop		Free			Free
Intersection Summary						
Control Type: Unsignalized			_	_		_
Intersection Capacity Utiliza	tion 32.5%			IC	U Level	of Service
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	0					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		₽			4
Traffic Vol, veh/h	0	0	550	3	3	410
Future Vol, veh/h	0	0	550	3	3	410
Conflicting Peds, #/hr	0	0	0	4	4	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	86	75	38	89
Heavy Vehicles, %	0	0	0	33	0	0
Mvmt Flow	0	0	640	4	8	461
WWW. Tiow	· ·	U	010	•	U	101
	Minor1		/lajor1	ľ	Major2	
Conflicting Flow All	1122	646	0	0	648	0
Stage 1	646	-	-	-	-	-
Stage 2	476	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	230	475	-	-	947	-
Stage 1	526	-	-	_	_	_
Stage 2	629	-	_	-	_	_
Platoon blocked, %	JL /		_	_		_
Mov Cap-1 Maneuver	227	473	_	-	947	_
Mov Cap-1 Maneuver	227	4/3	-		- 747	-
Stage 1	524	-	-	-	-	-
ū	622	-	-	-	-	-
Stage 2	022	-	-	-	-	-
Approach	WB		NB		SB	
					0.1	
HCM Control Delay, s	0		0		U. I	
HCM Control Delay, s HCM LOS	0 A		0		0.1	
HCM Control Delay, s HCM LOS	0 A		0		0.1	
HCM LOS	А	N.E.		MDI		
HCM LOS Minor Lane/Major Mvm	А	NBT		VBLn1	SBL	SBT
Minor Lane/Major Mvm Capacity (veh/h)	А	NBT -		-	SBL 947	SBT -
Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	A nt			-	SBL 947 0.008	
Minor Lane/Major Mvm Capacity (veh/h)	A nt	-	NBRV -	-	SBL 947	-
Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	A nt	-	NBRV - -	-	SBL 947 0.008	-

8: Route 138 & South Parking Lot Entrance

	•	•	†	~	\	↓
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations			ĵ»			ર્ન
Traffic Volume (vph)	0	0	1120	15	2	1270
Future Volume (vph)	0	0	1120	15	2	1270
Satd. Flow (prot)	0	0	1576	0	0	1580
Flt Permitted						
Satd. Flow (perm)	0	0	1576	0	0	1580
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	2%	2%	4%	8%	0%	4%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	1220	0	0	1368
Sign Control	Stop		Free			Free
Intersection Summary						

Control Type: Unsignalized Intersection Capacity Utilization 79.8%

ICU Level of Service D

9: Route 138 & South Parking Lot Exit

	•	•	†	~	\	↓
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		†			†
Traffic Volume (vph)	3	10	1130	0	0	1270
Future Volume (vph)	3	10	1130	0	0	1270
Satd. Flow (prot)	1453	0	1580	0	0	1595
Flt Permitted	0.989					
Satd. Flow (perm)	1453	0	1580	0	0	1595
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	0%	4%	2%	2%	3%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	14	0	1215	0	0	1366
Sign Control	Stop		Free			Free
Intersection Summary						

Control Type: Unsignalized Intersection Capacity Utilization 84.7%

ICU Level of Service E

Intersection						
Int Delay, s/veh	0.3					
		MES	NET	NDD	051	007
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥					
Traffic Vol, veh/h	3	10	1130	0	0	1270
Future Vol, veh/h	3	10	1130	0	0	1270
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e,# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	0	0	4	2	2	3
Mvmt Flow	3	11	1215	0	0	1366
WWW. Flow	· ·	• •	1210			1000
	Minor1		/lajor1	Λ	/lajor2	
Conflicting Flow All	2581	1215	0	-	-	-
Stage 1	1215	-	-	-	-	-
Stage 2	1366	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	-	-
Pot Cap-1 Maneuver	29	223	-	0	0	-
Stage 1	283	-	_	0	0	-
Stage 2	239	_	_	0	0	_
Platoon blocked, %	207		_	Ū		_
Mov Cap-1 Maneuver	29	223	_	_	_	_
Mov Cap-1 Maneuver	29	- 223	_	-	-	
Stage 1	283	-	-	-	-	-
	283	-	-	-	-	-
Stage 2	239	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	53.5		0		0	
HCM LOS	F					
Minor Lane/Major Mvn	nt	NBTV		SBT		
Capacity (veh/h)		-	88	-		
HCM Lane V/C Ratio		-	0.159	-		
HCM Control Delay (s))	-	53.5	-		
HCM Lane LOS		-	F	-		
HCM 95th %tile Q(veh	`	-	0.5	_		

10: Route 138 & North Parking Lot Entrance

	•	•	†	<i>></i>	\	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations			ĵ»			ર્ન
Traffic Volume (vph)	0	0	1120	9	6	1270
Future Volume (vph)	0	0	1120	9	6	1270
Satd. Flow (prot)	0	0	1593	0	0	1581
Flt Permitted						
Satd. Flow (perm)	0	0	1593	0	0	1581
Peak Hour Factor	0.92	0.92	0.98	0.45	0.50	0.97
Heavy Vehicles (%)	2%	2%	3%	0%	0%	4%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	1163	0	0	1321
Sign Control	Stop		Free			Free
Intersection Summary						

Control Type: Unsignalized Intersection Capacity Utilization 83.4%

ICU Level of Service E

	۶	→	•	•	•	•	•	†	<i>></i>	>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			ĵ.	
Traffic Volume (vph)	2	0	8	0	0	20	8	1120	0	0	1270	5
Future Volume (vph)	2	0	8	0	0	20	8	1120	0	0	1270	5
Satd. Flow (prot)	0	1451	0	0	1421	0	0	1593	0	0	1579	0
Flt Permitted		0.990										
Satd. Flow (perm)	0	1451	0	0	1421	0	0	1593	0	0	1579	0
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	0%	2%	0%	2%	2%	0%	25%	3%	0%	2%	4%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	10	0	0	21	0	0	1163	0	0	1314	0
Sign Control		Stop			Stop			Free			Free	

Intersection Summary

Control Type: Unsignalized

Intersection Capacity Utilization 85.0%

ICU Level of Service E

Intersection												
Int Delay, s/veh	0.5											
										001		
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			Þ	
Traffic Vol, veh/h	2	0	8	0	0	20	8	1120	0	0	1270	5
Future Vol, veh/h	2	0	8	0	0	20	8	1120	0	0	1270	5
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	97	97	97	97	97	97	97	97	97	97	97	97
Heavy Vehicles, %	0	2	0	2	2	0	25	3	0	2	4	0
Mvmt Flow	2	0	8	0	0	21	8	1155	0	0	1309	5
Major/Minor I	Minor2		ı	Minor1		ı	Major1		٨	/lajor2		
Conflicting Flow All	2493	2483	1312	2487	2485	1155	1314	0	- 1	najorz		0
Stage 1	1312	1312	1312	1171	1171	1100	1314	U	-	-	-	U
Stage 2	1181	1171	-	1316	1314	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.52	6.2	7.12	6.52	6.2	4.35	-	-	-	-	-
5		5.52		6.12	5.52	0.2	4.33	-	•	-	-	-
Critical Hdwy Stg 1	6.1		-			-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.52	2.2	6.12	5.52	2.2	2 425	-	-	-	-	-
Follow-up Hdwy	3.5	4.018		3.518	4.018	3.3	2.425	-	-	-	-	-
Pot Cap-1 Maneuver	20	30	196	20	29	242	457	-	0	0	_	-
Stage 1	197	228	-	235	267	-	-	-	0	0	-	-
Stage 2	234	267	-	194	228	-	-	-	0	0	-	-
Platoon blocked, %	10	00	10/	4.0	00	0.40	457	-			-	-
Mov Cap-1 Maneuver	18	29	196	18	28	242	457	-	-	-	-	-
Mov Cap-2 Maneuver	18	29	-	18	28	-	-	-	-	-	-	-
Stage 1	187	228	-	223	254	-	-	-	-	-	-	-
Stage 2	204	254	-	186	228	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	69.4			21.3			0.1			0		
HCM LOS	F			С								
	•											
Minor Long/Major May	.1	NDI	NDT	-DI1\	MDI1	CDT	CDD					
Minor Lane/Major Mvm	IU	NBL		EBLn1V		SBT	SBR					
Capacity (veh/h)		457	-	- 0	242	-	-					
HCM Lane V/C Ratio		0.018		0.156		-	-					
HCM Control Delay (s)		13	0	69.4	21.3	-	-					
HCM Lane LOS		В	Α	F	С	-	-					
HCM 95th %tile Q(veh))	0.1	-	0.5	0.3	-	-					

12: Route 138 & Summit Rd.

	•	•	†	~	\	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	M.		f)			4
Traffic Volume (vph)	1	1	790	340	10	1270
Future Volume (vph)	1	1	790	340	10	1270
Satd. Flow (prot)	1480	0	1523	0	0	1580
Flt Permitted	0.976					
Satd. Flow (perm)	1480	0	1523	0	0	1580
Confl. Peds. (#/hr)				1	1	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	0%	2%	5%	0%	2%	4%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	2	0	1165	0	0	1319
Sign Control	Stop		Free			Free
Intersection Summary						
Control Type: Unsignalized			_	_		_
Intersection Capacity Utiliza	tion 93.6%			IC	U Level o	of Service
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	0.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		₽			4
Traffic Vol, veh/h	1	1	790	340	10	1270
Future Vol, veh/h	1	1	790	340	10	1270
Conflicting Peds, #/hr	0	0	0	1	1	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	0	2	5	0	2	4
Mymt Flow	1	1	814	351	10	1309
IVIVIIICI IOVV		ı	017	331	10	1307
Major/Minor	Minor1	N	/lajor1	1	Major2	
Conflicting Flow All	2321	991	0	0	1166	0
Stage 1	991	-	-	-	-	-
Stage 2	1330	-	-	-	-	-
Critical Hdwy	6.4	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	_	-	-	_	-
Follow-up Hdwy	3.5	3.318	_	_	2.218	_
Pot Cap-1 Maneuver	42	299	_	_	599	_
Stage 1	362	-	_	_	-	_
Stage 2	249	_	_	-	_	_
Platoon blocked, %	Z47	-	-	-	-	-
	39	299		-	599	-
Mov Cap 2 Manager			-	-		
Mov Cap-2 Maneuver	39	-	-	-	-	-
Stage 1	362	-	-	-	-	-
Stage 2	234	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	58.8		0		0.1	
HCM LOS	F				0.1	
Minor Lane/Major Mvn	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	69	599	-
HCM Lane V/C Ratio		-	-	0.03	0.017	-
HCM Control Delay (s))	-	-	58.8	11.1	0
HCM Lane LOS		-	-	F	В	Α
HCM 95th %tile Q(veh)	-	-	0.1	0.1	-
/ 5 / 5 6	,			5	3.1	

13: Route 138 & Thacher Montessori School

	۶	•	4	†	ļ	✓
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			4	f)	
Traffic Volume (vph)	0	35	1	790	1260	3
Future Volume (vph)	0	35	1	790	1260	3
Satd. Flow (prot)	1421	0	0	1580	1579	0
Flt Permitted						
Satd. Flow (perm)	1421	0	0	1580	1579	0
Peak Hour Factor	0.92	0.39	0.25	0.96	0.97	0.38
Heavy Vehicles (%)	2%	0%	0%	4%	4%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	90	0	0	827	1307	0
Sign Control	Stop			Free	Free	
Intersection Summary						

Control Type: Unsignalized Intersection Capacity Utilization 84.3%

ICU Level of Service E

Intersection						
Intersection Int Delay, s/veh	1.5					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			सी	₽	
Traffic Vol, veh/h	0	35	1	790	1260	3
Future Vol, veh/h	0	35	1	790	1260	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e,# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	39	25	96	97	38
Heavy Vehicles, %	2	0	0	4	4	0
Mvmt Flow	0	90	4	823	1299	8
		, ,	•	020	,,	
	Minor2		/lajor1		Najor2	
Conflicting Flow All	2134	1303	1307	0	-	0
Stage 1	1303	-	-	-	-	-
Stage 2	831	-	-	-	-	-
Critical Hdwy	6.42	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	54	198	536	-	-	-
Stage 1	254	_	-	-	-	-
Stage 2	428	-	-	-	-	-
Platoon blocked, %	120			_	-	_
Mov Cap-1 Maneuver	53	198	536	_	_	_
Mov Cap-1 Maneuver	53	170	330		_	_
Stage 1	254	-	-	-	-	-
ū	422	-	-	-	-	-
Stage 2	422	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	37.4		0.1		0	
HCM LOS	Ε					
NA'		NDI	NOT	CDL 4	CDT	CDD
Minor Lane/Major Mvn	nt	NBL	MRT	EBLn1	SBT	SBR
Capacity (veh/h)		536	-	.,,	-	-
				0 152	_	_
HCM Lane V/C Ratio		0.007		0.453		
HCM Lane V/C Ratio HCM Control Delay (s)	0.007	0	37.4	-	-
HCM Lane V/C Ratio					-	-

	۶	→	•	•	←	•	1	†	<i>></i>	/	↓	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		*	f)						†	
Traffic Volume (vph)	15	0	370	140	70	10	0	790	0	0	770	0
Future Volume (vph)	15	0	370	140	70	10	0	790	0	0	770	0
Satd. Flow (prot)	0	1413	0	1546	1558	0	0	1580	0	0	1565	0
Flt Permitted		0.990		0.367								
Satd. Flow (perm)	0	1402	0	597	1558	0	0	1580	0	0	1565	0
Satd. Flow (RTOR)												
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	0%	2%	1%	1%	3%	8%	0%	4%	2%	2%	5%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	396	0	144	82	0	0	814	0	0	794	0
Turn Type	Perm	NA		Perm	NA			NA			NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8								
Detector Phase	4	4		8	8			2			6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0			10.0			10.0	
Minimum Split (s)	20.0	20.0		20.0	20.0			45.0			45.0	
Total Split (s)	30.0	30.0		30.0	30.0			45.0			45.0	
Total Split (%)	40.0%	40.0%		40.0%	40.0%			60.0%			60.0%	
Yellow Time (s)	4.0	4.0		4.0	4.0			5.0			5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0			1.0			1.0	
Lost Time Adjust (s)		0.0		0.0	0.0			0.0			0.0	
Total Lost Time (s)		5.0		5.0	5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None			Min			Min	
Act Effct Green (s)		23.1		23.1	23.1			39.1			39.1	
Actuated g/C Ratio		0.32		0.32	0.32			0.53			0.53	
v/c Ratio		0.90		0.77	0.17			0.97			0.95	
Control Delay		49.0		50.9	18.8			43.5			40.8	
Queue Delay		0.0		0.0	0.0			0.0			0.0	
Total Delay		49.0		50.9	18.8			43.5			40.8	
LOS		D		D	В			D			D	
Approach Delay		49.0			39.3			43.5			40.8	
Approach LOS		D			D			D			D	
Queue Length 50th (ft)		169		58	26			350			336	
Queue Length 95th (ft)		#323		#151	57			#606			#589	
Internal Link Dist (ft)		906			198			187			2213	
Turn Bay Length (ft)												
Base Capacity (vph)		479		204	532			843			834	
Starvation Cap Reductn		0		0	0			0			0	
Spillback Cap Reductn		0		0	0			0			0	
Storage Cap Reductn		0		0	0			0			0	
Reduced v/c Ratio		0.83		0.71	0.15			0.97			0.95	

Intersection Summary

Cycle Length: 75

Actuated Cycle Length: 73.2

14: Route 138 & Brush Hill Rd.

Natural Cycle: 80
Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.97
Intersection Signal Delay: 43.1 Intersection LOS: D
Intersection Capacity Utilization 95.0% ICU Level of Service F
Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Splits and Phases: 14: Route 138 & Brush Hill Rd.



	•	→	•	•	—	•	1	†	<i>></i>	\	↓	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		7	₽			↑			↑	
Traffic Volume (veh/h)	15	0	370	140	70	10	0	790	0	0	770	0
Future Volume (veh/h)	15	0	370	140	70	10	0	790	0	0	770	0
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q, veh	0	20	0	10	10	0	0	10	0	0	40	0
Ped-Bike Adj (A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1700	1684	1700	1683	1641	1700	0	1635	0	0	1619	0
Adj Flow Rate, veh/h	15	0	381	144	72	10	0	814	0	0	794	0
Adj No. of Lanes	0	1	0	1	1	0	0	1	0	0	1	0
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	2	2	2	1	3	3	0	4	0	0	5	0
Opposing Right Turn Influence		11	205	No	F20	/0	No	050	0	No	0.40	0
Cap, veh/h	50	11	305	191	529	60	1.00	850	1.00	1.00	842	1.00
HCM Platoon Ratio	1.00	1.00	1.00 0.33	1.00	1.00 0.33	1.00 0.33	1.00	1.00 0.52	1.00	1.00	1.00 0.52	1.00
Prop Arrive On Green Ln Grp Delay, s/veh	0.33 290.7	0.00	0.33	119.8	0.33	21.3	0.00	62.3	0.00	0.00	181.9	0.00
Ln Grp LOS	290.7 F	0.0	0.0	119.8 F	0.0	21.3 C	0.0	02.3 E	0.0	0.0	181.9 F	0.0
Approach Vol, veh/h	Г	396		Г	226	C		814			794	
Approach Delay, s/veh		290.7			84.1			62.3			181.9	
Approach LOS		270.7 F			04.1 F			02.3 E			F	
• •											'	
Timer:		1	2	3	4	5	6	7	8			
Assigned Phs			2		4		6		8			
Case No			8.0		8.0		8.0		6.0			
Phs Duration (G+Y+Rc), s			44.7		29.6		44.7		29.6			
Change Period (Y+Rc), s			6.0		5.0		6.0		5.0			
Max Green (Gmax), s	_		39.0		25.0		39.0		25.0			
Max Allow Headway (MAH),	S		5.2		5.4		5.2		5.4			
Max Q Clear (g_c+l1), s			37.4 1.4		20.9 1.5		36.3 2.2		24.8			
Green Ext Time (g_e), s			1.4		1.00		1.00		0.1 0.99			
Prob of Phs Call (p_c) Prob of Max Out (p_x)			1.00		1.00		1.00		1.00			
•			1.00		1.00		1.00		1.00			
Left-Turn Movement Data												
Assigned Mvmt			5		7		1		3			
Mvmt Sat Flow, veh/h			0		23		0		1008			
Through Movement Data												
Assigned Mvmt			2		4		6		8			
Mvmt Sat Flow, veh/h			1635		32		1619		1410			
Right-Turn Movement Data												
Assigned Mvmt			12		14		16		18			
Mvmt Sat Flow, veh/h			0		1380		0		196			
Left Lane Group Data												
Assigned Mvmt		0	5	0	7	0	1	0	3			
Lane Assignment					L+T+R							

Lanes in Grp	0	0	0	1	0	0	0	1	
Grp Vol (v), veh/h	0	0	0	396	0	0	0	144	
Grp Sat Flow (s), veh/h/ln	0	0	0	1434	0	0	0	1008	
Q Serve Time (g_s), s	0.0	0.0	0.0	4.6	0.0	0.0	0.0	3.8	
Cycle Q Clear Time (g_c), s	0.0	0.0	0.0	18.9	0.0	0.0	0.0	22.8	
Perm LT Sat Flow (s_l), veh/h/ln	0	0	0	1337	0	0	0	1008	
Shared LT Sat Flow (s_sh), veh/h/ln	0	0	0	0	0	0	0	347	
Perm LT Eff Green (g_p), s	0.0	0.0	0.0	24.6	0.0	0.0	0.0	24.6	
Perm LT Serve Time (g_u), s	0.0	0.0	0.0	22.0	0.0	0.0	0.0	5.7	
Perm LT Q Serve Time (g_ps), s	0.0	0.0	0.0	4.6	0.0	0.0	0.0	3.8	
Time to First Blk (g_f), s	0.0	38.7	0.0	14.3	0.0	38.7	0.0	0.0	
Serve Time pre Blk (g_fs), s	0.0	0.0	0.0	14.3	0.0	0.0	0.0	0.0	
Prop LT Inside Lane (P_L)	0.00	0.00	0.00	0.04	0.00	0.00	0.00	1.00	
Lane Grp Cap (c), veh/h	0	0	0	329	0	0	0	191	
V/C Ratio (X)	0.00	0.00	0.00	1.20	0.00	0.00	0.00	0.76	
Avail Cap (c_a), veh/h	0	0	0	532	0	0	0	267	
Upstream Filter (I)	0.00	0.00	0.00	1.00	0.00	0.00	0.00	1.00	
Uniform Delay (d1), s/veh	0.0	0.0	0.0	22.5	0.0	0.0	0.0	31.2	
Incr Delay (d2), s/veh	0.0	0.0	0.0	108.6	0.0	0.0	0.0	7.6	
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	159.7	0.0	0.0	0.0	81.1	
Control Delay (d), s/veh	0.0	0.0	0.0	290.7	0.0	0.0	0.0	119.8	
1st-Term Q (Q1), veh/ln	0.0	0.0	0.0	6.8	0.0	0.0	0.0	3.8	
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	9.9	0.0	0.0	0.0	0.4	
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	20.0	0.0	0.0	0.0	4.3	
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	1.00	
%ile Back of Q (50%), veh/ln	0.0	0.0	0.0	36.7	0.0	0.0	0.0	8.5	
%ile Storage Ratio (RQ%)	0.00	0.00	0.00	0.98	0.00	0.00	0.00	1.20	
Initial Q (Qb), veh	0.0	0.0	0.0	20.0	0.0	0.0	0.0	10.0	
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	36.8	0.0	0.0	0.0	0.0	
Sat Delay (ds), s/veh	0.0	0.0	0.0	22.5	0.0	0.0	0.0	31.4	
Sat Q (Qs), veh	0.0	0.0	0.0	6.8	0.0	0.0	0.0	3.8	
Sat Cap (cs), veh/h	0	0	0	329	0	0	0	186	
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.4	0.0	0.0	0.0	0.2	
Middle Lane Group Data						,			
Assigned Mvmt	0	2	0	4	0	6	0	8	
Lane Assignment		T			•	T		•	
Lanes in Grp	0	1	0	0	0	1	0	0	
Grp Vol (v), veh/h	0	814	0	0	0	794	0	0	
Grp Sat Flow (s), veh/h/ln	0	1635	0	0	0	1619	0	0	
Q Serve Time (g_s), s	0.0	35.4	0.0	0.0	0.0	34.3	0.0	0.0	
Cycle Q Clear Time (g_c), s	0.0	35.4	0.0	0.0	0.0	34.3	0.0	0.0	
Lane Grp Cap (c), veh/h	0	850	0	0	0	842	0	0	
V/C Ratio (X)	0.00	0.96	0.00	0.00	0.00	0.94	0.00	0.00	
Avail Cap (c_a), veh/h	0	857	0	0	0	849	0	0	
Upstream Filter (I)	0.00	1.00	0.00	0.00	0.00	1.00	0.00	0.00	
Uniform Delay (d1), s/veh	0.0	18.0	0.0	0.0	0.0	18.0	0.0	0.0	
Incr Delay (d2), s/veh	0.0	21.0	0.0	0.0	0.0	18.5	0.0	0.0	
Initial Q Delay (d3), s/veh	0.0	23.3	0.0	0.0	0.0	145.4	0.0	0.0	
Control Delay (d), s/veh	0.0	62.3	0.0	0.0	0.0	181.9	0.0	0.0	
1st-Term Q (Q1), veh/ln	0.0	17.2	0.0	0.0	0.0	17.1	0.0	0.0	

2nd-Term Q (Q2), veh/ln	0.0	5.0	0.0	0.0	0.0	4.3	0.0	0.0	
3rd-Term Q (Q3), veh/ln	0.0	5.5	0.0	0.0	0.0	34.0	0.0	0.0	
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	1.00	
%ile Back of Q (50%), veh/ln	0.0	27.7	0.0	0.0	0.0	55.4	0.0	0.0	
%ile Storage Ratio (RQ%)	0.00	4.44	0.00	0.00	0.00	0.64	0.00	0.00	
Initial Q (Qb), veh	0.0	10.0	0.0	0.0	0.0	40.0	0.0	0.0	
Final (Residual) Q (Qe), veh	0.0	1.0	0.0	0.0	0.0	28.0	0.0	0.0	
Sat Delay (ds), s/veh	0.0	18.0	0.0	0.0	0.0	18.0	0.0	0.0	
Sat Q (Qs), veh	0.0	17.2	0.0	0.0	0.0	17.1	0.0	0.0	
Sat Cap (cs), veh/h	0	850	0	0	0	842	0	0	
Initial Q Clear Time (tc), h	0.0	0.3	0.0	0.0	0.0	0.3	0.0	0.0	
Right Lane Group Data									
Assigned Mvmt	0	12	0	14	0	16	0	18	
Lane Assignment								T+R	
Lanes in Grp	0	0	0	0	0	0	0	1	
Grp Vol (v), veh/h	0	0	0	0	0	0	0	82	
Grp Sat Flow (s), veh/h/ln	0	0	0	0	0	0	0	1606	
Q Serve Time (g_s), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	2.7	
Cycle Q Clear Time (g_c), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	2.7	
Prot RT Sat Flow (s_R), veh/h/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Prot RT Eff Green (g_R), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Prop RT Outside Lane (P_R)	0.00	0.00	0.00	0.96	0.00	0.00	0.00	0.12	
Lane Grp Cap (c), veh/h	0.00	0.00	0.00	0.70	0.00	0.00	0.00	533	
V/C Ratio (X)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.15	
Avail Cap (c_a), veh/h	0.00	0.00		0.00	0.00	0.00	0.00	540	
	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Upstream Filter (I)				0.00				1.00 18.2	
Uniform Delay (d1), s/veh	0.0	0.0	0.0		0.0	0.0	0.0		
Incr Delay (d2), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.1	
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	3.0	
Control Delay (d), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	21.3	
1st-Term Q (Q1), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	2.0	
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.4	
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	1.00	
%ile Back of Q (50%), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	2.5	
%ile Storage Ratio (RQ%)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.36	
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	10.0	
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	25.0	
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	11.2	
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	546	
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Intersection Summary									
HCM 2010 Ctrl Delay		147.7							
HCM 2010 LOS		F							
		•							

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations			7	+		7
Traffic Volume (vph)	0	0	90	270	0	140
Future Volume (vph)	0	0	90	270	0	140
Satd. Flow (prot)	0	0	1546	1627	0	1394
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1546	1627	0	1394
Peak Hour Factor	0.92	0.92	0.96	0.93	0.92	0.82
Heavy Vehicles (%)	2%	2%	1%	1%	2%	2%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	94	290	0	171
Sign Control	Free			Free	Free	
Intersection Summary						
Control Type: Uncignalized						

Control Type: Unsignalized Intersection Capacity Utilization 21.9%

ICU Level of Service A

Analysis Period (min) 15

16: Route 138 & Neponset Valley Pkwy.

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			ર્ન	ĵ»	
Traffic Volume (vph)	7	230	310	490	530	3
Future Volume (vph)	7	230	310	490	530	3
Satd. Flow (prot)	1343	0	0	1548	1564	0
Flt Permitted	0.999			0.981		
Satd. Flow (perm)	1343	0	0	1548	1564	0
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	14%	6%	6%	3%	5%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	244	0	0	825	549	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Control Type: Uncignalized						

Control Type: Unsignalized Intersection Capacity Utilization 105.7%

ICU Level of Service G

Analysis Period (min) 15

Intersection								
Int Delay, s/veh	48.1							
Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations	¥			र्स	f)			
Traffic Vol, veh/h	7	230	310	490	530	3		
Future Vol, veh/h	7	230	310	490	530	3		
Conflicting Peds, #/hr	0	0	0	0	0	0		
Sign Control	Stop	Stop	Free	Free	Free	Free		
RT Channelized	-	None	-	None	-	None		
Storage Length	0	-	-	-	-	-		
Veh in Median Storag	e,# 0	-	-	0	0	-		
Grade, %	0	-	-	0	0	-		
Peak Hour Factor	97	97	97	97	97	97		
Heavy Vehicles, %	14	6	6	3	5	0		
Mvmt Flow	7	237	320	505	546	3		
Major/Minor	Minor2	-	Major1	N	Major2			
Conflicting Flow All	1692	548	549	0	- viajoi 2	0		
Stage 1	548	-	-	-	_	-		
Stage 2	1144		-		_	_		
Critical Hdwy	6.54	6.26	4.16	-	-	-		
Critical Hdwy Stg 1	5.54	-	-	_	_	_		
Critical Hdwy Stg 2	5.54	-	-	-	-	_		
Follow-up Hdwy	3.626	3.354	2.254	_	-	_		
Pot Cap-1 Maneuver		*~ 214	*559	-	-	_		
Stage 1	556	-	_	-	-	-		
Stage 2	288	-	-	-	-	-		
Platoon blocked, %		1	1	-	-	-		
Mov Cap-1 Maneuver	20	*~ 214	*559	-	-	-		
Mov Cap-2 Maneuver		-	-	-	-	-		
Stage 1	556	-	-	-	-	-		
Stage 2	59	-	-	-	-	-		
Approach	EB		NB		SB			
HCM Control Delay, s			7.6		0			
HCM LOS	F		7.0		U			
TOW LOO	'							
Minor Lanc/Major Mu	mt	NDI	NDT	EBLn1	SBT	SBR		
Minor Lane/Major Mvi	III	* FFO			SDI	SDK		
Capacity (veh/h)		* 559	-	166	-	-		
HCM Control Doloy (.\	0.572		1.472	-	-		
HCM Lang LOS	<i>)</i>	19.7	0	293	-	-		
HCM DEth 9/tile O(vol	۵)	C	Α	F 15.4	-	-		
HCM 95th %tile Q(vel	I)	3.6	-	15.6	-	-		
Notes								
~: Volume exceeds ca	apacity	\$: De	elay exc	ceeds 30	00s	+: Com	putation Not Defined	*: All major volume in platoor

17: Route 138 & Milton St./Dollar Ln.

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	30	120	15	10	110	25	15	470	10	8	510	25
Future Volume (vph)	30	120	15	10	110	25	15	470	10	8	510	25
Satd. Flow (prot)	0	1574	0	0	1526	0	0	1587	0	0	1556	0
Flt Permitted		0.927			0.974			0.978			0.992	
Satd. Flow (perm)	0	1472	0	0	1492	0	0	1555	0	0	1545	0
Satd. Flow (RTOR)												
Confl. Peds. (#/hr)	1		1			1						
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	7%	1%	0%	9%	5%	0%	0%	3%	10%	0%	5%	4%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	174	0	0	153	0	0	522	0	0	571	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	7.0	7.0		7.0	7.0		30.0	30.0		30.0	30.0	
Minimum Split (s)	12.0	12.0		12.0	12.0		37.0	37.0		37.0	37.0	
Total Split (s)	25.0	25.0		25.0	25.0		45.0	45.0		45.0	45.0	
Total Split (%)	35.7%	35.7%		35.7%	35.7%		64.3%	64.3%		64.3%	64.3%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)		0.0			0.0			0.0		2.0	0.0	
Total Lost Time (s)		5.0			5.0			7.0			7.0	
Lead/Lag		0.0			0.0			,,,			,.0	
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		None	None	
Act Effct Green (s)	. 10.110	11.8			11.8			34.5			34.5	
Actuated g/C Ratio		0.22			0.22			0.65			0.65	
v/c Ratio		0.53			0.47			0.52			0.57	
Control Delay		26.2			24.4			10.7			11.7	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		26.2			24.4			10.7			11.7	
LOS		С			С			В			В	
Approach Delay		26.2			24.4			10.7			11.7	
Approach LOS		C			С			В			В	
Queue Length 50th (ft)		49			43			95			110	
Queue Length 95th (ft)		114			101			227			262	
Internal Link Dist (ft)		1017			791			1154			1181	
Turn Bay Length (ft)		1017			, , ,			1101			1101	
Base Capacity (vph)		575			582			1145			1138	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.30			0.26			0.46			0.50	
Intersection Summary												
Cycle Length: 70												

17: Route 138 & Milton St./Dollar Ln.

Actuated Cycle Length: 53.3

Natural Cycle: 55

Control Type: Actuated-Uncoordinated

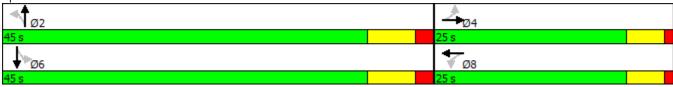
Maximum v/c Ratio: 0.57

Intersection Signal Delay: 14.4

Intersection Capacity Utilization 66.6%

Analysis Period (min) 15

Splits and Phases: 17: Route 138 & Milton St./Dollar Ln.



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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	30	120	15	10	110	25	15	470	10	8	510	25
Future Volume (veh/h)	30	120	15	10	110	25	15	470	10	8	510	25
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q, veh	0	25	0	0	15	0	0	15	0	0	40	0
Ped-Bike Adj (A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1700	1666	1700	1700	1628	1700	1700	1650	1700	1700	1621	1700
Adj Flow Rate, veh/h	32	126	16	11	116	26	16	495	11	8	537	26
Adj No. of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95 5	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, % Opposing Right Turn Influence	1 Voc	1	1	5 Voc	5	5	3 Yes	3	3	5 Voc	5	5
Cap, veh/h	Yes 99	276	19	Yes 80	274	35	80	921	16	Yes 65	895	35
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Prop Arrive On Green	0.17	0.17	0.17	0.17	0.17	0.17	0.59	0.59	0.59	0.59	0.59	0.59
Ln Grp Delay, s/veh	71.8	0.17	0.17	41.7	0.17	0.17	11.2	0.0	0.0	40.3	0.37	0.0
Ln Grp LOS	71.0 E	0.0	0.0	41.7 D	0.0	0.0	В	0.0	0.0	40.3 D	0.0	0.0
Approach Vol, veh/h		174		D	153		D	522		D	571	
Approach Delay, s/veh		71.8			41.7			11.2			40.3	
Approach LOS		F			D			В			D	
• •			2	2		_			0			
Timer:		1	2	3	4	5	6	7	8			
Assigned Phs					4		6		8			
Case No			8.0 37.0		8.0 13.5		8.0 37.0		8.0 13.5			
Phs Duration (G+Y+Rc), s Change Period (Y+Rc), s			7.0		5.0		7.0		5.0			
Max Green (Gmax), s			38.0		20.0		38.0		20.0			
Max Allow Headway (MAH), s			5.2		5.3		5.2		5.3			
Max Q Clear (g_c+l1), s			J.Z									
Green Ext Time (q_e), s			11 5									
Prob of Phs Call (p_c)			11.5 8.0		7.0		13.3		6.5			
Prob of Max Out (p_x)			8.0		7.0 1.5		13.3 7.9		6.5 1.6			
			8.0 1.00		7.0 1.5 0.99		13.3 7.9 1.00		6.5 1.6 0.99			
Loft Turn Mayamant Data			8.0		7.0 1.5		13.3 7.9		6.5 1.6			
Left-Turn Movement Data			8.0 1.00 0.13		7.0 1.5 0.99 0.07		13.3 7.9 1.00 0.15		6.5 1.6 0.99 0.06			
Assigned Mvmt			8.0 1.00 0.13		7.0 1.5 0.99 0.07		13.3 7.9 1.00 0.15		6.5 1.6 0.99 0.06			
Assigned Mvmt Mvmt Sat Flow, veh/h			8.0 1.00 0.13		7.0 1.5 0.99 0.07		13.3 7.9 1.00 0.15		6.5 1.6 0.99 0.06			
Assigned Mvmt Mvmt Sat Flow, veh/h Through Movement Data			8.0 1.00 0.13 5 17		7.0 1.5 0.99 0.07 7 186		13.3 7.9 1.00 0.15		6.5 1.6 0.99 0.06			
Assigned Mvmt Mvmt Sat Flow, veh/h Through Movement Data Assigned Mvmt			8.0 1.00 0.13 5 17		7.0 1.5 0.99 0.07 7 186		13.3 7.9 1.00 0.15		6.5 1.6 0.99 0.06			
Assigned Mvmt Mvmt Sat Flow, veh/h Through Movement Data			8.0 1.00 0.13 5 17		7.0 1.5 0.99 0.07 7 186		13.3 7.9 1.00 0.15		6.5 1.6 0.99 0.06			
Assigned Mvmt Mvmt Sat Flow, veh/h Through Movement Data Assigned Mvmt			8.0 1.00 0.13 5 17		7.0 1.5 0.99 0.07 7 186		13.3 7.9 1.00 0.15		6.5 1.6 0.99 0.06			
Assigned Mvmt Mvmt Sat Flow, veh/h Through Movement Data Assigned Mvmt Mvmt Sat Flow, veh/h			8.0 1.00 0.13 5 17		7.0 1.5 0.99 0.07 7 186		13.3 7.9 1.00 0.15		6.5 1.6 0.99 0.06			
Assigned Mvmt Mvmt Sat Flow, veh/h Through Movement Data Assigned Mvmt Mvmt Sat Flow, veh/h Right-Turn Movement Data			8.0 1.00 0.13 5 17 2 1575		7.0 1.5 0.99 0.07 7 186 4 1264		13.3 7.9 1.00 0.15 1 6 1524		6.5 1.6 0.99 0.06 3 52 8 1248			
Assigned Mvmt Mvmt Sat Flow, veh/h Through Movement Data Assigned Mvmt Mvmt Sat Flow, veh/h Right-Turn Movement Data Assigned Mvmt			8.0 1.00 0.13 5 17 2 1575		7.0 1.5 0.99 0.07 7 186 4 1264		13.3 7.9 1.00 0.15 1 6 1524		6.5 1.6 0.99 0.06 3 52 8 1248			
Assigned Mvmt Mvmt Sat Flow, veh/h Through Movement Data Assigned Mvmt Mvmt Sat Flow, veh/h Right-Turn Movement Data Assigned Mvmt Mvmt Sat Flow, veh/h		0	8.0 1.00 0.13 5 17 2 1575	0	7.0 1.5 0.99 0.07 7 186 4 1264	0	13.3 7.9 1.00 0.15 1 6 1524	0	6.5 1.6 0.99 0.06 3 52 8 1248			

Lanes in Grp	0	1	0	1	0	1	0	1	
Grp Vol (v), veh/h	0	522	0	174	0	571	0	153	
Grp Sat Flow (s), veh/h/ln	0	1627	0	1596	0	1603	0	1567	
Q Serve Time (g_s), s	0.0	0.0	0.0	0.5	0.0	0.0	0.0	0.0	
Cycle Q Clear Time (g_c), s	0.0	9.5	0.0	5.0	0.0	11.3	0.0	4.5	
Perm LT Sat Flow (s_l), veh/h/ln	0	861	0	1264	0	907	0	1264	
Shared LT Sat Flow (s_sh), veh/h/ln	0	1647	0	1651	0	1620	0	1622	
Perm LT Eff Green (g_p), s	0.0	30.0	0.0	8.5	0.0	30.0	0.0	8.5	
Perm LT Serve Time (g_u), s	0.0	18.7	0.0	3.9	0.0	20.5	0.0	3.4	
Perm LT Q Serve Time (g_ps), s	0.0	0.0	0.0	0.5	0.0	0.0	0.0	0.0	
Time to First Blk (q_f), s	0.0	19.4	0.0	3.0	0.0	22.1	0.0	4.5	
Serve Time pre Blk (g_fs), s	0.0	9.5	0.0	3.0	0.0	11.3	0.0	4.5	
Prop LT Inside Lane (P_L)	0.00	0.03	0.00	0.18	0.00	0.01	0.00	0.07	
Lane Grp Cap (c), veh/h	0	1026	0	400	0	982	0	368	
V/C Ratio (X)	0.00	0.51	0.00	0.43	0.00	0.58	0.00	0.42	
Avail Cap (c_a), veh/h	0	1294	0	705	0	1277	0	692	
Upstream Filter (I)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	1.00	
Uniform Delay (d1), s/veh	0.0	7.6	0.0	21.4	0.0	11.2	0.0	20.5	
Incr Delay (d2), s/veh	0.0	0.4	0.0	0.7	0.0	0.5	0.0	0.7	
Initial Q Delay (d3), s/veh	0.0	3.1	0.0	49.7	0.0	28.6	0.0	20.4	
Control Delay (d), s/veh	0.0	11.2	0.0	71.8	0.0	40.3	0.0	41.7	
1st-Term Q (Q1), veh/ln	0.0	6.2	0.0	4.5	0.0	10.7	0.0	3.3	
2nd-Term Q (Q2), veh/ln	0.0	0.1	0.0	0.1	0.0	0.1	0.0	0.1	
3rd-Term Q (Q3), veh/ln	0.0	0.9	0.0	5.5	0.0	7.8	0.0	2.1	
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	1.00	
%ile Back of Q (50%), veh/ln	0.0	7.2	0.0	10.1	0.0	18.6	0.0	5.4	
%ile Storage Ratio (RQ%)	0.00	0.16	0.00	0.24	0.00	0.40	0.00	0.17	
Initial Q (Qb), veh	0.0	15.0	0.0	25.0	0.0	40.0	0.0	15.0	
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Sat Delay (ds), s/veh	0.0	16.9	0.0	25.9	0.0	17.0	0.0	26.0	
Sat Q (Qs), veh	0.0	18.0	0.0	9.9	0.0	17.8	0.0	9.9	
Sat Cap (cs), veh/h	0	938	0	518	0	928	0	515	
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.1	0.0	0.1	0.0	0.1	
Middle Lane Group Data									
Assigned Mvmt	0	2	0	4	0	6	0	8	
Lane Assignment	•							•	
Lanes in Grp	0	0	0	0	0	0	0	0	
Grp Vol (v), veh/h	0	0	0	0	0	0	0	0	
Grp Sat Flow (s), veh/h/ln	0	0	0	0	0	0	0	0	
Q Serve Time (g_s), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Cycle Q Clear Time (g_c), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Lane Grp Cap (c), veh/h	0	0	0	0	0	0	0	0	
V/C Ratio (X)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Avail Cap (c_a), veh/h	0	0	0	0	0	0	0	0	
Upstream Filter (I)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Uniform Delay (d1), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Incr Delay (d2), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Control Delay (d), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
1st-Term Q (Q1), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	

2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	1.00	
%ile Back of Q (50%), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile Storage Ratio (RQ%)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0	
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Right Lane Group Data									
Assigned Mvmt	0	12	0	14	0	16	0	18	
Lane Assignment									
Lanes in Grp	0	0	0	0	0	0	0	0	
Grp Vol (v), veh/h	0	0	0	0	0	0	0	0	
Grp Sat Flow (s), veh/h/ln	0	0	0	0	0	0	0	0	
Q Serve Time (g_s), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Cycle Q Clear Time (g_c), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Prot RT Sat Flow (s_R), veh/h/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Prot RT Eff Green (g_R), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Prop RT Outside Lane (P_R)	0.00	0.02	0.00	0.09	0.00	0.05	0.00	0.17	
Lane Grp Cap (c), veh/h	0.00	0.02	0.00	0.07	0.00	0.03	0.00	0.17	
V/C Ratio (X)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Avail Cap (c_a), veh/h	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Upstream Filter (I)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Uniform Delay (d1), s/veh		0.0		0.0		0.0			
Incr Delay (d2), s/veh	0.0		0.0		0.0		0.0	0.0	
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Control Delay (d), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
1st-Term Q (Q1), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	1.00	
%ile Back of Q (50%), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile Storage Ratio (RQ%)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0	
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Intersection Summary									
HCM 2010 Ctrl Delay		33.6							
HCM 2010 LOS		С							

18: Route 138 & Blue Jay Way (Curry College)

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			4	₽	
Traffic Volume (vph)	50	100	80	460	440	25
Future Volume (vph)	50	100	80	460	440	25
Satd. Flow (prot)	1472	0	0	1589	1544	0
Flt Permitted	0.984			0.993		
Satd. Flow (perm)	1472	0	0	1589	1544	0
Confl. Peds. (#/hr)			1			1
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	0%	1%	3%	6%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	162	0	0	581	500	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utilizat	ion 79.6%			IC	U Level c	of Service D
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	6.6					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	₩.	LDIX	NDL	4) }	אומכ
Traffic Vol, veh/h	5 0	100	80	460	440	25
Future Vol, veh/h	50	100	80	460	440	25
Conflicting Peds, #/hr		0	1	0	0	1
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	- -	None	-	None	-	None
Storage Length	0	-	-	-	_	-
Veh in Median Storag		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	0	0	1	3	6	0
Mvmt Flow	54	108	86	495	473	27
N A = 1 = /N A1 =	N4'		M-!1		1-!	
Major/Minor	Minor2		Major1		/lajor2	
Conflicting Flow All	1155	488	501	0	-	0
Stage 1	488	-	-	-	-	-
Stage 2	667	- / 2	-	-	-	-
Critical Hdwy	6.4	6.2	4.11	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	2 200	-	-	-
Follow-up Hdwy	3.5		2.209	-	-	-
Pot Cap-1 Maneuver	220	*327	*488	-	-	-
Stage 1	621	-	-	-	-	-
Stage 2	514	- 1	1	-	-	-
Platoon blocked, %	. 1//	*227	*400	-	-	-
Mov Cap-1 Maneuver		*327	*488	-	-	-
Mov Cap-2 Maneuve		-	-	-	-	-
Stage 1	620	-	-	-	-	-
Stage 2	389	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	3 43.4		2.1		0	
HCM LOS	Е					
Minor Lane/Major Mv	mt	NBL	MRTI	EBLn1	SBT	SBR
	IIIL					SDK
Capacity (veh/h)		* 488	-	247	-	-
HCM Control Doloy (0.176		0.653	-	-
HCM Control Delay (s HCM Lane LOS	5)	13.9	0	43.4	-	-
HCM 95th %tile Q(ve	h)	0.6	А	4.1	-	-
·	11)	0.0	-	4.1	-	-
Notes						
~: Volume exceeds ca	apacity	\$: De	elay exc	eeds 30)0s	+: Com

19: Route 138 & Atherton St. & Bradlee Rd.

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Lane Group	EBL	EBR	NBL2	NBL	NBT	NBR	SBL	SBT	SBR2	SEL	SET	SER
Lane Configurations	M				4			4			4	
Traffic Volume (vph)	2	2	2	35	440	35	7	410	6	6	30	15
Future Volume (vph)	2	2	2	35	440	35	7	410	6	6	30	15
Satd. Flow (prot)	1495	0	0	0	1578	0	0	1544	0	0	1514	0
Flt Permitted	0.976				0.947			0.990			0.955	
Satd. Flow (perm)	1495	0	0	0	1500	0	0	1530	0	0	1455	0
Satd. Flow (RTOR)												
Confl. Peds. (#/hr)				1					1			
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	0%	0%	0%	3%	3%	14%	6%	0%	0%	6%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	4	0	0	0	551	0	0	455	0	0	54	0
Turn Type	Prot		Perm	Perm	NA		Perm	NA		Perm	NA	
Protected Phases	10				2			6			4	
Permitted Phases			2	2			6			4		
Detector Phase	10		2	2	2		6	6		4	4	
Switch Phase												
Minimum Initial (s)	4.0		22.0	22.0	22.0		22.0	22.0		10.0	10.0	
Minimum Split (s)	9.0		28.0	28.0	28.0		28.0	28.0		15.0	15.0	
Total Split (s)	10.0		40.0	40.0	40.0		40.0	40.0		20.0	20.0	
Total Split (%)	14.3%		57.1%	57.1%	57.1%		57.1%	57.1%		28.6%	28.6%	
Yellow Time (s)	4.0		5.0	5.0	5.0		5.0	5.0		4.0	4.0	
All-Red Time (s)	1.0		1.0	1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0				0.0			0.0			0.0	
Total Lost Time (s)	5.0				6.0			6.0			5.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None		Min	Min	Min		Min	Min		None	None	
Act Effct Green (s)	5.2				34.0			34.0			10.9	
Actuated g/C Ratio	0.11				0.71			0.71			0.23	
v/c Ratio	0.02				0.52			0.42			0.16	
Control Delay	25.2				10.8			8.9			19.5	
Queue Delay	0.0				0.0			0.0			0.0	
Total Delay	25.2				10.8			8.9			19.5	
LOS	С				В			Α			В	
Approach Delay	25.3				10.8			8.9			19.5	
Approach LOS	С				В			А			В	
Queue Length 50th (ft)	1				91			67			11	
Queue Length 95th (ft)	10				#299			217			48	
Internal Link Dist (ft)	1290				1586			2667			689	
Turn Bay Length (ft)												
Base Capacity (vph)	163				1179			1203			477	
Starvation Cap Reductn	0				0			0			0	
Spillback Cap Reductn	0				0			0			0	
Storage Cap Reductn	0				0			0			0	
Reduced v/c Ratio	0.02				0.47			0.38			0.11	
Intersection Summary Cycle Length: 70												

19: Route 138 & Atherton St. & Bradlee Rd.

	€	*	×	*
Lane Group	NWL2	NWL	NWT	NWR
Lane Configurations	1111122		4	14441
Traffic Volume (vph)	25	2	45	10
Future Volume (vph)	25	2	45	10
Satd. Flow (prot)	0	0	1547	0
Flt Permitted	U	U	0.869	U
	0	0	1367	0
Satd. Flow (perm)	U	U	1307	U
Satd. Flow (RTOR)				
Confl. Peds. (#/hr)	0.00	0.00	0.00	0.03
Peak Hour Factor	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	0%	5%	0%
Shared Lane Traffic (%)				
Lane Group Flow (vph)	0	0	88	0
Turn Type	Perm	Perm	NA	
Protected Phases			8	
Permitted Phases	8	8		
Detector Phase	8	8	8	
Switch Phase				
Minimum Initial (s)	10.0	10.0	10.0	
Minimum Split (s)	15.0	15.0	15.0	
Total Split (s)	20.0	20.0	20.0	
Total Split (%)	28.6%	28.6%	28.6%	
Yellow Time (s)	4.0	4.0	4.0	
All-Red Time (s)	1.0	1.0	1.0	
Lost Time Adjust (s)	1.0		0.0	
Total Lost Time (s)			5.0	
Lead/Lag			0.0	
Lead-Lag Optimize?				
Recall Mode	None	None	None	
Act Effct Green (s)	NOHE	NOHE	10.9	
Actuated g/C Ratio			0.23	
v/c Ratio				
			0.28	
Control Delay			21.0	
Queue Delay			0.0	
Total Delay			21.0	
LOS			С	
Approach Delay			21.0	
Approach LOS			С	
Queue Length 50th (ft)			18	
Queue Length 95th (ft)			71	
Internal Link Dist (ft)			1410	
Turn Bay Length (ft)				
Base Capacity (vph)			448	
Starvation Cap Reductn			0	
Spillback Cap Reductn			0	
Storage Cap Reductn			0	
Reduced v/c Ratio			0.20	
			0.20	
Intersection Summary				

19: Route 138 & Atherton St. & Bradlee Rd.

Actuated Cycle Length: 47.7

Natural Cycle: 60

Control Type: Semi Act-Uncoord

Maximum v/c Ratio: 0.52

Intersection Signal Delay: 11.3

Intersection LOS: B

Intersection Capacity Utilization 79.3%

ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 19: Route 138 & Atherton St. & Bradlee Rd.



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	5	35	3	7	35	5	1	450	20	5	410	9
Future Volume (vph)	5	35	3	7	35	5	1	450	20	5	410	9
Satd. Flow (prot)	0	1581	0	0	1574	0	0	1587	0	0	1561	0
Flt Permitted		0.953			0.935			0.999			0.995	
Satd. Flow (perm)	0	1514	0	0	1483	0	0	1585	0	0	1554	0
Satd. Flow (RTOR)												
Confl. Peds. (#/hr)							2		1	1		2
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	3%	0%	0%	3%	0%	0%	3%	0%	0%	5%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	46	0	0	51	0	0	507	0	0	456	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase	•	•					_	_		J.		
Minimum Initial (s)	7.0	7.0		7.0	7.0		20.0	20.0		20.0	20.0	
Minimum Split (s)	12.0	12.0		12.0	12.0		45.0	45.0		45.0	45.0	
Total Split (s)	20.0	20.0		20.0	20.0		45.0	45.0		45.0	45.0	
Total Split (%)	23.5%	23.5%		23.5%	23.5%		52.9%	52.9%		52.9%	52.9%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	1.0	0.0		1.0	0.0		1.0	0.0		1.0	0.0	
Total Lost Time (s)		5.0			5.0			6.0			6.0	
Lead/Lag		0.0			0.0			0.0			0.0	
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		Min	Min		Min	Min	
Act Effct Green (s)	110110	8.5		110110	8.5		141111	37.0		141111	37.0	
Actuated g/C Ratio		0.18			0.18			0.80			0.80	
v/c Ratio		0.17			0.19			0.40			0.37	
Control Delay		21.8			22.1			8.4			8.0	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		21.8			22.1			8.4			8.0	
LOS		C			C			A			A	
Approach Delay		21.8			22.1			8.4			8.0	
Approach LOS		C			C			A			A	
Queue Length 50th (ft)		5			6			0			0	
Queue Length 95th (ft)		52			56			298			259	
Internal Link Dist (ft)		939			647			711			3441	
Turn Bay Length (ft)		737			047			711			5441	
Base Capacity (vph)		549			538			1389			1362	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductin		0			0			0			0	
Reduced v/c Ratio		0.08			0.09			0.37			0.33	
Intersection Summary												
Cycle Length: 85												

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Lane Group	Ø9	
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Satd. Flow (RTOR)		
Confl. Peds. (#/hr)		
Peak Hour Factor		
Heavy Vehicles (%)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	
Permitted Phases		
Detector Phase		
Switch Phase		
Minimum Initial (s)	2.0	
Minimum Split (s)	20.0	
Total Split (s)	20.0	
Total Split (%)	24%	
Yellow Time (s)	2.0	
All-Red Time (s)	0.0	
Lost Time Adjust (s)		
Total Lost Time (s)		
Lead/Lag		
Lead-Lag Optimize?		
Recall Mode	None	
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (ft)		
Queue Length 95th (ft)		
Internal Link Dist (ft)		
Turn Bay Length (ft)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

20: Route 138 & Robbins St.

Actuated Cycle Length: 46.3

Natural Cycle: 80

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.40

Intersection Signal Delay: 9.5

Intersection Capacity Utilization 43.9%

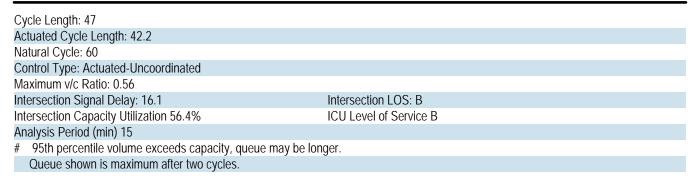
Analysis Period (min) 15

Splits and Phases: 20: Route 138 & Robbins St.

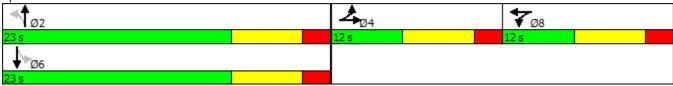


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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	•
Traffic Volume (vph)	3	6	3	35	9	15	3	450	45	15	420	7
Future Volume (vph)	3	6	3	35	9	15	3	450	45	15	420	7
Satd. Flow (prot)	0	1440	0	0	1426	0	0	1375	0	0	1354	0
Flt Permitted		0.988			0.971			0.997			0.975	
Satd. Flow (perm)	0	1431	0	0	1422	0	0	1371	0	0	1323	0
Satd. Flow (RTOR)												
Confl. Peds. (#/hr)	6		1	1		6	3		5	5		3
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	0%	0%	33%	9%	0%	6%	0%	3%	4%	0%	6%	0%
Parking (#/hr)								5			5	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	12	0	0	63	0	0	530	0	0	470	0
Turn Type	Split	NA		Split	NA		Perm	NA		Perm	NA	
Protected Phases	4	4		. 8	8			2			6	
Permitted Phases							2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	4.0	4.0		4.0	4.0		6.0	6.0		6.0	6.0	
Minimum Split (s)	11.0	11.0		11.0	11.0		20.0	20.0		20.0	20.0	
Total Split (s)	12.0	12.0		12.0	12.0		23.0	23.0		23.0	23.0	
Total Split (%)	25.5%	25.5%		25.5%	25.5%		48.9%	48.9%		48.9%	48.9%	
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		7.0			7.0			7.0			7.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		Min	Min		Min	Min	
Act Effct Green (s)		5.1			5.1			29.2			29.2	
Actuated g/C Ratio		0.12			0.12			0.69			0.69	
v/c Ratio		0.07			0.37			0.56			0.51	
Control Delay		18.8			25.1			15.8			15.0	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		18.8			25.1			15.8			15.0	
LOS		В			С			В			В	
Approach Delay		18.8			25.1			15.8			15.0	
Approach LOS		В			С			В			В	
Queue Length 50th (ft)		3			18			78			66	
Queue Length 95th (ft)		15			#47			#324			#286	
Internal Link Dist (ft)		838			877			3441			407	
Turn Bay Length (ft)												
Base Capacity (vph)		173			171			949			916	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.07			0.37			0.56			0.51	
Intersection Summary												

21: Route 138 & Cheever St./Blue Hill Terrace St.



Splits and Phases: 21: Route 138 & Cheever St./Blue Hill Terrace St.



	•	→	•	•	←	•	•	†	<u></u>	<u> </u>	Ţ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	3	6	3	35	9	15	3	450	45	15	420	7
Future Volume (veh/h)	3	6	3	35	9	15	3	450	45	15	420	7
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q, veh	0	0	0	0	0	0	0	5	0	0	5	0
Ped-Bike Adj (A_pbT)	1.00		0.97	1.00		0.97	1.00		0.99	1.00		0.99
Parking Bus Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1700	1570	1700	1700	1592	1700	1700	1649	1700	1700	1608	1700
Adj Flow Rate, veh/h	3	6	3	37	10	16	3	479	48	16	447	7
Adj No. of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	0	0	0	0	0	0	3	3	3	6	6	6
Opposing Right Turn Influence	e Yes			Yes			Yes			Yes		
Cap, veh/h	5	9	5	44	12	19	95	578	54	104	606	9
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Prop Arrive On Green	0.01	0.01	0.01	0.05	0.05	0.05	0.39	0.39	0.39	0.39	0.39	0.39
Ln Grp Delay, s/veh	51.4	0.0	0.0	39.3	0.0	0.0	15.4	0.0	0.0	13.2	0.0	0.0
Ln Grp LOS	D			D			В			В		
Approach Vol, veh/h		12			63			530			470	
Approach Delay, s/veh		51.4			39.3			15.4			13.2	
Approach LOS		D			D			В			В	
Timer:		1	2	3	4	5	6	7	8			
Assigned Phs			2	8	4		6					
Case No			8.0	12.0	12.0		8.0					
Phs Duration (G+Y+Rc), s			21.9	9.0	7.5		21.9					
Change Period (Y+Rc), s			7.0	7.0	7.0		7.0					
Max Green (Gmax), s			16.0	5.0	5.0		16.0					
Max Allow Headway (MAH), s	S		5.2	5.4	5.4		5.2					
Max Q Clear (g_c+l1), s			13.4	3.6	2.3		11.7					
Green Ext Time (g_e), s			1.5	0.0	0.0		2.4					
Prob of Phs Call (p_c)			1.00	0.51	0.15		1.00					
Prob of Max Out (p_x)			1.00	1.00	1.00		1.00					
Left-Turn Movement Data												
Assigned Mvmt			5	3	7		1					
Mvmt Sat Flow, veh/h			2	863	369		20					
Through Movement Data												
Assigned Mvmt			2	8	4		6					
Mvmt Sat Flow, veh/h			1472	233	737		1549					
Right-Turn Movement Data												
Assigned Mvmt			12	18	14		16					
Mvmt Sat Flow, veh/h			147	373	369		24					
Left Lane Group Data												
Assigned Mvmt		0	5	3	7	0	1	0	0			
Lane Assignment		U	L+T+R	L+T+R	L+T+R	U	L+T+R	U	U			
Earlo / 1331grilliont			Z I I I I I	LITIN	L 1 1 1 1 1 1 1		LITIN					

Lanes in Grp	0	1	1	1	0	1	0	0	
Grp Vol (v), veh/h	0	530	63	12	0	470	0	0	
Grp Sat Flow (s), veh/h/ln	0	1621	1470	1474	0	1593	0	0	
Q Serve Time (g_s), s	0.0	0.0	1.6	0.3	0.0	0.0	0.0	0.0	
Cycle Q Clear Time (g_c), s	0.0	11.4	1.6	0.3	0.0	9.7	0.0	0.0	
Perm LT Sat Flow (s_l), veh/h/ln	0	950	0	0	0	888	0	0	
Shared LT Sat Flow (s_sh), veh/h/ln	0	1649	0	0	0	1605	0	0	
Perm LT Eff Green (g_p), s	0.0	14.9	0.0	0.0	0.0	14.9	0.0	0.0	
Perm LT Serve Time (g_u), s	0.0	5.2	0.0	0.0	0.0	3.5	0.0	0.0	
Perm LT Q Serve Time (g_ps), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Time to First Blk (g_f), s	0.0	11.5	0.0	0.0	0.0	9.2	0.0	0.0	
Serve Time pre Blk (g_fs), s	0.0	11.4	0.0	0.0	0.0	9.2	0.0	0.0	
Prop LT Inside Lane (P_L)	0.00	0.01	0.59	0.25	0.00	0.03	0.00	0.00	
Lane Grp Cap (c), veh/h	0	727	75	18	0	718	0	0	
V/C Ratio (X)	0.00	0.73	0.84	0.65	0.00	0.65	0.00	0.00	
Avail Cap (c_a), veh/h	0	771	192	192	0	761	0	0	
Upstream Filter (I)	0.00	1.00	1.00	1.00	0.00	1.00	0.00	0.00	
Uniform Delay (d1), s/veh	0.0	10.8	18.1	18.9	0.0	10.3	0.0	0.0	
Incr Delay (d2), s/veh	0.0	3.3	21.2	32.5	0.0	1.9	0.0	0.0	
Initial Q Delay (d3), s/veh	0.0	1.3	0.0	0.0	0.0	1.0	0.0	0.0	
Control Delay (d), s/veh	0.0	15.4	39.3	51.4	0.0	13.2	0.0	0.0	
1st-Term Q (Q1), veh/ln	0.0	5.3	0.6	0.1	0.0	4.4	0.0	0.0	
2nd-Term Q (Q2), veh/ln	0.0	0.7	0.4	0.2	0.0	0.4	0.0	0.0	
3rd-Term Q (Q3), veh/ln	0.0	0.3	0.0	0.0	0.0	0.2	0.0	0.0	
%ile Back of Q Factor (f_B%)	0.00	1.00	1.00	1.00	0.00	1.00	0.00	0.00	
%ile Back of Q (50%), veh/ln	0.0	6.2	1.1	0.3	0.0	5.0	0.0	0.0	
%ile Storage Ratio (RQ%)	0.00	0.05	0.04	0.01	0.00	0.36	0.00	0.00	
Initial Q (Qb), veh	0.0	5.0	0.0	0.0	0.0	5.0	0.0	0.0	
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Sat Delay (ds), s/veh	0.0	12.7	18.6	19.4	0.0	12.7	0.0	0.0	
Sat Q (Qs), veh	0.0	8.0	0.7	0.1	0.0	7.8	0.0	0.0	
Sat Cap (cs), veh/h	0	757	74	18	0	742	0	0	
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Middle Lane Group Data									
Assigned Mvmt	0	2	8	4	0	6	0	0	
Lane Assignment	, ,	_		•					
Lanes in Grp	0	0	0	0	0	0	0	0	
Grp Vol (v), veh/h	0	0	0	0	0	0	0	0	
Grp Sat Flow (s), veh/h/ln	0	0	0	0	0	0	0	0	
Q Serve Time (q_s), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Cycle Q Clear Time (g_c), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Lane Grp Cap (c), veh/h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
V/C Ratio (X)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Avail Cap (c_a), veh/h	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Upstream Filter (I)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Uniform Delay (d1), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Incr Delay (d2), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Control Delay (d), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
1st-Term Q (Q1), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
	0.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	

2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile Back of Q Factor (f_B%)	0.00	1.00	1.00	1.00	0.00	1.00	0.00	0.00	
%ile Back of Q (50%), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile Storage Ratio (RQ%)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0	
nitial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Right Lane Group Data									
Assigned Mvmt	0	12	18	14	0	16	0	0	
Lane Assignment	-				-			-	
Lanes in Grp	0	0	0	0	0	0	0	0	
Grp Vol (v), veh/h	0	0	0	0	0	0	0	0	
Grp Sat Flow (s), veh/h/ln	0	0	0	0	0	0	0	0	
Q Serve Time (g_s), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Cycle Q Clear Time (g_c), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Prot RT Sat Flow (s_R), veh/h/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Prot RT Eff Green (g_R), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
rop RT Outside Lane (P_R)	0.00	0.09	0.25	0.25	0.00	0.01	0.00	0.00	
ane Grp Cap (c), veh/h	0	0	0	0	0	0	0	0	
/C Ratio (X)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
vail Cap (c_a), veh/h	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
pstream Filter (I)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Iniform Delay (d1), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
ncr Delay (d2), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
nitial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Control Delay (d), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Ist-Term Q (Q1), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Brd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile Back of Q Factor (f_B%)	0.00	1.00	1.00	1.00	0.00	1.00	0.00	0.00	
%ile Back of Q (50%), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile Storage Ratio (RQ%)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0	
nitial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
ntersection Summary									
HCM 2010 Ctrl Delay		16.2							
HCM 2010 LOS		В							
I IOWI ZUTU LUJ		D							

22: Route 138 & Aberdeen Rd.

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W	2011	.,,,,	4	1	0211
Traffic Volume (vph)	9	3	4	460	450	15
Future Volume (vph)	9	3	4	460	450	15
Satd. Flow (prot)	1414	0	0	1581	1576	0
Flt Permitted	0.963					
Satd. Flow (perm)	1414	0	0	1581	1576	0
Confl. Peds. (#/hr)	3	11	5			5
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	11%	0%	0%	4%	4%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	13	0	0	504	505	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utiliza				IC	U Level c	of Service A
Analysis Period (min) 15						

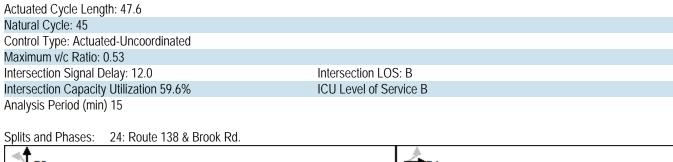
Intersection								
Int Delay, s/veh	0.4							
Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations	¥			4	ĵ.			
Traffic Vol, veh/h	9	3	4	460	450	15		
Future Vol, veh/h	9	3	4	460	450	15		
Conflicting Peds, #/hr		11	5	0	0	5		
Sign Control	Stop	Stop	Free	Free	Free	Free		
RT Channelized	-	None	-	None	-	None		
Storage Length	0	-	-	-	-	-		
Veh in Median Storag	je,# 0	-	-	0	0	-		
Grade, %	0	-	-	0	0	-		
Peak Hour Factor	92	92	92	92	92	92		
Heavy Vehicles, %	11	0	0	4	4	0		
Mvmt Flow	10	3	4	500	489	16		
Major/Minor	Minor2	Λ	/lajor1	I.	/lajor2			
Conflicting Flow All	1014	513	510	0	-	0		
Stage 1	502	-	-	-	-	-		
Stage 2	512		_	_	_	_		
Critical Hdwy	6.51	6.2	4.1	-	-	-		
Critical Hdwy Stg 1	5.51	-	-	-	-	-		
Critical Hdwy Stg 2	5.51	-	-	-	-	-		
Follow-up Hdwy	3.599	3.3	2.2	-	_	_		
Pot Cap-1 Maneuver	254	*109	*163	-	-	-		
Stage 1	590	-	-	-	-	-		
Stage 2	584	-	-	-	-	-		
Platoon blocked, %		1	1	-	-	-		
Mov Cap-1 Maneuver	243	*107	*162	-	-	-		
Mov Cap-2 Maneuver		-	-	-	-	-		
Stage 1	587	-	-	-	-	-		
Stage 2	562	-	-	-	-	-		
, and the second								
Approach	EB		NB		SB			
HCM Control Delay, s			0.2		0			
HCM LOS	D		0.2		U			
TIOWI LOG	U							
Minor Lane/Major Mvr	mt	NBL	NIDT	EBLn1	SBT	SBR		
	iiit				SDT	SDK		
Capacity (veh/h)		* 162	-	184	-	-		
HCM Cantral Dalay (c	.)	0.027		0.071	-	-		
HCM Long LOS	>)	27.8	0	26.1	-	-		
HCM Lane LOS	h)	D	Α	D	-	-		
HCM 95th %tile Q(veh	11)	0.1	-	0.2	-	-		
Notes								
~: Volume exceeds ca	anacity	¢. De	lav evo	eeds 30	nns	+. Comi	outation Not Defined	*: All major volume in platoor

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	3	3	10	3	1	5	10	460	9	7	440	1
Future Volume (vph)	3	3	10	3	1	5	10	460	9	7	440	1
Satd. Flow (prot)	0	1412	0	0	1496	0	0	1592	0	0	1565	0
Flt Permitted		0.991			0.984			0.999			0.999	
Satd. Flow (perm)	0	1412	0	0	1496	0	0	1592	0	0	1565	0
Confl. Peds. (#/hr)	66					66	23		66	66		23
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	0%	0%	9%	0%	0%	0%	0%	3%	0%	0%	5%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	16	0	0	9	0	0	493	0	0	462	0
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilizat	ion 52.9%			IC	CU Level	of Service	Α					
Analysis Period (min) 15												

Intersection													
Int Delay, s/veh	1.2												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4			4			4			4		
Traffic Vol, veh/h	3	3	10	3	1	5	10	460	9	7	440	1	
Future Vol, veh/h	3	3	10	3	1	5	10	460	9	7	440	1	
Conflicting Peds, #/hr	66	0	0	0	0	66	23	0	66	66	0	23	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-	
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	97	97	97	97	97	97	97	97	97	97	97	97	
Heavy Vehicles, %	0	0	9	0	0	0	0	3	0	0	5	0	
Mvmt Flow	3	3	10	3	1	5	10	474	9	7	454	1	
Major/Minor I	Minor2		N	Minor1			Major1		N	/lajor2			
Conflicting Flow All	1061	1062	477	1040	1057	611	478	0	0	550	0	0	
Stage 1	492	492	-	565	565	-		-	-	-	-	-	
Stage 2	569	570	_	475	492	_	_	_	_	_	_	_	
Critical Hdwy	7.1	6.5	6.29	7.1	6.5	6.2	4.1	_	_	4.1	_	_	
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	- 0.2		_	_		_	_	
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	_	_	_	_	_	
Follow-up Hdwy	3.5	4	3.381	3.5	4	3.3	2.2	_	_	2.2	_	_	
Pot Cap-1 Maneuver	203	225	*128	210	227	*131	*245	_	_	*245	_	_	
Stage 1	562	551	-	513	511	-	210	_	_	210	_	_	
Stage 2	511	509	-	574	551	_	_	_	_	_	_	_	
Platoon blocked, %	311	307	1	017	001	1	1	_	_	1	_	_	
Mov Cap-1 Maneuver	165	188	*125	166	190	*116	*245	_	_	*231	_	_	
Mov Cap-2 Maneuver	165	188	-	166	190	-	240	_	_	201	_	_	
Stage 1	520	518	_	456	455	_	_	_	_	_	_	_	
Stage 2	433	453	_	502	518	_	_	_	_	_	_	_	
Stage 2	733	700		302	310								
Approach	EB			WB			NB			SB			
	34.1			33.6			0.4			0.3			
HCM Control Delay, s HCM LOS	34.1 D			33.6 D			0.4			0.3			
HCIVI LUS	U			U									
Minor Lane/Major Mvm	nt .	NBL	NBT	NIDDI	EBLn1V	M/RI n1	SBL	SBT	SBR				
	IL		INDI	NDK				SDI	SDK				
Capacity (veh/h)		* 245	-	-	140	135	* 231	-	-				
HCM Control Doloy (c)		0.042	-			0.069		-	-				
HCM Control Delay (s)		20.3	0	-	34.1	33.6	21.1	0	-				
HCM Lane LOS	\	C	Α	-	D	D	C	Α	-				
HCM 95th %tile Q(veh))	0.1	-	-	0.4	0.2	0.1	-	-				
Notes													
~: Volume exceeds cap	pacity	\$: De	elay exc	eeds 3	00s	+: Com	putatior	Not D	efined	*: All	major v	olume i	n platoon

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4						4			4	
Traffic Volume (vph)	5	160	9	0	0	0	2	410	40	10	450	20
Future Volume (vph)	5	160	9	0	0	0	2	410	40	10	450	20
Satd. Flow (prot)	0	1562	0	0	0	0	0	1578	0	0	1549	0
Flt Permitted		0.999						0.998			0.990	
Satd. Flow (perm)	0	1562	0	0	0	0	0	1574	0	0	1535	0
Satd. Flow (RTOR)												
Confl. Peds. (#/hr)	4		4	2		2	2		2	1		6
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	0%	4%	11%	2%	2%	2%	0%	3%	0%	17%	5%	5%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	181	0	0	0	0	0	471	0	0	500	0
Turn Type	Perm	NA					Perm	NA		Perm	NA	
Protected Phases		4						2			6	
Permitted Phases	4						2			6		
Detector Phase	4	4					2	2		6	6	
Switch Phase												
Minimum Initial (s)	7.0	7.0					24.0	24.0		24.0	24.0	
Minimum Split (s)	12.0	12.0					30.0	30.0		30.0	30.0	
Total Split (s)	25.0	25.0					35.0	35.0		35.0	35.0	
Total Split (%)	41.7%	41.7%					58.3%	58.3%		58.3%	58.3%	
Yellow Time (s)	4.0	4.0					5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0					1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0						0.0			0.0	
Total Lost Time (s)		5.0						6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None					Min	Min		Min	Min	
Act Effct Green (s)		10.8						29.5			29.5	
Actuated g/C Ratio		0.23						0.62			0.62	
v/c Ratio		0.51						0.48			0.53	
Control Delay		21.4						9.9			10.6	
Queue Delay		0.0						0.0			0.0	
Total Delay		21.4						9.9			10.6	
LOS		С						Α			В	
Approach Delay		21.4						9.9			10.6	
Approach LOS		С						Α			В	
Queue Length 50th (ft)		41						72			79	
Queue Length 95th (ft)		96						177			197	
Internal Link Dist (ft)		967			1001			519			1021	
Turn Bay Length (ft)												
Base Capacity (vph)		661						1078			1052	
Starvation Cap Reductn		0						0			0	
Spillback Cap Reductn		0						0			0	
Storage Cap Reductn		0						0			0	
Reduced v/c Ratio		0.27						0.44			0.48	
Intersection Summary												
Cycle Length: 60												

24: Route 138 & Brook Rd.



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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4						4			4	
Traffic Volume (veh/h)	5	160	9	0	0	0	2	410	40	10	450	20
Future Volume (veh/h)	5	160	9	0	0	0	2	410	40	10	450	20
Number	7	4	14				5	2	12	1	6	16
Initial Q, veh	0	10	0				0	15	0	0	15	0
Ped-Bike Adj (A_pbT)	1.00		0.99				1.00		1.00	1.00		0.99
Parking Bus Adj	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1700	1631	1700				1700	1655	1700	1700	1615	1700
Adj Flow Rate, veh/h	5	167	9				2	427	42	10	469	21
Adj No. of Lanes	0	1	0				0	1	0	0	1	0
Peak Hour Factor	0.96	0.96	0.96				0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	0	4	0				3	3	3	5	5	5
Opposing Right Turn Influence	Yes	070	44				Yes	0.10	7.5	Yes	0.17	0.5
Cap, veh/h	6	273	11				84	849	75	88	867	35
HCM Platoon Ratio	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Prop Arrive On Green	0.15	0.15	0.15				0.58	0.58	0.58	0.58	0.58	0.58
Ln Grp Delay, s/veh	46.2	0.0	0.0				10.2	0.0	0.0	11.1	0.0	0.0
Ln Grp LOS	D	1.01					В	471		В	F00	
Approach Vol, veh/h		181						471			500	
Approach LOS		46.2						10.2			11.1	
Approach LOS		D						В			В	
Timer:		1	2	3	4	5	6	7	8			
Assigned Phs			2		4		6					
Case No			8.0		12.0		8.0					
Phs Duration (G+Y+Rc), s			30.0		11.3		30.0					
Change Period (Y+Rc), s			6.0		5.0		6.0					
Max Green (Gmax), s			29.0		20.0		29.0					
Max Allow Headway (MAH), s			5.2		5.3		5.2					
Max Q Clear (g_c+l1), s			9.0 6.2		6.4 0.8		9.8					
Green Ext Time (g_e), s			1.00		0.88		6.1					
Prob of Phs Call (p_c)			0.17				1.00 0.19					
Prob of Max Out (p_x)			0.17		0.01		0.19					
Left-Turn Movement Data												
Assigned Mvmt			5		7		1					
Mvmt Sat Flow, veh/h			1		45		9					
Through Movement Data												
Assigned Mvmt			2		4		6					
Mvmt Sat Flow, veh/h			1482		1489		1520					
Right-Turn Movement Data												
Assigned Mvmt			12		14		16					
Mvmt Sat Flow, veh/h			145		80		67					
Left Lane Group Data												
Assigned Mvmt		0	5	0	7	0	1	0	0			
Lane Assignment		U	L+T+R		L+T+R		L+T+R	J	U			
Earlo / toolgrimont			Z I I I I I		E I I I I		LITTE					

-									
Lanes in Grp	0	1	0	1	0	1	0	0	
Grp Vol (v), veh/h	0	471	0	181	0	500	0	0	
Grp Sat Flow (s), veh/h/ln	0	1628	0	1613	0	1596	0	0	
Q Serve Time (g_s), s	0.0	0.0	0.0	4.4	0.0	0.0	0.0	0.0	
Cycle Q Clear Time (g_c), s	0.0	7.0	0.0	4.4	0.0	7.8	0.0	0.0	
Perm LT Sat Flow (s_l), veh/h/ln	0	919	0	0	0	938	0	0	
Shared LT Sat Flow (s_sh), veh/h/ln	0	1655	0	0	0	1614	0	0	
Perm LT Eff Green (g_p), s	0.0	24.0	0.0	0.0	0.0	24.0	0.0	0.0	
Perm LT Serve Time (g_u), s	0.0	16.2	0.0	0.0	0.0	17.0	0.0	0.0	
Perm LT Q Serve Time (g_ps), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Time to First Blk (g_f), s	0.0	20.2	0.0	0.0	0.0	17.3	0.0	0.0	
Serve Time pre Blk (g_fs), s	0.0	7.0	0.0	0.0	0.0	7.8	0.0	0.0	
Prop LT Inside Lane (P_L)	0.00	0.00	0.00	0.03	0.00	0.02	0.00	0.00	
Lane Grp Cap (c), veh/h	0	1007	0	278	0	985	0	0	
V/C Ratio (X)	0.00	0.47	0.00	0.65	0.00	0.51	0.00	0.00	
Avail Cap (c_a), veh/h	0	1231	0	782	0	1208	0	0	
Upstream Filter (I)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00	
Uniform Delay (d1), s/veh	0.0	6.9	0.0	17.1	0.0	7.3	0.0	0.0	
Incr Delay (d2), s/veh	0.0	0.3	0.0	2.6	0.0	0.4	0.0	0.0	
Initial Q Delay (d3), s/veh	0.0	3.0	0.0	26.6	0.0	3.4	0.0	0.0	
Control Delay (d), s/veh	0.0	10.2	0.0	46.2	0.0	11.1	0.0	0.0	
1st-Term Q (Q1), veh/ln	0.0	4.7	0.0	2.7	0.0	5.2	0.0	0.0	
2nd-Term Q (Q2), veh/ln	0.0	0.1	0.0	0.2	0.0	0.1	0.0	0.0	
3rd-Term Q (Q3), veh/ln	0.0	0.8	0.0	2.1	0.0	0.9	0.0	0.0	
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00	
%ile Back of Q (50%), veh/ln	0.0	5.7	0.0	5.0	0.0	6.2	0.0	0.0	
%ile Storage Ratio (RQ%)	0.00	0.30	0.00	0.13	0.00	0.15	0.00	0.00	
Initial Q (Qb), veh	0.0	15.0	0.0	10.0	0.0	15.0	0.0	0.0	
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Sat Delay (ds), s/veh	0.0	16.5	0.0	20.0	0.0	16.5	0.0	0.0	
Sat Q (Qs), veh	0.0	14.0	0.0	8.9	0.0	13.7	0.0	0.0	
Sat Cap (cs), veh/h	0	857	0	543	0	838	0	0	
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.1	0.0	0.0	0.0	0.0	
Middle Lane Group Data									
Assigned Mvmt	0	2	0	4	0	6	0	0	
Lane Assignment	U	Z	U	4	U	O	U	U	
Lanes in Grp	0	0	0	0	0	0	0	0	
Grp Vol (v), veh/h	0	0	0	0	0	0	0	0	
Grp Vol (v), ven/n Grp Sat Flow (s), veh/h/ln	0	0	0	0	0	0	0	0	
Q Serve Time (q_s), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Cycle Q Clear Time (g_c), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Lane Grp Cap (c), veh/h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
V/C Ratio (X)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Avail Cap (c_a), veh/h	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Upstream Filter (I)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Uniform Delay (d1), s/veh	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Incr Delay (d2), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Control Delay (d), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
1st-Term Q (Q1), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	

2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00	
%ile Back of Q (50%), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile Storage Ratio (RQ%)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0	
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Right Lane Group Data									
Assigned Mvmt	0	12	0	14	0	16	0	0	
Lane Assignment									
Lanes in Grp	0	0	0	0	0	0	0	0	
Grp Vol (v), veh/h	0	0	0	0	0	0	0	0	
Grp Sat Flow (s), veh/h/ln	0	0	0	0	0	0	0	0	
Q Serve Time (g_s), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Cycle Q Clear Time (g_c), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Prot RT Sat Flow (s_R), veh/h/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Prot RT Eff Green (g_R), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Prop RT Outside Lane (P_R)	0.00	0.09	0.00	0.05	0.00	0.04	0.00	0.00	
Lane Grp Cap (c), veh/h	0	0	0	0	0	0	0	0	
V/C Ratio (X)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Avail Cap (c_a), veh/h	0	0	0	0	0	0	0	0	
Upstream Filter (I)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Uniform Delay (d1), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Incr Delay (d2), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Control Delay (d), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
1st-Term Q (Q1), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00	
%ile Back of Q (50%), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile Storage Ratio (RQ%)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0	
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Intersection Summary									
HCM 2010 Ctrl Delay		16.2							
HCM 2010 LOS		В							

102: Route 138 & Concord Baptist Church

	•	•	†	/	\	↓
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		f)			ર્ન
Traffic Volume (vph)	1	1	460	1	5	450
Future Volume (vph)	1	1	460	1	5	450
Satd. Flow (prot)	1495	0	1642	0	0	1642
Flt Permitted	0.976					0.999
Satd. Flow (perm)	1495	0	1642	0	0	1642
Confl. Peds. (#/hr)	3			11	11	
Peak Hour Factor	0.25	0.25	0.92	0.25	0.63	0.88
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	8	0	504	0	0	519
Sign Control	Stop		Free			Free
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utiliza				IC	U Level	of Service
Analysis Period (min) 15						
<i>J</i> = 2 = 2 = (·····)						

Intersection						
Int Delay, s/veh	0.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	, A		₽			4
Traffic Vol, veh/h	1	1	460	1	5	450
Future Vol, veh/h	1	1	460	1	5	450
Conflicting Peds, #/hr	3	0	0	11	11	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	25	25	92	25	63	88
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	4	4	500	4	8	511
	•	•	000	•		0
	Minor1		/lajor1	N	Major2	
Conflicting Flow All	1043	513	0	0	515	0
Stage 1	513	-	-	-	-	-
Stage 2	530	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	256	565	-	-	1061	-
Stage 1	605	-	_	-	-	-
Stage 2	594	-	-	-	-	-
Platoon blocked, %			_	-		_
Mov Cap-1 Maneuver	250	560	_	-	1061	_
Mov Cap-1 Maneuver	250	-	_		-	_
Stage 1	599					
Stage 2	586	_		_		
Staye 2	500	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	15.7		0		0.1	
HCM LOS	С					
Minor Long/Major M.	\ 1	NDT	MDD	M/DI1	CDI	CDT
Minor Lane/Major Mvm	Il	NBT		VBLn1	SBL	SBT
Capacity (veh/h)		-	-	0.0	1061	-
HCM Lane V/C Ratio		-		0.023		-
HCM Control Delay (s)		-	-		8.4	0
HCM Lane LOS		-	-	С	Α	Α
HCM 95th %tile Q(veh)	1	-	-	0.1	0	-

8: Route 138 & South Parking Lot Entrance

	•	•	†	<i>></i>	\	↓
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations			f)			ર્ન
Traffic Volume (vph)	0	0	1210	50	8	1260
Future Volume (vph)	0	0	1210	50	8	1260
Satd. Flow (prot)	0	0	1791	0	0	1819
Flt Permitted						
Satd. Flow (perm)	0	0	1791	0	0	1819
Confl. Peds. (#/hr)				6	6	
Peak Hour Factor	0.92	0.92	0.91	0.78	0.67	0.97
Heavy Vehicles (%)	2%	2%	2%	0%	0%	1%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	1394	0	0	1311
Sign Control	Stop		Free			Free
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utilizati	on 76.0%			IC	U Level o	of Service
Analysis Period (min) 15						

9: Route 138 & South Parking Lot Exit

	•	•	†	/	-	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	M.		†			†
Traffic Volume (vph)	10	70	1200	0	0	1260
Future Volume (vph)	10	70	1200	0	0	1260
Satd. Flow (prot)	1610	0	1818	0	0	1818
Flt Permitted	0.994					
Satd. Flow (perm)	1610	0	1818	0	0	1818
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	0%	1%	2%	2%	1%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	86	0	1290	0	0	1355
Sign Control	Stop		Free			Free
Intersection Summary						

Control Type: Unsignalized Intersection Capacity Utilization 77.9%

ICU Level of Service D

3.4					
	MED	Not	NDD	001	ODT
	WBR		NRK	SBL	SBT
					↑
					1260
					1260
					0
Stop		Free		Free	Free
-	None	-	None	-	None
0	-	-	-	-	-
e, # 0	-	0	-	-	0
0	-	0	-	-	0
93	93	93	93	93	93
0	0	1	2	2	1
11	75	1290	0	0	1355
Minor1	Ι.	Noior1	Λ.	//oior?	
			IV		
			-	-	-
			-	-	-
		-	-	-	-
	6.2	-	-	-	-
	-	-	-	-	-
	-	-	-	-	-
		-	-	-	-
	202	-	0	0	-
	-	-	0	0	-
242	-	-	0	0	-
		-			-
26	202	-	-	-	-
26	-	-	-	-	-
261	-	-	-	-	-
242	-	-	-	-	-
WR		NB		SB	
		0		0	
				U	
108.8		U			
108.8 F		0			
F					
	NBTV	VBLn1	SBT		
F	-	VBLn1 109	SBT -		
F	-	VBLn1			
F	-	VBLn1 109	-		
F nt	-	VBLn1 109 0.789	-		
	0 e, # 0 0 93 0 111 Minor1 2645 1290 1355 6.4 5.4 3.5 26 261 242 26 26 261	WBL WBR 10 70 10 70 0 0 0 Stop Stop - None 0 e, # 0 93 93 0 0 11 75 Minor1 N 2645 1290 1290 1355 6.4 6.2 5.4 5.4 3.5 3.3 26 202 261 242 26 202 26 242	WBL WBR NBT 10 70 1200 10 70 1200 0 0 0 Stop Stop Free None - 0 0 - 0 93 93 93 0 0 1 11 75 1290 Minor1 Major1 2645 1290 0 1290 - - 1355 - - 6.4 6.2 - 5.4 - - 3.5 3.3 - 26 202 - 261 - - 26 202 - 26 - - 26 202 - 26 - - 26 - - 26 - - 26 - -	WBL WBR NBT NBR 10 70 1200 0 10 70 1200 0 0 0 0 0 Stop Stop Free Free - None - None 0 - - - 0 - 0 - 93 93 93 93 0 0 1 2 11 75 1290 0 1290 - - - 1355 - - - 5.4 - - - 5.4 - - - 3.5 3.3 - - 26 202 - 0 261 - - 0 266 - - - 261 - - - 261 - -	WBL WBR NBT NBR SBL V Image: Control of the control of th

10: Route 138 & North Parking Lot Entrance

	_	•	†	<i>></i>	\	1
	•		'	′		•
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations			₽			र्स
Traffic Volume (vph)	0	0	1230	25	25	1250
Future Volume (vph)	0	0	1230	25	25	1250
Satd. Flow (prot)	0	0	1810	0	0	1817
Flt Permitted						0.999
Satd. Flow (perm)	0	0	1810	0	0	1817
Confl. Peds. (#/hr)				1	1	
Peak Hour Factor	0.92	0.92	0.89	0.50	0.72	0.96
Heavy Vehicles (%)	2%	2%	1%	0%	0%	1%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	1432	0	0	1337
Sign Control	Stop		Free			Free
Intersection Summary						
Control Type: Unsignalized	lon 00 20/			IC	III ovol	of Condoc
Intersection Capacity Utilizat	1011 89.2%			IC	o Level	of Service
Analysis Period (min) 15						

	٠	→	•	•	+	•	4	†	<i>></i>	/	+	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			ર્ન			ĵ»	
Traffic Volume (vph)	5	0	10	6	0	70	4	1220	0	0	1260	7
Future Volume (vph)	5	0	10	6	0	70	4	1220	0	0	1260	7
Satd. Flow (prot)	0	1645	0	0	1601	0	0	1819	0	0	1817	0
Flt Permitted		0.984			0.996							
Satd. Flow (perm)	0	1645	0	0	1601	0	0	1819	0	0	1817	0
Confl. Peds. (#/hr)	5					5						
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	0%	2%	0%	0%	2%	0%	0%	1%	0%	2%	1%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	15	0	0	78	0	0	1262	0	0	1306	0
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilization 80.6% ICU Level of Service D												
Analysis Period (min) 15												

Intersection												
Int Delay, s/veh	2.7											
		EDT	EDD	WDI	WDT	WDD	NDI	NDT	NDD	CDI	CDT	CDD
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	_	4			4			र्स			f)	_
Traffic Vol, veh/h	5	0	10	6	0	70	4	1220	0	0	1260	7
Future Vol, veh/h	5	0	10	6	0	70	4	1220	0	0	1260	7
Conflicting Peds, #/hr	5	0	0	0	0	5	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	2,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	97	97	97	97	97	97	97	97	97	97	97	97
Heavy Vehicles, %	0	2	0	0	2	0	0	1	0	2	1	0
Mvmt Flow	5	0	10	6	0	72	4	1258	0	0	1299	7
Major/Minor	Minor2		N	Minor1		N	/lajor1		N	/lajor2		
Conflicting Flow All	2610	2569	1303	2574	2572	1263	1306	0		najorz		0
Stage 1	1303	1303	1303	1266	1266	1203	1300	-	-	-	-	Ū
Stage 2	1303	1266	-	1308	1306	-	-	-	-	-	-	-
Critical Hdwy	6.5	6.52	6.2	6.5	10	6.2	4.1	-	-	-	-	-
Critical Hdwy Stg 1	6.1	5.52	0.2	6.1	5.52	0.2	4.1	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.52		6.1	5.52	-	-	-	-	-	-	-
	3.5	4.018	3.3		4.018	3.3	2.2	-	-	-	-	-
Follow-up Hdwy	25	4.018	198	27	4.018	209	537	-	-	- 0	-	-
Pot Cap-1 Maneuver		231			240			-	0	0	-	-
Stage 1	199 198	240	-	209		-	-	-	0	0	-	-
Stage 2	198	240	-	198	230	-	-	-	U	0	-	-
Platoon blocked, %	1/	٦F	100	٦٢	2	200	E27	-			-	-
Mov Cap-1 Maneuver	16	25	198	25	2	208	537	-	-	-	-	-
Mov Cap-2 Maneuver	16	25	-	25	2	-	-	-	-	-	-	-
Stage 1	194	231	-	204	234	-	-	-	-	-	-	-
Stage 2	126	234	-	188	230	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	138.5			65.9			0			0		
HCM LOS	F			F								
Minor Lane/Major Mvm	nt	NBL	MRTI	EBLn1V	VRI n1	SBT	SBR					
	π		INDII			JDT	אטכ					
Capacity (veh/h)		537	-	41	132	-	-					
HCM Central Delay (c)		0.008		0.377		-	-					
HCM Control Delay (s)		11.8		138.5	65.9	-	-					
HCM Lane LOS	\	В	Α	F	F	-	-					
HCM 95th %tile Q(veh)	0	-	1.3	3	-	-					

	•	•	٤	†	7	/	₩	/	ļ	4	t	
Lane Group	WBL	WBR	WBR2	NBT	NBR	NBR2	SBL2	SBL	SBT	SWL	SWR	
Lane Configurations	W			f.					4			
Traffic Volume (vph)	0	1	1	850	450	9	2	0	1260	0	0	
Future Volume (vph)	0	1	1	850	450	9	2	0	1260	0	0	
Satd. Flow (prot)	1589	0	0	1722	0	0	0	0	1819	0	0	
Flt Permitted												
Satd. Flow (perm)	1589	0	0	1722	0	0	0	0	1819	0	0	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	
Heavy Vehicles (%)	0%	0%	0%	2%	1%	0%	0%	2%	1%	2%	2%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	2	0	0	1349	0	0	0	0	1301	0	0	
Sign Control	Stop			Free					Free	Stop		

Control Type: Unsignalized

Intersection Capacity Utilization 82.7%

ICU Level of Service E

Intersection								
Int Delay, s/veh	0							
Movement	WBL	WBR	NBT	NBR	SBL	SBT	SWL	SWR
Lane Configurations	¥		₽			4		
Traffic Vol, veh/h	0	1	850	450	0	1260	0	0
Future Vol, veh/h	0	1	850	450	0	1260	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	-	-	-	-	None	-	None
Storage Length	0	-	-	-	-	-	-	-
Veh in Median Storage,	# 0	-	0	-	-	0	-	-
Grade, %	0	-	0	-	-	0	0	-
Peak Hour Factor	97	97	97	97	97	97	97	97
Heavy Vehicles, %	0	0	2	1	2	1	2	2
Mvmt Flow	0	1	876	464	0	1299	0	0
		•	0,0	.01		,,		
	1inor1		/lajor1		/lajor2			
Conflicting Flow All	0	0	0	0	473	0		
Stage 1	0	-	-	-	-	-		
Stage 2	0	-	-	-	-	-		
Critical Hdwy	-	-	-	-	-	-		
Critical Hdwy Stg 1	-	-	-	-	-	-		
Critical Hdwy Stg 2	-	-	-	-	-	-		
Follow-up Hdwy	-	_	-	-	-	_		
Pot Cap-1 Maneuver	-	-	-	-	-	-		
Stage 1		_	_	_	_	_		
Stage 2	_	_	-	_	-	_		
Platoon blocked, %			_	_				
Mov Cap-1 Maneuver	-		-	-	_	-		
Mov Cap-2 Maneuver	-	_	-	-	-	-		
Stage 1	-	-	-	-	-	-		
	-	-	-	-	-	-		
Stage 2	-	-	-	-	-	-		
Approach	WB		NB		SB			
HCM Control Delay, s								
HCM LOS	-							
Minor Lane/Major Mvmt		NBT	NBR	NBR2W	/BLn1	SBL2	SBL	SBT
Capacity (veh/h)		-	-	-	-	-	-	-
HCM Lane V/C Ratio		-	-	-	-	-	-	-
HCM Control Delay (s)		-	-	-	-	-	-	-
HCM Lane LOS		-	-	-	-	-	-	-
HCM 95th %tile Q(veh)		-	-	-	-	-	-	-
, , c 2(1011)								

13: Route 138 & Thacher Montessori School

	•	\rightarrow	4	†	ļ	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			4	f)	
Traffic Volume (vph)	0	6	6	850	1270	0
Future Volume (vph)	0	6	6	850	1270	0
Satd. Flow (prot)	1589	0	0	1801	1818	0
Flt Permitted						
Satd. Flow (perm)	1589	0	0	1801	1818	0
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	2%	0%	0%	2%	1%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	6	0	0	891	1323	0
Sign Control	Stop			Free	Free	
Intersection Summary						

Control Type: Unsignalized Intersection Capacity Utilization 76.8%

ICU Level of Service D

Intersection						
Int Delay, s/veh	0.1					
		EDD.	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			र्स	₽	
Traffic Vol, veh/h	0	6	6	850	1270	0
Future Vol, veh/h	0	6	6	850	1270	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	96	96	96	96	96	96
Heavy Vehicles, %	2	0	0	2	1	0
Mvmt Flow	0	6	6	885	1323	0
					5	
	Minor2		Major1		/lajor2	
Conflicting Flow All	2221	1323	1323	0	-	0
Stage 1	1323	-	-	-	-	-
Stage 2	898	-	-	-	-	-
Critical Hdwy	6.42	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	48	193	529	-	-	-
Stage 1	249	-	-	-	-	-
Stage 2	398	-	-	-	-	-
Platoon blocked, %	0.0			_	_	_
Mov Cap-1 Maneuver	47	193	529	_	_	_
Mov Cap-1 Maneuver Mov Cap-2 Maneuver	47	175	JZ 7 -	_		
Stage 1	249	-	-	-	-	-
		-	-	-	-	-
Stage 2	389	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	24.3		0.1		0	
HCM LOS	C		3.1			
1.000 200						
Minor Lane/Major Mvn	nt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		529	-	193	-	-
HCM Lane V/C Ratio		0.012	-	0.032	-	-
HCM Control Delay (s))	11.9	0	24.3	-	-
HCM Lane LOS		В	Α	С	-	-
HCM 95th %tile Q(veh)	0	-	0.1	-	-
	•					

	۶	→	•	•	←	•	•	†	<i>></i>	/	+	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		7	f)			ર્ન			ĵ»	
Traffic Volume (vph)	5	0	330	170	120	0	1	850	0	0	770	5
Future Volume (vph)	5	0	330	170	120	0	1	850	0	0	770	5
Satd. Flow (prot)	0	1591	0	1728	1837	0	0	1801	0	0	1799	0
Flt Permitted		0.997		0.393				0.999				
Satd. Flow (perm) Satd. Flow (RTOR)	0	1588	0	715	1837	0	0	1799	0	0	1799	0
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	0%	2%	0%	1%	0%	13%	0%	2%	2%	2%	2%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	352	0	179	126	0	0	896	0	0	816	0
Turn Type	Perm	NA		Perm	NA		Perm	NA			NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2					
Detector Phase	4	4		8	8		2	2			6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0			10.0	
Minimum Split (s)	20.0	20.0		20.0	20.0		45.0	45.0			45.0	
Total Split (s)	30.0	30.0		30.0	30.0		45.0	45.0			45.0	
Total Split (%)	40.0%	40.0%		40.0%	40.0%		60.0%	60.0%			60.0%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0			5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0			1.0	
Lost Time Adjust (s)		0.0		0.0	0.0			0.0			0.0	
Total Lost Time (s)		5.0		5.0	5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?	None	None		Mono	None		None	None			Mono	
Recall Mode	None	None 20.1		None 20.1	None		None	None 37.1			None 37.1	
Act Effct Green (s)		0.29		0.29	20.1 0.29			0.54			0.54	
Actuated g/C Ratio v/c Ratio		0.29		0.29	0.29			0.92			0.84	
Control Delay		33.4		59.3	19.7			32.6			24.3	
Queue Delay		0.0		0.0	0.0			0.0			0.0	
Total Delay		33.4		59.3	19.7			32.6			24.3	
LOS		33.4 C		59.5 E	19.7 B			32.0 C			24.3 C	
Approach Delay		33.4		L	42.9			32.6			24.3	
Approach LOS		C			42.7 D			32.0 C			24.3 C	
Queue Length 50th (ft)		138		73	41			346			290	
Queue Length 95th (ft)		229		#176	80			#637			#553	
Internal Link Dist (ft)		906		#170	198			187			2213	
Turn Bay Length (ft)		700			170			107			2210	
Base Capacity (vph)		592		266	685			1046			1046	
Starvation Cap Reductn		0		0	0			0			0	
Spillback Cap Reductn		0		0	0			0			0	
Storage Cap Reductn		0		0	0			0			0	
Reduced v/c Ratio		0.59		0.67	0.18			0.86			0.78	
Intersection Summary				-	-						-	

Cycle Length: 75

Actuated Cycle Length: 68.4

14: Route 138 & Brush Hill Rd.

Natural Cycle: 70 Control Type: Actuated-Uncoordinated Maximum v/c Ratio: 0.92 Intersection Signal Delay: 31.2 Intersection LOS: C Intersection Capacity Utilization 89.0% ICU Level of Service E Analysis Period (min) 15 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles. Splits and Phases: 14: Route 138 & Brush Hill Rd.



		→	•	•	—	•	•	†	<i>></i>	/	ţ	√
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		ň	ĵ»			ર્ન			ĵ»	
Traffic Volume (veh/h)	5	0	330	170	120	0	1	850	0	0	770	5
Future Volume (veh/h)	5	0	330	170	120	0	1	850	0	0	770	5
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	15	0	5	5	0	0	5	0	0	20	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1900	1900	1900	1881	1900	1900	1900	1863	0	0	1863	1900
Adj Flow Rate, veh/h	5	0	347	179	126	0	1	895	0	0	811	5
Adj No. of Lanes	0	1	0	1	1	0	0	1	0	0	1	0
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	2	2	2	1	0	0	2	2	0	0	2	2
Cap, veh/h	50	5	427	190	590	0	48	901	0	0	973	4
Arrive On Green	0.31	0.00	0.31	0.31	0.31	0.00	0.53	0.53	0.00	0.00	0.53	0.53
Sat Flow, veh/h	6	17	1593	1098	1900	0	0	1862	0	0	1850	11
Grp Volume(v), veh/h	352	0	0	179	126	0	896	0	0	0	0	816
Grp Sat Flow(s),veh/h/ln	1616	0	0	1098	1900	0	1862	0	0	0	0	1861
Q Serve(g_s), s	0.0	0.0	0.0	4.6	3.5	0.0	0.0	0.0	0.0	0.0	0.0	25.6
Cycle Q Clear(g_c), s	13.5	0.0	0.0	18.1	3.5	0.0	30.4	0.0	0.0	0.0	0.0	25.6
Prop In Lane	0.01	0	0.99	1.00	F00	0.00	0.00		0.00	0.00	0	0.01
Lane Grp Cap(c), veh/h	482	0	0	190	590	0	897	0	0	0	0	980
V/C Ratio(X)	0.73	0.00	0.00	0.94	0.21	0.00	1.00	0.00	0.00	0.00	0.00	0.83
Avail Cap(c_a), veh/h	625	0	1.00	379	674	0	1082	0	0	0	0	1030
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00 21.5	0.00	0.00	1.00 31.3	1.00 18.2	0.00	1.00	0.00	0.00	0.00	0.00	1.00 15.9
Uniform Delay (d), s/veh Incr Delay (d2), s/veh	3.1	0.0	0.0	19.0	0.2	0.0	17.6 25.5	0.0	0.0	0.0	0.0	5.7
Initial Q Delay(d3),s/veh	25.9	0.0	0.0	68.2	0.2	0.0	19.7	0.0	0.0	0.0	0.0	17.9
%ile BackOfQ(50%),veh/ln	10.0	0.0	0.0	8.5	2.4	0.0	28.4	0.0	0.0	0.0	0.0	22.9
LnGrp Delay(d),s/veh	50.6	0.0	0.0	118.5	19.1	0.0	62.8	0.0	0.0	0.0	0.0	39.5
LnGrp LOS	D	0.0	0.0	F	В	0.0	02.0 E	0.0	0.0	0.0	0.0	37.3 D
Approach Vol, veh/h		352		'	305			896			816	
Approach Vol, ven/ii Approach Delay, s/veh		50.6			77.4			62.8			39.5	
Approach LOS		D			77.4 E			02.0 E			D	
• •			0			,	-					
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		43.6		26.8		43.6		26.8				
Change Period (Y+Rc), s		6.0		5.0		6.0		5.0				
Max Green Setting (Gmax), s		39.0		25.0		39.0		25.0				
Max Q Clear Time (g_c+l1), s		32.4		15.5		27.6		20.1 1.7				
Green Ext Time (p_c), s		5.2		2.7		8.2		1.7				
Intersection Summary												
HCM 2010 Ctrl Delay			54.8									
HCM 2010 LOS			D									

	۶	\rightarrow	•	†	ļ	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations			ሻ	^		7
Traffic Volume (vph)	0	0	170	280	0	140
Future Volume (vph)	0	0	170	280	0	140
Satd. Flow (prot)	0	0	1745	1818	0	1573
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1745	1818	0	1573
Peak Hour Factor	0.92	0.92	0.86	0.79	0.92	0.94
Heavy Vehicles (%)	2%	2%	0%	1%	2%	1%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	198	354	0	149
Sign Control	Free			Free	Free	
Interesetion Cummers						

Control Type: Unsignalized

Intersection Capacity Utilization 24.8%

ICU Level of Service A

16: Route 138 & Neponset Valley Pkwy.

	٠	\rightarrow	4	†	ļ	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	**			र्स	f)	
Traffic Volume (vph)	4	200	390	490	570	1
Future Volume (vph)	4	200	390	490	570	1
Satd. Flow (prot)	1562	0	0	1755	1801	0
Flt Permitted	0.999			0.978		
Satd. Flow (perm)	1562	0	0	1755	1801	0
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	0%	2%	4%	1%	2%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	210	0	0	907	589	0
Sign Control	Stop			Free	Free	
Intersection Summary						

Control Type: Unsignalized Intersection Capacity Utilization 100.0%

ICU Level of Service G

Intersection						
Int Delay, s/veh	5.6					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	, M			र्स	Þ	
Traffic Vol, veh/h	4	200	390	490	570	1
Future Vol, veh/h	4	200	390	490	570	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storag	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	0	2	4	1	2	0
Mvmt Flow	4	206	402	505	588	1
	•	200	.02		000	•
	Minor2		Major1		/lajor2	
Conflicting Flow All	1897	588	589	0	-	0
Stage 1	588	-	-	-	-	-
Stage 2	1309	-	-	-	-	-
Critical Hdwy	6.4	6.22	4.14	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.318	2.236	-	-	-
Pot Cap-1 Maneuver	77	509	977	-	-	-
Stage 1	559	-	-	-	-	-
Stage 2	255	_	-	-	-	-
Platoon blocked, %				_	_	_
Mov Cap-1 Maneuver	33	509	977	-	_	_
Mov Cap 1 Maneuver		-	-	_	_	
Stage 1	559			_		
Stage 2	109					
Staye 2	109	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	23.8		5		0	
HCM LOS	С					
Minor Long/Major M.	m+	NDI	NDT	ΓDI ∽1	CDT	CDD
Minor Lane/Major Mvr	TIL	NBL		EBLn1	SBT	SBR
Capacity (veh/h)		977	-	0	-	-
HCM Lane V/C Ratio		0.412	-	0.53	-	-
HCM Control Delay (s)	11.2	0	23.8	-	-
HCM Lane LOS		В	Α	С	-	-
HCM 95th %tile Q(veh	1)	2	-	3	-	-

SBR

25

25

0

0

0.95

0%

17: Route 138 & Milton St./Dollar Ln. Ť NBR **EBL EBT EBR WBL WBT** WBR **NBL** NBT SBL **SBT** Lane Group Lane Configurations 4 4 4 4 Traffic Volume (vph) 30 100 8 15 70 20 15 460 540 Future Volume (vph) 30 100 8 15 70 20 15 460 1 540 Satd. Flow (prot) 0 1751 0 0 0 0 0 0 1808 1776 1812 Flt Permitted 0.919 0.950 0.977 0.999 Satd. Flow (perm) 1627 0 0 1699 0 0 1774 0 1807 Satd. Flow (RTOR) Confl. Peds. (#/hr) 1 Peak Hour Factor 0.95 0.95 0.95 0.95 0.95 0.95 0.95 0.95 0.95 0.95 0.95 Heavy Vehicles (%) 0% 4% 0% 0% 0% 0% 0% 1% 0% 0% 1% Shared Lane Traffic (%) Lane Group Flow (vph) 0 145 111 509 595 Perm Turn Type NA NA NA Perm Perm Perm NA Protected Phases 4 8 2 6 Permitted Phases 8 2 4 6 **Detector Phase** 4 8 8 2 6 6 Switch Phase Minimum Initial (s) 7.0 7.0 7.0 7.0 30.0 30.0 30.0 30.0 Minimum Split (s) 12.0 12.0 12.0 12.0 37.0 37.0 37.0 37.0 25.0 25.0 Total Split (s) 25.0 25.0 45.0 45.0 45.0 45.0 Total Split (%) 35.7% 35.7% 35.7% 35.7% 64.3% 64.3% 64.3% 64.3% Yellow Time (s) 4.0 4.0 4.0 4.0 5.0 5.0 5.0 5.0 All-Red Time (s) 1.0 1.0 1.0 1.0 2.0 2.0 2.0 2.0 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 Total Lost Time (s) 5.0 5.0 7.0 7.0 Lead/Lag Lead-Lag Optimize? Recall Mode None None None None Min Min Min Min Act Effct Green (s) 10.2 35.0 10.2 35.0 Actuated g/C Ratio 0.19 0.19 0.66 0.66 v/c Ratio 0.47 0.34 0.44 0.50 Control Delay 24.1 21.4 8.2 9.0 Queue Delay 0.0 0.0 0.0 0.0 **Total Delay** 21.4 8.2 9.0 24.1 LOS С С Α Α Approach Delay 24.1 21.4 8.2 9.0 Approach LOS C C Α Α Queue Length 50th (ft) 40 30 78 98 Queue Length 95th (ft) 175 86 67 216 Internal Link Dist (ft) 1017 791 1154 1181 Turn Bay Length (ft)

639

0

0

0

0.17

1364

0

0

0

0.37

Addressing Priority Corridors from the LRTP Needs Assessment: Route 138 in Milton
2018 Existing Conditions - Weekend Peak Hour

612

0

0

0

0.24

Base Capacity (vph)

Starvation Cap Reductn

Spillback Cap Reductn

Storage Cap Reductn

Intersection Summary
Cycle Length: 70

Reduced v/c Ratio

1389

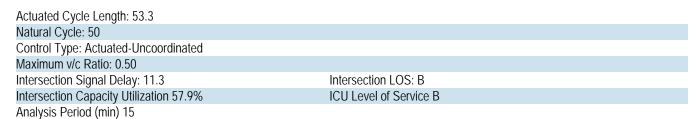
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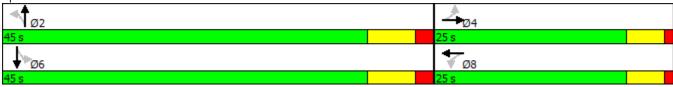
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17: Route 138 & Milton St./Dollar Ln.



Splits and Phases: 17: Route 138 & Milton St./Dollar Ln.



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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			44			4			4	
Traffic Volume (veh/h)	30	100	8	15	70	20	15	460	9	1	540	25
Future Volume (veh/h)	30	100	8	15	70	20	15	460	9	1	540	25
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	20	0	0	10	0	0	10	0	0	20	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1900	1847	1900	1900	1900	1900	1900	1882	1900	1900	1882	1900
Adj Flow Rate, veh/h	32	105	8	16	74	21	16	484	9	1	568	26
Adj No. of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	4	4	4	0	0	0	1	1	1	1	1	1
Cap, veh/h	113	234	12	101	227	42	86	1094	18	70	1086	46
Arrive On Green	0.14	0.14	0.14	0.14	0.14	0.14	0.61	0.61	0.61	0.61	0.61	0.61
Sat Flow, veh/h	262	1388	96	147	1313	341	20	1801	33	0	1785	82
Grp Volume(v), veh/h	145	0	0	111	0	0	509	0	0	595	0	0
Grp Sat Flow(s),veh/h/ln	1747	0	0	1800	0	0	1854	0	0	1867	0	0
Q Serve(g_s), s	1.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	3.7	0.0	0.0	2.7	0.0	0.0	7.0	0.0	0.0	8.8	0.0	0.0
Prop In Lane	0.22		0.06	0.14		0.19	0.03		0.02	0.00		0.04
Lane Grp Cap(c), veh/h	377	0	0	356	0	0	1205	0	0	1197	0	0
V/C Ratio(X)	0.38	0.00	0.00	0.31	0.00	0.00	0.42	0.00	0.00	0.50	0.00	0.00
Avail Cap(c_a), veh/h	792	0	0	807	0	0	1513	0	0	1528	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	20.8	0.0	0.0	19.8	0.0	0.0	5.8	0.0	0.0	7.3	0.0	0.0
Incr Delay (d2), s/veh	0.6	0.0	0.0	0.5	0.0	0.0	0.2	0.0	0.0	0.3	0.0	0.0
Initial Q Delay(d3),s/veh	32.8	0.0	0.0	8.3	0.0	0.0	0.9	0.0	0.0	4.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	7.0	0.0	0.0	3.1 28.5	0.0	0.0	5.1 6.9	0.0	0.0	8.6	0.0	0.0
LnGrp Delay(d),s/veh	54.3 D	0.0	0.0	28.5 C	0.0	0.0	0.9 A	0.0	0.0	11.6 B	0.0	0.0
LnGrp LOS	D	1 4 5		C	111		A	F00		D	ГОГ	
Approach Vol, veh/h		145			111			509			595	
Approach LOS		54.3			28.5 C			6.9			11.6 B	
Approach LOS		D						A			Б	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		37.0		11.8		37.0		11.8				
Change Period (Y+Rc), s		7.0		5.0		7.0		5.0				
Max Green Setting (Gmax), s		38.0		20.0		38.0		20.0				
Max Q Clear Time (g_c+l1), s		9.0		5.7		10.8		4.7				
Green Ext Time (p_c), s		8.3		1.2		8.2		1.2				
Intersection Summary			45.0									
HCM 2010 Ctrl Delay			15.8									
HCM 2010 LOS			В									

18: Route 138 & Blue Jay Way (Curry College)

	•	\rightarrow	4	†	ļ	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	f.	
Traffic Volume (vph)	0	0	0	0	0	0
Future Volume (vph)	0	0	0	0	0	0
Satd. Flow (prot)	1837	0	0	1701	1733	0
Flt Permitted						
Satd. Flow (perm)	1837	0	0	1701	1733	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	7%	1%	8%	6%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	0	0
Sign Control	Stop			Free	Free	
Intersection Summary						

Control Type: Unsignalized Intersection Capacity Utilization 0.0%

ICU Level of Service A

Intersection						
Int Delay, s/veh	0					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	, A			र्स	₽	
Traffic Vol, veh/h	0	0	0	0	0	0
Future Vol, veh/h	0	0	0	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	_	_	0	0	_
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	7	1	8	6	0
Mvmt Flow	0	0	0	0	0	0
IVIVIIIL I IOVV	U	U	U	U	U	U
Major/Minor I	Minor2		Major1	N	Major2	
Conflicting Flow All	1	1	1	0	-	0
Stage 1	1	-	-	-	-	-
Stage 2	0	_	_	-	-	_
Critical Hdwy	6.4	6.27	4.11	_	-	_
Critical Hdwy Stg 1	5.4	0.27	-	_	_	_
Critical Hdwy Stg 2	5.4					
Follow-up Hdwy	3.5	3.363	2.209	_	_	
Pot Cap-1 Maneuver	1027	1069	1628	-	-	-
•		1009	1028	-	-	-
Stage 1	1028	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	1027	1069	1628	-	-	-
Mov Cap-2 Maneuver	1027	-	-	-	-	-
Stage 1	1028	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Annroach	ED		MP		CD	
Approach	EB		NB		SB	
HCM Control Delay, s	0		0		0	
HCM LOS	Α					
Minor Lane/Major Mvm	nt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1628	-		051	ODIN
HCM Lane V/C Ratio		1020		-	-	-
			-			-
HCM Long LOS		0	-	0	-	-
HCM Lane LOS		A	-	Α	-	-
HCM 95th %tile Q(veh))	0	-	-	-	-

19: Route 138 & Atherton St. & Bradlee Rd.

15. Route 150 & 7tt		O Q L	aaio	,a.	
	-	•	†	↓	\
Lane Group	EBT	WBT	NBT	SBT	SEL
Lane Configurations	4	4	4	- 4	M.
Traffic Volume (vph)	0	0	0	0	0
Future Volume (vph)	0	0	0	0	0
Satd. Flow (prot)	1837	1837	1701	1733	1749
Flt Permitted	1037	1037	1701	1733	1/47
Satd. Flow (perm)	1837	1837	1701	1733	1749
Satd. Flow (RTOR)	1037	1037	1701	1733	1/47
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0.92	0.92	8%	6%	5%
Shared Lane Traffic (%)	U /0	U /0	0 /0	0 /0	5 /0
Lane Group Flow (vph)	0	0	0	0	0
	0	U	0	U	Prot
Turn Type Protected Phases	Л		2	L	
Permitted Phases	4	8	2	6	5
Detector Phase	Λ		2	L	5
Switch Phase	4	8	2	6	5
	ΕΛ	ΕO	ΕO	ΕO	ΕΛ
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	20.0	20.0	20.0	20.0	20.0
Total Split (s)	20.0	20.0	20.0	20.0	20.0
Total Split (%)	20.0%	20.0%	20.0%	20.0%	20.0%
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5
Lead/Lag				Lag	Lead
Lead-Lag Optimize?	N.	NI	8.41	Yes	Yes
Recall Mode	None	None	Min	Min	None
Act Effct Green (s)					
Actuated g/C Ratio					
v/c Ratio					
Control Delay					
Queue Delay					
Total Delay					
LOS					
Approach Delay					
Approach LOS					
Queue Length 50th (ft)					
Queue Length 95th (ft)	4000		4501	04:-	
Internal Link Dist (ft)	1290	1410	1586	2667	689
Turn Bay Length (ft)					
Base Capacity (vph)					
Starvation Cap Reductn					
Spillback Cap Reductn					
Storage Cap Reductn					
Reduced v/c Ratio					
Intersection Summary					
Cycle Length: 100					
Actuated Cycle Length: 20					
Astudiou Oyolo Longin. 20					

19: Route 138 & Atherton St. & Bradlee Rd.

Natural Cycle: 100
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.00
Intersection Signal Delay: 0.0
Intersection Capacity Utilization 0.0%
ICU Level of Service A
Analysis Period (min) 15

Splits and Phases: 19: Route 138 & Atherton St. & Bradlee Rd.

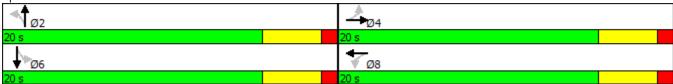


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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Future Volume (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Satd. Flow (prot)	0	1837	0	0	1837	0	0	1701	0	0	1717	0
Flt Permitted												
Satd. Flow (perm)	0	1837	0	0	1837	0	0	1701	0	0	1717	0
Satd. Flow (RTOR)												
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	3%	0%	13%	0%	8%	8%	0%	7%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Turn Type												
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	20.0	20.0		20.0	20.0		20.0	20.0		20.0	20.0	
Total Split (s)	20.0	20.0		20.0	20.0		20.0	20.0		20.0	20.0	
Total Split (%)	50.0%	50.0%		50.0%	50.0%		50.0%	50.0%		50.0%	50.0%	
Yellow Time (s)	3.5	3.5		3.5	3.5		3.5	3.5		3.5	3.5	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	110	0.0		1.0	0.0		1.0	0.0		1.0	0.0	
Total Lost Time (s)		4.5			4.5			4.5			4.5	
Lead/Lag		.,,						1,0				
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		Min	Min		Min	Min	
Act Effct Green (s)	140110	140110		140110	110110							
Actuated g/C Ratio												
v/c Ratio												
Control Delay												
Queue Delay												
Total Delay												
LOS												
Approach Delay												
Approach LOS												
Queue Length 50th (ft)												
Queue Length 95th (ft)												
Internal Link Dist (ft)		939			647			711			3441	
Turn Bay Length (ft)		737			047			711			ודדנ	
Base Capacity (vph)												
Starvation Cap Reductn												
Spillback Cap Reductn												
Storage Cap Reductn												
Reduced v/c Ratio												
Intersection Summary Cycle Length: 40												
Actuated Cycle Length: 25												
Actuated Cycle Length: 25												

20: Route 138 & Robbins St.

Natural Cycle: 40
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.00
Intersection Signal Delay: 0.0
Intersection Capacity Utilization 0.0%
ICU Level of Service A
Analysis Period (min) 15

Splits and Phases: 20: Route 138 & Robbins St.



	•	→	•	•	←	•	1	†	/	\	†	✓
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	0	0	0	0	0	0	0	0	0	0	0	0
Future Volume (veh/h)	0	0	0	0	0	0	0	0	0	0	0	0
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1900	1900	1900	1900	1900	1900	1900	1759	1900	1900	1776	1900
Adj Flow Rate, veh/h	0	0	0	0	0	0	0	0	0	0	0	0
Adj No. of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	0	0	0	0	0 9999	0	8	8 9999	8	7	7 9999	7
Cap, veh/h Arrive On Green	0.00	9999 0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Sat Flow, veh/h	0.00	-85500	0.00	0.00	-85500	0.00	0.00	-79167	0.00	0.00	-79907	0.00
	0		0	0		0	0					0
Grp Volume(v), veh/h Grp Sat Flow(s), veh/h/ln	0	0 1900	0	0	0 1900	0	0	0 1759	0	0	0 1776	0
Q Serve(g_s), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop In Lane	0.00	0.0	0.00	0.00	0.0	0.00	0.00	0.0	0.00	0.00	0.0	0.00
Lane Grp Cap(c), veh/h		1378624	0.00		4378624	0.00		9608320	0.00		5775104	0.00
V/C Ratio(X)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	126486078			12648607			117117039				4936320	0.00
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay (d), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
LnGrp Delay(d),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
LnGrp LOS												
Approach Vol, veh/h		0			0			0			0	
Approach Delay, s/veh		0.0			0.0			0.0			0.0	
Approach LOS												
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		0.0		0.0		0.0		0.0				
Change Period (Y+Rc), s		4.5		4.5		4.5		4.5				
Max Green Setting (Gmax), s		15.5		15.5		15.5		15.5				
Max Q Clear Time (g_c+I1), s	S	0.0		0.0		0.0		0.0				
Green Ext Time (p_c), s		0.0		0.0		0.0		0.0				
Intersection Summary												
HCM 2010 Ctrl Delay			0.0									
HCM 2010 LOS			Α									

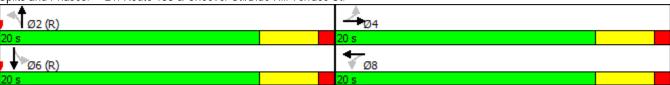
21: Route 138 & Cr	ite 138 & Cheever St./Blue Hill Terrace St.										20/2018	
	۶	→	•	•	←	•	•	†	~	/	ļ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Future Volume (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Satd. Flow (prot)	0	1837	0	0	1837	0	0	1701	0	0	1701	0
Flt Permitted	_		-	-	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	-	_			_		
Satd. Flow (perm)	0	1837	0	0	1837	0	0	1701	0	0	1701	0
Satd. Flow (RTOR)	J	1007			1007	J	•	1701		J	1701	J
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	1%	0%	0%	0%	8%	0%	13%	8%	0%
Shared Lane Traffic (%)	070	070	070	170	070	070	070	070	070	1070	070	070
Lane Group Flow (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Turn Type	U	0	U	- U	U	U	U	0	- O	U	U	J
Protected Phases		4			8			2			6	
Permitted Phases	4			8	U		2			6	U	
Minimum Split (s)	20.0	20.0		20.0	20.0		20.0	20.0		20.0	20.0	
Total Split (s)	20.0	20.0		20.0	20.0		20.0	20.0		20.0	20.0	
Total Split (%)	50.0%	50.0%		50.0%	50.0%		50.0%	50.0%		50.0%	50.0%	
Yellow Time (s)	3.5	3.5		3.5	3.5		3.5	3.5		3.5	3.5	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
. ,	1.0			1.0			1.0	0.0		1.0		
Lost Time Adjust (s)		0.0			0.0						0.0	
Total Lost Time (s)		4.5			4.5			4.5			4.5	
Lead/Lag												
Lead-Lag Optimize?												
Act Effet Green (s)												
Actuated g/C Ratio												
v/c Ratio												
Control Delay												
Queue Delay												
Total Delay												
LOS												
Approach Delay												
Approach LOS												
Queue Length 50th (ft)												
Queue Length 95th (ft)		000			077			0.4.44			407	
Internal Link Dist (ft)		838			877			3441			407	
Turn Bay Length (ft)												
Base Capacity (vph)												
Starvation Cap Reductn												
Spillback Cap Reductn												
Storage Cap Reductn												
Reduced v/c Ratio												
Intersection Summary												
Cycle Length: 40												
Actuated Cycle Length: 40												
Offset: 0 (0%), Referenced	to phase 2	::NBTL an	d 6:SBTL	., Start of	Green							
Natural Cycle: 40												
Control Type: Pretimed												
Maximum v/c Datio: 0.00												

Maximum v/c Ratio: 0.00

08/20/2018

Intersection Signal Delay: 0.0 Intersection LOS: A
Intersection Capacity Utilization 0.0% ICU Level of Service A
Analysis Period (min) 15

Splits and Phases: 21: Route 138 & Cheever St./Blue Hill Terrace St.



	۶	→	•	•	←	•	•	†	/	/	 	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	0	0	0	0	0	0	0	0	0	0	0	0
Future Volume (veh/h)	0	0	0	0	0	0	0	0	0	0	0	0
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1900	1900	1900	1900	1900	1900	1900	1759	1900	1900	1759	1900
Adj Flow Rate, veh/h	0	0	0	0	0	0	0	0	0	0	0	0
Adj No. of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	0	0	0	0	0	0	8	8	8	8	8	8
Cap, veh/h	0	9999	0	0	9999	0	0	9999	0	0	9999	0
Arrive On Green	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Sat Flow, veh/h	0	-85500	0	0	-85500	0	0	-79167	0	0	-79167	0
Grp Volume(v), veh/h	0	0	0	0	0	0	0	0	0	0	0	0
Grp Sat Flow(s), veh/h/ln	0	1900	0	0	1900	0	0	1759	0	0	1759	0
Q Serve(g_s), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop In Lane	0.00		0.00	0.00		0.00	0.00		0.00	0.00		0.00
Lane Grp Cap(c), veh/h	816004	4378624	0	816004	4378624	0	755059	608320	0	755059	9608320	0
V/C Ratio(X)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Avail Cap(c_a), veh/h 1	26486078	3686720	0 1	26486078	3686720	0 1	17117039	330560	0 1	177117039	9330560	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay (d), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
LnGrp Delay(d),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
LnGrp LOS												
Approach Vol, veh/h		0			0			0			0	
Approach Delay, s/veh		0.0			0.0			0.0			0.0	
Approach LOS												
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		0.0		0.0		0.0		0.0				
Change Period (Y+Rc), s		4.5		4.5		4.5		4.5				
Max Green Setting (Gmax), s		15.5		15.5		15.5		15.5				
Max Q Clear Time (g_c+l1), s		0.0		0.0		0.0		0.0				
Green Ext Time (p_c), s		0.0		0.0		0.0		0.0				
Intersection Summary												
HCM 2010 Ctrl Delay			0.0									
HCM 2010 LOS			Α									

22: Route 138 & Aberdeen Rd.

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			4	ĵ»	
Traffic Volume (vph)	1	2	2	270	350	15
Future Volume (vph)	1	2	2	270	350	15
Satd. Flow (prot)	1645	0	0	1801	1808	0
Flt Permitted	0.984					
Satd. Flow (perm)	1645	0	0	1801	1808	0
Confl. Peds. (#/hr)	8		5			5
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	0%	0%	2%	1%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	3	0	0	292	392	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utiliza				IC	III evel c	of Service A
Analysis Period (min) 15	111011 27.470			10	O LOVEI C	JOHNICE F
Analysis i Giba (IIIII) 13						

Intersection						
Int Delay, s/veh	0.1					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			र्स	₽	
Traffic Vol, veh/h	1	2	2	270	350	15
Future Vol, veh/h	1	2	2	270	350	15
Conflicting Peds, #/hr	8	0	5	0	0	5
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	_
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	0	0	0	2	1	0
Mvmt Flow	1	2	2	290	376	16
IVIVIIIL I IOVV				270	370	10
	/linor2		/lajor1	١	/lajor2	
Conflicting Flow All	692	389	397	0	-	0
Stage 1	389	-	-	-	-	-
Stage 2	303	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	_	-
Pot Cap-1 Maneuver	413	664	1173	_	-	_
Stage 1	689	- 004		_	_	_
Stage 2	754	-	-	-	-	-
Platoon blocked, %	754	-	-	-	-	-
	400	<u>//1</u>	1170	-	-	-
Mov Cap-1 Maneuver	409	661	1173	-	-	-
Mov Cap-2 Maneuver	409	-	-	-	-	-
Stage 1	686	-	-	-	-	-
Stage 2	749	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	11.6		0.1		0	
HCM LOS	В		311			
Notice and Language and American		ND	NICT	EDL 4	CDT	CDD
Minor Lane/Major Mvmt	t	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	t	1173	-	548	SBT -	SBR -
Capacity (veh/h) HCM Lane V/C Ratio	t	1173 0.002	-	548 0.006		SBR -
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	t	1173	-	548	-	-
Capacity (veh/h) HCM Lane V/C Ratio	t	1173 0.002	-	548 0.006	- -	-

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Future Volume (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Satd. Flow (prot)	0	1837	0	0	1837	0	0	1717	0	0	1701	0
Flt Permitted												
Satd. Flow (perm)	0	1837	0	0	1837	0	0	1717	0	0	1701	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	25%	0%	0%	0%	7%	0%	0%	8%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control		Stop			Stop			Free			Free	

Control Type: Unsignalized

Intersection Capacity Utilization 0.0%

ICU Level of Service A

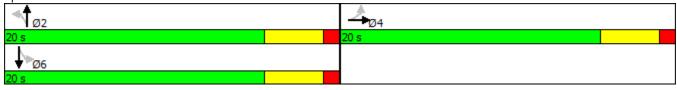
Intersection												
Int Delay, s/veh	0											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	0	0	0	0	0	0	0	0	0	0	0
Future Vol, veh/h	0	0	0	0	0	0	0	0	0	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	25	0	0	0	7	0	0	8	0
Mvmt Flow	0	0	0	0	0	0	0	0	0	0	0	0
Major/Minor N	Minor2		١	Minor1		N	/lajor1		N	Major2		
Conflicting Flow All	1	1	1	1	1	0	1	0	0	0	0	0
Stage 1	1	1	-	0	0	-	-	-	-	-	-	-
Stage 2	0	0	-	1	1	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.35	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.35	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.35	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.725	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	1027	899	1090	965	899	-	1635	-	-	-	-	-
Stage 1	1027	899	-	-	-	-	-	-	-	-	-	-
Stage 2	-	-	-	965	899	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	-	899	1090	965	899	-	1635	-	-	-	-	-
Mov Cap-2 Maneuver	-	899	-	965	899	-	-	-	-	-	-	-
Stage 1	1027	899	-	-	-	-	-	-	-	-	-	-
Stage 2	-	-	-	965	899	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0			0			0		
HCM LOS	A			A								
Minor Lane/Major Mvm	nt	NBL	NBT	MRRI	EBLn1V	VRI n1	SBL	SBT	SBR			
Capacity (veh/h)		1635	-	NOIN	-DEIIIV	*DLIII	JDL -	301	JUIC			
HCM Lane V/C Ratio		1033	-	-	-	-	-	-	-			
HCM Control Delay (s)		0	-	-	0	0	0	-	-			
HCM Lane LOS		A	-	-	A	A	A	-	-			
HCM 95th %tile Q(veh))	0	-	-	- A	- -	-	-	-			
HOW 75W 76WE Q(VEH)		U	_		-		-					

Lane Group EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL SBT SBR SBT Taffic Volume (ptph) 0	24. Roule 136 & Di	OUK INC	١.									00/2	20/2010
Lane Configurations		ᄼ	→	\rightarrow	•	←	•	4	†	<i>></i>	>	ļ	4
Traffic Volume (vph)	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph)	Lane Configurations		4						4			4	
Futher Volume (vph) 0 0 0 0 0 0 0 0 0 0 0 0 1717 0 0 1701 0 FIL Permitted Satd. Flow (prem) 0 1818 0 0 0 0 0 0 1717 0 0 1701 0 7101 0 Satd. Flow (prem) 0 1818 0 0 0 0 0 0 1717 0 0 1701 0 7101 0 Satd. Flow (prem) 0 1818 0 0 0 0 0 0 1717 0 0 1701 0 Satd. Flow (prem) 0 1818 0 0 0 0 0 0 1717 0 0 1701 0 Satd. Flow (prem) 0 1818 0 0 0 0 0 0 1717 0 0 1701 0 Satd. Flow (prem) 0 1818 0 0 0 0 0 0 0 1717 0 0 1701 0 Satd. Flow (prem) 0 1818 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		0		0	0	0	0	0		0	0		0
Fil Permitted Satd. Flow (perm) 0 1818 0 0 0 0 0 1717 0 0 1701 0 Satd. Flow (RTOR) Satd. Flow (RTOR) Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92	Future Volume (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Sald. Flow (perm) Sald. Flow (perm) Sald. Flow (perm) Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92	Satd. Flow (prot)	0	1818	0	0	0	0	0	1717	0	0	1701	0
Satd Flow (RTOR)	Flt Permitted												
Peak Hour Factor 0.92	Satd. Flow (perm)	0	1818	0	0	0	0	0	1717	0	0	1701	0
Heavy Vehicles (%)	Satd. Flow (RTOR)												
Shared Lane Traffic (%) Lane Group Flow (ph) O O O O O O O O O	Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Lane Group Flow (vph) 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Heavy Vehicles (%)	13%	1%	0%	2%	2%	2%	20%	7%	9%	0%	8%	0%
Turn Type Protected Phases	Shared Lane Traffic (%)												
Protected Phases	Lane Group Flow (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Protected Phases	Turn Type												
Detector Phase 4			4						2			6	
Switch Phase Minimum Initial (s) 5.0 5.0 5.0 5.0 5.0 5.0 5.0 Minimum Spit (s) 20.0 20.0 20.0 20.0 20.0 20.0 20.0 Total Spitt (s) 20.0 20.0 20.0 20.0 20.0 20.0 20.0 Total Spitt (s) 50.0%	Permitted Phases	4						2			6		
Minimum Initial (s) 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 Minimum Spiti (s) 20.0 20.0 20.0 20.0 20.0 20.0 20.0 20.	Detector Phase	4	4					2	2		6	6	
Minimum Split (s) 20.0 20.0 20.0 20.0 20.0 20.0 20.0 20.	Switch Phase												
Total Split (s) 20.0 20.0 20.0 20.0 20.0 20.0 20.0 20.	Minimum Initial (s)	5.0	5.0					5.0	5.0		5.0	5.0	
Total Split (%) 50.0% 50.0% 50.0% 50.0% 50.0% 50.0% 50.0% 90.0% 90.0% 90.0% 90.0% 90.0% 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0	Minimum Split (s)	20.0	20.0					20.0	20.0		20.0	20.0	
Total Split (%) 50.0% 50.0% 50.0% 50.0% 50.0% 50.0% 50.0% 90.0% 91.00 Time (s) 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5	Total Split (s)	20.0	20.0					20.0	20.0		20.0	20.0	
Yellow Time (s) 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5		50.0%	50.0%					50.0%	50.0%		50.0%	50.0%	
All-Red Time (s) 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0		3.5	3.5					3.5	3.5		3.5	3.5	
Total Lost Time (s)		1.0	1.0					1.0	1.0		1.0	1.0	
Total Lost Time (s)	Lost Time Adjust (s)		0.0						0.0			0.0	
Lead-Lag Optimize? Recall Mode None None Min Min Min Min Min Act Effet Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (ft) Queue Length 95th (ft) Internal Link Dist (ft) 967 1001 519 1021 Turn Bay Length (ft) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Reduced v/c Ratio Intersection Summary Cycle Length: 40			4.5						4.5			4.5	
Recall Mode None None Min Min Min Min Min Act Effet Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (ft) Queue Length 95th (ft) Internal Link Dist (ft) 967 1001 519 1021 Turn Bay Length (ft) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio Intersection Summary Cycle Length: 40	Lead/Lag												
Act Effet Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (ft) Queue Length 95th (ft) Internal Link Dist (ft) 967 1001 519 1021 Turn Bay Length (ft) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio Intersection Summary Cycle Length: 40													
Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (ft) Queue Length 95th (ft) Internal Link Dist (ft) 967 1001 519 1021 Turn Bay Length (ft) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio Intersection Summary Cycle Length: 40	Recall Mode	None	None					Min	Min		Min	Min	
v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (ft) Queue Length 95th (ft) Internal Link Dist (ft) 967 1001 519 1021 Turn Bay Length (ft) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio Intersection Summary Cycle Length: 40	Act Effct Green (s)												
Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (ft) Queue Length 95th (ft) Internal Link Dist (ft) 967 1001 519 1021 Turn Bay Length (ft) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio Intersection Summary Cycle Length: 40	Actuated g/C Ratio												
Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (ft) Queue Length 95th (ft) Internal Link Dist (ft) 967 1001 519 1021 Turn Bay Length (ft) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio Intersection Summary Cycle Length: 40	v/c Ratio												
Total Delay LOS Approach Delay Approach LOS Queue Length 50th (ft) Queue Length 95th (ft) Internal Link Dist (ft) 967 1001 519 1021 Turn Bay Length (ft) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio Intersection Summary Cycle Length: 40	Control Delay												
Approach Delay Approach LOS Queue Length 50th (ft) Queue Length 95th (ft) Internal Link Dist (ft) 967 1001 519 1021 Turn Bay Length (ft) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio Intersection Summary Cycle Length: 40	Queue Delay												
Approach Delay Approach LOS Queue Length 50th (ft) Queue Length 95th (ft) Internal Link Dist (ft) 967 1001 519 1021 Turn Bay Length (ft) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio Intersection Summary Cycle Length: 40	Total Delay												
Approach LOS Queue Length 50th (ft) Queue Length 95th (ft) Internal Link Dist (ft) 967 1001 519 1021 Turn Bay Length (ft) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio Intersection Summary Cycle Length: 40	LOS												
Oueue Length 50th (ft) Queue Length 95th (ft) Internal Link Dist (ft) 967 1001 519 1021 Turn Bay Length (ft) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio Intersection Summary Cycle Length: 40	Approach Delay												
Oueue Length 95th (ft) Internal Link Dist (ft) 967 1001 519 1021 Turn Bay Length (ft) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio Intersection Summary Cycle Length: 40	Approach LOS												
Internal Link Dist (ft) 967 1001 519 1021 Turn Bay Length (ft) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio Intersection Summary Cycle Length: 40	Queue Length 50th (ft)												
Turn Bay Length (ft) Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio Intersection Summary Cycle Length: 40	Queue Length 95th (ft)												
Base Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio Intersection Summary Cycle Length: 40	Internal Link Dist (ft)		967			1001			519			1021	
Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio Intersection Summary Cycle Length: 40													
Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn Reduced v/c Ratio Intersection Summary Cycle Length: 40	Base Capacity (vph)												
Storage Cap Reductn Reduced v/c Ratio Intersection Summary Cycle Length: 40													
Reduced v/c Ratio Intersection Summary Cycle Length: 40													
Intersection Summary Cycle Length: 40	Storage Cap Reductn												
Cycle Length: 40	Reduced v/c Ratio												
	Intersection Summary												
Actuated Cycle Length: 25													
	Actuated Cycle Length: 25												

24: Route 138 & Brook Rd.

Natural Cycle: 40
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.00
Intersection Signal Delay: 0.0
Intersection Capacity Utilization 0.0%
Analysis Period (min) 15

Splits and Phases: 24: Route 138 & Brook Rd.



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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4						4			4	
Traffic Volume (veh/h)	0	0	0	0	0	0	0	0	0	0	0	0
Future Volume (veh/h)	0	0	0	0	0	0	0	0	0	0	0	0
Number	7	4	14				5	2	12	1	6	16
Initial Q (Qb), veh	0	0	0				0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00				1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1900	1881	1900				1900	1776	1900	1900	1759	1900
Adj Flow Rate, veh/h	0	0	0				0	0	0	0	0	0
Adj No. of Lanes	0	1	0				0	1	0	0	1	0
Peak Hour Factor	0.92	0.92	0.92				0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	0	1	0				7	7	7	8	8	8
Cap, veh/h	0	9999	0				0	9999	0	0	9999	0
Arrive On Green	0.00	0.00	0.00				0.00	0.00	0.00	0.00	0.00	0.00
Sat Flow, veh/h	0	1881	0				0	-79907	0	0	-79167	0
Grp Volume(v), veh/h	0	0	0				0	0	0	0	0	0
Grp Sat Flow(s),veh/h/ln	0	1881	0				0	1776	0	0	1759	0
Q Serve(g_s), s	0.0	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	0.0	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
Prop In Lane	0.00	410040	0.00				0.00	775104	0.00	0.00	2/00220	0.00
Lane Grp Cap(c), veh/h		418048	0					5775104	0		9608320	0.00
V/C Ratio(X)	0.00	0.00	0.00			-	0.00 1821 10 94	0.00	0.00	0.00 1711 70 39	0.00	0.00
Avail Cap(c_a), veh/h HCM Platoon Ratio	12523 40 44 1.00	1.00	0 1.00			l	1.00	1.00	1.00	1.00		1.00
Upstream Filter(I)	0.00	0.00	0.00				0.00	0.00	0.00	0.00	1.00	0.00
Uniform Delay (d), s/veh	0.00	0.00	0.00				0.00	0.00	0.00	0.00	0.00	0.00
Incr Delay (d2), s/veh	0.0	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.0	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
LnGrp Delay(d),s/veh	0.0	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
LnGrp LOS	0.0	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
Approach Vol, veh/h		0						0			0	
Approach Delay, s/veh		0.0						0.0			0.0	
Approach LOS		0.0						0.0			0.0	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6						
Phs Duration (G+Y+Rc), s		0.0		0.0		0.0						
Change Period (Y+Rc), s		4.5		4.5		4.5						
Max Green Setting (Gmax), s		15.5		15.5		15.5						
Max Q Clear Time (q_c+l1), s		0.0		0.0		0.0						
Green Ext Time (p_c), s		0.0		0.0		0.0						
Intersection Summary												
HCM 2010 Ctrl Delay			0.0									
HCM 2010 LOS			Α									

102: Route 138 & Concord Baptist Church

	•	•	†	/	\	↓
Lane Group	• WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		f)			ર્ન
Traffic Volume (vph)	0	0	270	30	100	250
Future Volume (vph)	0	0	270	30	100	250
Satd. Flow (prot)	1837	0	1813	0	0	1806
Flt Permitted						0.986
Satd. Flow (perm)	1837	0	1813	0	0	1806
Confl. Peds. (#/hr)	19			53	53	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	0%	0%	0%	1%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	322	0	0	377
Sign Control	Stop		Free			Free
Intersection Summary						
Control Type: Unsignalized	tion 41 00/			10	اللمينوا	of Comile
Intersection Capacity Utiliza	uon 41.8%			IC	u Level	of Service
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	1.3					
		14/55		NES	05:	05=
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		- ₽			4
Traffic Vol, veh/h	0	0	270	30	100	250
Future Vol, veh/h	0	0	270	30	100	250
Conflicting Peds, #/hr	19	0	0	53	53	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,	, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	0	0	0	0	1	0
Mvmt Flow	0	0	290	32	108	269
WWW. Tiow	U	U	270	02	100	207
	/linor1		/lajor1		Major2	
Conflicting Flow All	862	359	0	0	376	0
Stage 1	359	-	-	-	-	-
Stage 2	503	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.11	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3		_	2.209	_
Pot Cap-1 Maneuver	328	690	_	_	1188	_
Stage 1	711	-	_	_	-	_
Stage 2	612	_	_	_	_	_
Platoon blocked, %	UIZ					
Mov Cap-1 Maneuver	275	658	-	-	1188	-
	275	000	-	-	1100	-
Mov Cap-2 Maneuver		-	-	-	-	-
Stage 1	678	-	-	-	-	-
Stage 2	537	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	0		0		2.4	
HCM LOS	A		J			
	,,					
Minor Lane/Major Mvm	t	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	-	1188	-
HCM Lane V/C Ratio		-	-	-	0.091	-
HCM Control Delay (s)			-	0	8.3	0
HCM Lane LOS		-	-	Α	Α	Α
HCM 95th %tile Q(veh)		_	_	_	0.3	-

Part 3: Future Intersection Levels of Service

8: Route 138 & South Parking Lot Entrance

	•	•	†	<i>></i>	\	↓
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations			ĵ»			ર્ન
Traffic Volume (vph)	0	0	1490	1	1	1070
Future Volume (vph)	0	0	1490	1	1	1070
Satd. Flow (prot)	0	0	1749	0	0	1733
Flt Permitted						
Satd. Flow (perm)	0	0	1749	0	0	1733
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	2%	2%	5%	0%	0%	6%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	1569	0	0	1127
Sign Control	Stop		Free			Free
Intersection Summary						

Control Type: Unsignalized Intersection Capacity Utilization 81.8% Analysis Period (min) 15

ICU Level of Service D

9: Route 138 & South Parking Lot Exit

	•	•	†	~	>	↓
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	M.		†			†
Traffic Volume (vph)	0	1	1490	0	0	1080
Future Volume (vph)	0	1	1490	0	0	1080
Satd. Flow (prot)	1589	0	1749	0	0	1733
Flt Permitted						
Satd. Flow (perm)	1589	0	1749	0	0	1733
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	0%	0%	5%	2%	2%	6%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	1	0	1568	0	0	1137
Sign Control	Stop		Free			Free
Intersection Summary						

Control Type: Unsignalized Intersection Capacity Utilization 88.4% Analysis Period (min) 15

ICU Level of Service E

Intersection						
Int Delay, s/veh	0					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	, A					
Traffic Vol, veh/h	0	1	1490	0	0	1080
Future Vol, veh/h	0	1	1490	0	0	1080
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	_	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	0	5	2	2	6
Mymt Flow	0	1	1568	0	0	1137
IVIVIII I IOVV	U	•	1300	U	U	1107
	Minor1		/lajor1	Λ	/lajor2	
Conflicting Flow All	2705	1568	0	-	-	-
Stage 1	1568	-	-	-	-	-
Stage 2	1137	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	-	-
Pot Cap-1 Maneuver	24	138	-	0	0	_
Stage 1	191	-	_	0	0	_
Stage 2	309	_	_	0	0	_
Platoon blocked, %	007		_	U	U	_
Mov Cap-1 Maneuver	24	138			_	
Mov Cap-1 Maneuver	24	130	_	_	-	_
	191	-	-	-	-	-
Stage 1		-	-	-	-	-
Stage 2	309	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	31.3		0		0	
HCM LOS	D					
Minor Lane/Major Mvm	nt	NBTV		SBT		
Capacity (veh/h)		-		-		
HCM Lane V/C Ratio		-	800.0	-		
HCM Control Delay (s)		-	31.3	-		
HCM Lane LOS		-	D	-		
HCM 95th %tile Q(veh)	-	0	-		

10: Route 138 & North Parking Lot Entrance

	•	•	†	<i>></i>	>	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations			ĵ∍			ર્ન
Traffic Volume (vph)	0	0	1490	7	6	1090
Future Volume (vph)	0	0	1490	7	6	1090
Satd. Flow (prot)	0	0	1746	0	0	1733
Flt Permitted						
Satd. Flow (perm)	0	0	1746	0	0	1733
Peak Hour Factor	0.92	0.92	0.96	0.58	0.75	0.94
Heavy Vehicles (%)	2%	2%	5%	14%	0%	6%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	1564	0	0	1168
Sign Control	Stop		Free			Free
Intersection Summary						

Control Type: Unsignalized Intersection Capacity Utilization 82.2% Analysis Period (min) 15

ICU Level of Service E

	۶	→	•	•	←	4	4	†	<i>></i>	/	+	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			ર્ન			1}•	
Traffic Volume (vph)	7	0	7	0	0	10	1	1480	0	0	1090	7
Future Volume (vph)	7	0	7	0	0	10	1	1480	0	0	1090	7
Satd. Flow (prot)	0	1671	0	0	1444	0	0	1766	0	0	1732	0
Flt Permitted		0.976										
Satd. Flow (perm)	0	1671	0	0	1444	0	0	1766	0	0	1732	0
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	0%	2%	0%	2%	2%	10%	0%	4%	0%	2%	6%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	14	0	0	11	0	0	1559	0	0	1154	0
Sign Control		Stop			Stop			Free			Free	

Intersection Summary

Control Type: Unsignalized

Intersection Capacity Utilization 92.5%

ICU Level of Service F

Analysis Period (min) 15

Interception												
Intersection	1.6											
Int Delay, s/veh	1.0											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			Þ	
Traffic Vol, veh/h	7	0	7	0	0	10	1	1480	0	0	1090	7
Future Vol, veh/h	7	0	7	0	0	10	1	1480	0	0	1090	7
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	0	2	0	2	2	10	0	4	0	2	6	0
Mvmt Flow	7	0	7	0	0	11	1	1558	0	0	1147	7
Major/Minor	Minor2			Minor1			Major1		Λ	/lajor2		
Conflicting Flow All	2716	2711	1151	2715	2715	1558	1155	0		-	_	0
Stage 1	1151	1151	-	1560	1560	-	-	-	_			-
Stage 2	1565	1560	_	1155	1155	_	_	_	_	_	_	_
Critical Hdwy	7.1	6.52	6.2	7.12	6.52	6.3	4.1	_	_	_	_	_
Critical Hdwy Stg 1	6.1	5.52	- 0.2	6.12	5.52	-	-	_	_	_	_	_
Critical Hdwy Stg 2	6.1	5.52	_	6.12	5.52	-	_		_	_	_	_
Follow-up Hdwy	3.5	4.018	3.3	3.518	4.018	3.39	2.2	_	_	_	_	_
Pot Cap-1 Maneuver	14	21	243	14	21	133	612	_	0	0	-	_
Stage 1	243	272	2 10	141	173	00		_	0	0	_	_
Stage 2	141	173	-	240	271	-	-	-	0	0	-	-
Platoon blocked, %		.,,		_ 10	-/ '			_			_	_
Mov Cap-1 Maneuver	13	21	243	13	21	133	612	-	-	-	-	-
Mov Cap-2 Maneuver	13	21	2 10	13	21	00		_		_	_	_
Stage 1	240	272	-	139	171	-	-	-	-	-	-	-
Stage 2	128	171	_	233	271	-	_	_		_	_	_
Clago L	.20	.,,		200								
Approach	ED			MD			ND			CD		
Approach	272.0			WB			NB			SB		
HCM Control Delay, s				34.4			0			0		
HCM LOS	F			D								
Minor Lane/Major Mvm	nt	NBL	NBT	EBLn1V	WBLn1	SBT	SBR					
Capacity (veh/h)		612	-	25	133	-	-					
HCM Lane V/C Ratio		0.002	-	0.589		-	-					
HCM Control Delay (s)		10.9		272.8	34.4	-	-					
HCM Lane LOS		В	Ā	F	D	_	_					
HCM 95th %tile Q(veh)	0	-	1.8	0.3	-	-					

12: Route 138 & Summit Rd.

	€	•	†	~	-	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	M.		†			ર્ન
Traffic Volume (vph)	0	1	970	550	7	1090
Future Volume (vph)	0	1	970	550	7	1090
Satd. Flow (prot)	1558	0	1672	0	0	1717
Flt Permitted						
Satd. Flow (perm)	1558	0	1672	0	0	1717
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	0%	2%	7%	0%	2%	7%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	1	0	1600	0	0	1154
Sign Control	Stop		Free			Free
Intersection Cummery						

Intersection Summary

Control Type: Unsignalized

Intersection Capacity Utilization 94.6%

ICU Level of Service F

Analysis Period (min) 15

13: Route 138 & Thacher Montessori School

	۶	\rightarrow	4	†	ļ	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	**			4	f)	
Traffic Volume (vph)	0	9	3	960	1080	45
Future Volume (vph)	0	9	3	960	1080	45
Satd. Flow (prot)	1589	0	0	1733	1728	0
Flt Permitted						
Satd. Flow (perm)	1589	0	0	1733	1728	0
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	2%	0%	0%	6%	6%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	10	0	0	1024	1197	0
Sign Control	Stop			Free	Free	
Intersection Summary						

Control Type: Unsignalized Intersection Capacity Utilization 69.6% Analysis Period (min) 15

ICU Level of Service C

Intersection								
Int Delay, s/veh	0.2							
	EBL	EBR	NBL	NBT	SBT	CDD		
Movement Lanc Configurations	FBL	ERK	INRL			SBR		
Lane Configurations Traffic Vol, veh/h	T	9	3	र्स 960	1 080	45		
Future Vol, veh/h	0	9	3	960	1080	45		
Conflicting Peds, #/hr		0	0	0	0	0		
Sign Control	Stop	Stop	Free	Free	Free	Free		
RT Channelized	310p -	None	-	None	-	None		
Storage Length	0	-	_	-	_	TVOIC		
Veh in Median Storag		_	_	0	0	_		
Grade, %	0	_	_	0	0	_		
Peak Hour Factor	94	94	94	94	94	94		
Heavy Vehicles, %	2	0	0	6	6	0		
Mvmt Flow	0	10	3	1021	1149	48		
WWW.CT IOW		10	Ū	1021	1117	10		
		_		_				
Major/Minor	Minor2		/lajor1		Major2			
Conflicting Flow All	2201	1173	1197	0	-	0		
Stage 1	1173	-	-	-	-	-		
Stage 2	1028	-	-	-	-	-		
Critical Hdwy	6.42	6.2	4.1	-	-	-		
Critical Hdwy Stg 1	5.42	-	-	-	-	-		
Critical Hdwy Stg 2	5.42	-	-	-	-	-		
Follow-up Hdwy	3.518	3.3	2.2	-	-	-		
Pot Cap-1 Maneuver	49	*109	*327	-	-	-		
Stage 1	294	-	-	-	-	-		
Stage 2	345	-	-	-	-	-		
Platoon blocked, %		1	1	-	-	-		
Mov Cap-1 Maneuver		*109	*327	-	-	-		
Mov Cap-2 Maneuver		-	-	-	-	-		
Stage 1	294	-	-	-	-	-		
Stage 2	338	-	-	-	-	-		
Approach	EB		NB		SB			
HCM Control Delay, s			0.1		0			
HCM LOS	E		0.1		U			
110111 200	_							
				·	6==	65-		
Minor Lane/Major Mvi	mt	NBL	NBT	EBLn1	SBT	SBR		
Capacity (veh/h)		* 327	-	107	-	-		
HCM Lane V/C Ratio		0.01		0.088	-	-		
HCM Control Delay (s	5)	16.1	0	41.2	-	-		
HCM Lane LOS		С	Α	Е	-	-		
HCM 95th %tile Q(vel	n)	0	-	0.3	-	-		
Notes								
~: Volume exceeds ca	anacity	\$· Do	lav evo	eeds 30	00s	+. Com	outation Not Defined	*· ΔII
Volume exceeds Ca	apacity	φ. De	iay ext	ceus si	003	T. CUIII	Julation Not Delineu	*: All ma

	۶	→	•	•	+	•	•	†	<i>></i>	/	+	✓
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		ሻ	ĥ			†			ĵ.	
Traffic Volume (vph)	6	0	310	160	110	15	0	970	0	0	660	7
Future Volume (vph)	6	0	310	160	110	15	0	970	0	0	660	7
Satd. Flow (prot)	0	1545	0	1728	1760	0	0	1733	0	0	1683	0
Flt Permitted		0.995		0.377								
Satd. Flow (perm) Satd. Flow (RTOR)	0	1539	0	686	1760	0	0	1733	0	0	1683	0
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	0%	2%	3%	1%	1%	13%	0%	6%	2%	2%	9%	14%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	329	0	167	131	0	0	1010	0	0	695	0
Turn Type	Perm	NA		Perm	NA			NA			NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8								
Detector Phase	4	4		8	8			2			6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		10.0	10.0			10.0			10.0	
Minimum Split (s)	23.0	23.0		24.0	24.0			45.0			45.0	
Total Split (s)	30.0	30.0		30.0	30.0			50.0			50.0	
Total Split (%)	37.5%	37.5%		37.5%	37.5%			62.5%			62.5%	
Yellow Time (s)	4.0	4.0		5.0	5.0			4.0			4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0			1.0			1.0	
Lost Time Adjust (s)		0.0		0.0	0.0			0.0			0.0	
Total Lost Time (s)		5.0		6.0	6.0			5.0			5.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None			None			None	
Act Effct Green (s)		21.3		20.3	20.3			45.2			45.2	
Actuated g/C Ratio		0.28		0.27	0.27			0.59			0.59	
v/c Ratio		0.77		0.92	0.28			0.99			0.70	
Control Delay		38.2		79.1	23.5			44.4			16.8	
Queue Delay		0.0		0.0	0.0			0.0			0.0	
Total Delay		38.2		79.1	23.5			44.4			16.8	
LOS		D		Е	С			D			В	
Approach Delay		38.2			54.6			44.4			16.8	
Approach LOS		D			D			D			В	
Queue Length 50th (ft)		142		77	49			~552			239	
Queue Length 95th (ft)		235		#186	92			#779			378	
Internal Link Dist (ft)		906			198			187			2213	
Turn Bay Length (ft)		E0.4		645	FF.4			4000			001	
Base Capacity (vph)		504		215	554			1023			994	
Starvation Cap Reductn		0		0	0			0			0	
Spillback Cap Reductn		0		0	0			0			0	
Storage Cap Reductn		0		0	0			0			0	
Reduced v/c Ratio		0.65		0.78	0.24			0.99			0.70	
Intersection Summary												

Intersection Summary

Cycle Length: 80

Actuated Cycle Length: 76.5

14: Route 138 & Brush Hill Rd.

Natural Cycle: 90
Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.99
Intersection Signal Delay: 36.6 Intersection LOS: D
Intersection Capacity Utilization 92.8% ICU Level of Service F

Analysis Period (min) 15

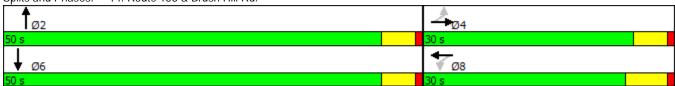
Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 14: Route 138 & Brush Hill Rd.



	•	→	•	•	←	•	•	†	~	/	+	
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		ሻ	ĵ₃			↑			₽	
Traffic Volume (veh/h)	6	0	310	160	110	15	0	970	0	0	660	7
Future Volume (veh/h)	6	0	310	160	110	15	0	970	0	0	660	7
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	10	0	5	10	0	0	20	0	0	15	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1900	1846	1900	1881	1854	1900	0	1792	0	0	1742	1900
Adj Flow Rate, veh/h	6	0	323	167	115	16	0	1010	0	0	688	7
Adj No. of Lanes	0	1	0	1	1	0	0	1	0	0	1	0
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	2	2	2	1	1	1	0	6	0	0	9	9
Cap, veh/h	46	6	380	177	525	59	0	996	0	0	971	9
Arrive On Green	0.29	0.00	0.29	0.29	0.29	0.29	0.00	0.57	0.00	0.00	0.57	0.57
Sat Flow, veh/h	9	20	1541	1063	1593	222	0	1792	0	0	1722	18
Grp Volume(v), veh/h	329	0	0	167	0	131	0	1010	0	0	0	695
Grp Sat Flow(s),veh/h/ln	1570	0	0	1063	0	1815	0	1792	0	0	0	1739
Q Serve(g_s), s	0.0	0.0	0.0	5.4	0.0	4.4	0.0	43.7	0.0	0.0	0.0	22.6
Cycle Q Clear(g_c), s	14.8	0.0	0.0	20.2	0.0	4.4	0.0	43.7	0.0	0.0	0.0	22.6
Prop In Lane	0.02		0.98	1.00		0.12	0.00		0.00	0.00		0.01
Lane Grp Cap(c), veh/h	422	0	0	177	0	531	0	996	0	0	0	987
V/C Ratio(X)	0.78	0.00	0.00	0.94	0.00	0.25	0.00	1.01	0.00	0.00	0.00	0.70
Avail Cap(c_a), veh/h	544	0	0	303	0	552	0	1022	0	0	0	992
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00	0.00	1.00
Uniform Delay (d), s/veh	24.9	0.0	0.0	34.3	0.0	22.0	0.0	18.0	0.0	0.0	0.0	13.4
Incr Delay (d2), s/veh	5.4	0.0	0.0	25.0	0.0	0.2	0.0	31.8	0.0	0.0	0.0	2.3
Initial Q Delay(d3),s/veh	18.3	0.0	0.0	75.9	0.0	3.4	0.0	68.4	0.0	0.0	0.0	5.6
%ile BackOfQ(50%),veh/ln	9.2	0.0	0.0	8.9	0.0	3.7	0.0	50.7	0.0	0.0	0.0	15.1
LnGrp Delay(d),s/veh	48.6	0.0	0.0	135.2	0.0	25.7	0.0	118.3	0.0	0.0	0.0	21.3
LnGrp LOS	D			F		С		F				С
Approach Vol, veh/h		329			298			1010			695	
Approach Delay, s/veh		48.6			87.0			118.3			21.3	
Approach LOS		D			F			F			С	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		50.0		28.9		50.0		28.9				
Change Period (Y+Rc), s		5.0		* 6		5.0		6.0				
Max Green Setting (Gmax), s		45.0		* 25		45.0		24.0				
Max Q Clear Time (g_c+I1), s		45.7		16.8		24.6		22.2				
Green Ext Time (p_c), s		0.0		2.4		13.4		0.7				
Intersection Summary												
HCM 2010 Ctrl Delay			75.6									
HCM 2010 LOS			E									
Notes												

	٠	\rightarrow	4	†	ļ	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations			ሻ	↑		7
Traffic Volume (vph)	0	0	160	390	0	130
Future Volume (vph)	0	0	160	390	0	130
Satd. Flow (prot)	0	0	1711	1801	0	1573
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1711	1801	0	1573
Peak Hour Factor	0.92	0.92	0.84	0.86	0.92	0.85
Heavy Vehicles (%)	2%	2%	2%	2%	2%	1%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	190	453	0	153
Sign Control	Free			Free	Free	
Intersection Summary						

Control Type: Unsignalized Intersection Capacity Utilization 23.9% Analysis Period (min) 15

ICU Level of Service A

16: Route 138 & Neponset Valley Pkwy.

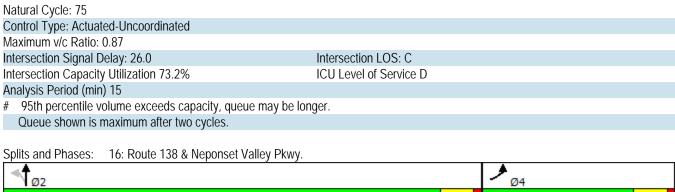
	•	\searrow	1	†	↓	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥		*	†	1 >	
Traffic Volume (vph)	4	170	440	550	490	3
Future Volume (vph)	4	170	440	550	490	3
Satd. Flow (prot)	1348	0	1662	1717	1745	0
Flt Permitted	0.999		0.178			
Satd. Flow (perm)	1348	0	311	1717	1745	0
Satd. Flow (RTOR)	1010	Ū	0		.,	
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98
Heavy Vehicles (%)	25%	18%	5%	7%	5%	33%
Shared Lane Traffic (%)	2070	.0,0	0,0	,,,	0,0	00.0
Lane Group Flow (vph)	177	0	449	561	503	0
Turn Type	Prot		pm+pt	NA	NA	
Protected Phases	4		5	2	6	
Permitted Phases	7		2		U	
Detector Phase	4		5	2	6	
Switch Phase	4		J		U	
Minimum Initial (s)	5.0		5.0	5.0	5.0	
Minimum Split (s)	23.0		9.5	20.0	23.0	
Total Split (s)	23.0		24.0	57.0	33.0	
Total Split (%)	28.8%		30.0%	71.3%	41.3%	
Yellow Time (s)	4.0		3.5	4.0	41.370	
All-Red Time (s)	1.0		1.0	1.0	1.0	
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	
Total Lost Time (s)	5.0		4.5	5.0	5.0	
	3.0		Lead	5.0		
Lead/Lag			Yes		Lag Yes	
Lead-Lag Optimize? Recall Mode	None		None	None	None	
	13.8		47.6	None	23.7	
Actuated a/C Patio				47.1		
Actuated g/C Ratio	0.19		0.67	0.66	0.33	
v/c Ratio	0.68		0.80	0.49	0.87	
Control Delay	41.7		26.0	8.3	40.1	
Queue Delay	0.0		0.0	0.0	0.0	
Total Delay	41.7		26.0	8.3	40.1	
LOS	D		С	A	D	
Approach Delay	41.7			16.2	40.1	
Approach LOS	D			В	D	
Queue Length 50th (ft)	79		123	111	212	
Queue Length 95th (ft)	145		#299	199	#390	
Internal Link Dist (ft)	1200			2213	1154	
Turn Bay Length (ft)			200			
Base Capacity (vph)	350		588	1288	704	
Starvation Cap Reductn	0		0	0	0	
Spillback Cap Reductn	0		0	0	0	
Storage Cap Reductn	0		0	0	0	
Reduced v/c Ratio	0.51		0.76	0.44	0.71	
Intersection Summary						

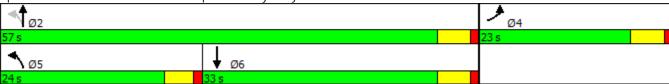
Intersection Summary

Cycle Length: 80

Actuated Cycle Length: 71.2

16: Route 138 & Neponset Valley Pkwy.





	•	•	•	†	 	4
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W		ች	†	1>	
Traffic Volume (veh/h)	4	170	440	550	490	3
Future Volume (veh/h)	4	170	440	550	490	3
Number	7	14	5	2	6	16
Initial Q (Qb), veh	5	0	5	10	10	0
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00			1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1608	1900	1810	1776	1807	1900
Adj Flow Rate, veh/h	4	173	449	561	500	3
Adj No. of Lanes	0	0	1	1	1	0
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	0.70	0.70	5	7	5	5
Cap, veh/h	12	208	579	1172	682	3
Arrive On Green	0.16	0.16	0.19	0.66	0.38	0.38
	31			1776		0.38
Sat Flow, veh/h		1333	1723		1794	
Grp Volume(v), veh/h	178	0	449	561	0	503
Grp Sat Flow(s), veh/h/ln	1371	0	1723	1776	0	1805
Q Serve(g_s), s	6.8	0.0	7.4	8.6	0.0	13.0
Cycle Q Clear(g_c), s	6.8	0.0	7.4	8.6	0.0	13.0
Prop In Lane	0.02	0.97	1.00			0.01
Lane Grp Cap(c), veh/h	222	0	579	1172	0	685
V/C Ratio(X)	0.80	0.00	0.77	0.48	0.00	0.73
Avail Cap(c_a), veh/h	455	0	875	1700	0	931
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	23.5	0.0	11.9	5.1	0.0	16.6
Incr Delay (d2), s/veh	6.7	0.0	2.5	0.3	0.0	2.0
Initial Q Delay(d3),s/veh	0.0	0.0	2.4	1.0	0.0	5.8
%ile BackOfQ(50%),veh/ln	3.3	0.0	5.8	5.7	0.0	9.7
LnGrp Delay(d),s/veh	30.2	0.0	16.8	6.4	0.0	24.4
LnGrp LOS	C	3.0	В	A	3.0	C
Approach Vol, veh/h	178			1010	503	
Approach Delay, s/veh	30.2			11.0	24.4	
11						
Approach LOS	С			В	С	
Timer	1	2	3	4	5	6
Assigned Phs		2		4	5	6
Phs Duration (G+Y+Rc), s		40.7		13.6	15.0	25.7
Change Period (Y+Rc), s		5.0		5.0	4.5	5.0
Max Green Setting (Gmax), s		52.0		18.0	19.5	28.0
Max Q Clear Time (q_c+l1), s		10.6		8.8	9.4	15.0
Green Ext Time (p_c), s		8.9		0.4	1.1	5.8
•		0.7		0.7	1.1	5.0
Intersection Summary						
HCM 2010 Ctrl Delay			17.0			
HCM 2010 LOS			В			
Notes						

17: Route 138 & Milton St./Dollar Ln.

	۶	→	•	•	←	•	•	†	<i>></i>	/	↓	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		7	f)			4	
Traffic Volume (vph)	30	90	15	30	180	35	9	540	5	1	460	35
Future Volume (vph)	30	90	15	30	180	35	9	540	5	1	460	35
Satd. Flow (prot)	0	1731	0	0	1758	0	1745	1716	0	0	1719	0
Flt Permitted		0.906			0.948		0.480				0.999	
Satd. Flow (perm)	0	1586	0	0	1676	0	882	1716	0	0	1717	0
Satd. Flow (RTOR)												
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	3%	4%	0%	0%	2%	3%	0%	7%	0%	0%	6%	3%
Shared Lane Traffic (%)	_		_	_		_	_		_	_		
Lane Group Flow (vph)	0	139	0	0	253	0	9	562	0	0	511	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4		0	8		0	2		,	6	
Permitted Phases	4			8	0		2	0		6	,	
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase	7.0	7.0		7.0	7.0		20.0	20.0		20.0	20.0	
Minimum Initial (s)	7.0	7.0		7.0	7.0		30.0	30.0		30.0	30.0	
Minimum Split (s)	23.0	23.0		23.0	23.0		37.0	37.0		37.0	37.0	
Total Split (s)	23.0	23.0		23.0	23.0		37.0	37.0		37.0	37.0	
Total Split (%)	38.3%	38.3%		38.3%	38.3%		61.7%	61.7%		61.7%	61.7%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)		0.0 5.0			0.0 5.0		0.0 7.0	0.0 7.0			0.0 7.0	
Total Lost Time (s) Lead/Lag		5.0			5.0		7.0	7.0			7.0	
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		None	None	
Act Effct Green (s)	None	14.3		None	14.7		30.1	30.1		None	30.1	
Actuated g/C Ratio		0.25			0.26		0.53	0.53			0.53	
v/c Ratio		0.25			0.58		0.02	0.62			0.56	
Control Delay		19.5			24.2		7.7	13.9			12.7	
Queue Delay		0.0			0.0		0.0	0.0			0.0	
Total Delay		19.5			24.2		7.7	13.9			12.7	
LOS		В			C		Α.,	В			В	
Approach Delay		19.5			24.2		, ,	13.8			12.7	
Approach LOS		В			C			В			В	
Queue Length 50th (ft)		38			75		1	124			108	
Queue Length 95th (ft)		78			136		7	237			207	
Internal Link Dist (ft)		1017			791			1154			1181	
Turn Bay Length (ft)							200					
Base Capacity (vph)		503			532		466	908			908	
Starvation Cap Reductn		0			0		0	0			0	
Spillback Cap Reductn		0			0		0	0			0	
Storage Cap Reductn		0			0		0	0			0	
Reduced v/c Ratio		0.28			0.48		0.02	0.62			0.56	
Intersection Summary												

Intersection Summary

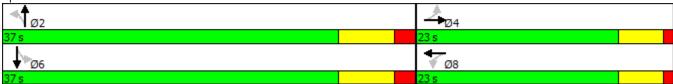
Cycle Length: 60

Actuated Cycle Length: 56.8

17: Route 138 & Milton St./Dollar Ln.

Natural Cycle: 60
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.62
Intersection Signal Delay: 15.7
Intersection Capacity Utilization 54.1%
Analysis Period (min) 15

Splits and Phases: 17: Route 138 & Milton St./Dollar Ln.



	۶	→	•	•	←	•	1	†	<i>></i>	/	+	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		7	4î			4	
Traffic Volume (veh/h)	30	90	15	30	180	35	9	540	5	1	460	35
Future Volume (veh/h)	30	90	15	30	180	35	9	540	5	1	460	35
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	10	0	0	15	0	0	15	0	0	10	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1900	1838	1900	1900	1865	1900	1900	1777	1900	1900	1796	1900
Adj Flow Rate, veh/h	31	93	15	31	186	36	9	557	5	1	474	36
Adj No. of Lanes	0	1	0	0	1	0	1	1	0	0	1	0
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	4	4	4	2	2	2	0	7	7	6	6	6
Cap, veh/h	123	314	34	96	327	47	561	976	8	67	906	63
Arrive On Green	0.21	0.21	0.21	0.21	0.21	0.21	0.56	0.56	0.56	0.56	0.56	0.56
Sat Flow, veh/h	219	1320	186	124	1389	251	904	1758	16	0	1648	125
Grp Volume(v), veh/h	139	0	0	253	0	0	9	0	562	511	0	0
Grp Sat Flow(s), veh/h/ln	1725	0	0	1764	0	0	904	0	1774	1774	0	0
Q Serve(g_s), s	0.0	0.0	0.0	2.3	0.0	0.0	0.0	0.0	10.8	0.0	0.0	0.0
Cycle Q Clear(g_c), s	3.5	0.0	0.0	6.9	0.0	0.0	0.3	0.0	10.8	9.4	0.0	0.0
Prop In Lane	0.22		0.11	0.12		0.14	1.00		0.01	0.00		0.07
Lane Grp Cap(c), veh/h	460	0	0	476	0	0	561	0	979	1039	0	0
V/C Ratio(X)	0.30	0.00	0.00	0.53	0.00	0.00	0.02	0.00	0.57	0.49	0.00	0.00
Avail Cap(c_a), veh/h	649	0	0	667	0	0	614	0	1000	1067	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00	1.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	18.3	0.0	0.0	19.7	0.0	0.0	5.5	0.0	8.8	8.0	0.0	0.0
Incr Delay (d2), s/veh	8.0	0.0	0.0	2.0	0.0	0.0	0.0	0.0	8.0	0.4	0.0	0.0
Initial Q Delay(d3),s/veh	4.9	0.0	0.0	15.3	0.0	0.0	0.0	0.0	4.0	1.3	0.0	0.0
%ile BackOfQ(50%),veh/ln	3.2	0.0	0.0	6.8	0.0	0.0	0.1	0.0	8.2	6.0	0.0	0.0
LnGrp Delay(d),s/veh	23.9	0.0	0.0	37.0	0.0	0.0	5.5	0.0	13.6	9.7	0.0	0.0
LnGrp LOS	С			D			Α		В	Α		
Approach Vol, veh/h		139			253			571			511	
Approach Delay, s/veh		23.9			37.0			13.4			9.7	
Approach LOS		С			D			В			А	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		37.0		16.2		37.0		16.2				
Change Period (Y+Rc), s		7.0		5.0		7.0		5.0				
Max Green Setting (Gmax), s		30.0		18.0		30.0		18.0				
Max Q Clear Time (g_c+I1), s		12.8		5.5		11.4		8.9				
Green Ext Time (p_c), s		6.9		3.2		7.1		2.6				
Intersection Summary												
HCM 2010 Ctrl Delay			17.2									
HCM 2010 LOS			В									

18: Route 138 & Blue Jay Way (Curry College)

	•	•	•	†	ļ	1
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			4	ĥ	
Traffic Volume (vph)	5	15	130	480	480	60
Future Volume (vph)	5	15	130	480	480	60
Satd. Flow (prot)	1596	0	0	1705	1717	0
Flt Permitted	0.979			0.989		
Satd. Flow (perm)	1596	0	0	1705	1717	0
Confl. Peds. (#/hr)	1		16			16
Peak Hour Factor	0.42	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	7%	1%	8%	6%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	28	0	0	663	587	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utiliza				IC	U Level c	of Service I
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	2.4					
	EBL	EBR	NBL	NBT	SBT	SBR
Movement Lang Configurations	EBL	EDK	INDL			SDK
Lane Configurations		15	130	4 480	190	60
Traffic Vol, veh/h Future Vol, veh/h	5	15		480	480	
Conflicting Peds, #/hr	5	15	130 16	480	480 0	60 16
Sign Control RT Channelized	Stop	Stop None	Free -	Free None	Free -	Free None
Storage Length	0	None -	-	none -	-	None
Veh in Median Storag		-		0	0	-
Grade, %	je, # 0 0	-	-	0	0	
Peak Hour Factor	42	92	92	92	92	92
Heavy Vehicles, %	0	7	1 1 1 1	8	6	0
Mvmt Flow	12	16	141	522	522	65
Major/Minor	Minor2	ľ	Major1	N	/lajor2	
Conflicting Flow All	1375	570	603	0	-	0
Stage 1	570	-	-	-	-	-
Stage 2	805	-	-	-	-	-
Critical Hdwy	6.4	6.27	4.11	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.363	2.209	-	-	-
Pot Cap-1 Maneuver	162	*321	*488	-	-	-
Stage 1	570	-	-	-	-	-
Stage 2	443	-	-	-	-	-
Platoon blocked, %		1	1	-	-	-
Mov Cap-1 Maneuve	r 93	*316	*488	-	-	_
Mov Cap-2 Maneuver		-	-	-	_	_
Stage 1	562	-	_	-	-	_
Stage 2	259	_	_	_	_	_
Olugo 2	207					
Approach	EB		NB		SB	
HCM Control Delay, s			3.3		0	
HCM LOS	D					
Minor Lane/Major Mv	mt	NBL	NBT F	EBLn1	SBT	SBR
Capacity (veh/h)		* 488	-	157	-	-
HCM Lane V/C Ratio		0.29	-	0.18	_	-
HCM Control Delay (15.4	0	32.9	-	-
HCM Lane LOS		C	A	J2.7	_	_
HCM 95th %tile Q(ve	h)	1.2	-	0.6	_	
	,	1.2		0.0		
Notes						
~: Volume exceeds c	apacity	\$: De	elay exc	eeds 30	00s	+: Com

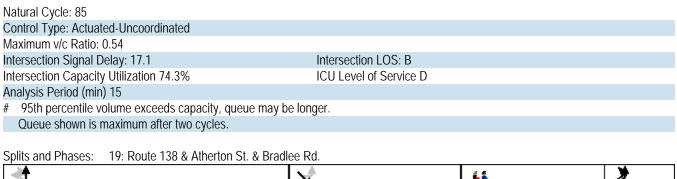
19: Route 138 & Atherton St. & Bradlee Rd.

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Lane Group	EBL	EBR	EBR2	NBL2	NBL	NBT	NBR	SBL	SBT	SBR	SBR2	SEL
Lane Configurations	M					4			4			
Traffic Volume (vph)	2	2	3	2	25	450	9	2	460	1	1	5
Future Volume (vph)	2	2	3	2	25	450	9	2	460	1	1	5
Satd. Flow (prot)	1432	0	0	0	0	1689	0	0	1732	0	0	0
Flt Permitted	0.986					0.959			0.998			
Satd. Flow (perm)	1432	0	0	0	0	1625	0	0	1728	0	0	0
Satd. Flow (RTOR)												_
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	50%	0%	0%	50%	7%	8%	11%	0%	6%	0%	0%	0%
Shared Lane Traffic (%)	00.0	0,0	0.0	00,0	,,,	0,0		0.0	0.0	0.0	0.0	0,0
Lane Group Flow (vph)	7	0	0	0	0	506	0	0	483	0	0	0
Turn Type	Prot			Perm	Perm	NA		Perm	NA			Perm
Protected Phases	10			1 01111	1 01111	2		1 CIIII	6			1 01111
Permitted Phases	10			2	2			6	J			4
Detector Phase	10			2	2	2		6	6			4
Switch Phase	10			2		2		U	U			7
Minimum Initial (s)	4.0			22.0	22.0	22.0		22.0	22.0			10.0
Minimum Split (s)	9.0			28.0	28.0	28.0		28.0	28.0			24.0
Total Split (s)	10.0			40.0	40.0	40.0		40.0	40.0			24.0
Total Split (%)	10.6%			42.6%	42.6%	42.6%		42.6%	42.6%			25.5%
Yellow Time (s)	4.0			5.0	5.0	5.0		5.0	5.0			4.0
All-Red Time (s)	1.0			1.0	1.0	1.0		1.0	1.0			1.0
Lost Time Adjust (s)	0.0			1.0	1.0	0.0		1.0	0.0			1.0
Total Lost Time (s)	5.0					6.0			6.0			
Lead/Lag	Lag					0.0			0.0			
Lead-Lag Optimize?	Lay											
Recall Mode	None			None	None	None		None	None			None
Act Effct Green (s)	5.8			None	None	29.6		None	29.6			None
Actuated g/C Ratio	0.11					0.58			0.58			
v/c Ratio	0.11					0.56			0.36			
	31.4					16.5			15.0			
Control Delay	0.0					0.0						
Queue Delay	31.4					16.5			0.0 15.0			
Total Delay LOS	31.4 C											
						B			B			
Approach LOS	31.4					16.5			15.0			
Approach LOS	С					В			В			
Queue Length 50th (ft)	2					76			70			
Queue Length 95th (ft)	18					#456			#382			
Internal Link Dist (ft)	1228					1586			2667			
Turn Bay Length (ft)	1/0					1100			10/0			
Base Capacity (vph)	163					1193			1269			
Starvation Cap Reductn	0					0			0			
Spillback Cap Reductn	0					0			0			
Storage Cap Reductn	0					0			0			
Reduced v/c Ratio	0.04					0.42			0.38			
Intersection Summary												
Cycle Length: 94												
Actuated Cycle Length: 50.	9											

19: Route 138 & Atherton St. & Bradlee Rd.

	\mathbf{x}	>	€	*	×	*			
Lane Group	SET	SER	NWL2	NWL	NWT	NWR	Ø9		
Lane Configurations	4				4				
Traffic Volume (vph)	20	30	45	1	50	30			
Future Volume (vph)	20	30	45	1	50	30			
Satd. Flow (prot)	1639	0	0	0	1734	0			
Flt Permitted	0.966	· ·		J	0.855	J			
Satd. Flow (perm)	1589	0	0	0	1509	0			
Satd. Flow (RTOR)	1007	U	U	U	1007	U			
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96			
Heavy Vehicles (%)	5%	3%	0.70	0.70	0%	3%			
Shared Lane Traffic (%)	J 70	370	070	0 70	070	370			
Lane Group Flow (vph)	57	0	0	0	131	0			
	NA	U			NA	U			
Turn Type			Perm	Perm			0		
Protected Phases	4		0	0	8		9		
Permitted Phases	4		8	8	0				
Detector Phase	4		8	8	8				
Switch Phase			4.0.0	4.5.5	4		4.5		
Minimum Initial (s)	10.0		10.0	10.0	10.0		1.0		
Minimum Split (s)	24.0		24.0	24.0	24.0		20.0		
Total Split (s)	24.0		24.0	24.0	24.0		20.0		
Total Split (%)	25.5%		25.5%	25.5%	25.5%		21%		
Yellow Time (s)	4.0		4.0	4.0	4.0		2.0		
All-Red Time (s)	1.0		1.0	1.0	1.0		0.0		
Lost Time Adjust (s)	0.0				0.0				
Total Lost Time (s)	5.0				5.0				
Lead/Lag							Lead		
Lead-Lag Optimize?									
Recall Mode	None		None	None	None		None		
Act Effct Green (s)	12.8				12.8				
Actuated g/C Ratio	0.25				0.25				
v/c Ratio	0.14				0.34				
Control Delay	22.1				24.5				
Queue Delay	0.0				0.0				
Total Delay	22.1				24.5				
LOS	C				C				
Approach Delay	22.1				24.5				
Approach LOS	C				24.5 C				
Queue Length 50th (ft)	11				27				
Queue Length 95th (ft)	63				126				
Internal Link Dist (ft)	689				1410				
` ,	009				1410				
Turn Bay Length (ft)	400				455				
Base Capacity (vph)	690				655				
Starvation Cap Reductn	0				0				
Spillback Cap Reductn	0				0				
Storage Cap Reductn	0				0				
Reduced v/c Ratio	0.08				0.20				
Intersection Summary									

19: Route 138 & Atherton St. & Bradlee Rd.



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	10	15	3	30	30	15	4	490	10	0	470	7
Future Volume (vph)	10	15	3	30	30	15	4	490	10	0	470	7
Satd. Flow (prot)	0	1778	0	0	1690	0	0	1696	0	0	1715	0
Flt Permitted		0.845			0.855			0.996				
Satd. Flow (perm)	0	1530	0	0	1474	0	0	1690	0	0	1715	0
Satd. Flow (RTOR)												
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	3%	0%	13%	0%	8%	8%	0%	7%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	30	0	0	82	0	0	548	0	0	519	0
Turn Type	Perm	NA		Perm	NA		Perm	NA			NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	7.0	7.0		7.0	7.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	23.0	23.0		23.0	23.0		24.0	24.0		24.0	24.0	
Total Split (s)	24.0	24.0		24.0	24.0		56.0	56.0		56.0	56.0	
Total Split (%)	30.0%	30.0%		30.0%	30.0%		70.0%	70.0%		70.0%	70.0%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.0			5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		None	None	
Act Effct Green (s)		10.5			10.5			24.4			24.4	
Actuated g/C Ratio		0.30			0.30			0.71			0.71	
v/c Ratio		0.06			0.18			0.46			0.43	
Control Delay		14.4			14.9			8.3			7.9	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		14.4			14.9			8.3			7.9	
LOS		В			В			А			А	
Approach Delay		14.4			14.9			8.3			7.9	
Approach LOS		В			В			Α			А	
Queue Length 50th (ft)		5			13			71			65	
Queue Length 95th (ft)		25			52			221			201	
Internal Link Dist (ft)		939			647			711			3441	
Turn Bay Length (ft)												
Base Capacity (vph)		1005			968			1627			1651	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.03			0.08			0.34			0.31	
Intersection Summary												

Cycle Length: 80

Actuated Cycle Length: 34.6

20: Route 138 & Robbins St.

Natural Cycle: 55
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.46
Intersection Signal Delay: 8.8
Intersection Capacity Utilization 44.6%
Analysis Period (min) 15

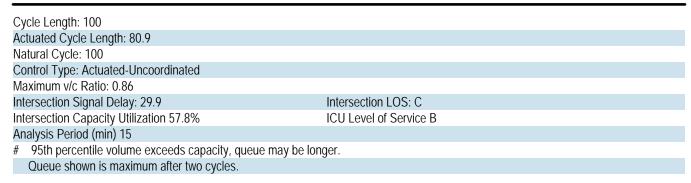
Splits and Phases: 20: Route 138 & Robbins St.



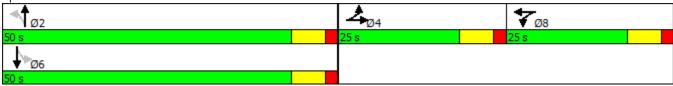
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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		₩.			4			4			4	
Traffic Volume (veh/h)	10	15	3	30	30	15	4	490	10	0	470	7
Future Volume (veh/h)	10	15	3	30	30	15	4	490	10	0	470	7
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	2	0	0	2	0	0	10	0	0	10	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1900	1900	1900	1900	1831	1900	1900	1760	1900	1900	1777	1900
Adj Flow Rate, veh/h	11	16	3	33	33	16	4	533	11	0	511	8
Adj No. of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	0	0	0	0	0	0	8	8	8	7	7	7
Cap, veh/h	198	175	22	216	126	41	104	907	18	0	936	14
Arrive On Green	0.14	0.14	0.14	0.14	0.14	0.14	0.53	0.53	0.53	0.00	0.53	0.53
Sat Flow, veh/h	379	1175	173	489	805	314	3	1713	35	0	1745	27
Grp Volume(v), veh/h	30	0	0	82	0	0	548	0	0	0	0	519
Grp Sat Flow(s), veh/h/ln	1726	0	0	1609	0	0	1751	0	0	0	0	1773
Q Serve(g_s), s	0.0	0.0	0.0	0.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0	6.4
Cycle Q Clear(g_c), s	0.5	0.0	0.0	1.4	0.0	0.0	7.0	0.0	0.0	0.0	0.0	6.4
Prop In Lane	0.37		0.10	0.40		0.20	0.01		0.02	0.00		0.02
Lane Grp Cap(c), veh/h	386	0	0	374	0	0	1025	0	0	0	0	953
V/C Ratio(X)	0.08	0.00	0.00	0.22	0.00	0.00	0.53	0.00	0.00	0.00	0.00	0.54
Avail Cap(c_a), veh/h	1105	0	0	1058	0	0	2749	0	0	0	0	2683
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00	1.00
Uniform Delay (d), s/veh	12.9	0.0	0.0	13.3	0.0	0.0	6.7	0.0	0.0	0.0	0.0	5.8
Incr Delay (d2), s/veh	0.1	0.0	0.0	0.3	0.0	0.0	0.4	0.0	0.0	0.0	0.0	0.5
Initial Q Delay(d3),s/veh	0.2	0.0	0.0	0.3	0.0	0.0	1.5	0.0	0.0	0.0	0.0	1.7
%ile BackOfQ(50%),veh/ln	0.5	0.0	0.0	0.9	0.0	0.0	5.6	0.0	0.0	0.0	0.0	5.0
LnGrp Delay(d),s/veh	13.1	0.0	0.0	13.9	0.0	0.0	8.6	0.0	0.0	0.0	0.0	8.0
LnGrp LOS	В			В			А					Α
Approach Vol, veh/h		30			82			548			519	
Approach Delay, s/veh		13.1			13.9			8.6			8.0	
Approach LOS		В			В			А			А	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs	•	2	<u> </u>	4	<u> </u>	6	,	8				
Phs Duration (G+Y+Rc), s		23.5		9.5		23.5		9.5				
Change Period (Y+Rc), s		6.0		5.0		6.0		5.0				
Max Green Setting (Gmax), s		50.0		19.0		50.0		19.0				
Max Q Clear Time (g_c+l1), s		9.0		2.5		8.4		3.4				
Green Ext Time (p_c), s		8.5		0.5		8.5		0.4				
Intersection Summary												
HCM 2010 Ctrl Delay			8.8									
HCM 2010 Car belay			Α									
HOW ZUTU LUS			Α.									

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	4	15	3	100	30	15	1	530	50	8	410	8
Future Volume (vph)	4	15	3	100	30	15	1	530	50	8	410	8
Satd. Flow (prot)	0	1778	0	0	1735	0	0	1519	0	0	1525	0
Flt Permitted		0.991			0.967						0.985	
Satd. Flow (perm)	0	1777	0	0	1732	0	0	1519	0	0	1504	0
Satd. Flow (RTOR)												
Confl. Peds. (#/hr)	1		1	1		1			2	2		
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82
Heavy Vehicles (%)	0%	0%	0%	1%	0%	0%	0%	8%	0%	13%	8%	0%
Parking (#/hr)							0	0	0	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	27	0	0	177	0	0	708	0	0	520	0
Turn Type	Split	NA		Split	NA		Perm	NA		Perm	NA	
Protected Phases	4	4		8	8			2			6	
Permitted Phases							2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase	•	•					_	_				
Minimum Initial (s)	4.0	4.0		4.0	4.0		6.0	6.0		6.0	6.0	
Minimum Split (s)	25.0	25.0		25.0	25.0		25.0	25.0		25.0	25.0	
Total Split (s)	25.0	25.0		25.0	25.0		50.0	50.0		50.0	50.0	
Total Split (%)	25.0%	25.0%		25.0%	25.0%		50.0%	50.0%		50.0%	50.0%	
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	2.0	0.0		2.0	0.0		2.0	0.0		2.0	0.0	
Total Lost Time (s)		7.0			7.0			7.0			7.0	
Lead/Lag		7.0			7.0			7.0			7.0	
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		None	None	
Act Effct Green (s)	NOTIC	8.4		NOTIC	13.1		NOTIC	44.1		None	44.1	
Actuated g/C Ratio		0.10			0.16			0.55			0.55	
v/c Ratio		0.15			0.63			0.86			0.63	
Control Delay		37.4			44.4			32.4			21.3	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		37.4			44.4			32.4			21.3	
LOS		37.4 D			D			J2.4			C C	
Approach Delay		37.4			44.4			32.4			21.3	
Approach LOS		37.4 D			D			J2.4			C C	
Queue Length 50th (ft)		14			89			327			196	
Queue Length 95th (ft)		35			156			#626			365	
Internal Link Dist (ft)		838			877			3441			407	
Turn Bay Length (ft)		030			0//			3441			407	
		406			396			828			820	
Base Capacity (vph)								828				
Starvation Cap Reductn		0			0						0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn Reduced v/c Ratio		0 0.07			0 45			0			0 62	
		0.07			0.45			0.86			0.63	
Intersection Summary												

21: Route 138 & Cheever St./Blue Hill Terrace St.



Splits and Phases: 21: Route 138 & Cheever St./Blue Hill Terrace St.



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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	4	15	3	100	30	15	1	530	50	8	410	8
Future Volume (veh/h)	4	15	3	100	30	15	1	530	50	8	410	8
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	0	0	0	2	0	0	5	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.99	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.90	1.00	1.00	0.90
Adj Sat Flow, veh/h/ln	1900	1900	1900	1900	1887	1900	1900	1771	1900	1900	1760	1900
Adj Flow Rate, veh/h	5	18	4	122	37	18	1	646	61	10	500	10
Adj No. of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82
Percent Heavy Veh, %	0	0	0	0	0	0	8	8	8	8	8	8
Cap, veh/h	9	32	7	157	77	22	53	759	67	59	803	16
Arrive On Green	0.03	0.03	0.03	0.13	0.13	0.13	0.53	0.53	0.53	0.53	0.53	0.53
Sat Flow, veh/h	339	1222	271	1235	375	182	0	1434	135	9	1528	30
Grp Volume(v), veh/h	27	0	0	177	0	0	708	0	0	520	0	0
Grp Sat Flow(s), veh/h/ln	1832	0	0	1792	0	0	1569	0	0	1567	0	0
Q Serve(g_s), s	1.0	0.0	0.0	6.3	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	1.0	0.0	0.0	6.3	0.0	0.0	25.8	0.0	0.0	15.4	0.0	0.0
Prop In Lane	0.19		0.15	0.69		0.10	0.00		0.09	0.02		0.02
Lane Grp Cap(c), veh/h	47	0	0	240	0	0	878	0	0	878	0	0
V/C Ratio(X)	0.57	0.00	0.00	0.74	0.00	0.00	0.81	0.00	0.00	0.59	0.00	0.00
Avail Cap(c_a), veh/h	496	0	0	485	0	0	1068	0	0	1064	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	32.8	0.0	0.0	28.1	0.0	0.0	14.6	0.0	0.0	11.4	0.0	0.0
Incr Delay (d2), s/veh	10.4	0.0	0.0	4.4	0.0	0.0	3.8	0.0	0.0	0.6	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	1.9	0.0	0.0	1.2	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.6	0.0	0.0	3.8	0.0	0.0	13.4	0.0	0.0	7.0	0.0	0.0
LnGrp Delay(d),s/veh	43.2	0.0	0.0	34.4	0.0	0.0	19.7	0.0	0.0	12.0	0.0	0.0
LnGrp LOS	D			С			В			В		
Approach Vol, veh/h		27			177			708			520	
Approach Delay, s/veh		43.2			34.4			19.7			12.0	
Approach LOS		D			С			В			В	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		42.1		8.7		42.1		15.7				
Change Period (Y+Rc), s		7.0		7.0		7.0		7.0				
Max Green Setting (Gmax), s		43.0		18.0		43.0		18.0				
Max Q Clear Time (q_c+l1), s		27.8		3.0		17.4		8.3				
Green Ext Time (p_c), s		7.3		0.1		9.4		0.6				
Intersection Summary												
HCM 2010 Ctrl Delay			19.2									
HCM 2010 LOS			В									

22: Route 138 & Aberdeen Rd.

	•	\rightarrow	•	†	↓	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			4	1≽	
Traffic Volume (vph)	10	4	4	540	410	30
Future Volume (vph)	10	4	4	540	410	30
Satd. Flow (prot)	1701	0	0	1546	1638	0
Flt Permitted	0.967					
Satd. Flow (perm)	1701	0	0	1546	1638	0
Confl. Peds. (#/hr)	2					
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88
Heavy Vehicles (%)	0%	0%	0%	7%	0%	0%
Parking (#/hr)			0	0	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	16	0	0	619	500	0
Sign Control	Stop			Free	Free	
Intersection Summary						

Control Type: Unsignalized Intersection Capacity Utilization 41.6%

ICU Level of Service A

Analysis Period (min) 15

Intersection						
Int Delay, s/veh	0.3					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
	EBL W	EDK	INDL			SDK
Lane Configurations Traffic Vol, veh/h	1 0	4	4	₄1 540	♣ 410	30
Future Vol, veh/h	10	4	4	540	410	30
Conflicting Peds, #/hr		0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storag	e,# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	88	88	88	88	88	88
Heavy Vehicles, %	0	0	0	7	0	0
Mvmt Flow	11	5	5	614	466	34
Major/Minor	Minor2	N	/lajor1	N	/lajor2	
Conflicting Flow All	1108	483	500	0	-	0
Stage 1	483	-	-	-	-	-
Stage 2	625	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1		-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	234	*327	*491	-	-	-
Stage 1	625	-	-	-	-	-
Stage 2	537	-	-	-	-	-
Platoon blocked, %		1	1	-	-	-
Mov Cap-1 Maneuver		*327	*491	-	-	-
Mov Cap-2 Maneuver		-	-	-	-	-
Stage 1	625	-	-	-	-	-
Stage 2	529	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	20.3		0.1		0	
HCM LOS	С					
Minor Lane/Major Mvr	mt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		* 491	-		-	
HCM Lane V/C Ratio		0.009		0.063	-	-
HCM Control Delay (s	;)	12.4	0	20.3	-	-
HCM Lane LOS		В	A	20.5 C	-	-
HCM 95th %tile Q(veh	1)	0	-	0.2	-	-
·	,					
Notes	nnae!t.	ф D-	Jourse	20 de 20	200	0 = ==
~: Volume exceeds ca	apacity	\$: De	lay exc	ceeds 30	JUS	+: Com

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	2	6	2	4	6	7	3	540	35	10	400	1
Future Volume (vph)	2	6	2	4	6	7	3	540	35	10	400	1
Satd. Flow (prot)	0	1775	0	0	1616	0	0	1710	0	0	1702	0
Flt Permitted		0.991			0.988						0.999	
Satd. Flow (perm)	0	1775	0	0	1616	0	0	1710	0	0	1702	0
Confl. Peds. (#/hr)	7					7	1		16	16		1
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Heavy Vehicles (%)	0%	0%	0%	25%	0%	0%	0%	7%	0%	0%	8%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	11	0	0	20	0	0	664	0	0	472	0
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilizat	ion 44.1%			IC	U Level	of Service	Α					
Analysis Period (min) 15												

Intersection													
Int Delay, s/veh	0.7												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4			4			4			4		
Traffic Vol, veh/h	2	6	2	4	6	7	3	540	35	10	400	1	
Future Vol, veh/h	2	6	2	4	6	7	3	540	35	10	400	1	
Conflicting Peds, #/hr	7	0	0	0	0	7	1	0	16	16	0	1	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	_	None	-	-	None	-	-	None	_	-	None	
Storage Length	_	_	-	_	_	-	_	_	-	_	_	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	_	0	_	
Grade, %	_	0	_	_	0	_	_	0	-	_	0	_	
Peak Hour Factor	87	87	87	87	87	87	87	87	87	87	87	87	
Heavy Vehicles, %	0	0	0	25	0	0	0	7	0	0	8	0	
Mvmt Flow	2	7	2	5	7	8	3	621	40	11	460	1	
IVIVIIIL I IOW	2	,	2	J	,	0	J	021	40	- 11	400		
Major/Minor N	/linor2			Minor1			Major1		N	/aior?			
		11/0		Minor1	1140		Major1	0		Major2	0	0	
Conflicting Flow All	1146	1168	461	1152	1149	664	462	0	0	677	0	0	
Stage 1	484	484	-	664	664	-	-	-	-	-	-	-	
Stage 2	662	684	-	488	485	-	-	-	-	-	-	-	
Critical Hdwy	7.1	6.5	6.2	7.35	6.5	6.2	4.1	-	-	4.1	-	-	
Critical Hdwy Stg 1	6.1	5.5	-	6.35	5.5	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.1	5.5	-	6.35	5.5	-	-	-	-	-	-	-	
Follow-up Hdwy	3.5	4	3.3	3.725	4	3.3	2.2	-	-	2.2	-	-	
Pot Cap-1 Maneuver	178	195	*436	157	200	*436	1110	-	-	924	-	-	
Stage 1	568	555	-	414	461	-	-	-	-	-	-	-	
Stage 2	454	452	-	520	555	-	-	-	-	-	-	-	
Platoon blocked, %			1			1		-	-		-	-	
Mov Cap-1 Maneuver	166	188	*436	147	193	*427	1110	-	-	918	-	-	
Mov Cap-2 Maneuver	166	188	-	147	193	-	-	-	-	-	-	-	
Stage 1	565	546	-	407	453	-	-	-	-	-	-	-	
Stage 2	434	444	-	503	546	-	-	-	-	-	-	-	
.													
Approach	EB			WB			NB			SB			
HCM Control Delay, s	23.5			22.3			0			0.2			
HCM LOS	C			C			U			0.2			
TIOW EOS	C												
NA!I /NA ! NA		NDI	NET	NDD	EDI 41	NDL 4	CDI	CDT	CDD				
Minor Lane/Major Mvm	l	NBL	NBT	NRK	EBLn1V		SBL	SBT	SBR				
Capacity (veh/h)		1110	-	-	206	228	918	-	-				
HCM Lane V/C Ratio		0.003	-	-		0.086	0.013	-	-				
HCM Control Delay (s)		8.3	0	-	23.5	22.3	9	0	-				
HCM Lane LOS		Α	Α	-	С	С	Α	Α	-				
HCM 95th %tile Q(veh)		0	-	-	0.2	0.3	0	-	-				
Notes													
~: Volume exceeds cap	acity	\$· De	lav evo	ceeds 3	00s	+. Com	putation	Not D	efined	*· ΔII	maior	/olume i	in platoon
. Volume exceeds cap	acity	ψ. DC	hay CAL	ocus 3	003	i. Cuili	pulation	ו ואטנ טי	cillicu	. 📶	major (rolume I	iii piatuuii

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4						4			4	
Traffic Volume (vph)	8	190	4	0	0	0	5	500	45	9	400	20
Future Volume (vph)	8	190	4	0	0	0	5	500	45	9	400	20
Satd. Flow (prot)	0	1802	0	0	0	0	0	1688	0	0	1695	0
Flt Permitted		0.998						0.996			0.987	
Satd. Flow (perm)	0	1802	0	0	0	0	0	1681	0	0	1675	0
Satd. Flow (RTOR)												
Confl. Peds. (#/hr)	1		1				1		7	7		1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	13%	1%	0%	2%	2%	2%	20%	7%	9%	0%	8%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	224	0	0	0	0	0	612	0	0	476	0
Turn Type	Perm	NA					Perm	NA		Perm	NA	
Protected Phases		4						2			6	
Permitted Phases	4						2			6		
Detector Phase	4	4					2	2		6	6	
Switch Phase												
Minimum Initial (s)	7.0	7.0					30.0	30.0		30.0	30.0	
Minimum Split (s)	23.0	23.0					36.0	36.0		36.0	36.0	
Total Split (s)	29.0	29.0					71.0	71.0		71.0	71.0	
Total Split (%)	29.0%	29.0%					71.0%	71.0%		71.0%	71.0%	
Yellow Time (s)	4.0	4.0					5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0					1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0						0.0			0.0	
Total Lost Time (s)		5.0						6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None					Min	Min		Min	Min	
Act Effct Green (s)		14.3						32.7			32.7	
Actuated g/C Ratio		0.25						0.56			0.56	
v/c Ratio		0.51						0.65			0.51	
Control Delay		23.9						13.2			10.6	
Queue Delay		0.0						0.0			0.0	
Total Delay		23.9						13.2			10.6	
LOS		С						В			В	
Approach Delay		23.9						13.2			10.6	
Approach LOS		С						В			В	
Queue Length 50th (ft)		62						124			86	
Queue Length 95th (ft)		144						272			189	
Internal Link Dist (ft)		967			1001			519			1021	
Turn Bay Length (ft)												
Base Capacity (vph)		756						1644			1639	
Starvation Cap Reductn		0						0			0	
Spillback Cap Reductn		0						0			0	
Storage Cap Reductn		0						0			0	
Reduced v/c Ratio		0.30						0.37			0.29	
Intersection Summary												
Cycle Length: 100												

24: Route 138 & Brook Rd.

Actuated Cycle Length: 58.1

Natural Cycle: 60

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.65

Intersection Signal Delay: 14.1

Intersection LOS: B

Intersection Capacity Utilization 51.6%

Analysis Period (min) 15

Splits and Phases: 24: Route 138 & Brook Rd.



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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4						4			4	
Traffic Volume (veh/h)	8	190	4	0	0	0	5	500	45	9	400	20
Future Volume (veh/h)	8	190	4	0	0	0	5	500	45	9	400	20
Number	7	4	14				5	2	12	1	6	16
Initial Q (Qb), veh	0	10	0				0	15	0	0	15	0
Ped-Bike Adj(A_pbT)	1.00		1.00				1.00		0.99	1.00		0.99
Parking Bus, Adj	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1900	1873	1900				1900	1771	1900	1900	1768	1900
Adj Flow Rate, veh/h	9	211	4				6	556	50	10	444	22
Adj No. of Lanes	0	1	0				0	1	0	0	1	0
Peak Hour Factor	0.90	0.90	0.90				0.90	0.90	0.90	0.90	0.90	0.90
Percent Heavy Veh, %	0	1	0				7	7	7	8	8	8
Cap, veh/h	12	337	5				71	972	78	76	1002	44
Arrive On Green	0.18	0.18	0.18				0.60	0.60	0.60	0.60	0.60	0.60
Sat Flow, veh/h	75	1755	33				4	1595	142	10	1649	80
Grp Volume(v), veh/h	224	0	0				612	0	0	476	0	0
Grp Sat Flow(s),veh/h/ln	1863	0	0				1741	0	0	1740	0	0
Q Serve(g_s), s	5.6	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	5.6	0.0	0.0				10.9	0.0	0.0	7.5	0.0	0.0
Prop In Lane	0.04		0.02				0.01		0.08	0.02		0.05
Lane Grp Cap(c), veh/h	361	0	0				1119	0	0	1118	0	0
V/C Ratio(X)	0.62	0.00	0.00				0.55	0.00	0.00	0.43	0.00	0.00
Avail Cap(c_a), veh/h	890	0	0				2316	0	0	2302	0	0
HCM Platoon Ratio	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	0.00				1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	22.4	0.0	0.0				7.6	0.0	0.0	6.7	0.0	0.0
Incr Delay (d2), s/veh	3.7	0.0	0.0				0.4	0.0	0.0	0.3	0.0	0.0
Initial Q Delay(d3),s/veh	14.5	0.0	0.0				2.9	0.0	0.0	2.3	0.0	0.0
%ile BackOfQ(50%),veh/ln	6.4	0.0	0.0				9.0	0.0	0.0	6.8 9.2	0.0	0.0
LnGrp Delay(d),s/veh	40.6 D	0.0	0.0				10.8 B	0.0	0.0		0.0	0.0
LnGrp LOS	D	224					D	/10		A	17/	
Approach Vol, veh/h		224						612			476 9.2	
Approach LOS		40.6 D						10.8 B			9.2 A	
Approach LOS											А	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6						
Phs Duration (G+Y+Rc), s		36.0		14.2		36.0						
Change Period (Y+Rc), s		6.0		5.0		6.0						
Max Green Setting (Gmax), s		65.0		24.0		65.0						
Max Q Clear Time (g_c+I1), s		12.9		7.6		9.5						
Green Ext Time (p_c), s		9.2		2.0		9.3						
Intersection Summary			45.0									
HCM 2010 Ctrl Delay			15.3									
HCM 2010 LOS			В									

102: Route 138 & Concord Baptist Church

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥	WDIX	1	NDIC	ODL	<u> </u>
Traffic Volume (vph)	0	0	550	3	3	410
Future Volume (vph)	0	0	550	3	3	410
Satd. Flow (prot)	1837	0	1831	0	0	1835
Flt Permitted						0.999
Satd. Flow (perm)	1837	0	1831	0	0	1835
Confl. Peds. (#/hr)				4	4	
Peak Hour Factor	0.92	0.92	0.86	0.75	0.38	0.89
Heavy Vehicles (%)	0%	0%	0%	33%	0%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	644	0	0	469
Sign Control	Stop		Free			Free
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utiliza				IC	III evel	of Service
Analysis Period (min) 15	111011 32.370			10	O LEVEL	OI JOI VICE
Analysis i chou (min) 15						

Intersection						
Int Delay, s/veh	0					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	, A		₽			4
Traffic Vol, veh/h	0	0	550	3	3	410
Future Vol, veh/h	0	0	550	3	3	410
Conflicting Peds, #/hr	0	0	0	4	4	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	86	75	38	89
Heavy Vehicles, %	0	0	0	33	0	0
Mvmt Flow	0	0	640	4	8	461
	, and the second		0.0	•		
	Minor1		/lajor1	Λ	/lajor2	
Conflicting Flow All	1122	646	0	0	648	0
Stage 1	646	-	-	-	-	-
Stage 2	476	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	230	475	-	-	947	-
Stage 1	526	_	-	-	_	_
Stage 2	629	_	-	-	-	_
Platoon blocked, %	02,		_	_		_
Mov Cap-1 Maneuver	227	473	_	_	947	_
Mov Cap-1 Maneuver	227	- 475	_	_	- 777	_
Stage 1	524	_				
Stage 2	622	-	_		-	-
Slaye 2	UZZ	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	0		0		0.1	
HCM LOS	A					
Minor Lanc/Major Mun	\ 1	NDT	NDD	MDI n1	CDI	CDT
Minor Lane/Major Mvm	IL	NBT	MRKA	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	-	947	-
HCM Lane V/C Ratio		-	-		0.008	-
HCM Control Delay (s)		-	-	0	8.8	0
HCM Lane LOS		-	-	A	Α	Α
HCM 95th %tile Q(veh)	1	_	_	-	0	-

8: Route 138 & South Parking Lot Entrance

	•	•	†	<i>></i>	-	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations			ĵ»			ર્ન
Traffic Volume (vph)	0	0	1120	15	2	1270
Future Volume (vph)	0	0	1120	15	2	1270
Satd. Flow (prot)	0	0	1576	0	0	1580
Flt Permitted						
Satd. Flow (perm)	0	0	1576	0	0	1580
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	2%	2%	4%	8%	0%	4%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	1220	0	0	1368
Sign Control	Stop		Free			Free
Intersection Summary						
Control Type: Unsignalized						

Control Type: Unsignalized Intersection Capacity Utilization 79.8%

ICU Level of Service D

9: Route 138 & South Parking Lot Exit

	•	•	†	~	\	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	**		^			†
Traffic Volume (vph)	3	10	1130	0	0	1270
Future Volume (vph)	3	10	1130	0	0	1270
Satd. Flow (prot)	1453	0	1580	0	0	1595
Flt Permitted	0.989					
Satd. Flow (perm)	1453	0	1580	0	0	1595
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	0%	4%	2%	2%	3%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	14	0	1215	0	0	1366
Sign Control	Stop		Free			Free
Intersection Summary						

Control Type: Unsignalized Intersection Capacity Utilization 84.7% Analysis Period (min) 15

ICU Level of Service E

Intersection						
Int Delay, s/veh	0.3					
		14/55		NES	051	05=
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W					
Traffic Vol, veh/h	3	10	1130	0	0	1270
Future Vol, veh/h	3	10	1130	0	0	1270
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	0	0	4	2	2	3
Mvmt Flow	3	11	1215	0	0	1366
Major/Minor	Minor1	N	Anior1	Ι.	//nior?	
	Minor1		//ajor1		/lajor2	
Conflicting Flow All	2581	1215	0	-	-	-
Stage 1	1215	-	-	-	-	-
Stage 2	1366	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	-	-
Pot Cap-1 Maneuver	29	223	-	0	0	-
Stage 1	283	-	-	0	0	-
Stage 2	239	-	-	0	0	-
Platoon blocked, %			-			-
Mov Cap-1 Maneuver	29	223	-	-	-	-
Mov Cap-2 Maneuver	29	-	-	-	-	-
Stage 1	283	-	-	-	-	-
Stage 2	239	-	-	-	-	-
J -						
Approach	WB		NB		SB	
	53.5		0		0	
HCM Control Delay, s			U		U	
HCM LOS	F					
Minor Lane/Major Mvm	nt	NBTV	VBLn1	SBT		
Capacity (veh/h)		-	88	-		
HCM Lane V/C Ratio		-	0.159	-		
HCM Control Delay (s)		-	53.5	-		
HCM Lane LOS			F	-		
HCM 95th %tile Q(veh)	-	0.5	-		
HOW FOUT FOUT QUEL	,		0.0			

10: Route 138 & North Parking Lot Entrance

	•	•	†	/	-	↓
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations			ĵ»			ની
Traffic Volume (vph)	0	0	1120	9	6	1270
Future Volume (vph)	0	0	1120	9	6	1270
Satd. Flow (prot)	0	0	1593	0	0	1581
Flt Permitted						
Satd. Flow (perm)	0	0	1593	0	0	1581
Peak Hour Factor	0.92	0.92	0.98	0.45	0.50	0.97
Heavy Vehicles (%)	2%	2%	3%	0%	0%	4%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	1163	0	0	1321
Sign Control	Stop		Free			Free
Intersection Summary						

Control Type: Unsignalized Intersection Capacity Utilization 83.4% Analysis Period (min) 15

ICU Level of Service E

	۶	→	•	•	←	•	•	†	<i>></i>	>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			ĵ.	
Traffic Volume (vph)	2	0	8	0	0	20	8	1120	0	0	1270	5
Future Volume (vph)	2	0	8	0	0	20	8	1120	0	0	1270	5
Satd. Flow (prot)	0	1451	0	0	1421	0	0	1593	0	0	1579	0
Flt Permitted		0.990										
Satd. Flow (perm)	0	1451	0	0	1421	0	0	1593	0	0	1579	0
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	0%	2%	0%	2%	2%	0%	25%	3%	0%	2%	4%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	10	0	0	21	0	0	1163	0	0	1314	0
Sign Control		Stop			Stop			Free			Free	

Intersection Summary

Control Type: Unsignalized

Intersection Capacity Utilization 85.0%

ICU Level of Service E

Intersection												
Int Delay, s/veh	0.5											
										001		000
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			Þ	
Traffic Vol, veh/h	2	0	8	0	0	20	8	1120	0	0	1270	5
Future Vol, veh/h	2	0	8	0	0	20	8	1120	0	0	1270	5
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	97	97	97	97	97	97	97	97	97	97	97	97
Heavy Vehicles, %	0	2	0	2	2	0	25	3	0	2	4	0
Mvmt Flow	2	0	8	0	0	21	8	1155	0	0	1309	5
Major/Minor I	Minor2		1	Minor1			Major1			/lajor2		
Conflicting Flow All	2493	2483	1312	2487	2485	1155	1314	0				0
Stage 1	1312	1312	1312	1171	1171	1100	1314	U	-	-	-	U
Stage 2	1181	1171	-	1316	1314	-	_		_	_	_	
Critical Hdwy	7.1	6.52	6.2	7.12	6.52	6.2	4.35	-	-	-	-	-
Critical Hdwy Stg 1	6.1	5.52	0.2	6.12	5.52	0.2	4.55		_	_	_	
Critical Hdwy Stg 2	6.1	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4.018	3.3	3.518	4.018	3.3	2.425		-	_	_	
Pot Cap-1 Maneuver	20	30	196	20	29	242	457	-	0	0	-	-
Stage 1	197	228	190	235	267	242	407	-	0	0	-	-
Stage 2	234	267	-	194	207	-	-	-	0	0	-	-
Platoon blocked, %	234	207	-	174	220	-	-	-	U	U	-	•
Mov Cap-1 Maneuver	18	29	196	18	28	242	457	-			-	-
Mov Cap-1 Maneuver	18	29	190	18	28	242	437	-	-	-	-	•
Stage 1	187	29	-	223	254	-	-	-	-	-	-	-
Stage 2	204	254	-	186	228	-	-	-	-	-	-	-
Staye 2	204	204	-	100	220	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	69.4			21.3			0.1			0		
HCM LOS	F			С								
Minor Lane/Major Mvm	nt	NBL	NBT	EBLn1V	VBLn1	SBT	SBR					
Capacity (veh/h)		457	_		242	_	_					
HCM Lane V/C Ratio		0.018		0.156		_	_					
HCM Control Delay (s)		13	0	69.4	21.3	_	_					
HCM Lane LOS		В	A	F	C C	_	_					
HCM 95th %tile Q(veh))	0.1	-	0.5	0.3	-	_					
HOW FOUT FOUT Q(VCH)	,	0.1		0.0	0.5							

12: Route 138 & Summit Rd.

	•	•	†	~	>	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	M		₽			ર્ન
Traffic Volume (vph)	1	1	790	340	10	1270
Future Volume (vph)	1	1	790	340	10	1270
Satd. Flow (prot)	1480	0	1523	0	0	1580
Flt Permitted	0.976					
Satd. Flow (perm)	1480	0	1523	0	0	1580
Confl. Peds. (#/hr)				1	1	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	0%	2%	5%	0%	2%	4%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	2	0	1165	0	0	1319
Sign Control	Stop		Free			Free
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utiliza	ation 93.6%			IC	U Level o	of Service
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	0.1					
		MDD	NET	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		ĵ.			4
Traffic Vol, veh/h	1	1	790	340	10	1270
Future Vol, veh/h	1	1	790	340	10	1270
Conflicting Peds, #/hr	0	0	0	1	1	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	0	2	5	0	2	4
Mvmt Flow	1	1	814	351	10	1309
	Minor1		/lajor1		Major2	
Conflicting Flow All	2321	991	0	0	1166	0
Stage 1	991	-	-	-	-	-
Stage 2	1330	-	-	-	-	-
Critical Hdwy	6.4	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	42	299	-	-	599	-
Stage 1	362	-	-	-	-	-
Stage 2	249	-	-	_	-	-
Platoon blocked, %			_			
Mov Cap-1 Maneuver	39	299	_	_	599	_
Mov Cap-1 Maneuver	39		_	_	- 377	_
Stage 1	362	_	-	-		_
Stage 2	234	_	-		-	-
Staye 2	234	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	58.8		0		0.1	
HCM LOS	F					
		NET	NICO	VD.	051	057
Minor Lane/Major Mvn	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	69	599	-
HCM Lane V/C Ratio		-	-	0.03	0.017	-
HCM Control Delay (s))	-	-	58.8	11.1	0
HCM Lane LOS		-	-	F	В	Α
HCM 95th %tile Q(veh	1)	-	-	0.1	0.1	-

13: Route 138 & Thacher Montessori School

	۶	\rightarrow	4	†	ļ	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			4	f)	
Traffic Volume (vph)	0	35	1	790	1260	3
Future Volume (vph)	0	35	1	790	1260	3
Satd. Flow (prot)	1421	0	0	1580	1579	0
Flt Permitted						
Satd. Flow (perm)	1421	0	0	1580	1579	0
Peak Hour Factor	0.92	0.39	0.25	0.96	0.97	0.38
Heavy Vehicles (%)	2%	0%	0%	4%	4%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	90	0	0	827	1307	0
Sign Control	Stop			Free	Free	
Intersection Summary						

Control Type: Unsignalized Intersection Capacity Utilization 84.3%

ICU Level of Service E

Intersection						
Int Delay, s/veh	1.5					
		EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	M	٥٦	1	4	12(0	2
Traffic Vol, veh/h	0	35	1	790	1260	3
Future Vol, veh/h	0	35	1	790	1260	3
Conflicting Peds, #/hr	0	0	0	0	0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	39	25	96	97	38
Heavy Vehicles, %	2	0	0	4	4	0
Mvmt Flow	0	90	4	823	1299	8
Major/Minor	Minor2	N	/lajor1	N	/lajor2	
						^
Conflicting Flow All	2134	1303	1307	0	-	0
Stage 1	1303	-	-	-	-	-
Stage 2	831	-	-	-	-	-
Critical Hdwy	6.42	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	54	198	536	-	-	-
Stage 1	254	-	-	-	-	-
Stage 2	428	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	53	198	536		-	-
Mov Cap-2 Maneuver	53	-	-	-	-	-
Stage 1	254	-	-	-	-	-
Stage 2	422	-	-	-	-	-
J						
Annroach	ED		ND		CD	
Approach	EB		NB		SB	
HCM Control Delay, s	37.4		0.1		0	
HCM LOS	E					
Minor Lane/Major Mvn	nt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		536	-		-	-
HCM Lane V/C Ratio		0.007		0.453	-	-
HCM Control Delay (s)	\	11.8	0	37.4	-	-
HCM Lane LOS			A	37.4 E	-	-
HCM 95th %tile Q(veh	.\	B 0	A -	2.1		
HON ADM WING MINCS)	U	-	Z. I	-	-

	۶	→	*	•	←	4	1	†	<i>></i>	/		4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		ሻ	ĥ			^			1>	
Traffic Volume (vph)	15	0	370	140	70	10	0	790	0	0	770	0
Future Volume (vph)	15	0	370	140	70	10	0	790	0	0	770	0
Satd. Flow (prot)	0	1413	0	1546	1558	0	0	1580	0	0	1565	0
Flt Permitted		0.990		0.367								
Satd. Flow (perm)	0	1402	0	597	1558	0	0	1580	0	0	1565	0
Satd. Flow (RTOR)												
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	0%	2%	1%	1%	3%	8%	0%	4%	2%	2%	5%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	396	0	144	82	0	0	814	0	0	794	0
Turn Type	Perm	NA		Perm	NA			NA			NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8								
Detector Phase	4	4		8	8			2			6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0			10.0			10.0	
Minimum Split (s)	23.0	23.0		23.0	23.0			45.0			45.0	
Total Split (s)	30.0	30.0		30.0	30.0			45.0			45.0	
Total Split (%)	40.0%	40.0%		40.0%	40.0%			60.0%			60.0%	
Yellow Time (s)	4.0	4.0		4.0	4.0			5.0			5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0			1.0			1.0	
Lost Time Adjust (s)		0.0		0.0	0.0			0.0			0.0	
Total Lost Time (s)		5.0		5.0	5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None			None			None	
Act Effct Green (s)		23.1		23.1	23.1			39.1			39.1	
Actuated g/C Ratio		0.32		0.32	0.32			0.53			0.53	
v/c Ratio		0.90		0.77	0.17			0.97			0.95	
Control Delay		49.0		50.9	18.8			43.5			40.8	
Queue Delay		0.0		0.0	0.0			0.0			0.0	
Total Delay		49.0		50.9	18.8			43.5			40.8	
LOS		D		D	В			D			D	
Approach Delay		49.0			39.3			43.5			40.8	
Approach LOS		D			D			D			D	
Queue Length 50th (ft)		169		58	26			350			336	
Queue Length 95th (ft)		#323		#151	57			#606			#589	
Internal Link Dist (ft)		906			198			187			2213	
Turn Bay Length (ft)												
Base Capacity (vph)		479		204	532			843			834	
Starvation Cap Reductn		0		0	0			0			0	
Spillback Cap Reductn		0		0	0			0			0	
Storage Cap Reductn		0		0	0			0			0	
Reduced v/c Ratio		0.83		0.71	0.15			0.97			0.95	

Intersection Summary

Cycle Length: 75

Actuated Cycle Length: 73.2

14: Route 138 & Brush Hill Rd.

Natural Cycle: 80

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.97

Intersection Signal Delay: 43.1 Intersection LOS: D

Intersection Capacity Utilization 95.0% ICU Level of Service F

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 14: Route 138 & Brush Hill Rd.



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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		ሻ	₽			†			f)	
Traffic Volume (veh/h)	15	0	370	140	70	10	0	790	0	0	770	0
Future Volume (veh/h)	15	0	370	140	70	10	0	790	0	0	770	0
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	20	0	10	10	0	0	10	0	0	40	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1700	1684	1700	1683	1641	1700	0	1635	0	0	1619	1700
Adj Flow Rate, veh/h	15	0	381	144	72	10	0	814	0	0	794	0
Adj No. of Lanes	0	1	0	1	1	0	0	1	0	0	1	0
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	2	2	2	1	3	3	0	4	0	0	5	5
Cap, veh/h	50	11	306	190	529	60	0	850	0	0	842	0
Arrive On Green	0.33	0.00	0.33	0.33	0.33	0.33	0.00	0.52	0.00	0.00	0.52	0.00
Sat Flow, veh/h	23	32	1377	902	1410	196	0	1635	0	0	1619	0
Grp Volume(v), veh/h	396	0	0	144	0	82	0	814	0	0	794	0
Grp Sat Flow(s),veh/h/ln	1431	0	0	902	0	1606	0	1635	0	0	1619	0
Q Serve(g_s), s	4.4	0.0	0.0	4.4	0.0	2.7	0.0	35.7	0.0	0.0	34.6	0.0
Cycle Q Clear(g_c), s	19.0	0.0	0.0	23.4	0.0	2.7	0.0	35.7	0.0	0.0	34.6	0.0
Prop In Lane	0.04		0.96	1.00		0.12	0.00		0.00	0.00		0.00
Lane Grp Cap(c), veh/h	329	0	0	190	0	538	0	850	0	0	842	0
V/C Ratio(X)	1.20	0.00	0.00	0.76	0.00	0.15	0.00	0.96	0.00	0.00	0.94	0.00
Avail Cap(c_a), veh/h	528	0	0	257	0	537	0	852	0	0	844	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00	1.00	0.00
Uniform Delay (d), s/veh	22.5	0.0	0.0	31.2	0.0	18.1	0.0	18.0	0.0	0.0	18.0	0.0
Incr Delay (d2), s/veh	108.8	0.0	0.0	8.5	0.0	0.1	0.0	21.1	0.0	0.0	18.6	0.0
Initial Q Delay(d3),s/veh	159.7	0.0	0.0	81.8	0.0	2.9	0.0	23.3	0.0	0.0	145.4	0.0
%ile BackOfQ(50%),veh/ln	36.7	0.0	0.0	8.6	0.0	2.5	0.0	27.7	0.0	0.0	55.4	0.0
LnGrp Delay(d),s/veh	290.9	0.0	0.0	121.5	0.0	21.2	0.0	62.4	0.0	0.0	182.0	0.0
LnGrp LOS	F			F		С		E			F	
Approach Vol, veh/h		396			226			814			794	
Approach Delay, s/veh		290.9			85.1			62.4			182.0	
Approach LOS		F			F			E			F	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		44.8		30.0		44.8		30.0				
Change Period (Y+Rc), s		6.0		5.0		6.0		5.0				
Max Green Setting (Gmax), s		39.0		25.0		39.0		25.0				
Max Q Clear Time (g_c+I1), s		37.7		21.0		36.6		25.4				
Green Ext Time (p_c), s		1.1		1.5		1.9		0.0				
Intersection Summary												
HCM 2010 Ctrl Delay			147.9									
HCM 2010 LOS			F									
Notes												

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations			7	^		7
Traffic Volume (vph)	0	0	90	270	0	140
Future Volume (vph)	0	0	90	270	0	140
Satd. Flow (prot)	0	0	1546	1627	0	1394
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1546	1627	0	1394
Peak Hour Factor	0.92	0.92	0.96	0.93	0.92	0.82
Heavy Vehicles (%)	2%	2%	1%	1%	2%	2%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	94	290	0	171
Sign Control	Free			Free	Free	

Intersection Summary

Control Type: Unsignalized Intersection Capacity Utilization 21.9%

ICU Level of Service A

16: Route 138 & Neponset Valley Pkwy.

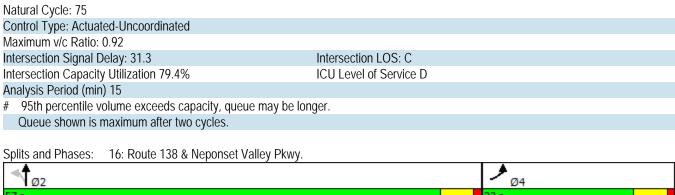
Lane Group		•	\rightarrow	1	†	↓	4
Lane Configurations	Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Traffic Volume (vph) 7 230 310 490 530 3 Future Volume (vph) 7 230 310 490 530 3 Satd. Flow (prot) 1343 0 1473 1595 1564 0 Fit Permitted 0.999 0.185 Satd. Flow (perm) 1343 0 287 1595 1564 0 Satd. Flow (perm) 144 0 320 1595 1564 0 Satd. Flow (perm) 144 0 320 1595 1564 0 Satd. Flow (perm) 144 0 320 1595 1564 0 Satd. Flow (perm) 144 0 320 1595 1564 0 Satd. Flow (perm) 144 153 1595 1564 0 S							
Future Volume (vph) 7 230 310 490 530 3 Satd. Flow (prot) 1343 0 1473 1595 1564 0 Fit Permitted 0.999 0.185 Satd. Flow (perm) 1343 0 287 1595 1564 0 Satd. Flow (perm) 1343 0 287 1595 1564 0 Satd. Flow (RTOR) Peak Hour Factor 0.97 0.97 0.97 0.97 0.97 0.97 Heavy Vehicles (%) 14% 6% 6% 3% 5% 0% Shared Lane Traffic (%) Lane Group Flow (vph) 244 0 320 505 549 0 Turn Type Prot pm+pt NA NA Protected Phases 2 0 Permitted Phases 2 2 Detector Phase 4 5 2 6 Switch Phase Minimum Initial (s) 5.0 5.0 5.0 5.0 5.0 Minimum Split (s) 23.0 10.0 23.0 23.0 10.1 23.0 23.0 10.1 23.0 23.0 10.1 23.0 23.0 10.1 23.0 23.0 10.1 23.0 23.0 10.1 23.0 23.0 10.1 23.0 23.0 10.1 23.0 23.0 10.1 23.0 23.0 10.1 23.0 23.0 10.1 23.0 25.0 57.0 37.0 10.1 25.1 10.1 10.1 1.0 1.0 1.0 1.0 1.0 1.0 1.0			230				3
Satd. Flow (prot) 1343 0 1473 1595 1564 0 Flt Permitted 0.999 0.185 0 1595 1564 0 Satd. Flow (RTOR) 0.97 0.	` ' '						
Fit Permitted 0.999 0.185 Satd. Flow (perm) 1343 0 287 1595 1564 0 Satd. Flow (RTOR) Peak Hour Factor 0.97 <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>							
Satd. Flow (Perm) 1343 0 287 1595 1564 0 Satd. Flow (RTOR) 0.97<							
Satd. Flow (RTOR) Peak Hour Factor 0.97			0		1595	1564	0
Peak Hour Factor 0.97 0.97 0.97 0.97 0.97 0.97 Heavy Vehicles (%) 14% 6% 6% 3% 5% 0% Shared Lane Traffic (%) Same Group Flow (vph) 244 0 320 505 549 0 Turn Type Prot pm+pt NA NA Protected Phases 4 5 2 6 Permitted Phases 2 2 6 Detector Phase 4 5 2 6 Switch Phase 8 2 6 8 Minimum Initial (s) 5.0 5.0 5.0 5.0 5.0 Minimum Split (s) 23.0 10.0 23.0 <td>4 ,</td> <td>1010</td> <td></td> <td>20.</td> <td>.070</td> <td></td> <td></td>	4 ,	1010		20.	.070		
Heavy Vehicles (%)		0.97	0.97	0.97	0.97	0.97	0.97
Shared Lane Traffic (%) Lane Group Flow (vph) 244 0 320 505 549 0 Turn Type Prot pm+pt NA NA Protected Phases 4 5 2 6 Permitted Phases 2 2 6 Detector Phase 4 5 2 6 Switch Phase 4 5 2 6 Minimum Initial (s) 5.0 5.0 5.0 5.0 Minimum Split (s) 23.0 20.0 57.0 37.0 Total Split (s) 23.0 20.0 57.0 37.0 Total Split (s) 28.8% 25.0% 71.3% 46.3% Yellow Time (s) 4.0 4.0 4.0 4.0 40.3 40.3% Yellow Time (s) 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1							
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Turn Type	, ,	244	0	320	505	549	0
Protected Phases			U				U
Permitted Phases 2							
Detector Phase 4 5 2 6 Switch Phase Minimum Initial (s) 5.0 5.0 5.0 5.0 Minimum Split (s) 23.0 10.0 23.0 23.0 Total Split (s) 23.0 20.0 57.0 37.0 Total Split (%) 28.8% 25.0% 71.3% 46.3% Yellow Time (s) 4.0 4.0 4.0 4.0 All-Red Time (s) 1.0 1.0 1.0 1.0 All-Red Time (s) 0.0 0.0 0.0 0.0 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 Total Lost Time (s) 5.0 5.0 5.0 5.0 Lead/Lag Lead Lag Lead Lag Lead-Lag Optimize? Yes Yes Yes Recall Mode None None None None Actuated g/C Ratio 0.22 0.65 0.65 0.38 V/c Ratio 0.22 0.65		7				U	
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Turn Bay Length (ft) 200 Base Capacity (vph) 327 426 1123 677 Starvation Cap Reductn 0 0 0 0 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0 Reduced v/c Ratio 0.75 0.75 0.45 0.81		1200			2213	1154	
Base Capacity (vph) 327 426 1123 677 Starvation Cap Reductn 0 0 0 0 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0 Reduced v/c Ratio 0.75 0.75 0.45 0.81	Turn Bay Length (ft)			200			
Starvation Cap Reductn 0 0 0 0 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0 Reduced v/c Ratio 0.75 0.75 0.45 0.81		327		426	1123	677	
Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0 Reduced v/c Ratio 0.75 0.75 0.45 0.81							
Storage Cap Reductn 0 0 0 0 Reduced v/c Ratio 0.75 0.75 0.45 0.81							
Reduced v/c Ratio 0.75 0.45 0.81				0	0		
	Intersection Summary						

Intersection Summary

Cycle Length: 80

Actuated Cycle Length: 74.9

16: Route 138 & Neponset Valley Pkwy.



					1	,
	•	*	-	T	¥	*
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥		J.	†	f)	
Traffic Volume (veh/h)	7	230	310	490	530	3
Future Volume (veh/h)	7	230	310	490	530	3
Number	7	14	5	2	6	16
Initial Q (Qb), veh	5	0	10	15	15	0
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00			1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1600	1700	1604	1650	1619	1700
Adj Flow Rate, veh/h	7	237	320	505	546	3
Adj No. of Lanes	0	0	1	1	1	0
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	0	0	6	3	5	5
Cap, veh/h	31	265	388	1053	655	2
Arrive On Green	0.21	0.21	0.15	0.64	0.41	0.41
Sat Flow, veh/h	39	1321	1527	1650	1609	9
Grp Volume(v), veh/h	245	0	320	505	0	549
Grp Sat Flow(s), veh/h/ln	1365	0	1527	1650	0	1618
Q Serve(g_s), s	11.1	0.0	7.0	10.3	0.0	19.4
Cycle Q Clear(g_c), s	11.1	0.0	7.0	10.3	0.0	19.4
Prop In Lane	0.03	0.97	1.00	,	,,,	0.01
Lane Grp Cap(c), veh/h	297	0	388	1053	0	655
V/C Ratio(X)	0.83	0.00	0.82	0.48	0.00	0.84
Avail Cap(c_a), veh/h	383	0	549	1337	0	807
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	27.3	0.0	20.1	6.9	0.0	21.2
Incr Delay (d2), s/veh	10.9	0.0	6.9	0.3	0.0	6.5
Initial Q Delay(d3),s/veh	0.0	0.0	27.1	2.8	0.0	23.4
%ile BackOfQ(50%),veh/ln	6.1	0.0	10.4	7.5	0.0	17.4
LnGrp Delay(d),s/veh	38.3	0.0	54.1	10.1	0.0	51.2
LnGrp LOS	30.3 D	0.0	54.1 D	В	0.0	51.2 D
	245		D		549	D
Approach Vol, veh/h				825		
Approach LOS	38.3			27.1	51.2	
Approach LOS	D			С	D	
Timer	1	2	3	4	5	6
Assigned Phs		2		4	5	6
Phs Duration (G+Y+Rc), s		45.9		18.3	14.5	31.4
Change Period (Y+Rc), s		5.0		5.0	5.0	5.0
Max Green Setting (Gmax), s		52.0		18.0	15.0	32.0
Max Q Clear Time (q_c+I1), s		12.3		13.1	9.0	21.4
Green Ext Time (p_c), s		8.7		0.4	0.5	5.0
•		0.7		0.7	0.0	5.0
Intersection Summary						
HCM 2010 Ctrl Delay			37.0			
HCM 2010 LOS			D			
Notes						
NOTES						

17: Route 138 & Milton St./Dollar Ln.

	۶	→	•	•	+	•	•	†	/	/	↓	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ሻ	f.			4	
Traffic Volume (vph)	30	120	15	10	110	25	15	470	10	8	510	25
Future Volume (vph)	30	120	15	10	110	25	15	470	10	8	510	25
Satd. Flow (prot)	0	1574	0	0	1525	0	1561	1588	0	0	1556	0
Flt Permitted		0.926			0.970		0.462				0.992	
Satd. Flow (perm)	0	1470	0	0	1486	0	759	1588	0	0	1545	0
Satd. Flow (RTOR)												
Confl. Peds. (#/hr)	1		1			1						
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	7%	1%	0%	9%	5%	0%	0%	3%	10%	0%	5%	4%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	174	0	0	153	0	16	506	0	0	571	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	7.0	7.0		7.0	7.0		30.0	30.0		30.0	30.0	
Minimum Split (s)	23.0	23.0		23.0	23.0		37.0	37.0		37.0	37.0	
Total Split (s)	25.0	25.0		25.0	25.0		55.0	55.0		55.0	55.0	
Total Split (%)	31.3%	31.3%		31.3%	31.3%		68.8%	68.8%		68.8%	68.8%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)		0.0			0.0		0.0	0.0			0.0	
Total Lost Time (s)		5.0			5.0		7.0	7.0			7.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		None	None	
Act Effct Green (s)		12.0			12.0		32.5	32.5			32.5	
Actuated g/C Ratio		0.21			0.21		0.57	0.57			0.57	
v/c Ratio		0.56			0.49		0.04	0.56			0.65	
Control Delay		28.0			25.8		6.7	11.2			13.2	
Queue Delay		0.0			0.0		0.0	0.0			0.0	
Total Delay		28.0			25.8		6.7	11.2			13.2	
LOS		С			С		Α	В			В	
Approach Delay		28.0			25.8			11.0			13.2	
Approach LOS		С			С			В			В	
Queue Length 50th (ft)		49			43		2	90			110	
Queue Length 95th (ft)		123			109		11	214			263	
Internal Link Dist (ft)		1017			791			1154			1181	
Turn Bay Length (ft)							200					
Base Capacity (vph)		528			534		655	1370			1333	
Starvation Cap Reductn		0			0		0	0			0	
Spillback Cap Reductn		0			0		0	0			0	
Storage Cap Reductn		0			0		0	0			0	
Reduced v/c Ratio		0.33			0.29		0.02	0.37			0.43	
Intersection Summary												_
Cycle Length: 80												

17: Route 138 & Milton St./Dollar Ln.

Actuated Cycle Length: 56.7

Natural Cycle: 60

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.65

Intersection Signal Delay: 15.6

Intersection Capacity Utilization 67.6%

Analysis Period (min) 15

Splits and Phases: 17: Route 138 & Milton St./Dollar Ln.



	۶	→	•	•	←	•	1	†	/	/	+	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		7	₽			4	
Traffic Volume (veh/h)	30	120	15	10	110	25	15	470	10	8	510	25
Future Volume (veh/h)	30	120	15	10	110	25	15	470	10	8	510	25
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	25	0	0	15	0	0	15	0	0	30	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1700	1666	1700	1700	1628	1700	1700	1648	1700	1700	1621	1700
Adj Flow Rate, veh/h	32	126	16	11	116	26	16	495	11	8	537	26
Adj No. of Lanes	0	1	0	0	1	0	1	1	0	0	1	0
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	1	1	1	5	5	5	0	3	3	5	5	5
Cap, veh/h	92	280	17	77	279	31	412	963	19	53	821	31
Arrive On Green	0.17	0.17	0.17	0.17	0.17	0.17	0.59	0.59	0.59	0.59	0.59	0.59
Sat Flow, veh/h	181	1250	145	51	1243	265	770	1606	36	6	1523	73
Grp Volume(v), veh/h	174	0	0	153	0	0	16	0	506	571	0	0
Grp Sat Flow(s),veh/h/ln	1576	0	0	1559	0	0	770	0	1642	1602	0	0
Q Serve(g_s), s	0.5	0.0	0.0	0.0	0.0	0.0	0.0	0.0	9.2	0.0	0.0	0.0
Cycle Q Clear(g_c), s	5.0	0.0	0.0	4.5	0.0	0.0	0.6	0.0	9.2	11.3	0.0	0.0
Prop In Lane	0.18	0	0.09	0.07	0	0.17	1.00	0	0.02	0.01	_	0.05
Lane Grp Cap(c), veh/h	388	0	0	363	0	0	412	0	975	803	0	0
V/C Ratio(X)	0.45	0.00	0.00	0.42	0.00	0.00	0.04	0.00	0.52	0.71	0.00	0.00
Avail Cap(c_a), veh/h	691	0	0	685	0	0	823	0	1557	1585	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00	1.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	23.6	0.0	0.0	21.7	0.0	0.0	5.4	0.0	7.3	16.7	0.0	0.0
Incr Delay (d2), s/veh	0.8 54.1	0.0	0.0	0.8 21.2	0.0	0.0	0.0	0.0	0.4 3.5	1.2 34.9	0.0	0.0
Initial Q Delay(d3),s/veh	10.9	0.0	0.0	5.8	0.0	0.0	0.0	0.0	7.3	20.6	0.0	0.0
%ile BackOfQ(50%),veh/ln LnGrp Delay(d),s/veh	78.5	0.0	0.0	43.6	0.0	0.0	5.4	0.0	11.3	52.7	0.0	0.0
LnGrp LOS	76.5 E	0.0	0.0	43.0 D	0.0	0.0	3.4 A	0.0	11.3 B	32.7 D	0.0	0.0
	<u> </u>	174		D	153			522	Ь	U	571	
Approach Vol, veh/h		78.5			43.6			11.1			52.7	
Approach Delay, s/veh Approach LOS		76.5 E			43.0 D			В			52.7 D	
											D	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		37.0		13.6		37.0		13.6				
Change Period (Y+Rc), s		7.0		5.0		7.0		5.0				
Max Green Setting (Gmax), s		48.0		20.0		48.0		20.0				
Max Q Clear Time (g_c+I1), s		11.2		7.0		13.3		6.5				
Green Ext Time (p_c), s		8.6		1.5		8.5		1.6				
Intersection Summary			00.1									
HCM 2010 Ctrl Delay			39.6									
HCM 2010 LOS			D									

18: Route 138 & Blue Jay Way (Curry College)

	•	•	4	†	↓	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	f)	
Traffic Volume (vph)	50	100	80	460	440	25
Future Volume (vph)	50	100	80	460	440	25
Satd. Flow (prot)	1472	0	0	1589	1544	0
Flt Permitted	0.984			0.993		
Satd. Flow (perm)	1472	0	0	1589	1544	0
Confl. Peds. (#/hr)			1			1
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	0%	1%	3%	6%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	162	0	0	581	500	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utiliza				IC	'III ovol c	of Service D
Analysis Period (min) 15	111011 / 9.0%			IC	o rever	ii Seivice D
Analysis Penou (IIIII) 15						

Intersection						
Int Delay, s/veh	6.6					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	₩.	LDIX	NDL	4) }	אומכ
Traffic Vol, veh/h	5 0	100	80	460	440	25
Future Vol, veh/h	50	100	80	460	440	25
Conflicting Peds, #/hr		0	1	0	0	1
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	- -	None	-		-	None
Storage Length	0	-	_	-	_	-
Veh in Median Storag		-	-	0	0	-
Grade, %	0	_	_	0	0	_
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	0	0		3	6	0
Mvmt Flow	54	108	86	495	473	27
	0.			.,,	., 0	= '
N A = 1 = 1/N A1 = 1 = 1	NA'		NA-!1		1-!	
Major/Minor	Minor2		Major1		/lajor2	
Conflicting Flow All	1155	488	501	0	-	0
Stage 1	488	-	-	-	-	-
Stage 2	667	- / 2	-	-	-	-
Critical Hdwy	6.4	6.2	4.11	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	2 200	-	-	-
Follow-up Hdwy	3.5			-	-	-
Pot Cap-1 Maneuver	220 621	*327	*488	-	-	-
Stage 1	514	-	-	-	-	-
Stage 2 Platoon blocked, %	514	- 1	- 1	-	-	-
	- 144	-	*488	-	-	-
Mov Cap-1 Maneuver		*327		-	-	-
Mov Cap-2 Maneuver	620	-	-	-		-
Stage 1	389	-	-	-	-	-
Stage 2	389	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	43.4		2.1		0	
HCM LOS	Е					
Minor Lane/Major Mvi	mt	NBL	NRT	EBLn1	SBT	SBR
Capacity (veh/h)	iiit	* 488	-		-	JUIN
HCM Lane V/C Ratio		0.176		0.653	-	-
HCM Control Delay (s	-)	13.9		43.4	-	-
HCM Lane LOS	P)	13.9 B	A	43.4 E	-	-
HCM 95th %tile Q(vel	h)	0.6	- A	4.1	-	- -
	'')	0.0		7.1		
Notes						
~: Volume exceeds ca	apacity	\$: De	elay exc	ceeds 30	00s	+: Com

19: Route 138 & Atherton St. & Bradlee Rd.

	۶	74	•	ሻ	†	r*	Ļ	↓	¥J	•	`*	\
Lane Group	EBL	EBR	NBL2	NBL	NBT	NBR	SBL	SBT	SBR2	SEL	SET	SER
Lane Configurations	M				4			4			4	
Traffic Volume (vph)	2	2	2	35	440	35	7	410	6	6	30	15
Future Volume (vph)	2	2	2	35	440	35	7	410	6	6	30	15
Satd. Flow (prot)	1495	0	0	0	1578	0	0	1544	0	0	1514	0
Flt Permitted	0.976	Ū	, and the second	J	0.947		J	0.990	J	, and the second	0.949	J
Satd. Flow (perm)	1495	0	0	0	1500	0	0	1530	0	0	1446	0
Satd. Flow (RTOR)		Ū	, and the second	J			J	.000	J	, and the second		J
Confl. Peds. (#/hr)				1					1			
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	0%	0%	0%	3%	3%	14%	6%	0%	0%	6%	0%
Shared Lane Traffic (%)	070	070	070	070	070	370	1170	070	070	070	070	070
Lane Group Flow (vph)	4	0	0	0	551	0	0	455	0	0	54	0
Turn Type	Prot	U	Perm	Perm	NA	U	Perm	NA	U	Perm	NA	J
Protected Phases	10		1 01111	1 01111	2		1 01111	6		1 01111	4	
Permitted Phases	10		2	2			6	U		4	-	
Detector Phase	10		2	2	2		6	6		4	4	
Switch Phase	10			2	2		U	U		_	-	
Minimum Initial (s)	4.0		22.0	22.0	22.0		22.0	22.0		10.0	10.0	
Minimum Split (s)	9.0		28.0	28.0	28.0		28.0	28.0		15.0	15.0	
Total Split (s)	9.0		55.0	55.0	55.0		55.0	55.0		16.0	16.0	
Total Split (%)	11.3%		68.8%	68.8%	68.8%		68.8%	68.8%		20.0%	20.0%	
Yellow Time (s)	4.0		5.0	5.0	5.0		5.0	5.0		4.0	4.0	
All-Red Time (s)	1.0		1.0	1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0		1.0	1.0	0.0		1.0	0.0		1.0	0.0	
Total Lost Time (s)	5.0				6.0			6.0			5.0	
Lead/Lag	3.0				0.0			0.0			3.0	
Lead-Lag Optimize?												
Recall Mode	None		None	None	None		None	None		None	None	
Act Effct Green (s)	4.5		None	None	31.6		None	31.6		None	11.4	
Actuated g/C Ratio	0.10				0.73			0.73			0.26	
v/c Ratio	0.10				0.73			0.73			0.20	
Control Delay	27.0				9.3			7.9			20.6	
Queue Delay	0.0				0.0			0.0			0.0	
Total Delay	27.0				9.3			7.9			20.6	
LOS	27.0 C				7.5 A			Α.			20.0 C	
Approach Delay	27.0				9.3			7.9			20.6	
Approach LOS	27.0 C				7.3 A			7. 7			20.0 C	
Queue Length 50th (ft)	1				91			67			11	
Queue Length 95th (ft)	11				255			188			53	
Internal Link Dist (ft)	1290				1586			2667			689	
Turn Bay Length (ft)	1270				1300			2007			007	
Base Capacity (vph)	155				1403			1431			413	
Starvation Cap Reductn	0				1403			1431			413	
	0							0				
Spillback Cap Reductn Storage Cap Reductn	0				0			0			0	
Reduced v/c Ratio	0.03				0.39			0.32			0.13	
	0.03				0.39			0.32			0.13	
Intersection Summary												
Cycle Length: 80												

19: Route 138 & Atherton St. & Bradlee Rd.

	€	*	×	*
Lane Group	NWL2	NWL	NWT	NWR
Lane Configurations	1444 LZ		4	14441
Traffic Volume (vph)	25	2	45	10
Future Volume (vph)	25	2	45	10
Satd. Flow (prot)	0	0	1547	0
Flt Permitted	U	U	0.869	U
Satd. Flow (perm)	0	0	1367	0
Satd. Flow (RTOR)	U	U	1307	U
Confl. Peds. (#/hr)				
Peak Hour Factor	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	0%	5%	0.73
Shared Lane Traffic (%)	070	070	370	070
Lane Group Flow (vph)	0	0	88	0
Turn Type	Perm	Perm	NA	U
Protected Phases	i Giiii	i Cilli	8	
Permitted Phases	8	8	0	
Detector Phase	8	8	8	
Switch Phase	U	U	0	
Minimum Initial (s)	10.0	10.0	10.0	
Minimum Split (s)	15.0	15.0	15.0	
Total Split (s)	16.0	16.0	16.0	
Total Split (%)	20.0%	20.0%	20.0%	
Yellow Time (s)	4.0	4.0	4.0	
All-Red Time (s)	1.0	1.0	1.0	
Lost Time Adjust (s)	1.0	1.0	0.0	
Total Lost Time (s)			5.0	
Lead/Lag			3.0	
Lead-Lag Optimize?				
Recall Mode	None	None	None	
Act Effct Green (s)	NOTIC	NOTIC	11.4	
Actuated g/C Ratio			0.26	
v/c Ratio			0.25	
Control Delay			21.6	
Queue Delay			0.0	
Total Delay			21.6	
LOS			21.0 C	
			21.6	
Approach Delay Approach LOS			21.6 C	
Queue Length 50th (ft)			18	
Queue Length 95th (ft)			78 1410	
Internal Link Dist (ft)			1410	
Turn Bay Length (ft)			201	
Base Capacity (vph)			391	
Starvation Cap Reductn			0	
Spillback Cap Reductn			0	
Storage Cap Reductn			0	
Reduced v/c Ratio			0.23	
Intersection Summary				

19: Route 138 & Atherton St. & Bradlee Rd.

Actuated Cycle Length: 43.5

Natural Cycle: 60

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.51

Intersection Signal Delay: 10.3

Intersection Capacity Utilization 79.3%

Analysis Period (min) 15

Splits and Phases: 19: Route 138 & Atherton St. & Bradlee Rd.



	•	-	\rightarrow	•	←	*	4	†	/	-	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	5	35	3	7	35	5	1	450	20	5	410	9
Future Volume (vph)	5	35	3	7	35	5	1	450	20	5	410	9
Satd. Flow (prot)	0	1581	0	0	1574	0	0	1587	0	0	1561	0
Flt Permitted								0.999			0.995	
Satd. Flow (perm)	0	1589	0	0	1587	0	0	1585	0	0	1554	0
Satd. Flow (RTOR)												
Confl. Peds. (#/hr)							2		1	1		2
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	3%	0%	0%	3%	0%	0%	3%	0%	0%	5%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	46	0	0	51	0	0	507	0	0	456	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	7.0	7.0		7.0	7.0		20.0	20.0		20.0	20.0	
Minimum Split (s)	12.0	12.0		12.0	12.0		45.0	45.0		45.0	45.0	
Total Split (s)	12.0	12.0		12.0	12.0		48.0	48.0		48.0	48.0	
Total Split (%)	15.0%	15.0%		15.0%	15.0%		60.0%	60.0%		60.0%	60.0%	
Yellow Time (s)	4.0	4.0		4.0	4.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		5.0			5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		None	None	
Act Effct Green (s)		8.4			8.4			31.2			31.2	
Actuated g/C Ratio		0.22			0.22			0.81			0.81	
v/c Ratio		0.13			0.15			0.40			0.36	
Control Delay		21.7			21.7			7.5			7.1	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		21.7			21.7			7.5			7.1	
LOS		С			С			Α			Α	
Approach Delay		21.7			21.7			7.5			7.1	
Approach LOS		С			С			Α			Α	
Queue Length 50th (ft)		4			5			0			0	
Queue Length 95th (ft)		53			57			267			233	
Internal Link Dist (ft)		939			647			711			3441	
Turn Bay Length (ft)												
Base Capacity (vph)		343			343			1439			1411	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.13			0.15			0.35			0.32	
Intersection Summary Cycle Length: 80												

Lane Group	Ø9	
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Satd. Flow (RTOR)		
Confl. Peds. (#/hr)		
Peak Hour Factor		
Heavy Vehicles (%)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	
Permitted Phases		
Detector Phase		
Switch Phase		
Minimum Initial (s)	2.0	
Minimum Split (s)	20.0	
Total Split (s)	20.0	
Total Split (%)	25%	
Yellow Time (s)	2.0	
All-Red Time (s)	0.0	
Lost Time Adjust (s)	0.0	
Total Lost Time (s)		
Lead/Lag		
Lead-Lag Optimize?		
Recall Mode	None	
Act Effct Green (s)	140110	
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (ft)		
Queue Length 95th (ft)		
Internal Link Dist (ft)		
Turn Bay Length (ft)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		

20: Route 138 & Robbins St.

Actuated Cycle Length: 38.7

Natural Cycle: 80

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.40

Intersection Signal Delay: 8.6

Intersection Capacity Utilization 43.9%

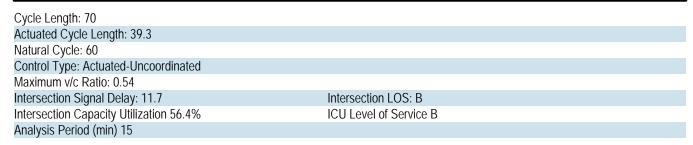
Analysis Period (min) 15

Splits and Phases: 20: Route 138 & Robbins St.



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	3	6	3	35	9	15	3	450	45	15	420	7
Future Volume (vph)	3	6	3	35	9	15	3	450	45	15	420	7
Satd. Flow (prot)	0	1439	0	0	1426	0	0	1375	0	0	1354	0
Flt Permitted		0.988			0.971			0.997			0.973	
Satd. Flow (perm)	0	1421	0	0	1423	0	0	1370	0	0	1320	0
Satd. Flow (RTOR)												
Confl. Peds. (#/hr)	6		1	1		6	3		5	5		3
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	0%	0%	33%	9%	0%	6%	0%	3%	4%	0%	6%	0%
Parking (#/hr)								5			5	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	12	0	0	63	0	0	530	0	0	470	0
Turn Type	Split	NA		Split	NA		Perm	NA		Perm	NA	
Protected Phases	4	4		8	8			2			6	
Permitted Phases							2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	4.0	4.0		4.0	4.0		6.0	6.0		6.0	6.0	
Minimum Split (s)	11.0	11.0		11.0	11.0		25.0	25.0		25.0	25.0	
Total Split (s)	11.0	11.0		15.0	15.0		44.0	44.0		44.0	44.0	
Total Split (%)	15.7%	15.7%		21.4%	21.4%		62.9%	62.9%		62.9%	62.9%	
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		7.0			7.0			7.0			7.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		None	None	
Act Effct Green (s)		5.6			8.9			28.2			28.2	
Actuated g/C Ratio		0.14			0.23			0.72			0.72	
v/c Ratio		0.06			0.20			0.54			0.50	
Control Delay		26.9			23.2			11.2			10.4	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		26.9			23.2			11.2			10.4	
LOS		С			С			В			В	
Approach Delay		26.9			23.2			11.2			10.4	
Approach LOS		С			С			В			В	
Queue Length 50th (ft)		3			14			94			79	
Queue Length 95th (ft)		21			61			287			243	
Internal Link Dist (ft)		838			877			3441			407	
Turn Bay Length (ft)												
Base Capacity (vph)		203			402			1131			1090	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.06			0.16			0.47			0.43	
Intersection Summary												

21: Route 138 & Cheever St./Blue Hill Terrace St.



Splits and Phases: 21: Route 138 & Cheever St./Blue Hill Terrace St.



	۶	→	•	•	←	•	1	†	/	/	ţ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	3	6	3	35	9	15	3	450	45	15	420	7
Future Volume (veh/h)	3	6	3	35	9	15	3	450	45	15	420	7
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	0	0	0	0	0	0	5	0	0	5	0
Ped-Bike Adj(A_pbT)	1.00		0.97	1.00		0.97	1.00		0.99	1.00		0.99
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1700	1570	1700	1700	1592	1700	1700	1649	1700	1700	1608	1700
Adj Flow Rate, veh/h	3	6	3	37	10	16	3	479	48	16	447	7
Adj No. of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	0	0	0	0	0	0	3	3	3	6	6	6
Cap, veh/h	5	9	5	42	11	18	82	691	66	92	721	11
Arrive On Green	0.01	0.01	0.01	0.05	0.05	0.05	0.46	0.46	0.46	0.46	0.46	0.46
Sat Flow, veh/h	368	736	368	862	233	373	2	1472	147	19	1539	24
Grp Volume(v), veh/h	12	0	0	63	0	0	530	0	0	470	0	0
Grp Sat Flow(s),veh/h/ln	1472	0	0	1468	0	0	1621	0	0	1582	0	0
Q Serve(g_s), s	0.4	0.0	0.0	1.9	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	0.4	0.0	0.0	1.9	0.0	0.0	11.5	0.0	0.0	9.8	0.0	0.0
Prop In Lane	0.25		0.25	0.59		0.25	0.01		0.09	0.03		0.01
Lane Grp Cap(c), veh/h	18	0	0	72	0	0	840	0	0	823	0	0
V/C Ratio(X)	0.66	0.00	0.00	0.88	0.00	0.00	0.63	0.00	0.00	0.57	0.00	0.00
Avail Cap(c_a), veh/h	134	0	0	266	0	0	1440	0	0	1399	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	22.0	0.0	0.0	21.1	0.0	0.0	9.6	0.0	0.0	9.2	0.0	0.0
Incr Delay (d2), s/veh	33.7	0.0	0.0	26.5	0.0	0.0	8.0	0.0	0.0	0.6	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.7	0.0	0.0	0.6	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.3	0.0	0.0	1.3	0.0	0.0	5.8	0.0	0.0	5.1	0.0	0.0
LnGrp Delay(d),s/veh	55.7	0.0	0.0	47.6	0.0	0.0	11.1	0.0	0.0	10.4	0.0	0.0
LnGrp LOS	Е			D			В			В		
Approach Vol, veh/h		12			63			530			470	
Approach Delay, s/veh		55.7			47.6			11.1			10.4	
Approach LOS		E			D			В			В	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		27.4		7.5		27.4		9.2				
Change Period (Y+Rc), s		7.0		7.0		7.0		7.0				
Max Green Setting (Gmax), s		37.0		4.0		37.0		8.0				
Max Q Clear Time (g_c+I1), s		13.5		2.4		11.8		3.9				
Green Ext Time (p_c), s		6.9		0.0		7.0		0.1				
Intersection Summary												
HCM 2010 Ctrl Delay			13.4									
HCM 2010 LOS			В									

22: Route 138 & Aberdeen Rd.

	•	•	•	†	Ţ	1
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W	2011	.,,,,	4	1	02.1
Traffic Volume (vph)	9	3	4	460	450	15
Future Volume (vph)	9	3	4	460	450	15
Satd. Flow (prot)	1414	0	0	1581	1576	0
Flt Permitted	0.963					
Satd. Flow (perm)	1414	0	0	1581	1576	0
Confl. Peds. (#/hr)	3	11	5			5
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	11%	0%	0%	4%	4%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	13	0	0	504	505	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utiliza				IC	U Level c	of Service A
Analysis Period (min) 15						

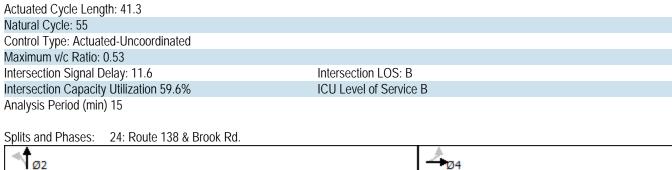
Intersection								
Int Delay, s/veh	0.4							
Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations	¥			4	ĵ.			
Traffic Vol, veh/h	9	3	4	460	450	15		
Future Vol, veh/h	9	3	4	460	450	15		
Conflicting Peds, #/hr		11	5	0	0	5		
Sign Control	Stop	Stop	Free	Free	Free	Free		
RT Channelized	-	None	-	None	-	None		
Storage Length	0	-	-	-	-	-		
Veh in Median Storag	je,# 0	-	-	0	0	-		
Grade, %	0	-	-	0	0	-		
Peak Hour Factor	92	92	92	92	92	92		
Heavy Vehicles, %	11	0	0	4	4	0		
Mvmt Flow	10	3	4	500	489	16		
Major/Minor	Minor2	Λ	/lajor1	I.	/lajor2			
Conflicting Flow All	1014	513	510	0	-	0		
Stage 1	502	-	-	-	-	-		
Stage 2	512		_	_	_	_		
Critical Hdwy	6.51	6.2	4.1	-	-	-		
Critical Hdwy Stg 1	5.51	-	-	-	-	-		
Critical Hdwy Stg 2	5.51	-	-	-	-	-		
Follow-up Hdwy	3.599	3.3	2.2	-	_	_		
Pot Cap-1 Maneuver	254	*109	*163	-	-	-		
Stage 1	590	-	-	-	-	-		
Stage 2	584	-	-	-	-	-		
Platoon blocked, %		1	1	-	-	-		
Mov Cap-1 Maneuver	243	*107	*162	-	-	-		
Mov Cap-2 Maneuver		-	-	-	-	-		
Stage 1	587	-	-	-	-	-		
Stage 2	562	-	-	-	-	-		
, and the second								
Approach	EB		NB		SB			
HCM Control Delay, s			0.2		0			
HCM LOS	D		0.2		U			
TIOWI LOG	U							
Minor Lane/Major Mvr	mt	NBL	NIDT	EBLn1	SBT	SBR		
	iiit				SDT	SDK		
Capacity (veh/h)		* 162	-	184	-	-		
HCM Cantral Dalay (c	.)	0.027		0.071	-	-		
HCM Long LOS	>)	27.8	0	26.1	-	-		
HCM Lane LOS	h)	D	Α	D	-	-		
HCM 95th %tile Q(veh	11)	0.1	-	0.2	-	-		
Notes								
~: Volume exceeds ca	anacity	¢. De	lav evo	eeds 30	nns	+. Comi	outation Not Defined	*: All major volume in platoor

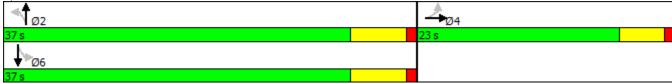
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	3	3	10	3	1	5	10	460	9	7	440	1
Future Volume (vph)	3	3	10	3	1	5	10	460	9	7	440	1
Satd. Flow (prot)	0	1412	0	0	1496	0	0	1592	0	0	1565	0
Flt Permitted		0.991			0.984			0.999			0.999	
Satd. Flow (perm)	0	1412	0	0	1496	0	0	1592	0	0	1565	0
Confl. Peds. (#/hr)	66					66	23		66	66		23
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	0%	0%	9%	0%	0%	0%	0%	3%	0%	0%	5%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	16	0	0	9	0	0	493	0	0	462	0
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilizat	ion 52.9%			IC	CU Level	of Service	Α					
Analysis Period (min) 15												

Intersection													
Int Delay, s/veh	1.2												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4			4			4			4		
Traffic Vol, veh/h	3	3	10	3	1	5	10	460	9	7	440	1	
Future Vol, veh/h	3	3	10	3	1	5	10	460	9	7	440	1	
Conflicting Peds, #/hr	66	0	0	0	0	66	23	0	66	66	0	23	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-	
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	97	97	97	97	97	97	97	97	97	97	97	97	
Heavy Vehicles, %	0	0	9	0	0	0	0	3	0	0	5	0	
Mvmt Flow	3	3	10	3	1	5	10	474	9	7	454	1	
Major/Minor	Minor2		N	Minor1		ı	Major1		ı	/lajor2			
		1062	477		1057	611	478	^		550	0	0	
Conflicting Flow All	1061			1040				0	0		0	0	
Stage 1	492	492	-	565	565	-	-	-	-	-	-	-	
Stage 2	569	570	- / 20	475	492	- / 2	- 11	-	-	- 11	-	-	
Critical Hdwy	7.1	6.5	6.29	7.1	6.5	6.2	4.1	-	-	4.1	-	-	
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-	
Follow-up Hdwy	3.5	4	3.381	3.5	4	3.3	2.2	-	-	2.2	-	-	
Pot Cap-1 Maneuver	203	225	*128	210	227	*131	*245	-	-	*245	-	-	
Stage 1	562	551	-	513	511	-	-	-	-	-	-	-	
Stage 2	511	509	-	574	551	-	-	-	-	-	-	-	
Platoon blocked, %	4.5	100	1	4//	100	1	1	-	-	1	-	-	
Mov Cap-1 Maneuver	165	188	*125	166	190	*116	*245	-	-	*231	-	-	
Mov Cap-2 Maneuver	165	188	-	166	190	-	-	-	-	-	-	-	
Stage 1	520	518	-	456	455	-	-	-	-	-	-	-	
Stage 2	433	453	-	502	518	-	-	-	-	-	-	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s	34.1			33.6			0.4			0.3			
HCM LOS	D			D									
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1V	VBI n1	SBL	SBT	SBR				
Capacity (veh/h)		* 245		, tort	140	135	* 231	- 051	ODIT				
HCM Lane V/C Ratio		0.042	-			0.069		-	-				
HCM Control Delay (s)		20.3	0	-	34.1	33.6	21.1	0	-				
HCM Lane LOS		20.3 C	A	-	34.1 D	33.0 D	21.1 C	A	-				
HCM 95th %tile Q(veh)	0.1	- A	-	0.4	0.2	0.1	- A	-				
	1	0.1	_		0.4	0.2	U. I						
Notes													
~: Volume exceeds ca	pacity	\$: De	elay exc	eeds 3	00s	+: Com	putatior	Not De	efined	*: All	major v	olume i	in platoon

	•	→	•	€	+	•	•	†	~	/	+	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4						4			4	
Traffic Volume (vph)	5	160	9	0	0	0	2	410	40	10	450	20
Future Volume (vph)	5	160	9	0	0	0	2	410	40	10	450	20
Satd. Flow (prot)	0	1562	0	0	0	0	0	1577	0	0	1549	0
Flt Permitted		0.999						0.998			0.987	
Satd. Flow (perm)	0	1562	0	0	0	0	0	1574	0	0	1530	0
Satd. Flow (RTOR)												
Confl. Peds. (#/hr)	4		4	2		2	2		2	1		6
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	0%	4%	11%	2%	2%	2%	0%	3%	0%	17%	5%	5%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	181	0	0	0	0	0	471	0	0	500	0
Turn Type	Perm	NA					Perm	NA		Perm	NA	
Protected Phases		4						2			6	
Permitted Phases	4						2			6		
Detector Phase	4	4					2	2		6	6	
Switch Phase												
Minimum Initial (s)	7.0	7.0					24.0	24.0		24.0	24.0	
Minimum Split (s)	23.0	23.0					30.0	30.0		30.0	30.0	
Total Split (s)	23.0	23.0					37.0	37.0		37.0	37.0	
Total Split (%)	38.3%	38.3%					61.7%	61.7%		61.7%	61.7%	
Yellow Time (s)	4.0	4.0					5.0	5.0		5.0	5.0	
All-Red Time (s)	1.0	1.0					1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0						0.0			0.0	
Total Lost Time (s)		5.0						6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None					None	None		None	None	
Act Effct Green (s)	110110	14.0						25.6		110110	25.6	
Actuated g/C Ratio		0.34						0.62			0.62	
v/c Ratio		0.34						0.48			0.53	
Control Delay		17.9						10.1			10.9	
Queue Delay		0.0						0.0			0.0	
Total Delay		17.9						10.1			10.9	
LOS		В						В			В	
Approach Delay		17.9						10.1			10.9	
Approach LOS		В						В			В	
Queue Length 50th (ft)		41						72			79	
Queue Length 95th (ft)		99						188			210	
Internal Link Dist (ft)		967			1001			519			1021	
Turn Bay Length (ft)		701			1001			017			1021	
Base Capacity (vph)		777						1121			1089	
Starvation Cap Reductn		0						0			0	
Spillback Cap Reductn		0						0			0	
Storage Cap Reductn		0						0			0	
Reduced v/c Ratio		0.23						0.42			0.46	
Intersection Summary												
Cycle Length: 60												

24: Route 138 & Brook Rd.





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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4						4			4	
Traffic Volume (veh/h)	5	160	9	0	0	0	2	410	40	10	450	20
Future Volume (veh/h)	5	160	9	0	0	0	2	410	40	10	450	20
Number	7	4	14				5	2	12	1	6	16
Initial Q (Qb), veh	0	10	0				0	15	0	0	15	0
Ped-Bike Adj(A_pbT)	1.00		0.99				1.00		1.00	1.00		0.99
Parking Bus, Adj	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1700	1631	1700				1700	1655	1700	1700	1615	1700
Adj Flow Rate, veh/h	5	167	9				2	427	42	10	469	21
Adj No. of Lanes	0	1	0				0	1	0	0	1	0
Peak Hour Factor	0.96	0.96	0.96				0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	0	4	0				3	3	3	5	5	5
Cap, veh/h	6	268	11				84	853	75	89	870	35
Arrive On Green	0.16	0.16	0.16				0.58	0.58	0.58	0.58	0.58	0.58
Sat Flow, veh/h	45	1489	80				1	1482	145	9	1518	67
Grp Volume(v), veh/h	181	0	0				471	0	0	500	0	0
Grp Sat Flow(s), veh/h/ln	1613	0	0				1628	0	0	1594	0	0
Q Serve(g_s), s	4.4	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	4.4	0.0	0.0				7.1	0.0	0.0	7.9	0.0	0.0
Prop In Lane	0.03	0	0.05				0.00	0	0.09	0.02	0	0.04
Lane Grp Cap(c), veh/h	283	0	0				1013	0	0	992	0	0
V/C Ratio(X)	0.64	0.00	0.00				0.47	0.00	0.00	0.50	0.00	0.00
Avail Cap(c_a), veh/h	700	1.00	1.00				1302	1.00	1.00	1275	1.00	1.00
HCM Platoon Ratio	1.00	1.00	1.00 0.00				1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00 17.2	0.00	0.00				1.00 6.6	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh Incr Delay (d2), s/veh	2.4	0.0	0.0				0.0	0.0	0.0	0.4	0.0	0.0
Initial Q Delay(d3),s/veh	24.8	0.0	0.0				3.0	0.0	0.0	3.3	0.0	0.0
%ile BackOfQ(50%),veh/ln	4.9	0.0	0.0				5.7	0.0	0.0	6.2	0.0	0.0
LnGrp Delay(d),s/veh	44.4	0.0	0.0				9.9	0.0	0.0	10.6	0.0	0.0
LnGrp LOS	D	0.0	0.0				7.7 A	0.0	0.0	В	0.0	0.0
Approach Vol, veh/h	<u> </u>	181						471		<u> </u>	500	
Approach Delay, s/veh		44.4						9.9			10.6	
Approach LOS		44.4 D						7.7 A			В	
• •											Ь	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6						
Phs Duration (G+Y+Rc), s		30.0		11.5		30.0						
Change Period (Y+Rc), s		6.0		5.0		6.0						
Max Green Setting (Gmax), s		31.0		18.0		31.0						
Max Q Clear Time (g_c+I1), s Green Ext Time (p_c), s		9.1 6.4		6.4 0.7		9.9 6.3						
		0.4		0.7		0.3						
Intersection Summary			45 '									
HCM 2010 Ctrl Delay			15.6									
HCM 2010 LOS			В									

102: Route 138 & Concord Baptist Church

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		₽			4
Traffic Volume (vph)	1	1	460	1	5	450
Future Volume (vph)	1	1	460	1	5	450
Satd. Flow (prot)	1495	0	1642	0	0	1642
Flt Permitted	0.976					0.999
Satd. Flow (perm)	1495	0	1642	0	0	1642
Confl. Peds. (#/hr)	3			11	11	
Peak Hour Factor	0.25	0.25	0.92	0.25	0.63	0.88
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	8	0	504	0	0	519
Sign Control	Stop		Free			Free
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utiliza	tion 40.9%			IC	U Level	of Service
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	0.2					
		MED	NET	NES	051	ODT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		\$		_	. ની
Traffic Vol, veh/h	1	1	460	1	5	450
Future Vol, veh/h	1	1	460	1	5	450
Conflicting Peds, #/hr	3	0	0	11	11	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	25	25	92	25	63	88
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	4	4	500	4	8	511
N A = ' =/N A' - =	\ A!1		4-!1		M-!	
	Minor1		/lajor1		Major2	
Conflicting Flow All	1043	513	0	0	515	0
Stage 1	513	-	-	-	-	-
Stage 2	530	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	256	565	-	-	1061	-
Stage 1	605	-	-	-	-	-
Stage 2	594	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	250	560	-	-	1061	-
Mov Cap-2 Maneuver	250	-	-	_	-	-
Stage 1	599	-	-	-	-	-
Stage 2	586	-	_	_	_	_
5.ago 2	500					
A	MD		ND		CD	
Approach	WB		NB		SB	
HCM Control Delay, s	15.7		0		0.1	
HCM LOS	С					
Minor Lane/Major Mvm	n†	NBT	NRR\	VBLn1	SBL	SBT
	IL.				1061	
Capacity (veh/h) HCM Lane V/C Ratio		-	-	346 0.023		-
		-				-
HCM Long LOS		-	-	15.7	8.4	0
HCM Lane LOS		-	-	C	A	Α
HCM 95th %tile Q(veh))	-	-	0.1	0	-

8: Route 138 & South Parking Lot Entrance

	•	*	†	~	\	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations			f)			4
Traffic Volume (vph)	0	0	1210	50	8	1260
Future Volume (vph)	0	0	1210	50	8	1260
Satd. Flow (prot)	0	0	1791	0	0	1819
Flt Permitted						
Satd. Flow (perm)	0	0	1791	0	0	1819
Confl. Peds. (#/hr)				6	6	
Peak Hour Factor	0.92	0.92	0.91	0.78	0.67	0.97
Heavy Vehicles (%)	2%	2%	2%	0%	0%	1%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	1394	0	0	1311
Sign Control	Stop		Free			Free
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utilizat	tion 76 0%			IC	III ovol o	of Service
Analysis Period (min) 15	1011 /0.0%			10	o rever	ii Selvice
Analysis Feriou (IIIII) 13						

9: Route 138 & South Parking Lot Exit

	•	•	†	/	>	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	M		†			†
Traffic Volume (vph)	10	70	1200	0	0	1260
Future Volume (vph)	10	70	1200	0	0	1260
Satd. Flow (prot)	1610	0	1818	0	0	1818
Flt Permitted	0.994					
Satd. Flow (perm)	1610	0	1818	0	0	1818
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	0%	1%	2%	2%	1%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	86	0	1290	0	0	1355
Sign Control	Stop		Free			Free
Intersection Summary						

Intersection Summary

Control Type: Unsignalized

Intersection Capacity Utilization 77.9%

ICU Level of Service D

Analysis Period (min) 15

3.4					
	MED	Not	NDD	001	ODT
	WBR		NRK	SBL	SBT
					↑
					1260
					1260
					0
Stop		Free		Free	Free
-	None	-	None	-	None
0	-	-	-	-	-
e,# 0	-	0	-	-	0
0	-	0	-	-	0
93	93	93	93	93	93
0	0	1	2	2	1
11	75	1290	0	0	1355
Minor1	Λ.	Noior1	Λ.	//oior?	
			IV		
			-	-	-
			-	-	-
		-	-	-	-
	6.2	-	-	-	-
	-	-	-	-	-
	-	-	-	-	-
		-	-	-	-
	202	-	0	0	-
	-	-	0	0	-
242	-	-	0	0	-
		-			-
26	202	-	-	-	-
26	-	-	-	-	-
261	-	-	-	-	-
242	-	-	-	-	-
WR		NB		SB	
		0		0	
				U	
108.8		U			
108.8 F		0			
F					
	NBTV	VBLn1	SBT		
F	-	VBLn1 109	SBT -		
F	-	VBLn1			
F	-	VBLn1 109	-		
F nt	-	VBLn1 109 0.789	-		
	0 e, # 0 0 93 0 111 Minor1 2645 1290 1355 6.4 5.4 3.5 26 261 242 26 26 261	WBL WBR 10 70 10 70 10 70 0 0 0 Stop Stop - None 0 e, # 0 93 93 0 0 11 75 Minor1 N 2645 1290 1290 1355 6.4 6.2 5.4 5.4 3.5 3.3 26 202 261 242 26 202 26 242	WBL WBR NBT 10 70 1200 10 70 1200 0 0 0 Stop Stop Free None - 0 0 - 0 93 93 93 0 0 1 11 75 1290 Minor1 Major1 2645 1290 0 1290 - - 1355 - - 6.4 6.2 - 5.4 - - 3.5 3.3 - 26 202 - 261 - - 26 202 - 26 - - 26 202 - 26 - - 26 - - 26 - - 26 - -	WBL WBR NBT NBR 10 70 1200 0 10 70 1200 0 0 0 0 0 Stop Stop Free Free - None - None 0 - - - 0 - 0 - 93 93 93 93 0 0 1 2 11 75 1290 0 1290 - - - 1355 - - - 5.4 - - - 5.4 - - - 5.4 - - - 5.4 - - - 5.4 - - - 26 202 - 0 242 - - 0 26 202 - <	WBL WBR NBT NBR SBL V Image: Control of the control of th

10: Route 138 & North Parking Lot Entrance

	•	•	†	<i>></i>	\	Ţ
Lane Group	WBL	WBR	NBT	• NBR	SBL	SBT
Lane Configurations			ĵ₃			4
Traffic Volume (vph)	0	0	1230	25	25	1250
Future Volume (vph)	0	0	1230	25	25	1250
Satd. Flow (prot)	0	0	1810	0	0	1817
Flt Permitted						0.999
Satd. Flow (perm)	0	0	1810	0	0	1817
Confl. Peds. (#/hr)				1	1	
Peak Hour Factor	0.92	0.92	0.89	0.50	0.72	0.96
Heavy Vehicles (%)	2%	2%	1%	0%	0%	1%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	1432	0	0	1337
Sign Control	Stop		Free			Free
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utilizat	ion 89.2%			IC	U Level	of Service
Analysis Period (min) 15						

	٠	→	*	•	←	•	1	<u></u>	/	/		4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			f)	
Traffic Volume (vph)	5	0	10	6	0	70	4	1220	0	0	1260	7
Future Volume (vph)	5	0	10	6	0	70	4	1220	0	0	1260	7
Satd. Flow (prot)	0	1645	0	0	1601	0	0	1819	0	0	1817	0
Flt Permitted		0.984			0.996							
Satd. Flow (perm)	0	1645	0	0	1601	0	0	1819	0	0	1817	0
Confl. Peds. (#/hr)	5					5						
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	0%	2%	0%	0%	2%	0%	0%	1%	0%	2%	1%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	15	0	0	78	0	0	1262	0	0	1306	0
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilizat	ion 80.6%			IC	:U Level	of Service	D					
Analysis Period (min) 15												

Intersection												
Int Delay, s/veh	2.7											
										001		000
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			र्स			Þ	
Traffic Vol, veh/h	5	0	10	6	0	70	4	1220	0	0	1260	7
Future Vol, veh/h	5	0	10	6	0	70	4	1220	0	0	1260	7
Conflicting Peds, #/hr	5	0	0	0	0	5	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	97	97	97	97	97	97	97	97	97	97	97	97
Heavy Vehicles, %	0	2	0	0	2	0	0	1	0	2	1	0
Mvmt Flow	5	0	10	6	0	72	4	1258	0	0	1299	7
Major/Minor	Minor2		N	Minor1		N	Major1		N	/lajor2		
Conflicting Flow All	2610	2569	1303	2574	2572	1263	1306	0		najorz		0
Stage 1	1303	1303	1303	1266	1266	1203	1300	U	-	-	-	U
Stage 2	1303	1266	-	1308	1306	-	-	-	-	-	-	•
	6.5	6.52	6.2	6.5	10	6.2	4.1	-	-	-	-	-
Critical Hdwy		5.52	0.2	6.1	5.52	0.2	4.1	-	-	-	-	-
Critical Hdwy Stg 1	6.1							-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.52	- 2 2	6.1	5.52	- 2 2	- 2.2	-	-	-	-	-
Follow-up Hdwy	3.5	4.018	3.3	3.5	4.018	3.3	2.2	-	-	-	-	-
Pot Cap-1 Maneuver	25	26	198	27	2	209	537	-	0	0	_	-
Stage 1	199	231	-	209	240	-	-	-	0	0	-	-
Stage 2	198	240	-	198	230	-	-	-	0	0	-	-
Platoon blocked, %	4.1		400	0.5	^	000	F07	-			-	-
Mov Cap-1 Maneuver	16	25	198	25	2	208	537	-	-	-	-	-
Mov Cap-2 Maneuver	16	25	-	25	2	-	-	-	-	-	-	-
Stage 1	194	231	-	204	234	-	-	-	-	-	-	-
Stage 2	126	234	-	188	230	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	138.5			65.9			0			0		
HCM LOS	F			F								
Minor Long /Maior M		NDI	NDT	TDL 41	MDI 1	CDT	CDD					
Minor Lane/Major Mvn	11	NBL		EBLn1\		SBT	SBR					
Capacity (veh/h)		537	-		132	-	-					
HCM Lane V/C Ratio		0.008		0.377		-	-					
HCM Control Delay (s)		11.8	0	138.5	65.9	-	-					
HCM Lane LOS		В	Α	F	F	-	-					
HCM 95th %tile Q(veh)	0	-	1.3	3	-	-					

	•	•	€_	†	1	/	(v	>	ļ	4	t	
Lane Group	WBL	WBR	WBR2	NBT	NBR	NBR2	SBL2	SBL	SBT	SWL	SWR	
Lane Configurations	W			f)					4			
Traffic Volume (vph)	0	1	1	850	450	9	2	0	1260	0	0	
Future Volume (vph)	0	1	1	850	450	9	2	0	1260	0	0	
Satd. Flow (prot)	1589	0	0	1722	0	0	0	0	1819	0	0	
Flt Permitted												
Satd. Flow (perm)	1589	0	0	1722	0	0	0	0	1819	0	0	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	
Heavy Vehicles (%)	0%	0%	0%	2%	1%	0%	0%	2%	1%	2%	2%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	2	0	0	1349	0	0	0	0	1301	0	0	
Sign Control	Stop			Free					Free	Stop		

Intersection Summary

Control Type: Unsignalized

Intersection Capacity Utilization 82.7%

ICU Level of Service E

Analysis Period (min) 15

Intersection								
Int Delay, s/veh	0							
Movement	WBL	WBR	NBT	NBR	SBL	SBT	SWL	SWR
Lane Configurations	¥		₽			4		
Traffic Vol, veh/h	0	1	850	450	0	1260	0	0
Future Vol, veh/h	0	1	850	450	0	1260	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	-	-	-	-	None	-	None
Storage Length	0	-	-	-	-	-	-	-
Veh in Median Storage,	# 0	-	0	-	-	0	-	-
Grade, %	0	-	0	-	-	0	0	-
Peak Hour Factor	97	97	97	97	97	97	97	97
Heavy Vehicles, %	0	0	2	1	2	1	2	2
Mvmt Flow	0	1	876	464	0	1299	0	0
		•	0,0	.01		,,		
	1inor1		/lajor1		/lajor2			
Conflicting Flow All	0	0	0	0	473	0		
Stage 1	0	-	-	-	-	-		
Stage 2	0	-	-	-	-	-		
Critical Hdwy	-	-	-	-	-	-		
Critical Hdwy Stg 1	-	-	-	-	-	-		
Critical Hdwy Stg 2	-	-	-	-	-	-		
Follow-up Hdwy	-	_	-	-	-	_		
Pot Cap-1 Maneuver	-	-	-	-	-	-		
Stage 1		_	_	_	_	_		
Stage 2	_	_	-	_	-	_		
Platoon blocked, %			_	_				
Mov Cap-1 Maneuver	-	_	-	-	_	-		
Mov Cap-2 Maneuver	-	_	-	-	-	-		
Stage 1	-	-	-	-	-	-		
	-	-	-	-	-	-		
Stage 2	-	-	-	-	-	-		
Approach	WB		NB		SB			
HCM Control Delay, s								
HCM LOS	-							
Minor Lane/Major Mvmt		NBT	NBR	NBR2W	/BLn1	SBL2	SBL	SBT
Capacity (veh/h)		-	-	-	-	-	-	-
HCM Lane V/C Ratio		-	-	-	-	-	-	-
HCM Control Delay (s)		-	-	-	-	-	-	-
HCM Lane LOS		-	-	-	-	-	-	-
HCM 95th %tile Q(veh)		-	-	-	-	-	-	-
, , c 2(1011)								

13: Route 138 & Thacher Montessori School

	۶	\rightarrow	4	†	ļ	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	N/			4	f.	
Traffic Volume (vph)	0	6	6	850	1270	0
Future Volume (vph)	0	6	6	850	1270	0
Satd. Flow (prot)	1589	0	0	1801	1818	0
Flt Permitted						
Satd. Flow (perm)	1589	0	0	1801	1818	0
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	2%	0%	0%	2%	1%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	6	0	0	891	1323	0
Sign Control	Stop			Free	Free	
Intersection Summary						

Control Type: Unsignalized Intersection Capacity Utilization 76.8% Analysis Period (min) 15

ICU Level of Service D

Intersection						
Int Delay, s/veh	0.1					
		EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			स	₽	
Traffic Vol, veh/h	0	6	6	850	1270	0
Future Vol, veh/h	0	6	6	850	1270	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e,# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	96	96	96	96	96	96
Heavy Vehicles, %	2	0	0	2	1	0
Mvmt Flow	0	6	6	885	1323	0
						-
		_				
	Minor2		/lajor1		/lajor2	
Conflicting Flow All	2221	1323	1323	0	-	0
Stage 1	1323	-	-	-	-	-
Stage 2	898	-	-	-	-	-
Critical Hdwy	6.42	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	48	193	529	-	-	-
Stage 1	249	-	-	-	-	-
Stage 2	398	-	_	-	_	-
Platoon blocked, %	0.0				_	_
Mov Cap-1 Maneuver	47	193	529	_	_	_
Mov Cap-2 Maneuver	47	1/3	JZ /	_	_	_
Stage 1	249	-	-	-		-
	389	-	-	-	-	-
Stage 2	369	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	24.3		0.1		0	
HCM LOS	С					
NA' 1 /NA ' NA		NIDI	NDT	EDL 4	CDT	CDD
Minor Lane/Major Mvr	nt	NBL	MBT	EBLn1	SBT	SBR
Capacity (veh/h)		529	-	. , .	-	-
HCM Lane V/C Ratio		0.012		0.032	-	-
HCM Control Delay (s)	11.9	0	24.3	-	-
HCM Lane LOS		В	Α	С	-	-
HCM 95th %tile Q(veh	1)	0	-	0.1	-	-

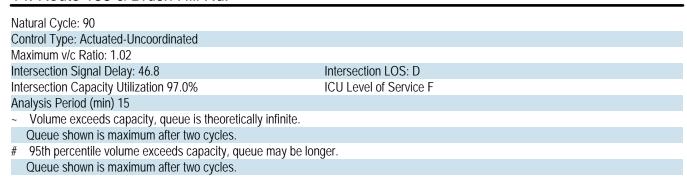
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		ሻ	ĥ			1			ĵ.	
Traffic Volume (vph)	5	0	330	170	120	0	0	850	0	0	770	5
Future Volume (vph)	5	0	330	170	120	0	0	850	0	0	770	5
Satd. Flow (prot)	0	1423	0	1546	1643	0	0	1611	0	0	1610	0
Flt Permitted		0.997		0.395								
Satd. Flow (perm) Satd. Flow (RTOR)	0	1420	0	643	1643	0	0	1611	0	0	1610	0
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	0%	2%	0%	1%	0%	13%	0%	2%	2%	2%	2%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	352	0	179	126	0	0	895	0	0	816	0
Turn Type	Perm	NA		Perm	NA			NA			NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8								
Detector Phase	4	4		8	8			2			6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0			10.0			10.0	
Minimum Split (s)	20.0	20.0		20.0	20.0			45.0			45.0	
Total Split (s)	30.0	30.0		30.0	30.0			45.0			45.0	
Total Split (%)	40.0%	40.0%		40.0%	40.0%			60.0%			60.0%	
Yellow Time (s)	4.0	4.0		4.0	4.0			5.0			5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0			1.0			1.0	
Lost Time Adjust (s)		0.0		0.0	0.0			0.0			0.0	
Total Lost Time (s)		5.0		5.0	5.0			6.0			6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None			None			None	
Act Effct Green (s)		21.8		21.8	21.8			39.1			39.1	
Actuated g/C Ratio		0.30		0.30	0.30			0.54			0.54	
v/c Ratio		0.82		0.92	0.25			1.02			0.93	
Control Delay		40.1		74.4	20.0			56.9			36.7	
Queue Delay		0.0		0.0	0.0			0.0			0.0	
Total Delay		40.1		74.4	20.0			56.9			36.7	
LOS		D		Ε	B			E			D	
Approach Delay		40.1			51.9			56.9			36.7	
Approach LOS		D		7.	D			E			D	
Queue Length 50th (ft)		143		76	42			~470			345	
Queue Length 95th (ft)		#269		#188	81			#684			#601	
Internal Link Dist (ft)		906			198			187			2213	
Turn Bay Length (ft)		405		224	F72			075			075	
Base Capacity (vph)		495		224	572			875			875	
Starvation Cap Reductn		0		0	0			0			0	
Spillback Cap Reductn		0		0	0			0			0	
Storage Cap Reductn		0		0	0			1.02			0 02	
Reduced v/c Ratio		0.71		0.80	0.22			1.02			0.93	
Intersection Summary												

Intersection Summary

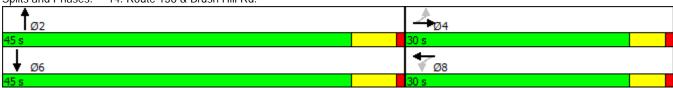
Cycle Length: 75

Actuated Cycle Length: 72

14: Route 138 & Brush Hill Rd.



Splits and Phases: 14: Route 138 & Brush Hill Rd.



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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		ň	f)			†			£	
Traffic Volume (veh/h)	5	0	330	170	120	0	0	850	0	0	770	5
Future Volume (veh/h)	5	0	330	170	120	0	0	850	0	0	770	5
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	20	0	10	10	0	0	10	0	0	40	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1700	1700	1700	1683	1700	1700	0	1667	0	0	1667	1700
Adj Flow Rate, veh/h	5	0	347	179	126	0	0	895	0	0	811	5
Adj No. of Lanes	0	1	0	1	1	0	0	1	0	0	1	0
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	2	2	2	1	0	0	0	2	0	0	2	2
Cap, veh/h	49	5	372	180	567	0	0	867	0	0	863	2
Arrive On Green	0.33	0.00	0.33	0.33	0.33	0.00	0.00	0.52	0.00	0.00	0.52	0.52
Sat Flow, veh/h	5	15	1426	982	1700	0	0	1667	0	0	1655	10
Grp Volume(v), veh/h	352	0	0	179	126	0	0	895	0	0	0	816
Grp Sat Flow(s),veh/h/ln	1446	0	0	982	1700	0	0	1667	0	0	0	1665
Q Serve(g_s), s	0.0	0.0	0.0	7.0	4.0	0.0	0.0	39.0	0.0	0.0	0.0	34.6
Cycle Q Clear(g_c), s	16.1	0.0	0.0	23.0	4.0	0.0	0.0	39.0	0.0	0.0	0.0	34.6
Prop In Lane	0.01		0.99	1.00		0.00	0.00		0.00	0.00		0.01
Lane Grp Cap(c), veh/h	402	0	0	180	567	0	0	867	0	0	0	866
V/C Ratio(X)	0.87	0.00	0.00	0.99	0.22	0.00	0.00	1.03	0.00	0.00	0.00	0.94
Avail Cap(c_a), veh/h	531	0	0	296	567	0	0	867	0	0	0	866
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	0.00	1.00	1.00	0.00	0.00	1.00	0.00	0.00	0.00	1.00
Uniform Delay (d), s/veh	22.3	0.0	0.0	31.7	18.6	0.0	0.0	18.0	0.0	0.0	0.0	18.0
Incr Delay (d2), s/veh	12.1	0.0	0.0	39.7	0.2	0.0	0.0	39.3	0.0	0.0	0.0	18.1
Initial Q Delay(d3),s/veh	122.6	0.0	0.0	196.5	2.9	0.0	0.0	39.3	0.0	0.0	0.0	140.1
%ile BackOfQ(50%),veh/ln	21.4	0.0	0.0	15.5	3.2	0.0	0.0	37.0	0.0	0.0	0.0	55.6
LnGrp Delay(d),s/veh	157.0	0.0	0.0	267.9	21.7	0.0	0.0	96.6	0.0	0.0	0.0	176.2
LnGrp LOS	F			F	С			F				F
Approach Vol, veh/h		352			305			895			816	
Approach Delay, s/veh		157.0			166.2			96.6			176.2	
Approach LOS		F			F			F			F	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		45.0		30.0		45.0		30.0				
Change Period (Y+Rc), s		6.0		5.0		6.0		5.0				
Max Green Setting (Gmax), s		39.0		25.0		39.0		25.0				
Max Q Clear Time (g_c+I1), s		41.0		18.1		36.6		25.0				
Green Ext Time (p_c), s		0.0		2.3		2.0		0.0				
Intersection Summary												
HCM 2010 Ctrl Delay			142.0									
HCM 2010 LOS			F									

	۶	\rightarrow	4	†	ļ	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations			ሻ	^		7
Traffic Volume (vph)	0	0	170	280	0	140
Future Volume (vph)	0	0	170	280	0	140
Satd. Flow (prot)	0	0	1745	1818	0	1573
Flt Permitted			0.950			
Satd. Flow (perm)	0	0	1745	1818	0	1573
Peak Hour Factor	0.92	0.92	0.86	0.79	0.92	0.94
Heavy Vehicles (%)	2%	2%	0%	1%	2%	1%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	198	354	0	149
Sign Control	Free			Free	Free	
Intersection Summary						

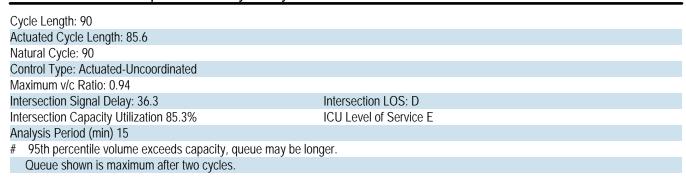
Control Type: Unsignalized Intersection Capacity Utilization 24.8% Analysis Period (min) 15

ICU Level of Service A

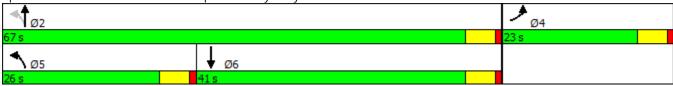
16: Route 138 & Neponset Valley Pkwy.

	۶	•	4	†	↓	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W		ሻ	<u> </u>	7	
Traffic Volume (vph)	4	200	390	490	570	1
Future Volume (vph)	4	200	390	490	570	1
Satd. Flow (prot)	1312	0	1501	1627	1611	0
Flt Permitted	0.999	U	0.150	1021	1011	U
Satd. Flow (perm)	1312	0	237	1627	1611	0
Satd. Flow (RTOR)	1312	U	231	1027	1011	0
Confl. Peds. (#/hr)		10				10
Confl. Bikes (#/hr)		10				10
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	0.97	2%	4%	1%	2%	0.97
Shared Lane Traffic (%)	U /0	∠ /0	4 /0	1 /0	∠ /0	U /0
Lane Group Flow (vph)	210	0	402	505	589	0
	Prot	U		NA	NA	U
Turn Type			pm+pt			
Protected Phases	4		5	2	6	
Permitted Phases	4		2	^	,	
Detector Phase	4		5	2	6	
Switch Phase	F 0		F 0	F 0	F 0	
Minimum Initial (s)	5.0		5.0	5.0	5.0	
Minimum Split (s)	23.0		10.0	23.0	23.0	
Total Split (s)	23.0		26.0	67.0	41.0	
Total Split (%)	25.6%		28.9%	74.4%	45.6%	
Yellow Time (s)	4.0		4.0	4.0	4.0	
All-Red Time (s)	1.0		1.0	1.0	1.0	
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	
Total Lost Time (s)	5.0		5.0	5.0	5.0	
Lead/Lag			Lead		Lag	
Lead-Lag Optimize?			Yes		Yes	
Recall Mode	None		None	None	None	
Act Effct Green (s)	16.4		59.1	59.1	33.3	
Actuated g/C Ratio	0.19		0.69	0.69	0.39	
v/c Ratio	0.84		0.86	0.45	0.94	
Control Delay	62.6		37.3	7.7	50.8	
Queue Delay	0.0		0.0	0.0	0.0	
Total Delay	62.6		37.3	7.7	50.8	
LOS	E		D	Α	D	
Approach Delay	62.6			20.8	50.8	
Approach LOS	62.6 E			C C	D	
Queue Length 50th (ft)	115		148	112	310	
Queue Length 95th (ft)	#232		#319	172	#520	
Internal Link Dist (ft)	1200		IF J 17	2213	1154	
Turn Bay Length (ft)	1200		200	2213	1104	
	279		477	1191	685	
Base Capacity (vph) Starvation Cap Reductn	0		0	0	080	
Spillback Cap Reductn	0		0	0	0	
Storage Cap Reductn	0		0 04	0 42	0	
Reduced v/c Ratio	0.75		0.84	0.42	0.86	
Intersection Summary						

16: Route 138 & Neponset Valley Pkwy.



Splits and Phases: 16: Route 138 & Neponset Valley Pkwy.

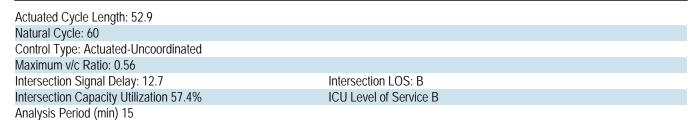


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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W		ħ	<u>↑</u>	7	UDIK
Traffic Volume (veh/h)	4	200	390	490	570	1
Future Volume (veh/h)	4	200	390	490	570	1
Number	7	14	5	2	6	16
Initial Q (Qb), veh	5	0	10	15	15	0
Ped-Bike Adj(A_pbT)	1.00	0.94	1.00	10	10	0.96
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1667	1700	1635	1683	1667	1700
Adj Flow Rate, veh/h	4	206	402	505	588	1700
Adj No. of Lanes	0	0	402	1	ງoo 1	0
			0.97	0.97	0.97	
Peak Hour Factor	0.97	0.97				0.97
Percent Heavy Veh, %	0	0	4	1117	2	2
Cap, veh/h	24	236	446	1127	684	1
Arrive On Green	0.19	0.19	0.17	0.67	0.42	0.42
Sat Flow, veh/h	25	1299	1557	1683	1663	3
Grp Volume(v), veh/h	211	0	402	505	0	589
Grp Sat Flow(s),veh/h/ln	1331	0	1557	1683	0	1666
Q Serve(g_s), s	10.7	0.0	9.2	10.0	0.0	22.1
Cycle Q Clear(g_c), s	10.7	0.0	9.2	10.0	0.0	22.1
Prop In Lane	0.02	0.98	1.00			0.00
Lane Grp Cap(c), veh/h	261	0	446	1127	0	679
V/C Ratio(X)	0.81	0.00	0.90	0.45	0.00	0.87
Avail Cap(c_a), veh/h	342	0	652	1489	0	856
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	31.9	0.0	25.8	6.3	0.0	24.5
Incr Delay (d2), s/veh	10.4	0.0	11.6	0.3	0.0	7.8
Initial Q Delay(d3),s/veh	0.0	0.0	36.6	2.3	0.0	26.5
%ile BackOfQ(50%),veh/ln	5.8	0.0	16.3	7.6	0.0	21.2
LnGrp Delay(d),s/veh	42.3	0.0	74.1	8.9	0.0	58.9
LnGrp LOS	42.3 D	0.0	74.1 E	Α	0.0	50.7 E
	211		<u> </u>	907	589	<u> </u>
Approach Vol, veh/h						
Approach LOS	42.3			37.8	58.9	
Approach LOS	D			D	Е	
Timer	1	2	3	4	5	6
Assigned Phs		2		4	5	6
Phs Duration (G+Y+Rc), s		51.8		18.3	17.1	34.7
Change Period (Y+Rc), s		5.0		5.0	5.0	5.0
Max Green Setting (Gmax), s		62.0		18.0	21.0	36.0
Max Q Clear Time (q_c+l1), s		12.0		12.7	11.2	24.1
Green Ext Time (p_c), s		9.5		0.3	0.9	5.6
		7.5		0.3	U.7	5.0
Intersection Summary						
HCM 2010 Ctrl Delay			45.6			
HCM 2010 LOS			D			
Notes						

4 t **EBT EBR WBL WBT NBL NBT** NBR **SBL** Lane Group **EBL WBR SBT SBR** Lane Configurations 4 4 Ъ 4 Traffic Volume (vph) 30 100 8 15 70 15 460 9 540 25 20 Future Volume (vph) 30 100 8 15 70 20 15 460 9 540 1 25 1622 1618 Satd. Flow (prot) 0 1567 0 0 0 0 0 1589 1561 0 Flt Permitted 0.916 0.948 0.465 0.999 Satd. Flow (perm) 1451 0 0 1517 0 764 1622 0 1616 Satd. Flow (RTOR) Confl. Peds. (#/hr) 1 0.95 0.95 0.95 0.95 Peak Hour Factor 0.95 0.95 0.95 0.95 0.95 0.95 0.95 0.95 Heavy Vehicles (%) 0% 4% 0% 0% 0% 0% 0% 1% 0% 0% 1% 0% Shared Lane Traffic (%) Lane Group Flow (vph) 0 145 493 595 111 16 Turn Type NA NA NA Perm Perm Perm Perm NA Protected Phases 4 8 2 6 Permitted Phases 8 2 4 6 **Detector Phase** 4 4 8 8 2 6 6 Switch Phase Minimum Initial (s) 7.0 7.0 7.0 7.0 30.0 30.0 30.0 30.0 23.0 Minimum Split (s) 23.0 23.0 23.0 37.0 37.0 37.0 37.0 Total Split (s) 30.0 30.0 30.0 30.0 50.0 50.0 50.0 50.0 Total Split (%) 37.5% 37.5% 37.5% 37.5% 62.5% 62.5% 62.5% 62.5% Yellow Time (s) 4.0 4.0 4.0 4.0 5.0 5.0 5.0 5.0 All-Red Time (s) 1.0 1.0 1.0 1.0 2.0 2.0 2.0 2.0 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 Total Lost Time (s) 5.0 5.0 7.0 7.0 7.0 Lead/Lag Lead-Lag Optimize? Recall Mode None None None None None None None None Act Effct Green (s) 11.1 11.1 34.7 34.7 34.7 Actuated g/C Ratio 0.21 0.66 0.21 0.66 0.66 v/c Ratio 0.48 0.35 0.03 0.46 0.56 Control Delay 25.6 22.7 6.3 9.2 10.8 Queue Delay 0.0 0.0 0.0 0.0 0.0 **Total Delay** 9.2 10.8 25.6 22.7 6.3 LOS C С Α Α В Approach Delay 25.6 22.7 9.1 10.8 Approach LOS C С Α В Queue Length 50th (ft) 40 30 2 80 107 Queue Length 95th (ft) 100 78 10 195 261 Internal Link Dist (ft) 791 1017 1154 1181 Turn Bay Length (ft) 200 714 747 1315 Base Capacity (vph) 622 1320 Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 Reduced v/c Ratio 0.20 0.15 0.03 0.37 0.45

Intersection Summary
Cycle Length: 80

17: Route 138 & Milton St./Dollar Ln.



Splits and Phases: 17: Route 138 & Milton St./Dollar Ln.



	۶	→	•	•	←	•	1	†	/	/	+	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		7	₽			4	
Traffic Volume (veh/h)	30	100	8	15	70	20	15	460	9	1	540	25
Future Volume (veh/h)	30	100	8	15	70	20	15	460	9	1	540	25
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	25	0	0	15	0	0	15	0	0	30	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1700	1652	1700	1700	1700	1700	1700	1683	1700	1700	1684	1700
Adj Flow Rate, veh/h	32	105	8	16	74	21	16	484	9	1	568	26
Adj No. of Lanes	0	1	0	0	1	0	1	1	0	0	1	0
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	4	4	4	0	0	0	0	1	1	1	1	1
Cap, veh/h	105	267	10	95	265	36	472	974	17	59	921	36
Arrive On Green	0.15	0.15	0.15	0.15	0.15	0.15	0.61	0.61	0.61	0.61	0.61	0.61
Sat Flow, veh/h	226	1252	86	126	1181	305	790	1647	31	0	1597	73
Grp Volume(v), veh/h	145	0	0	111	0	0	16	0	493	595	0	0
Grp Sat Flow(s), veh/h/ln	1564	0	0	1612	0	0	790	0	1678	1671	0	0
Q Serve(g_s), s	1.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	8.1	0.0	0.0	0.0
Cycle Q Clear(g_c), s	4.2	0.0	0.0	3.0	0.0	0.0	0.6	0.0	8.1	10.8	0.0	0.0
Prop In Lane	0.22		0.06	0.14		0.19	1.00		0.02	0.00		0.04
Lane Grp Cap(c), veh/h	380	0	0	358	0	0	472	0	1001	964	0	0
V/C Ratio(X)	0.38	0.00	0.00	0.31	0.00	0.00	0.03	0.00	0.49	0.62	0.00	0.00
Avail Cap(c_a), veh/h	862	0	0	880	0	0	785	0	1459	1525	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00	1.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	21.8	0.0	0.0	20.3	0.0	0.0	5.4	0.0	7.3	12.0	0.0	0.0
Incr Delay (d2), s/veh	0.6	0.0	0.0	0.5	0.0	0.0	0.0	0.0	0.4	0.6	0.0	0.0
Initial Q Delay(d3),s/veh	50.4	0.0	0.0	18.3	0.0	0.0	0.0	0.0	3.2	18.3	0.0	0.0
%ile BackOfQ(50%),veh/ln	9.8	0.0	0.0	4.7	0.0	0.0	0.1	0.0	7.0	15.9	0.0	0.0
LnGrp Delay(d),s/veh	72.8	0.0	0.0	39.1	0.0	0.0	5.4	0.0	10.9	30.9	0.0	0.0
LnGrp LOS	E			D			Α		В	С		
Approach Vol, veh/h		145			111			509			595	
Approach Delay, s/veh		72.8			39.1			10.7			30.9	
Approach LOS		Е			D			В			С	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		37.0		12.5		37.0		12.5				
Change Period (Y+Rc), s		7.0		5.0		7.0		5.0				
Max Green Setting (Gmax), s		43.0		25.0		43.0		25.0				
Max Q Clear Time (g_c+I1), s		10.1		6.2		12.8		5.0				
Green Ext Time (p_c), s		8.5		1.3		8.3		1.4				
Intersection Summary												
HCM 2010 Ctrl Delay			28.5									
HCM 2010 LOS			С									

18: Route 138 & Blue Jay Way (Curry College)

	۶	\rightarrow	4	†	ļ	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			र्स	ĵ»	
Traffic Volume (vph)	0	0	0	0	0	0
Future Volume (vph)	0	0	0	0	0	0
Satd. Flow (prot)	1837	0	0	1701	1733	0
Flt Permitted						
Satd. Flow (perm)	1837	0	0	1701	1733	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	7%	1%	8%	6%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	0	0	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Control Type, Uncignalized						

Control Type: Unsignalized Intersection Capacity Utilization 0.0% Analysis Period (min) 15

ICU Level of Service A

Intersection						
Int Delay, s/veh	0					
			NE		057	055
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			4	₽	
Traffic Vol, veh/h	0	0	0	0	0	0
Future Vol, veh/h	0	0	0	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	7	1	8	6	0
Mvmt Flow	0	0	0	0	0	0
	/linor2		Major1		/lajor2	
Conflicting Flow All	1	1	1	0	-	0
Stage 1	1	-	-	-	-	-
Stage 2	0	-	-	-	-	
Critical Hdwy	6.4	6.27	4.11	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.363	2.209	-	-	-
Pot Cap-1 Maneuver	1027	1069	1628	-	-	-
Stage 1	1028	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	1027	1069	1628	-	-	-
Mov Cap-2 Maneuver	1027	-	-		_	-
Stage 1	1028	_	_		_	_
Stage 2	1020					_
Jiage Z	-					
Approach	EB		NB		SB	
HCM Control Delay, s	0		0		0	
HCM LOS	Α					
Minor Lane/Major Mvmt	1	NBL	MRT	EBLn1	SBT	SBR
			NDT	LULIII	301	SDR
Capacity (veh/h)		1628	-		-	-
		-	-	-	-	-
HCM Lane V/C Ratio		^		^		
HCM Lane V/C Ratio HCM Control Delay (s)		0	-	0	-	-
HCM Lane V/C Ratio		0 A 0	-	0 A	-	-

19: Route 138 & Atherton St. & Bradlee Rd.

	→	+	†	+	\ ,
Lane Group	EBT	WBT	NBT	SBT	SEL
Lane Configurations	4	4	4	4	M
Traffic Volume (vph)	0	0	0	0	0
Future Volume (vph)	0 1837		1701	1733	1749
Satd. Flow (prot) Flt Permitted	183/	1837	1701	1/33	1/49
	1837	1027	1701	1733	1749
Satd. Flow (PTOP)	183/	1837	1701	1/33	1/49
Satd. Flow (RTOR)	0.00	0.00	0.00	0.00	0.00
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	8%	6%	5%
Shared Lane Traffic (%)	•	_			
Lane Group Flow (vph)	0	0	0	0	0
Turn Type					Prot
Protected Phases	4		2	6	5
Permitted Phases		8			
Detector Phase	4	8	2	6	5
Switch Phase					
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	20.0	20.0	20.0	20.0	20.0
Total Split (s)	20.0	20.0	20.0	20.0	20.0
Total Split (%)	20.0%	20.0%	20.0%	20.0%	20.0%
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5
Lead/Lag				Lag	Lead
Lead-Lag Optimize?				Yes	Yes
Recall Mode	None	None	Min	Min	None
Act Effct Green (s)					
Actuated g/C Ratio					
v/c Ratio					
Control Delay					
Queue Delay					
Total Delay					
LOS					
Approach Delay					
Approach LOS					
Queue Length 50th (ft)					
Queue Length 95th (ft)					
Internal Link Dist (ft)	1290	1410	1586	2667	689
Turn Bay Length (ft)					
Base Capacity (vph)					
Starvation Cap Reductn					
Spillback Cap Reductn					
Storage Cap Reductn					
Reduced v/c Ratio					
Intersection Summary					
Cycle Length: 100					
Actuated Cycle Length: 20					
Actuated Cycle Length: 20					

19: Route 138 & Atherton St. & Bradlee Rd.

Natural Cycle: 100
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.00
Intersection Signal Delay: 0.0 Intersection LOS: A
Intersection Capacity Utilization 0.0% ICU Level of Service A
Analysis Period (min) 15

Splits and Phases: 19: Route 138 & Atherton St. & Bradlee Rd.

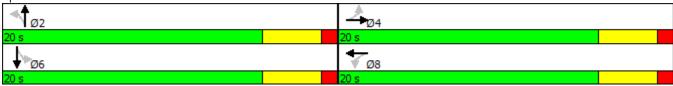


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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Future Volume (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Satd. Flow (prot)	0	1837	0	0	1837	0	0	1701	0	0	1717	0
Flt Permitted												
Satd. Flow (perm)	0	1837	0	0	1837	0	0	1701	0	0	1717	0
Satd. Flow (RTOR)												
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	3%	0%	13%	0%	8%	8%	0%	7%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Turn Type												
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	20.0	20.0		20.0	20.0		20.0	20.0		20.0	20.0	
Total Split (s)	20.0	20.0		20.0	20.0		20.0	20.0		20.0	20.0	
Total Split (%)	50.0%	50.0%		50.0%	50.0%		50.0%	50.0%		50.0%	50.0%	
Yellow Time (s)	3.5	3.5		3.5	3.5		3.5	3.5		3.5	3.5	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0			0.0			0.0	
Total Lost Time (s)		4.5			4.5			4.5			4.5	
Lead/Lag		1.0			.,,							
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		Min	Min		Min	Min	
Act Effct Green (s)												
Actuated g/C Ratio												
v/c Ratio												
Control Delay												
Queue Delay												
Total Delay												
LOS												
Approach Delay												
Approach LOS												
Queue Length 50th (ft)												
Queue Length 95th (ft)												
Internal Link Dist (ft)		939			647			711			3441	
Turn Bay Length (ft)												
Base Capacity (vph)												
Starvation Cap Reductn												
Spillback Cap Reductn												
Storage Cap Reductn												
Reduced v/c Ratio												
Intersection Summary												
Cycle Length: 40												
Actuated Cycle Length: 25												

20: Route 138 & Robbins St.

Natural Cycle: 40
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.00
Intersection Signal Delay: 0.0 Intersection LOS: A
Intersection Capacity Utilization 0.0% ICU Level of Service A
Analysis Period (min) 15

Splits and Phases: 20: Route 138 & Robbins St.



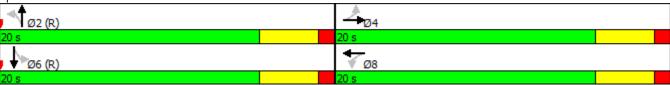
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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	0	0	0	0	0	0	0	0	0	0	0	0
Future Volume (veh/h)	0	0	0	0	0	0	0	0	0	0	0	0
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1900	1900	1900	1900	1900	1900	1900	1759	1900	1900	1776	1900
Adj Flow Rate, veh/h	0	0	0	0	0	0	0	0	0	0	0	0
Adj No. of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	0	0	0	0	0	0	8	8	8	7	7	7
Cap, veh/h	0	9999	0	0	9999	0	0	9999	0	0	9999	0
Arrive On Green	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Sat Flow, veh/h	0	-85500	0	0	-85500	0	0	-79167	0	0	-79907	0
Grp Volume(v), veh/h	0	0	0	0	0	0	0	0	0	0	0	0
Grp Sat Flow(s),veh/h/ln	0	1900	0	0	1900	0	0	1759	0	0	1776	0
Q Serve(g_s), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop In Lane	0.00		0.00	0.00		0.00	0.00		0.00	0.00		0.00
Lane Grp Cap(c), veh/h		4378624	0		1378624	0		9608320	0		5775104	0
V/C Ratio(X)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
		3686720		26486078			17117039			11821 10 9		0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay (d), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
LnGrp Delay(d),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
LnGrp LOS												
Approach Vol, veh/h		0			0			0			0	
Approach Delay, s/veh		0.0			0.0			0.0			0.0	
Approach LOS												
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		0.0		0.0		0.0		0.0				
Change Period (Y+Rc), s		4.5		4.5		4.5		4.5				
Max Green Setting (Gmax), s		15.5		15.5		15.5		15.5				
Max Q Clear Time (g_c+I1), s		0.0		0.0		0.0		0.0				
Green Ext Time (p_c), s		0.0		0.0		0.0		0.0				
Intersection Summary												
HCM 2010 Ctrl Delay			0.0									
HCM 2010 LOS			А									

21: Route 138 & Cr	neever	St./Diu	е пш	remac	e Si.						00/2	20/2018
	۶	→	•	•	←	•	•	†	~	/	ļ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Future Volume (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Satd. Flow (prot)	0	1837	0	0	1837	0	0	1701	0	0	1701	0
Flt Permitted	_		-	-	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	-	_			_		
Satd. Flow (perm)	0	1837	0	0	1837	0	0	1701	0	0	1701	0
Satd. Flow (RTOR)	J	1007			1007	J	•	1701	· ·	J	1701	J
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	1%	0%	0%	0%	8%	0%	13%	8%	0%
Shared Lane Traffic (%)	070	070	070	170	070	070	070	070	070	1070	070	070
Lane Group Flow (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Turn Type	U	0	U	- U	U	U	U	0	- O	U	U	J
Protected Phases		4			8			2			6	
Permitted Phases	4			8	U		2			6	U	
Minimum Split (s)	20.0	20.0		20.0	20.0		20.0	20.0		20.0	20.0	
Total Split (s)	20.0	20.0		20.0	20.0		20.0	20.0		20.0	20.0	
Total Split (%)	50.0%	50.0%		50.0%	50.0%		50.0%	50.0%		50.0%	50.0%	
Yellow Time (s)	3.5	3.5		3.5	3.5		3.5	3.5		3.5	3.5	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
. ,	1.0			1.0			1.0	0.0		1.0		
Lost Time Adjust (s)		0.0			0.0						0.0	
Total Lost Time (s)		4.5			4.5			4.5			4.5	
Lead/Lag												
Lead-Lag Optimize?												
Act Effet Green (s)												
Actuated g/C Ratio												
v/c Ratio												
Control Delay												
Queue Delay												
Total Delay												
LOS												
Approach Delay												
Approach LOS												
Queue Length 50th (ft)												
Queue Length 95th (ft)		000			077			0.4.44			407	
Internal Link Dist (ft)		838			877			3441			407	
Turn Bay Length (ft)												
Base Capacity (vph)												
Starvation Cap Reductn												
Spillback Cap Reductn												
Storage Cap Reductn												
Reduced v/c Ratio												
Intersection Summary												
Cycle Length: 40												
Actuated Cycle Length: 40												
Offset: 0 (0%), Referenced	to phase 2	::NBTL an	d 6:SBTL	., Start of	Green							
Natural Cycle: 40												
Control Type: Pretimed												
Maximum v/c Datio: 0.00												

Maximum v/c Ratio: 0.00

Intersection Signal Delay: 0.0 Intersection LOS: A
Intersection Capacity Utilization 0.0% ICU Level of Service A
Analysis Period (min) 15

Splits and Phases: 21: Route 138 & Cheever St./Blue Hill Terrace St.



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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	0	0	0	0	0	0	0	0	0	0	0	0
Future Volume (veh/h)	0	0	0	0	0	0	0	0	0	0	0	0
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1900	1900	1900	1900	1900	1900	1900	1759	1900	1900	1759	1900
Adj Flow Rate, veh/h	0	0	0	0	0	0	0	0	0	0	0	0
Adj No. of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	0	0	0	0	0	0	8	8	8	8	8	8
Cap, veh/h	0	9999	0	0	9999	0	0	9999	0	0	9999	0
Arrive On Green	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Sat Flow, veh/h	0	-85500	0	0	-85500	0	0	-79167	0	0	-79167	0
Grp Volume(v), veh/h	0	0	0	0	0	0	0	0	0	0	0	0
Grp Sat Flow(s), veh/h/ln	0	1900	0	0	1900	0	0	1759	0	0	1759	0
Q Serve(g_s), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop In Lane	0.00		0.00	0.00	1070/01	0.00	0.00		0.00	0.00		0.00
Lane Grp Cap(c), veh/h		4378624	0		4378624	0		9608320	0		9608320	0
V/C Ratio(X)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
1 \ - /	126486078			126486078			117117039				9330560	1.00
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay (d), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Initial Q Delay(d3),s/veh %ile BackOfQ(50%),veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
LnGrp Delay(d),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
LnGrp LOS	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Approach Vol, veh/h		0			0			0			0	
Approach Delay, s/veh		0.0			0.0			0.0			0.0	
Approach LOS		0.0			0.0			0.0			0.0	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		0.0		0.0		0.0		0.0				
Change Period (Y+Rc), s		4.5		4.5		4.5		4.5				
Max Green Setting (Gmax), s		15.5		15.5		15.5		15.5				
Max Q Clear Time (g_c+l1), s		0.0		0.0		0.0		0.0				
Green Ext Time (p_c), s		0.0		0.0		0.0		0.0				
Intersection Summary												
HCM 2010 Ctrl Delay			0.0									
HCM 2010 LOS			Α									

22: Route 138 & Aberdeen Rd.

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			ર્ન	f)	
Traffic Volume (vph)	1	2	2	270	350	15
Future Volume (vph)	1	2	2	270	350	15
Satd. Flow (prot)	1645	0	0	1801	1808	0
Flt Permitted	0.984					
Satd. Flow (perm)	1645	0	0	1801	1808	0
Confl. Peds. (#/hr)	8		5			5
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	0%	0%	2%	1%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	3	0	0	292	392	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Control Type: Unsignalized	tion 20 40/			10	III ovol m	of Comileo A
Intersection Capacity Utiliza	111011 29.4%			IC	U Level C	of Service A
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	0.1					
		EDD	ND	Not	ODT	000
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			स्	₽	
Traffic Vol, veh/h	1	2	2	270	350	15
Future Vol, veh/h	1	2	2	270	350	15
Conflicting Peds, #/hr	8	0	5	0	0	5
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	0	0	0	2	1	0
Mvmt Flow	1	2	2	290	376	16
	•	_				
		_				
	/linor2		/lajor1		/lajor2	
Conflicting Flow All	692	389	397	0	-	0
Stage 1	389	-	-	-	-	-
Stage 2	303	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	413	664	1173	-	-	-
Stage 1	689	-	-	-	-	-
Stage 2	754	-	-	-	-	-
Platoon blocked, %				-	-	_
Mov Cap-1 Maneuver	409	661	1173	_	-	-
Mov Cap-2 Maneuver	409	-	, 3	_	_	_
Stage 1	686		_	_	_	_
Stage 2	749			_		
Jiaye Z	147	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	11.6		0.1		0	
HCM LOS	В					
Minor Lang/Major Mund		NDI	NDT	EDI 51	CDT	CDD
Minor Lane/Major Mvmt	l e	NBL		EBLn1	SBT	SBR
Capacity (veh/h)		1173	-	0.0	-	-
HCM Lane V/C Ratio		0.002		0.006	-	-
HCM Control Delay (s)		8.1	0	11.6	-	-
HCM Lane LOS		Α	Α	В	-	-
HCM 95th %tile Q(veh)		0	-	0	-	-

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Future Volume (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Satd. Flow (prot)	0	1837	0	0	1837	0	0	1717	0	0	1701	0
Flt Permitted												
Satd. Flow (perm)	0	1837	0	0	1837	0	0	1717	0	0	1701	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	25%	0%	0%	0%	7%	0%	0%	8%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control		Stop			Stop			Free			Free	

Intersection Summary

Control Type: Unsignalized Intersection Capacity Utilization 0.0%

ICU Level of Service A

Analysis Period (min) 15

Intersection												
Int Delay, s/veh	0											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	0	0	0	0	0	0	0	0	0	0	0
Future Vol, veh/h	0	0	0	0	0	0	0	0	0	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	25	0	0	0	7	0	0	8	0
Mvmt Flow	0	0	0	0	0	0	0	0	0	0	0	0
Major/Minor	Minor2		ı	Minor1			/lajor1		Λ	/lajor2		
Conflicting Flow All	1	1	<u>'</u>		1	0	<u>пајог г. </u>	0	0	0	0	0
J	1	1		1	0			-	U			-
Stage 1		-	-			-	-	-	-	-	-	-
Stage 2	7.1	6.5	6.2	7.35	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy	6.1	5.5		6.35	5.5			-	-	4. I	-	-
Critical Hdwy Stg 1	6.1	5.5	-			-	-	-	-	-	-	-
Critical Hdwy Stg 2			3.3	6.35 3.725	5.5	3.3	2.2	-	-	2.2	-	-
Follow-up Hdwy	3.5	900		965	900	3.3	1635	-	-		-	-
Pot Cap-1 Maneuver	1027	899 899	1090		899	-	1033	-	-	-	-	-
Stage 1	1027	899	-	965	899	-	-	-	-	-	-	-
Stage 2 Platoon blocked, %	-	-	-	900	899	-	-	-	-	-	-	-
		000	1000	045	000		1405	-	-		-	-
Mov Cap-1 Maneuver	-	899	1090	965	899	-	1635	-	-	-	-	-
Mov Cap-2 Maneuver	1027	899	-	965	899	-	-	-	-	-	-	-
Stage 1	1027	899	-	- 04 E	-	-	-	-	-	-	-	-
Stage 2	-	-	-	965	899	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0			0			0		
HCM LOS	Α			Α								
Minor Lane/Major Mvm	nt	NBL	NBT	NRR F	EBLn1V	VRI n1	SBL	SBT	SBR			
Capacity (veh/h)		1635	1101	ואטויו	LULITIV	, DLIII	UDL	051	אפט			
HCM Lane V/C Ratio			-	-	-	-	-	-	-			
		-	-	-	-	-	-	-	-			
HCM Lang LOS		0	-	-	0	0	0	-	-			
HCM DEth %tile O(yeh)	١	A	-	-	А	А	А	-	-			
HCM 95th %tile Q(veh))	0	-	-	-	-	-	-	-			

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4						4			4	
Traffic Volume (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Future Volume (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Satd. Flow (prot)	0	1818	0	0	0	0	0	1717	0	0	1701	0
Flt Permitted												
Satd. Flow (perm)	0	1818	0	0	0	0	0	1717	0	0	1701	0
Satd. Flow (RTOR)												
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	13%	1%	0%	2%	2%	2%	20%	7%	9%	0%	8%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	0	0	0	0	0	0	0	0	0	C
Turn Type												
Protected Phases		4						2			6	
Permitted Phases	4						2			6		
Detector Phase	4	4					2	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0					5.0	5.0		5.0	5.0	
Minimum Split (s)	20.0	20.0					20.0	20.0		20.0	20.0	
Total Split (s)	20.0	20.0					20.0	20.0		20.0	20.0	
Total Split (%)	50.0%	50.0%					50.0%	50.0%		50.0%	50.0%	
Yellow Time (s)	3.5	3.5					3.5	3.5		3.5	3.5	
All-Red Time (s)	1.0	1.0					1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0						0.0			0.0	
Total Lost Time (s)		4.5						4.5			4.5	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None					Min	Min		Min	Min	
Act Effct Green (s)												
Actuated g/C Ratio												
v/c Ratio												
Control Delay												
Queue Delay												
Total Delay												
LOS												
Approach Delay												
Approach LOS												
Queue Length 50th (ft)												
Queue Length 95th (ft)												
Internal Link Dist (ft)		967			1001			519			1021	
Turn Bay Length (ft)		701			1001			017			1021	
Base Capacity (vph)												
Starvation Cap Reductn												
Spillback Cap Reductn												
Storage Cap Reductn												
Reduced v/c Ratio												
Intersection Summary												
Cycle Length: 40												
Actuated Cycle Length: 25												

24: Route 138 & Brook Rd.

Natural Cycle: 40
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.00
Intersection Signal Delay: 0.0
Intersection Capacity Utilization 0.0%
Analysis Period (min) 15

Splits and Phases: 24: Route 138 & Brook Rd.



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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4						4			4	
Traffic Volume (veh/h)	0	0	0	0	0	0	0	0	0	0	0	0
Future Volume (veh/h)	0	0	0	0	0	0	0	0	0	0	0	0
Number	7	4	14				5	2	12	1	6	16
Initial Q (Qb), veh	0	0	0				0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00				1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1900	1881	1900				1900	1776	1900	1900	1759	1900
Adj Flow Rate, veh/h	0	0	0				0	0	0	0	0	0
Adj No. of Lanes	0	1	0				0	1	0	0	1	0
Peak Hour Factor	0.92	0.92	0.92				0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	0	1	0				7	7	7	8	8	8
Cap, veh/h	0	9999	0				0	9999	0	0	9999	0
Arrive On Green	0.00	0.00	0.00				0.00	0.00	0.00	0.00	0.00	0.00
Sat Flow, veh/h	0	1881	0				0	-79907	0	0	-79167	0
Grp Volume(v), veh/h	0	0	0				0	0	0	0	0	0
Grp Sat Flow(s), veh/h/ln	0	1881	0				0	1776	0	0	1759	0
Q Serve(g_s), s	0.0	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	0.0	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
Prop In Lane	0.00	418048	0.00				0.00	5775104	0.00	0.00	9608320	0.00
Lane Grp Cap(c), veh/h V/C Ratio(X)	0.00	0.00	0.00				0.00	0.00	0.00	0.00	0.00	0.00
	0.00 12523 49 44		0.00			1	0.00 1821 109 2				9330560	0.00
HCM Platoon Ratio	1.00	1.00	1.00			l	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.00	0.00	0.00				0.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay (d), s/veh	0.0	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.0	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
LnGrp Delay(d),s/veh	0.0	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
LnGrp LOS												
Approach Vol, veh/h		0						0			0	
Approach Delay, s/veh		0.0						0.0			0.0	
Approach LOS												
•	1	2	2	4	_	,	7	0				
Timer		2	3	4	5	6	7	8				
Assigned Phs		2		4		6						
Phs Duration (G+Y+Rc), s		0.0		0.0		0.0						
Change Period (Y+Rc), s		4.5		4.5		4.5						
Max Green Setting (Gmax), s Max Q Clear Time (q_c+I1), s		15.5		15.5		15.5						
.0_ ,	5	0.0		0.0		0.0						
Green Ext Time (p_c), s		0.0		0.0		0.0						
Intersection Summary												
HCM 2010 Ctrl Delay			0.0									
HCM 2010 LOS			Α									

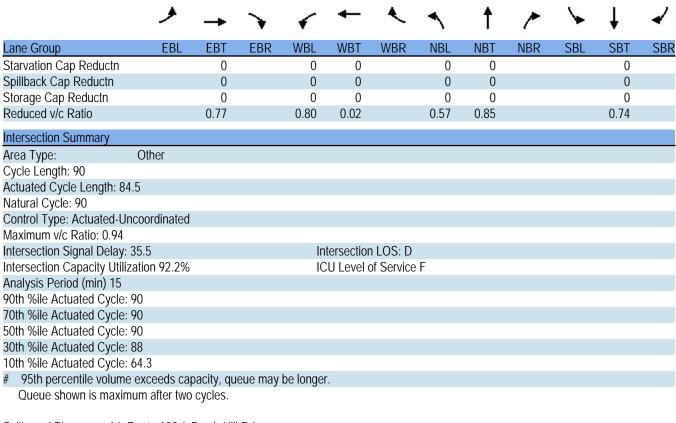
102: Route 138 & Concord Baptist Church

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	•		'	′		*
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		₽			र्स
Traffic Volume (vph)	0	0	270	30	100	250
Future Volume (vph)	0	0	270	30	100	250
Satd. Flow (prot)	1837	0	1813	0	0	1806
Flt Permitted						0.986
Satd. Flow (perm)	1837	0	1813	0	0	1806
Confl. Peds. (#/hr)	19			53	53	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	0%	0%	0%	1%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	322	0	0	377
Sign Control	Stop		Free			Free
Intersection Summary						
Control Type: Unsignalized				10	امنتمالا	-f C - m .!
Intersection Capacity Utiliza	111011 41.8%			IC	u Level	of Service
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	1.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	N/		- ₽			4
Traffic Vol, veh/h	0	0	270	30	100	250
Future Vol, veh/h	0	0	270	30	100	250
Conflicting Peds, #/hr	19	0	0	53	53	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,	# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	0	0	0	0	1	0
Mvmt Flow	0	0	290	32	108	269
WWW. Tiow	U	U	270	02	100	207
Major/Minor N	1inor1		/lajor1		Major2	
Conflicting Flow All	862	359	0	0	376	0
Stage 1	359	-	-	-	-	-
Stage 2	503	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.11	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	_	-	2.209	-
Pot Cap-1 Maneuver	328	690	-	-	1188	-
Stage 1	711	-	_	_	-	_
Stage 2	612	_	_	_	_	_
Platoon blocked, %	012		_	_		_
Mov Cap-1 Maneuver	275	658			1188	
Mov Cap-1 Maneuver	275	- 030	-		1100	-
	678		-	-	-	-
Stage 1		-			-	-
Stage 2	537	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	0		0		2.4	
HCM LOS	A					
Minor Lane/Major Mvmt		NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	-	1188	-
HCM Lane V/C Ratio		-	-	-	0.091	-
HCM Control Delay (s)		-	-	0	8.3	0
HCM Lane LOS		-	-	Α	Α	Α
HCM 95th %tile Q(veh)		-	-	-	0.3	-
					5.5	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		ň	£		Ť				f)	
Traffic Volume (vph)	6	0	310	125	5	5	160	970	0	0	660	7
Future Volume (vph)	6	0	310	125	5	5	160	970	0	0	660	7
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	200		0	0		0
Storage Lanes	0		0	1		0	1		0	0		0
Taper Length (ft)	0			0			0			0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.867			0.925						0.999	
Flt Protected		0.999		0.950			0.950					
Satd. Flow (prot)	0	1545	0	1728	1588	0	1745	1733	0	0	1683	0
Flt Permitted		0.997		0.339			0.167					
Satd. Flow (perm)	0	1542	0	616	1588	0	307	1733	0	0	1683	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		986			278			267			2293	
Travel Time (s)		22.4			6.3			6.1			52.1	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	0%	2%	3%	1%	1%	13%	0%	6%	2%	2%	9%	14%
Adj. Flow (vph)	6	0	323	130	5	5	167	1010	0	0	688	7
Shared Lane Traffic (%)		, in the second	020								555	
Lane Group Flow (vph)	0	329	0	130	10	0	167	1010	0	0	695	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)	20.0	0		20.0	11		20.1	11	. a.g. a.		11	
Link Offset(ft)		-3			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.04	1.04	1.04	1.04	1.04	1.04	1.04	1.04	1.04	1.04	1.04	1.04
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2	,	1	2	•	1	2			2	,
Detector Template	Left	Thru		Left	Thru		Left	Thru			Thru	
Leading Detector (ft)	20	100		20	100		20	100			100	
Trailing Detector (ft)	0	0		0	0		0	0			0	
Detector 1 Position(ft)	0	0		0	0		0	0			0	
Detector 1 Size(ft)	20	6		20	6		20	6			6	
Detector 1 Type	CI+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex			CI+Ex	
Detector 1 Channel	OITEX	OITEX		OITEX	OITEX		OITEX	OITEX			OITEX	
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0			0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0			0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0			0.0	
Detector 2 Position(ft)	0.0	94		0.0	94		0.0	94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Type Detector 2 Channel		CITEX			CITEX			CITEX			CITEX	
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
	Perm	NA		Perm	NA		nmint	NA			NA	
Turn Type Protected Phases	Pelill			Pellil			pm+pt					
FIUIECIEU PHASES		4			8		5	2			6	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Permitted Phases	4			8			2					
Detector Phase	4	4		8	8		5	2			6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		10.0	10.0		5.0	10.0			10.0	
Minimum Split (s)	23.0	23.0		24.0	24.0		10.0	45.0			45.0	
Total Split (s)	28.0	28.0		28.0	28.0		11.0	62.0			51.0	
Total Split (%)	31.1%	31.1%		31.1%	31.1%		12.2%	68.9%			56.7%	
Maximum Green (s)	23.0	23.0		22.0	22.0		6.0	57.0			46.0	
Yellow Time (s)	4.0	4.0		5.0	5.0		4.0	4.0			4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0			1.0	
Lost Time Adjust (s)		0.0		0.0	0.0		0.0	0.0			0.0	
Total Lost Time (s)		5.0		6.0	6.0		5.0	5.0			5.0	
Lead/Lag							Lead				Lag	
Lead-Lag Optimize?							Yes				Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0			3.0	
Recall Mode	None	None		None	None		None	None			None	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0			7.0	
Flash Dont Walk (s)	11.0	11.0		11.0	11.0			11.0			11.0	
Pedestrian Calls (#/hr)	10	10		10	10			10			10	
Act Effct Green (s)		21.9		20.9	20.9		52.4	52.4			41.2	
Actuated g/C Ratio		0.26		0.25	0.25		0.62	0.62			0.49	
v/c Ratio		0.82		0.86	0.03		0.57	0.94			0.85	
Control Delay		49.5		78.5	26.2		14.6	32.5			30.2	
Queue Delay		0.0		0.0	0.0		0.0	0.0			0.0	
Total Delay		49.5		78.5	26.2		14.6	32.5			30.2	
LOS		D		Е	С		В	С			С	
Approach Delay		49.5			74.8			30.0			30.2	
Approach LOS		D			Е			С			С	
90th %ile Green (s)	23.0	23.0		22.0	22.0		6.0	57.0			46.0	
90th %ile Term Code	Max	Max		Max	Max		Max	Max			Max	
70th %ile Green (s)	23.0	23.0		22.0	22.0		6.0	57.0			46.0	
70th %ile Term Code	Max	Max		Max	Max		Max	Max			Max	
50th %ile Green (s)	23.0	23.0		22.0	22.0		6.0	57.0			46.0	
50th %ile Term Code	Max	Max		Max	Max		Max	Max			Hold	
30th %ile Green (s)	23.0	23.0		22.0	22.0		6.0	55.0			44.0	
30th %ile Term Code	Hold	Hold		Max	Max		Max	Gap			Hold	
10th %ile Green (s)	17.3	17.3		16.3	16.3		6.0	37.0			26.0	
10th %ile Term Code	Hold	Hold		Gap	Gap		Max	Gap			Hold	
Stops (vph)		271		99	9		60	759			538	
Fuel Used(gal)		7		3	0		1	13			19	
CO Emissions (g/hr)		497		197	9		80	883			1328	
NOx Emissions (g/hr)		97		38	2		16	172			258	
VOC Emissions (g/hr)		115		46	2		18	205			308	
Dilemma Vehicles (#)		0		0	0		0	0			0	
Queue Length 50th (ft)		177		71	4		35	455			313	
Queue Length 95th (ft)		#321		#178	17		60	#787			#492	
Internal Link Dist (ft)		906			198			187			2213	
Turn Bay Length (ft)							200					
Base Capacity (vph)		427		163	421		294	1190			933	



Splits and Phases: 14: Route 138 & Brush Hill Rd.



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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		ሻ	f)		ሻ	†			ĵ»	
Traffic Volume (veh/h)	6	0	310	125	5	5	160	970	0	0	660	7
Future Volume (veh/h)	6	0	310	125	5	5	160	970	0	0	660	7
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	10	0	5	10	0	0	20	0	0	15	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1900	1846	1900	1881	1776	1900	1900	1792	0	0	1742	1900
Adj Flow Rate, veh/h	6	0	323	130	5	5	167	1010	0	0	688	7
Adj No. of Lanes	0	1	0	1	1	0	1	1	0	0	1	0
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	2	2	2	1	1	1	0	6	0	0	9	9
Cap, veh/h	40	5	320	175	344	188	252	1119	0	0	868	7
Arrive On Green	0.25	0.00	0.25	0.25	0.25	0.25	0.06	0.62	0.00	0.00	0.50	0.50
Sat Flow, veh/h	9	20	1542	1063	816	816	1810	1792	0	0	1722	18
Grp Volume(v), veh/h	329	0	0	130	0	10	167	1010	0	0	0	695
Grp Sat Flow(s), veh/h/ln	1571	0	0	1063	0	1632	1810	1792	0	0	0	1739
Q Serve(g_s), s	2.1	0.0	0.0	3.9	0.0	0.4	3.7	42.6	0.0	0.0	0.0	29.0
Cycle Q Clear(g_c), s	17.1	0.0	0.0	21.0	0.0	0.4	3.7	42.6	0.0	0.0	0.0	29.0
Prop In Lane	0.02		0.98	1.00		0.50	1.00		0.00	0.00		0.01
Lane Grp Cap(c), veh/h	311	0	0	175	0	417	252	1119	0	0	0	870
V/C Ratio(X)	1.06	0.00	0.00	0.74	0.00	0.02	0.66	0.90	0.00	0.00	0.00	0.80
Avail Cap(c_a), veh/h	458	0	0	223	0	414	333	1177	0	0	0	922
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	0.00	1.00	0.00	1.00	1.00	1.00	0.00	0.00	0.00	1.00
Uniform Delay (d), s/veh	32.3	0.0	0.0	38.2	0.0	25.2	19.8	16.2	0.0	0.0	0.0	19.6
Incr Delay (d2), s/veh	53.8	0.0	0.0	9.5	0.0	0.0	3.0	9.5	0.0	0.0	0.0	4.8
Initial Q Delay(d3),s/veh	102.4	0.0	0.0	22.9	0.0	4.2	0.0	23.6	0.0	0.0	0.0	10.6
%ile BackOfQ(50%),veh/ln	22.4	0.0	0.0	5.3	0.0	1.7	3.3	36.0	0.0	0.0	0.0	20.0
LnGrp Delay(d),s/veh	188.5	0.0	0.0	70.7	0.0	29.5	22.8	49.3	0.0	0.0	0.0	35.0
LnGrp LOS	F			Е		С	С	D				С
Approach Vol, veh/h		329			140			1177			695	
Approach Delay, s/veh		188.5			67.7			45.5			35.0	
Approach LOS		F			Е			D			С	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs	<u> </u>	2	J	4	5	6	/	8				
Phs Duration (G+Y+Rc), s		58.8		28.0	10.6	48.2		28.0				
Change Period (Y+Rc), s		5.0		* 6	5.0	5.0		6.0				
Max Green Setting (Gmax), s		57.0		* 23	6.0	46.0		22.0				
Max Q Clear Time (q_c+l1), s		44.6		19.1	5.7	31.0		23.0				
Green Ext Time (p_c), s		9.2		1.0	0.0	10.7		0.0				
		7.2		1.0	0.0	10.7		0.0				
Intersection Summary			(2.0									
HCM 2010 Ctrl Delay			63.8									
HCM 2010 LOS			E									
Notes												

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		ሻ	ĵ»		ሻ	†			ĵ.	•
Traffic Volume (vph)	15	0	370	140	5	5	90	790	0	0	770	0
Future Volume (vph)	15	0	370	140	5	5	90	790	0	0	770	0
Ideal Flow (vphpl)	1700	1700	1700	1700	1700	1700	1700	1700	1700	1700	1700	1700
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.870			0.925							
Flt Protected		0.998		0.950			0.950					
Satd. Flow (prot)	0	1413	0	1546	1441	0	1561	1580	0	0	1565	0
Flt Permitted		0.992		0.321			0.104					
Satd. Flow (perm)	0	1405	0	522	1441	0	171	1580	0	0	1565	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		30			30			35			35	
Link Distance (ft)		986			278			267			2293	
Travel Time (s)		22.4			6.3			5.2			44.7	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	0%	2%	1%	1%	3%	8%	0%	4%	2%	2%	5%	0%
Adj. Flow (vph)	15	0	381	144	5	5	93	814	0	0	794	0
Shared Lane Traffic (%)		J.	00.		· ·	· ·	, 0	0			,,,	J
Lane Group Flow (vph)	0	396	0	144	10	0	93	814	0	0	794	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)	Lort	0	rugiii	Lon	11	rugiii	Lon	11	rugin	Loit	11	rugin
Link Offset(ft)		-3			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.20	1.20	1.20	1.20	1.20	1.20	1.20	1.20	1.20	1.20	1.20	1.20
Turning Speed (mph)	15	1.20	9	15	1.20	9	15	1.20	9	15	1.20	9
Number of Detectors	1	2	,	1	2	,	1	2	,	10	2	,
Detector Template	Left	Thru		Left	Thru		Left	Thru			Thru	
Leading Detector (ft)	20	100		20	100		20	100			100	
Trailing Detector (ft)	0	0		0	0		0	0			0	
Detector 1 Position(ft)	0	0		0	0		0	0			0	
Detector 1 Size(ft)	20	6		20			20	6			6	
Detector 1 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 1 Channel	OI. EX	OI LX		OI LX	OI. EX		OI LX	OI · EX			OI. EX	
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0			0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0			0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0			0.0	
Detector 2 Position(ft)	0.0	94		0.0	94		0.0	94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel		OI LA			OI LA			OITEN			OITEN	
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA		pm+pt	NA			NA	
Protected Phases	. 61111	4		. 01111	8		5	2			6	
Permitted Phases	4	т		8	- 0		2				- 0	
Detector Phase	4	4		8	8		5	2			6	
Switch Phase	4	4		U	U		J				0	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Initial (s)	10.0	10.0		10.0	10.0		5.0	10.0			10.0	
Minimum Split (s)	23.0	23.0		23.0	23.0		9.5	45.0			45.0	
Total Split (s)	31.0	31.0		31.0	31.0		9.5	59.0			49.5	
Total Split (%)	34.4%	34.4%		34.4%	34.4%		10.6%	65.6%			55.0%	
Maximum Green (s)	26.0	26.0		26.0	26.0		5.0	53.0			43.5	
Yellow Time (s)	4.0	4.0		4.0	4.0		3.5	5.0			5.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0			1.0	
Lost Time Adjust (s)		0.0		0.0	0.0		0.0	0.0			0.0	
Total Lost Time (s)		5.0		5.0	5.0		4.5	6.0			6.0	
Lead/Lag							Lead				Lag	
Lead-Lag Optimize?							Yes				Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0			3.0	
Recall Mode	None	None		None	None		None	None			None	
Act Effct Green (s)		26.1		26.1	26.1		52.5	51.0			43.6	
Actuated g/C Ratio		0.30		0.30	0.30		0.60	0.58			0.49	
v/c Ratio		0.95		0.94	0.02		0.52	0.89			1.03	
Control Delay		67.6		93.2	23.3		17.8	30.1			64.1	
Queue Delay		0.0		0.0	0.0		0.0	0.0			0.0	
Total Delay		67.6		93.2	23.3		17.8	30.1			64.1	
LOS		Ε		F	С		В	С			Е	
Approach Delay		67.6			88.6			28.8			64.1	
Approach LOS		Ε			F			С			Ε	
90th %ile Green (s)	26.0	26.0		26.0	26.0		5.0	53.0			43.5	
90th %ile Term Code	Max	Max		Max	Max		Max	Max			Max	
70th %ile Green (s)	26.0	26.0		26.0	26.0		5.0	53.0			43.5	
70th %ile Term Code	Max	Max		Max	Max		Max	Max			Max	
50th %ile Green (s)	26.0	26.0		26.0	26.0		5.0	53.0			43.5	
50th %ile Term Code	Max	Max		Max	Max		Max	Max			Max	
30th %ile Green (s)	26.0	26.0		26.0	26.0		5.0	53.0			43.5	
30th %ile Term Code	Max	Max		Max	Max		Max	Hold			Max	
10th %ile Green (s)	26.0	26.0		26.0	26.0		0.0	43.5			43.5	
10th %ile Term Code	Max	Max		Max	Max		Skip	Hold			Max	
Stops (vph)		320		110	9		36	622			618	
Fuel Used(gal)		10		4	0		1	11			27	
CO Emissions (g/hr)		700		249	8		54	772			1919	
NOx Emissions (g/hr)		136		49	2		10	150			373	
VOC Emissions (g/hr)		162		58	2		12	179			445	
Dilemma Vehicles (#)		0		0	0		0	39			39	
Queue Length 50th (ft)		222		80	4		20	362			~497	
Queue Length 95th (ft)		#406		#199	16		41	#639			#717	
Internal Link Dist (ft)		906			198			187			2213	
Turn Bay Length (ft)												
Base Capacity (vph)		415		154	426		180	952			774	
Starvation Cap Reductn		0		0	0		0	0			0	
Spillback Cap Reductn		0		0	0		0	0			0	
Storage Cap Reductn		0		0	0		0	0			0	
Reduced v/c Ratio		0.95		0.94	0.02		0.52	0.86			1.03	
Intersection Summary												

_			
· · · · · / / · · ·	Other		
Cycle Length: 90			
Actuated Cycle Length: 88.1			
Natural Cycle: 100			
Control Type: Actuated-Unc	oordinated		
Maximum v/c Ratio: 1.03			
Intersection Signal Delay: 52	2.2	Intersection LOS: D	
Intersection Capacity Utilizat	tion 103.1%	ICU Level of Service G	
Analysis Period (min) 15			
90th %ile Actuated Cycle: 90	0		
70th %ile Actuated Cycle: 90	0		
50th %ile Actuated Cycle: 90	0		
30th %ile Actuated Cycle: 90	0		
10th %ile Actuated Cycle: 80	0.5		
 Volume exceeds capacit 	ty, queue is theoretically infinite.		
Queue shown is maximul	m after two cycles.		
# 95th percentile volume e	exceeds capacity, queue may be lor	nger.	
Queue shown is maximul	m after two cycles.		

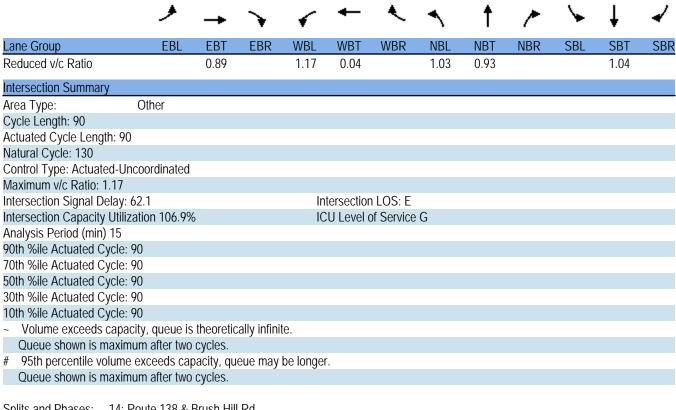
Splits and Phases: 14: Route 138 & Brush Hill Rd.



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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		7	1>		ሻ	†			ĵ.	
Traffic Volume (veh/h)	15	0	370	140	5	5	90	790	0	0	770	0
Future Volume (veh/h)	15	0	370	140	5	5	90	790	0	0	770	0
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	20	0	10	10	0	0	10	0	0	40	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1700	1684	1700	1683	1611	1700	1700	1635	0	0	1619	1700
Adj Flow Rate, veh/h	15	0	381	144	5	5	93	814	0	0	794	0
Adj No. of Lanes	0	1	0	1	1	0	1	1	0	0	1	0
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	2	2	2	1	3	3	0	4	0	0	5	5
Cap, veh/h	42	9	280	156	382	196	161	960	0	0	791	0
Arrive On Green	0.29	0.00	0.29	0.29	0.29	0.29	0.05	0.59	0.00	0.00	0.49	0.00
Sat Flow, veh/h	24	30	1381	1008	740	740	1619	1635	0	0	1619	0
Grp Volume(v), veh/h	396	0	0	144	0	10	93	814	0	0	794	0
Grp Sat Flow(s), veh/h/ln	1435	0	0	1008	0	1481	1619	1635	0	0	1619	0
Q Serve(g_s), s	9.8	0.0	0.0	1.8	0.0	0.4	2.4	36.7	0.0	0.0	43.5	0.0
Cycle Q Clear(g_c), s	24.2	0.0	0.0	26.0	0.0	0.4	2.4	36.7	0.0	0.0	43.5	0.0
Prop In Lane	0.04		0.96	1.00		0.50	1.00		0.00	0.00		0.00
Lane Grp Cap(c), veh/h	271	0	0	156	0	433	161	960	0	0	791	0
V/C Ratio(X)	1.46	0.00	0.00	0.92	0.00	0.02	0.58	0.85	0.00	0.00	1.00	0.00
Avail Cap(c_a), veh/h	459	0	0	176	0	430	171	968	0	0	787	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	0.00	1.00	0.00	1.00	1.00	1.00	0.00	0.00	1.00	0.00
Uniform Delay (d), s/veh	29.1	0.0	0.0	38.7	0.0	23.5	20.9	16.1	0.0	0.0	23.0	0.0
Incr Delay (d2), s/veh	223.0	0.0	0.0	44.1	0.0	0.0	4.2	7.1	0.0	0.0	32.8	0.0
Initial Q Delay(d3),s/veh	155.0	0.0	0.0	197.5	0.0	3.9	0.0	5.1	0.0	0.0	166.7	0.0
%ile BackOfQ(50%),veh/ln	43.5	0.0	0.0	14.3	0.0	1.6	1.4	21.5	0.0	0.0	66.8	0.0
LnGrp Delay(d),s/veh	407.1	0.0	0.0	280.2	0.0	27.4	25.1	28.4	0.0	0.0	222.5	0.0
LnGrp LOS	F			F		С	С	С			F	
Approach Vol, veh/h		396			154			907			794	
Approach Delay, s/veh		407.1			263.8			28.0			222.5	
Approach LOS		F			F			С			F	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4	5	6		8				
Phs Duration (G+Y+Rc), s		58.5		31.0	9.0	49.5		31.0				
Change Period (Y+Rc), s		6.0		5.0	4.5	6.0		5.0				
Max Green Setting (Gmax), s		53.0		26.0	5.0	43.5		26.0				
Max Q Clear Time (g_c+l1), s		38.7		26.2	4.4	45.5		28.0				
Green Ext Time (p_c), s		9.3		0.0	0.0	0.0		0.0				
Intersection Summary												
HCM 2010 Ctrl Delay			179.4									
HCM 2010 LOS			F									
Notes												
NOIGS												

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		ሻ	ĥ		ሻ	^			ĵ»	
Traffic Volume (vph)	5	0	330	170	10	5	170	850	0	0	770	5
Future Volume (vph)	5	0	330	170	10	5	170	850	0	0	770	5
Ideal Flow (vphpl)	1700	1700	1700	1700	1700	1700	1700	1700	1700	1700	1700	1700
Storage Length (ft)	0		0	0		0	200		0	0		0
Storage Lanes	0		0	1		0	1		0	0		0
Taper Length (ft)	0			0			0			0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.867			0.953						0.999	
Flt Protected		0.999		0.950			0.950					
Satd. Flow (prot)	0	1423	0	1546	1505	0	1561	1611	0	0	1610	0
Flt Permitted		0.998		0.340			0.087					
Satd. Flow (perm)	0	1422	0	553	1505	0	143	1611	0	0	1610	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		30			30			35			35	
Link Distance (ft)		986			278			267			2293	
Travel Time (s)		22.4			6.3			5.2			44.7	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	0%	2%	0%	1%	0%	13%	0%	2%	2%	2%	2%	0%
Adj. Flow (vph)	5	0	347	179	11	5	179	895	0	0	811	5
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	352	0	179	16	0	179	895	0	0	816	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			11			11			11	
Link Offset(ft)		-3			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.20	1.20	1.20	1.20	1.20	1.20	1.20	1.20	1.20	1.20	1.20	1.20
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2			2	
Detector Template	Left	Thru		Left	Thru		Left	Thru			Thru	
Leading Detector (ft)	20	100		20	100		20	100			100	
Trailing Detector (ft)	0	0		0	0		0	0			0	
Detector 1 Position(ft)	0	0		0	0		0	0			0	
Detector 1 Size(ft)	20	6		20	6		20	6			6	
Detector 1 Type	CI+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex			CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0			0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0			0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0			0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA		pm+pt	NA			NA	
Protected Phases		4			8		5	2			6	

Bame Group		ၨ	→	,		←	•	4	†	<i>></i>	>	ļ	4
Detector Phase 4	Lane Group	EBL	EBT	EBR W	/BL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase		4			8			2					
Switch Phase	Detector Phase		4		8	8		5	2			6	
Minimum Initial (s)													
Minimum Spilt (s) 20.0 20.0 20.0 20.0 9.5 45.0 45.0 Total Spilt (s) 30.0 30.0 30.0 30.0 10.0 60.0 50.0 Total Spilt (%) 33.3% 33.		10.0	10.0	1	0.0	10.0		5.0	10.0			10.0	
Total Spili (%) 30.0 30.0 30.0 10.0 60.0 50.0 50.0 Total Spili (%) 33.3% 33.3% 33.3% 33.3% 11.1% 66.7% 55.6% Maximum Green (s) 25.0 25.0 25.0 25.0 5.5 54.0 44.0 40.0 3.5 5.0 5.0 44.0 10.0 10.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0									45.0				
Total Split (%) 33.3% 33.3% 33.3% 33.3% 11.1% 66.7% 55.6% Maximum Green (s) 25.0 25.0 25.0 25.0 5.5 54.0 44.0 40.0 3.5 50.0 5.0 5.0 All-Red Time (s) 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0								10.0					
Maximum Green (s) 25.0 25.0 25.0 25.0 5.5 54.0 44.0 4.0 4.0 4.0 4.0 3.5 5.0 5.0 5.0 5.0 All-Red Time (s) 1.0 1													
Vellow Time (s)									54.0				
All-Red Time (s) Lost Time (s) LeadLag Lead Lag Lead Lag Lead Lag Lead-Lad Cybimize? Vers Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0													
Lost Time Adjust (s)	` ,												
Total Lost Time (s) 5.0 5.0 5.0 4.5 6.0 6.0	, ,												
Lead/Lag Lead-Lag Optimizer Ves Ve	•					5.0		4.5					
Lead-Lag Optimize? Yes Yes Yes Vehicle Extension (s) 3.0 4.0 4													
Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 Recal Mode None Add Add None													
Recall Mode		3.0	3.0		3.0	3.0			3.0				
Act Effet Green (s)													
Actuated g/C Ratio													
Vic Ratio 0.89 1.17 0.04 1.03 0.93 1.04 Control Delay 57.9 158.9 24.2 96.3 33.8 66.9 Queue Delay 0.0 0.0 0.0 0.0 0.0 Total Delay 57.9 158.9 24.2 96.3 33.8 66.9 LOS E F C F C E Approach LOS E F D E E 90th %ile Green (s) 25.0 25.0 25.0 25.0 5.5 54.0 44.0 90th %ile Green (s) 25.0 25.0 25.0 25.0 5.5 54.0 44.0 90th %ile Green (s) 25.0 25.0 25.0 25.0 25.0 25.0 44.0 90th %ile Green (s) 25.0 25.0 25.0 25.0 25.0 44.0 90th %ile Green (s) 25.0 25.0 25.0 25.0 5.5 54.0 44.0 30th													
Control Delay 57.9 158.9 24.2 96.3 33.8 66.9 Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 Total Delay 57.9 158.9 24.2 96.3 33.8 66.9 LOS E F F C F C E Approach LOS E F F C F C E Approach LOS E F F C F C E 90th %ile Green (s) 25.0 25.0 25.0 25.0 5.5 54.0 44.0 90th %ile Green (s) 25.0 25.0 25.0 25.0 5.5 54.0 44.0 70th %ile Green (s) 25.0 25.0 25.0 25.0 5.5 54.0 44.0 70th %ile Green (s) 25.0 25.0 25.0 25.0 5.5 54.0 44.0 70th %ile Green (s) 25.0 25.0 25.0 25.0 5.5 54.0 44.0 70th %ile Green (s) 25.0 25.0 25.0 25.0 5.5 54.0 44.0 70th %ile Green (s) 25.0 25.0 25.0 25.0 5.5 54.0 44.0 70th %ile Green (s) 25.0 25.0 25.0 25.0 5.5 54.0 44.0 70th %ile Green (s) 25.0 25.0 25.0 25.0 5.5 54.0 44.0 70th %ile Green (s) 25.0 25.0 25.0 55.0 54.0 44.0 70th %ile Green (s) 25.0 25.0 25.0 55.0 54.0 44.0 70th %ile Green (s) 25.0 25.0 25.0 55.0 54.0 44.0 30th %ile Green (s) 25.0 25.0 25.0 25.0 55.0 54.0 44.0 30th %ile Green (s) 25.0 25.0 25.0 25.0 55.0 54.0 44.0 30th %ile Green (s) 25.0 25.0 25.0 25.0 55.0 54.0 44.0 30th %ile Green (s) 25.0 25.0 25.0 25.0 55.0 54.0 44.0 30th %ile Green (s) 25.0 25.0 25.0 25.0 55.0 54.0 44.0 30th %ile Green (s) 25.0 25.0 25.0 25.0 55.0 54.0 44.0 30th %ile Green (s) 25.0 25.0 25.0 25.0 25.0 55.0 54.0 44.0 30th %ile Green (s) 25.0 25.0 25.0 25.0 25.0 55.0 54.0 44.0 30th %ile Green (s) 25.0 25.0 25.0 25.0 25.0 55.0 54.0 44.0 30th %ile Green (s) 25.0 25.0 25.0 25.0 25.0 25.0 35.0 55.0 54.0 44.0 30th %ile Green (s) 25.0 25.0 25.0 25.0 25.0 35.0 55.0 54.0 44.0 30th %ile Green (s) 25.0 25.0 25.0 25.0 25.0 35.0 55.0 55.0 55.0 55.0 55.0 55.0 5													
Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 Total Delay 57.9 158.9 24.2 96.3 33.8 66.9 LOS E F C F C E Approach Delay 57.9 147.9 44.3 66.9 Approach LOS E F D E 90th Wile Green (s) 25.0 25.0 25.0 55.5 54.0 44.0 90th Wile Green (s) 25.0 25.0 25.0 25.0 55.5 54.0 44.0 90th Wile Green (s) 25.0 25.0 25.0 25.0 55.5 54.0 44.0 70th Wile Green (s) 25.0 25.0 25.0 25.0 55.5 54.0 44.0 50th Wile Green (s) 25.0 25.0 25.0 25.0 55.5 54.0 44.0 30th Wile Green (s) 25.0 25.0 25.0 25.0 55.5 54.0 44.0 30th Wile G													
Total Delay													
LOS	•												
Approach Delay 57.9 147.9 44.3 66.9 Approach LOS E F D E 90th %ile Green (s) 25.0 25.0 25.0 5.5 54.0 44.0 90th %ile Term Code Max Max Max Max Max Max 70th %ile Green (s) 25.0 25.0 25.0 25.0 5.5 54.0 44.0 70th %ile Term Code Max Max Max Max Max Max Max 50th %ile Green (s) 25.0 25.0 25.0 25.0 5.5 54.0 44.0 50th %ile Green (s) 25.0 25.0 25.0 25.0 5.5 54.0 44.0 50th %ile Green (s) 25.0 25.0 25.0 25.0 5.5 54.0 44.0 30th %ile Term Code Max Max Max Max Max Max 44.0 30th %ile Term Code Max Max Max Max Max Hold													
Approach LOS					•			•					
Oith %ile Green (s) 25.0 25.0 25.0 25.0 5.5 54.0 44.0 90th %ile Term Code Max Max Max Max Max Max Max 70th %ile Green (s) 25.0 25.0 25.0 25.0 5.5 54.0 44.0 70th %ile Green (s) 25.0 25.0 25.0 5.5 54.0 44.0 50th %ile Green (s) 25.0 25.0 25.0 5.5 54.0 44.0 50th %ile Term Code Max Max Max Max Max Max 30th %ile Green (s) 25.0 25.0 25.0 25.0 25.0 44.0 30th %ile Green (s) 25.0 25.0 25.0 25.0 5.5 54.0 44.0 30th %ile Green (s) 25.0 25.0 25.0 25.0 5.5 54.0 44.0 30th %ile Green (s) 25.0 25.0 25.0 5.5 54.0 44.0 30th %ile Green (s) 25.0													
90th %ile Term Code Max Hold Max Max Max Hold Max Max Max Max Max		25.0		2	5.0			5.5					
70th %ile Green (s) 25.0 25.0 25.0 25.0 5.5 54.0 44.0 70th %ile Term Code Max Max Max Max Max Max Max 50th %ile Green (s) 25.0 25.0 25.0 25.0 5.5 54.0 44.0 50th %ile Green (s) 25.0 25.0 25.0 25.0 5.5 54.0 44.0 30th %ile Green (s) 25.0 25.0 25.0 5.5 54.0 44.0 30th %ile Green (s) 25.0 25.0 25.0 5.5 54.0 44.0 30th %ile Green (s) 25.0 25.0 25.0 25.0 5.5 54.0 44.0 30th %ile Green (s) 25.0 25.0 25.0 25.0 5.5 54.0 44.0 10th %ile Green (s) 25.0 25.0 25.0 25.0 25.0 25.0 25.0 25.0 25.0 25.0 25.0 25.0 25.0 25.0 25.0 25.0 25.0	. ,												
70th %ile Term Code Max													
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Splits and Phases: 14: Route 138 & Brush Hill Rd.



	۶	→	•	•	←	•	1	†	/	/	↓	✓
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		7	£		7	†			4î	
Traffic Volume (veh/h)	5	0	330	170	10	5	170	850	0	0	770	5
Future Volume (veh/h)	5	0	330	170	10	5	170	850	0	0	770	5
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	15	0	5	5	0	5	5	0	0	20	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1700	1700	1700	1683	1634	1700	1700	1667	0	0	1667	1700
Adj Flow Rate, veh/h	5	0	347	179	11	5	179	895	0	0	811	5
Adj No. of Lanes	0	1	0	1	1	0	1	1	0	0	1	0
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	2	2	2	1	0	0	0	2	0	0	2	2
Cap, veh/h	41	4	316	150	406	128	179	1000	0	0	811	2
Arrive On Green	0.28	0.00	0.28	0.28	0.28	0.28	0.06	0.60	0.00	0.00	0.49	0.49
Sat Flow, veh/h	6	15	1427	1040	1064	484	1619	1667	0	0	1655	10
Grp Volume(v), veh/h	352	0	0	179	0	16	179	895	0	0	0	816
Grp Sat Flow(s), veh/h/ln	1447	0	0	1040	0	1548	1619	1667	0	0	0	1665
Q Serve(g_s), s	3.1	0.0	0.0	4.1	0.0	0.7	5.5	41.8	0.0	0.0	0.0	44.0
Cycle Q Clear(g_c), s	20.9	0.0	0.0	25.0	0.0	0.7	5.5	41.8	0.0	0.0	0.0	44.0
Prop In Lane	0.01	•	0.99	1.00		0.31	1.00	1000	0.00	0.00	•	0.01
Lane Grp Cap(c), veh/h	327	0	0	150	0	431	179	1000	0	0	0	815
V/C Ratio(X)	1.08	0.00	0.00	1.19	0.00	0.04	1.00	0.89	0.00	0.00	0.00	1.00
Avail Cap(c_a), veh/h	443	0	0	197	0	430	179	1000	0	0	0	814
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	0.00	1.00	0.00	1.00	1.00	1.00	0.00	0.00	0.00	1.00
Uniform Delay (d), s/veh	30.9	0.0	0.0	39.2	0.0	24.1	23.8	16.0	0.0	0.0	0.0	23.0
Incr Delay (d2), s/veh	62.9 139.3	0.0	0.0	128.8 93.9	0.0	0.0 1.0	67.4	10.5 1.7	0.0	0.0	0.0	32.0 86.9
Initial Q Delay(d3),s/veh %ile BackOfQ(50%),veh/ln	28.8	0.0	0.0	93.9 14.1	0.0	0.9	100.6 12.8	23.3	0.0	0.0	0.0	47.4
	233.1	0.0	0.0	261.9	0.0	25.2	191.8	28.2	0.0	0.0	0.0	141.8
LnGrp Delay(d),s/veh LnGrp LOS	233.1 F	0.0	0.0	201.9 F	0.0	23.2 C	191.0 F	20.2 C	0.0	0.0	0.0	141.0 F
-	г	252		Г	105						014	
Approach Vol, veh/h		352			195			1074			816	
Approach LOS		233.1 F			242.4 F			55.5 E			141.8 F	
Approach LOS											Г	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4	5	6		8				
Phs Duration (G+Y+Rc), s		60.0		30.0	10.0	50.0		30.0				
Change Period (Y+Rc), s		6.0		5.0	4.5	6.0		5.0				
Max Green Setting (Gmax), s		54.0		25.0	5.5	44.0		25.0				
Max Q Clear Time (g_c+I1), s		43.8		22.9	7.5	46.0		27.0				
Green Ext Time (p_c), s		7.6		0.7	0.0	0.0		0.0				
Intersection Summary												
HCM 2010 Ctrl Delay			125.0									
HCM 2010 LOS			F									

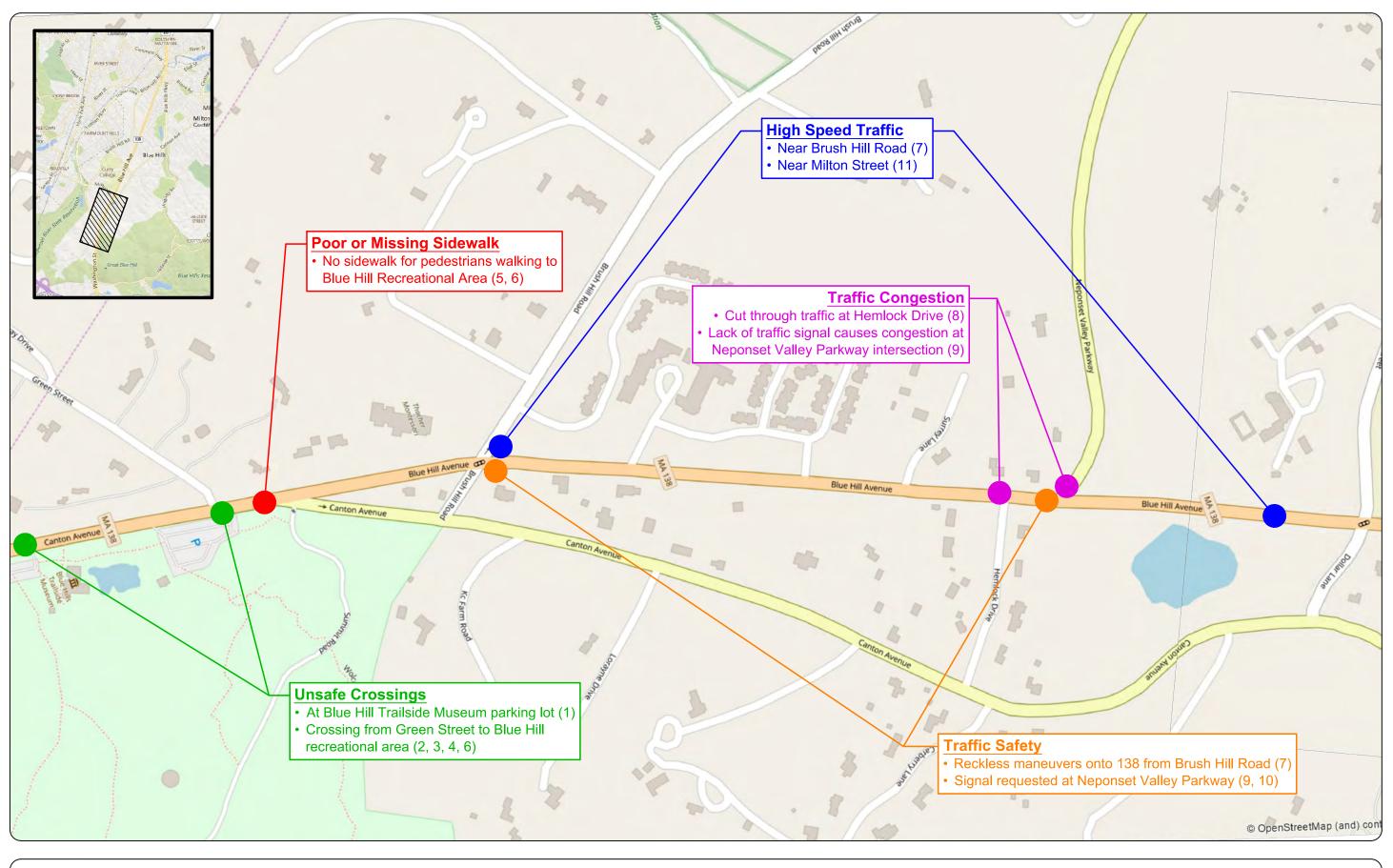
Appendix G: Survey Comments

Part 1: Common comments encountered in the Milton Complete Streets Prioritization

Plan at wikimapping.com

Part 2: Significant free-response feedback for one or more questions

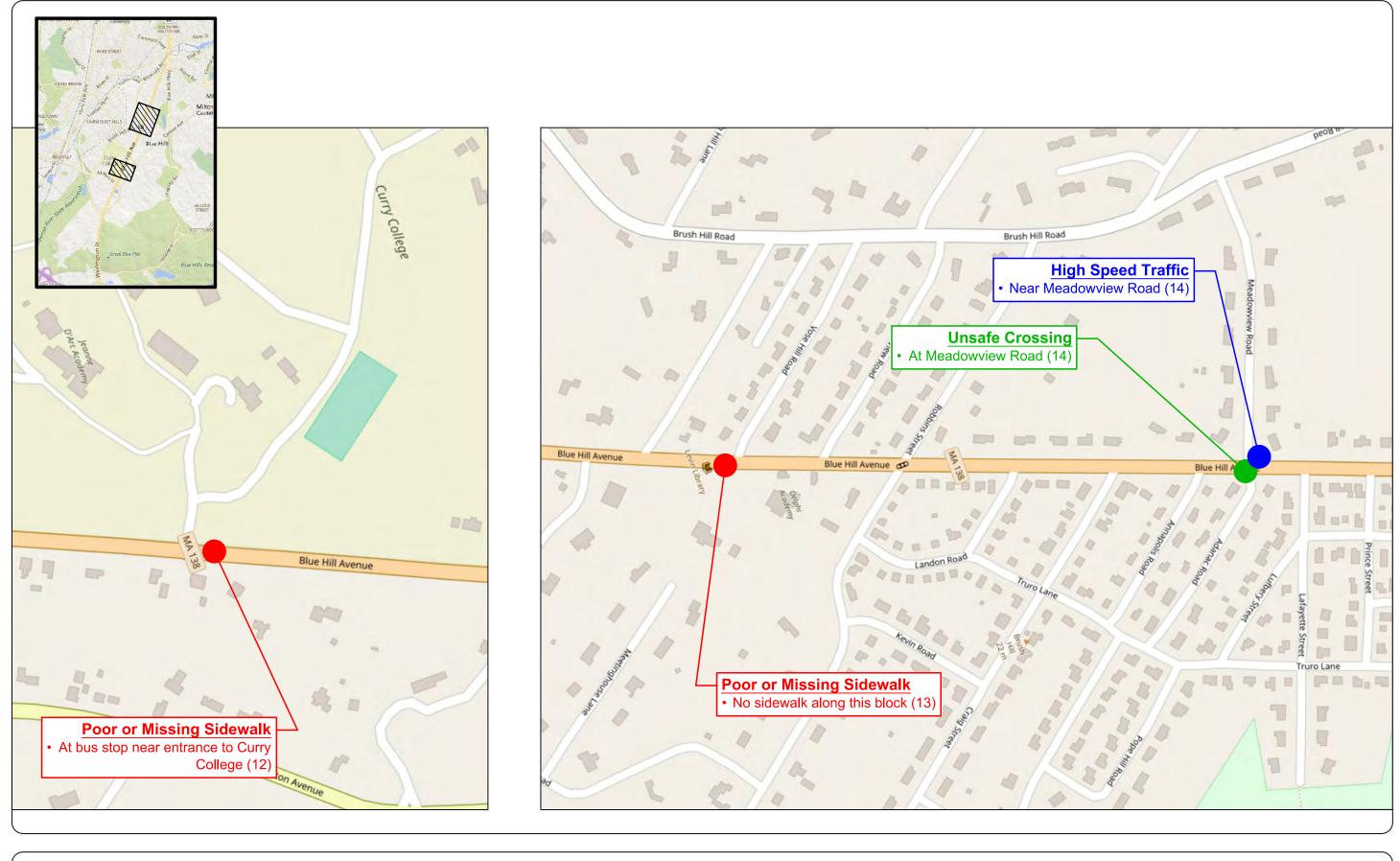
Part 1: Common Comments Encountered in the Milton Complete Streets
Prioritization Plan at Wikimapping.com



BOSTON
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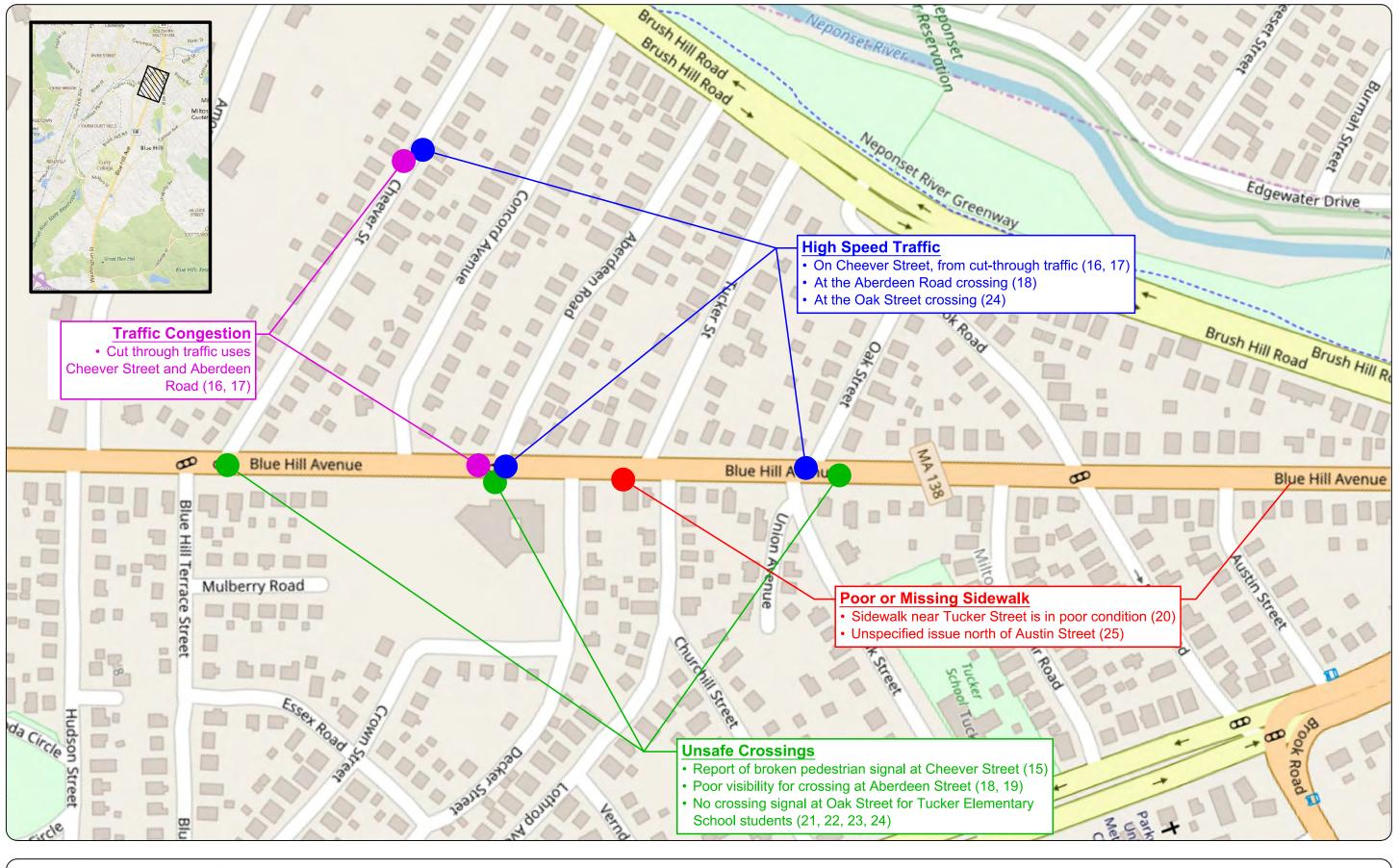
Figure G-1
Wikimap Issues
Route 138 Priority Corridor Study in Milton

Assessing Priority Corridors from the LRTP Needs Assessment: Route 138 in Milton



BOSTON REGION MPO





BOSTON REGION MPO



Figure G-3
Wikimap Issues
Route 138 Priority Corridor Study in Milton

Assessing Priority Corridors from the LRTP Needs Assessment: Route 138 in Milton

Part 2: Significant Free-Response Feedback for Questions

Survey #6 (3/22/2018)

8. Mattapan square (end of new neponset river greenway trail) to southbound rt 138 is very difficult and dangerous to navigate by bike. Signals are poorly timed, and trying to use the ped crossings brings bikes in direct conflict with high speed traffic.

Survey #10 (3/22/2018)

8. Brush hill road at robbins the wait time too long at light

Survey #13 (3/22/2018)

7. More frequent/better trash collection; better enforcement of littering laws

Survey #15 (3/22/2018)

8. I would like to see the royal street traffic stop backing up rt138 southbound traffic. One royal st car can stop a line of traffic for too much time. Let royal st wait like everyone else.

Survey #16 (3/22/2018)

4. Poor street lighting definitley #1, sidewalks in poor condition #2

Survey #18 (3/22/2018)

1. I would also be a cyclist on 138 if it felt safer.

Survey #20 (3/22/2018)

- 3. People come up on the right and it is not even a lane
- 6. I feel it has become a highway. Also need to install "don't block the box". Too many cars sit in the middle of an intersection when traffic is heavy

Survey #24 (3/22/2018)

Protect bike lanes on the whole corridor! So many people bike down this route to get to the blue hills. It is dangerous.

Survey #29 (3/22/2018)

- 2. Speed is a problem. Taking a turn is hair raising and you feel like you're going to be rear ended.
- 3. Poorly marked, people not knowing what lanes to me on coming out of mattapan.

Survey #32 (3/22/2018)

- 3. Poor sight distance on the hilltops only. Aggressive drivers pass in the shoulder and the double yellow line.
- 4. Sidewalks are lacking in the southern area, where people walk or bike to curry college. Sidewalks would help for their runners, too. Knotweed grows and blocks the shoulder between curry college and milton st.
- 5. I bike on route 138, but i try to avoid the area north of churchill st because of all the parked cars and the narrow road.
- 6. The i-93 interchange in canton, with its high-speed ramps is most in need of complete streets improvements. Elsewhere please reduce vehicle speeds or use separated bike-ped facilities.
- 7. Some drivers pass in the wide shoulders, please change the road to discourage this. A left turn lane to neponset valley pkwy would be helpful, but please do not eliminate the bike lane / shoulder.
- 8. Please add a signalized crosswalk and curb bumpouts at oak street for children going to tucker school. Consider lowering the speed limit north of blue hill terrace street and making the speed limits consistent south of there (varies between 35 and 45). A sidewalk or shared-use path would be preferred between blue hill terrace and robbins. North of blue hill terrace, if good bike lanes are not possible, consider wayfinding signage to direct bicyclists to blue hills parkway bike lanes

Survey #39 (3/22/2018)

3. Bicycle accommodations too meager. Need separated lanes. Also keep speed limit one speed entire stretch. Stupid to go 35, 45, 40 all in the stretch. 35 is fine with timed lights.

Survey #40 (3/22/2018)

- 7. Police presence at tucker school drop off and pick up. Crossing guard only responsible for crossing. Cars speed, drive on the wrong side of the road to avoid waiting for cars to turn on to oak street.
- 8. "oak street during tucker school drop off and pick up.

Survey #43 (3/22/2018)

6. After 4pm, it typically takes 25 minutes to travel less than 2 miles; from bradley rd to route 95!

Survey #46 (3/22/2018)

5. It's clear after living here for 35 years that milton is the cut through capital. People speed to cut through milton.

Survey #66 (3/23/2018)

1. I would like to be a bike rider but the road feels unsafe for bikes.

Survey #76 (3/23/2018)

8. The traffic jams on the road start at 230pm and continue until 630pm "

making the road almost impossible to use without massive waiting time in traffic.

Survey #78 (3/23/2018)

8. The section between royale and canton ave is part of a very popular bike route and should be of immediate concern. I am a member of the blue hills cycling club and ride that section several times per week and feel like a dedicated (with physical barriers) bike lane on the ski resort side of street would significantly increase safety

Survey #85 (3/23/2018)

- 3. Congestion from poorly configured student drop off at tucker in am. Back up to valentine road some days.
- 8. Regulation sidewalks the length of the road. Dedicated bike lanes the length of the road. Reduce congestion at signals due to poorly configured lights. Fix tucker school drop off so it does not have a major impact on traffic

Survey #89 (3/23/2018)

8. It is extremely frustrating for all involved pedestrians cyclists and drivers alike become frustrated with each other for the above reasons it's tough to share the road when we are all lacking in space pedestrians are definitely not accommodated especially near the blue hill area

Survey #90 (3/23/2018)

8. Slow down traffic. When i pull out of my street (pagoda street) cars are often traveling very fast. Fortunate to have blue hill terrace light nearby, but traffic coming inbound to boston travels very fast.

Survey #95 (3/23/2018)

- 3. The long wait is at the signals trying to get onto 138.
- 4. Cars usually drive to the right of the white line.
- Normally, when they say they are going to make it safer, it usually has the opposite effect. (see intersection at canton ave and brook rd.) Be sure to keep safety vehicles in mind when planning any changes.
- 8. At the end of vose hill rd, there must be some kind of bend in the road where vehicles always seem to crowd the side of the road where i'm walking.

Survey #96 (3/23/2018)

- 4. Student's safety walking to school, and crossing 138. There are guards on the parkway for high school and middle school students, but none for 138.
- 6. "the study should include how traffic on brush hill/ truman hwy affect 138. Many high park residents zoom through this corridor. As a resident of cheever street i monitor this activity and have some specific suggestions to remedy some of this behavior.

Survey #98 (3/23/2018)

- 4. I won't walk on route 138 with my children and very rarely with my dog. Also there is a ridiculous amount of trash along the side of the road
- 7. To me it is unacceptable that it is not possible to walk on that road safely.

Survey #113 (3/23/2018)

8. Don't know how anyone would dare walk along here; it is far too dangerous. Cycling is possible as most of blue hill ave has pretty good width for cycle lane, but safety could be improved tremendously as noted above.

Survey #114 (3/23/2018)

8. Fix left hand turn only out of parking lot at ski mountain. And left hand only lane to turn in.

Survey #115 (3/23/2018)

7. Even with a bike lane, i would not bike on that road. Drivers don't comply with keeping clear of bike lanes. Would bike on wide sidewalk

Survey #119 (3/23/2018)

8. Fix the lane leaving the ski mountain when you leave that you can only turn right and then on the entrance when you're trying to turn left into the parking lot make a left turn only lane.

Survey #121 (3/23/2018)

7. Ban bicycles for everyone's safety. The road is too narrow with too much traffic.

Survey #122 (3/23/2018)

3. Delays at blue hill ave/ brush hill rd, due to thacher montessori school start & end, blocking blue hill ave due to no access from brush hill rd, which would be safer for students & ease congestion on 138, which back traffic all the way down 138 towards mattapan sq

Survey #124 (3/23/2018)

3. Cars backed up going into thatcher montessori sometimes

Survey #125 (3/23/2018)

- 3. Drivers ignoring the yellow and red lights. And the trucks going to fast
- 8. Lights at brush hill and also at robbins streets need more enforcement.

Survey #128 (3/23/2018)

3. Wish we had two lines on blue hill instead of one. Kind of like randolph ave

Survey #129 (3/23/2018)

- 3. High volume at peak traffic times only-morning and evening commutes and when day care is getting out.
- 4. I do not bike or walk rte 138 because it is so dangerous for bicyclists and pedestrians. I checked the above boxes because when i see a pedestrian or bicyclist i know how insufficient the road is for them.

Survey #131 (3/23/2018)

3. I actually don't experience problems on the small part i use; typically the signal at blue hill terrace, cutting across to cheever.

Survey #134 (3/23/2018)

- 4. When making a left turn onto 138 southbound from brush hill road (canton ave side), there is no left turn arrow or concessions for bikes, so i'm often turning into traffic making a right turn from the opposite direction. Also, there are a lot of drivers that run that red light on 138 which makes pulling out dangerous.
- I still ride on 138, but generally its a meas to an end (better riding on canton ave, green st, hillside, etc...)

Survey #137 (3/23/2018)

- 3. Merging from 2 lanes into one northbound from rt128 there is no merge sign. Very dangerous.
- 7. Merge sign northbound going from 2 lanes into one.
- 8. Needmerge sign northbound from 128 from 2 lanes into 1

Survey #140 (3/23/2018)

8. Neponset valley pkwy intersection during peak times is dangerous, as those going onto or coming from neponset are very aggressive entering and exiting. Perhaps an officer directing traffic orr and onto 138? Also light at 138/canton ave/brush hill rd back up heading to 95 during afternoon commute.

Survey #141 (3/23/2018)

- 3. Biggest problem are the intersections congestion at them and the people turning from side streets don't abide by the turn on red rules. They think they can merge into slow moving oncoming traffic when in fact the turn on red rule is that you can stop and turn on red if nothing is coming.
- 4. I've walked on 138 a few times but won't do it unless there is a side walk.

Survey #144 (3/23/2018)

- 2. Between milton st and bradlee rd/atherton st. The curry college entrance is particularly dangerous.
- Safety concerns with the entering and exiting out of curry college on 138.
- 6. Between milton st and bradlee rd/atherton st. The curry college entrance is particularly dangerous.
- 8. Deal with the congestion, make it safer for people to walk from robbins to blue hill river rd

Survey #148 (3/23/2018)

- Coming from direction of robbins and moving towards 128, i'm always wary of the rise that happens at coulter and worry about any pedestrian/cyclist that might be just over the rise.
- 8. For me it's mostly about sidewalks. Even for the areas that have a useable shoulder, i do not trust the drivers to not veer into them and feel that a sidewalk is the only way that i would let my kids ride bikes/walk on this road. Even the short sections that i run along, i'm very mindful of the speed and aggressive nature of the traffic and never trust drivers. I'm happy to hear of this study and look forward to changes.

Survey #150 (3/23/2018)

4. I do a lot of running on rt. 138 between neponset and mattapan sq and to be fair find it very easy to navigate. Wide shoulders where there are no sidewalks and sidewalks where there are no shoulders.

Survey #153 (3/23/2018)

- 1. Homeowner with a driveway that has an entrance/exit on blue hill ave/rt138
- Not quite congestion, but high volume northbound at morning drivetime and southbound evening drivetime, which makes drivers more aggressive and creates dangerous conditions for entering and exiting sidestreets.
- 5. With lack of sidewalks or sidewalks only on one side of roadway, pedestrians are forced to either walk on roadway or make crossings. Frequency of aggressive drivers in travel lane passing slower cars by using the curbside/parking(or in many cases walking or bicycling)lane to pass illegally on the right creating a major danger for anyone who might be in that lane because of lack of sidewalks

Survey #155 (3/23/2018)

8. Physical barriers separating motor vehicles from pedestrians & bicyclists would allow cars and trucks to maintain high speed and everyone to maintain safety. Painting a picture of a bicycle on a road just puts bikers in danger. There needs to be jersey barrier walls or pylons separating car lanes and bike lanes.

Survey #160 (3/23/2018)

2. "pedestrian and cycling from robbins towards boston

Survey #162 (3/23/2018)

8. Turns into some driveways and side streets are too sharp, this makes for some quick crazy turns or the risk of being rear-ended

Survey #163 (3/23/2018)

8. I know it's not the most important thing, but it's really unpleasant to walk along 138 because of all of the trash/litter.

Survey #165 (3/23/2018)

8. "1/ sidewalks between valentine and amor road on other side so kids can safely walk to school and all can walk safely to the train station in mattapan

Survey #168 (3/23/2018)

8. 138 n near the blue hills is heavily congested and entering form the side streets is nearly impossible

Survey #171 (3/23/2018)

8. Better lighting, traffic light coordination, sidewalks and and bike/run/walk lanes would be a huge improvement! We have many kids that use 138 to get home and many live on the other side of 138. There are no cross walks between blue hills terrace and robbins street and is virtually impossible to cross. Our children are at risk on a daily basis.

Survey #175 (3/23/2018)

8. I consider 138 a highway, so i'm not sure adding bike lanes is such a good idea. Cyclists should be encouraged to take canton ave or brush hill road, which run parallel to 138. Adding turning lanes and second lanes at certain points would be a better use of resources.

Survey #177 (3/23/2018)

- 2. Brush hill intersects 138 in 2 locations: just over the mattapan bridge & just before trailside.
- 3. Poor sight distance just at the firehouse, where that firefighter was so terribly injured.
- 8. "nerve-wracking to take left into trailside parking lot.

Survey #181 (3/23/2018)

8. Designated bike lane markers on the existing shoulders. The road isms already wide enough. Law enforcement speed trap/write more speed tickets. I have never seen a police car positioned to monitor speed.

Survey #183 (3/23/2018)

- 6. Cars and trucks ignore the yellow and red lights on 138
- 7. More policing is needed to slow down the 18 wheelers and the cars that ignore the traffic lights and the speed limits

Survey #187 (3/23/2018)

- 3. The 18 wheelers and other trucks. They go fast down blue hill specifically between oak street and brook road. Pot holes don't help at night it sounds like the trucks are coming into our house as they go over the pot holes/dips in the street
- 8. Speeding and passing from the start of 138 after mattapan all the way up 138 pass the church. Specifically in front of our house always congested with traffic the trucks (18 wheelers) speed heavily and people are so impatient i get scared they will crash in our yard.

Survey #189 (3/23/2018)

- When trying to turn onto 138 from side streets, there are often cars parked on 138 which makes seeing oncoming vehicles very difficult.
- 7. Restrict street parking to reduce blind intersections.

Survey #196 (3/23/2018)

- 3. Milton street intersection must have a "no turn on red" sign. It used to. There is one for the opposite side of the intersection (dollar lane).
- 4. The entire area around route 138 should have sidewalks. No walk light at milton street/rte 138 intersection.
- 8. "1. No turn on red sign installed at milton street intersection with rte

Survey #201 (3/23/2018)

3. Too much development in milton leading to crowding, crime, litter

Survey #204 (3/23/2018)

Speeding on side streets by people cutting through from brush hill to 138. Very dangerous for children and people pulling out of driveways on vose hill rd and hillside rd

Survey #205 (3/23/2018)

3. I would like a bike path physically separated from cars

Survey #208 (3/23/2018)

- 2. Only between blue hills terrace and decker street.
- 3. Haven't had any problems in the short distance that i sometimes drive.
- 4. I don't have any problems but i worry about the families that park on blue hill avenue to attend our school (the campbell school in the concord baptist church).
- 5. I only walk to the mailbox in front of the building. No problems.

Survey #211 (3/23/2018)

8. Need to accommodate or provide viable alternative routes for the many cars using the very congested road. If bike lanes are added they must not further decrease room for automobiles.

Survey #213 (3/23/2018)

8. There are certain time (i.e. Pm rush hour) that the traffic is very, very heavy and other times (weekends) it is not a problem.

Survey #219 (3/23/2018)

8. Very clear pedestrian crossings near tucker school. Very dangerous area and my kids walk on 138 to get home

Survey #222 (3/23/2018)

5. I will continue to walk on 138. However, my husband and i are having discussions as to whether we feel comfortable letting our 11 year old daughter walk to pierce middle school (when she starts there in fall 2018) because she must cross 138 to do so.

Survey #223 (3/23/2018)

- 1. Just did this survey, didn't i? No acknowledgement.
- 4. Sidewalks. Driving by bikers and walkers always makes me nervous that they will fall unto road.
- 5. Sidewalks. Bad weather effects bikers and walkers

Survey #225 (3/23/2018)

3. People go through obvious red lights at an extremely high rate of speed at the intersection of blue hill river road and route 138.

Survey #228 (3/23/2018)

8. "intersection at 138 is consistently blocked by traffic at the light. Add a ââ,¬Ëœbox' to the intersection to restrict 138 traffic from blocking side road access to the roadway in all directions. Same applies to intersection at brush hill rd.

Survey #230 (3/23/2018)

- 2. Intersection of milton st / dollar lane and 138. I am a resident of milton st and use 138 as a driver. I use that intersection as a pedestrian. It is treacherous.
- Drivers speeding in and out of boston as fast as they can.Commuters using this roadway to cut through to boston causing congestion, blocking the box at every intersection.
- 4. No sidewalks at all where i live at the junction of 138 and milton st.
- Intersection of milton st and 138 and dollar lane. Need pedestrian crosswalk lights.
- 8. Police presanse everywhere. I think its a state rd so probably would be state police as opposed to milton police. Speed traps. Stop speeders, aggressive drivers. Give tickets. Patrol the intersection at milton st especially at rush hour which is backed up way past our intersection. People blocking the box we can't even get out of our street. You would meet your monthly quota of tickets in the first week of the month! Delay in traffic lights. Add pedestrian traffic lights at this intersection. Curry college kids jog along this area all the time. It is treacherous to all! I would venture to say that most of the drivers coming in and out of boston on 138 are not even milton residents! We should put up toll booths!

Survey #233 (3/23/2018)

- Inability to cross over rte 138 at dollar lane due to drivers consistently blocking the intersection by ignoring the changing traffic light.
- 7. Increased police presence to manage scofflaws (speed, obstructing intersections). Designate neighboring streets oneway at peak traffic time (2pm-7pm) to deter high speed cut through traffic.
- 8. "1. Intersection of rte 138 and dollar lane is obstructed daily at peak traffic times by vehicles ignoring the changing light. signage to remind drivers not to ""block the box"" and enforcement.

Survey #235 (3/23/2018)

3. Occasionally there are aggressive drivers, but 138 feels safer than it 28.

Survey #238 (3/23/2018)

- 3. As a parent parking and walking to tucker, i have been nearly sideswiped / run over getting out of my car. I am new to milton and disturbed by the aggressive, speedy drivers in an area where children are walking. This road needs speed bumps, as it's treated as a commuter cut-through. It's also completely unkept, and i'm surprised and disappointed that the town doesn't do more to encourage pride of ownership, litter clean-up, and tree planting.
- 4. The tucker crossing guard stays in her car, unless she notices that someone is coming. She needs to be out of her car and off of her phone. She is a very nice person, but i've had to try to get her attention in order to cross multiple times, and that's not what she's being paid for.
- 7. If the road was made to discourage people from speeding, with either more traffic lights, or speed bumps, there would be less tailgating, people would feel more comfortable biking, walking, etc. Make it friendlier for people walking and biking, less accommodating for people in their cars zooming down the road to their destination.

Survey #240 (3/23/2018)

4. Passing cars on the right when another car is stopped or taking a left turn is very dangerous for cyclists. High rate of speed is also dangerous to cyclists.

Survey #241 (3/23/2018)

The sidewalks stopped abruptly and there is no safe place to walk.The sidewalks built nearer to the fire station area should have a barrier from the traffic.

Survey #244 (3/23/2018)

- Also an abutting resident. Maintaining the sides of the roads would help both drivers and pedestrians. There is little to no maintenance currently between atherton and royall, i.e., grass not cut, debris such as fallen tree limps and animal carcasses not removed.
- 4. Maintaining the off road areas running along rte 138 (mowing, debris removal) between atherton and royall

Survey #246 (3/23/2018)

1. I would ride my ride, if the bike lane was more guarded

Survey #247 (3/23/2018)

- Only if i am driving to the mattapan station at rush hour. It is much better though than it used to be with the addition of the second lane down at the intersection with blue hills pkwy and brush hill road.
- 4. Please, please, please do not add bike lanes to the highway. It is too dangerous and the people who live along the street use those lanes for guest parking since the houses are small and have little parking in them.
- I do not bike and do not wish to see biking on the busy street.Sidewalks need some help, but are not too bad.
- No bikes!! Please no bikes on this road. It is a foolish idea as it was on truman hwy. Sooner or later someone will get killed or severley injured.
- 7. There is no bus that goes to either mattapan or ashmont.
- 8. I appreciate what you did with the intersection with blue hills terrace and brush hill road. That has made both driving and walking easier and better. Taking away the second lane at the intersection will make the traffic back up again as it used to. I find the right hand lane perfect for getting over to the right turn to the station. Do not change that for bike lanes that will not be used enough to justify taking away these and the parking lanes. Better marked crossing lanes especially at blue hills terrace would be good as a lot of kids cross over there before putting making the often dangerous trek on blue hills terrace to school.

Survey #249 (3/23/2018)

8. The traffic light congestion at brushhill road/138/canton ave intersection

Survey #250 (3/23/2018)

- 3. Do not add a bicycle lane. Speed limit not adhered too, aggressive drivers.
- 6. Sidewalks for upper blue hill avenue after robbins street not available for pedestrians on either side, making it dangerous to walk.
- 7. "accommodate pedestrians only! Reduce speed in areas from 45 mph to 35 mph.

Survey #256 (3/23/2018)

3. Therefore bottle necks at every intersection. Reducing lanes in an intersection will always make traffic problems.

Survey #260 (3/23/2018)

- 4. Terrifying road on a bicycle. I would be so grateful for any improvements.
- 8. Sure would be great if you could bicycle safely from blue hills to mattapan square.

Survey #262 (3/23/2018)

8. "as a milton resident i drive on 138 all the time. I try to avoid using 138 in the evenings due to traffic congestion.

Survey #265 (3/23/2018)

- 2. Tucker school area, sometimes no crossing guard for kids and parents.
- 4. Need crosswalks and pedestrian activated light at 138 and brush hill road, need enforcement of no left turn going north at brush hill road or a left turn signal.
- 8. "brush hill road pedestrian crossing and crosswalk.

Survey #269 (3/23/2018)

- 7. Increase police presence to subdue aggressive driving.
- 8. Blue hills ski area can be a nightmare during the winter.

Survey #270 (3/23/2018)

- 6. None, the issue relates to safety for drivers, not bikers.
- 7. The road needs to be reevaluated to reduce deadly crashes, possibly by reducing to 1 lane each way with a central turn lane (reducing multiple lanes of high speed traffic).

Survey #271 (3/23/2018)

8. Shoulder road conditions can be difficult; pot holes, rain/water pooling, debris, and markings.

Survey #272 (3/23/2018)

- 6. Specifically at brook road and 138; automotive speeding traffic to make the light, backup traffic to make left turn
- 8. The only truck (heavy vehicle), and other traffic pass-through access from 138 to route 28 is at brook road. There is heavy traffic at this intersection especially with all the trucks making the turn onto this one lane, heavily congested/populated street. The traffic from brook rd (starting at blue hills parkway) often backs up onto and up route 138, especially at rush hour.

Survey #275 (3/23/2018)

7. Improve timing of lights at both royal and brush hill

Survey #279 (3/23/2018)

3. The largest problem is there are too many vehicles traveling at ridiculously fast speeds.

Survey #281 (3/23/2018)

8. With the rise of technology and awareness, this road has become a pass through for boston to i-95... Congestion especially on fridays is brutal... I'm not sure how to solve...perhaps resident stickers? More hourly restrictions for usage

Survey #285 (3/23/2018)

Crossing in blue hills reservation from one section to other on either side of 138

Survey #286 (3/23/2018)

8. Every intersection with traffic lights needs to have a delayed green from the side streets because people run the red lights on 138 all the time

Survey #287 (3/23/2018)

- 7. Widening the roads in milton to add left hand turns will just make milton more of a cut through town. Slow traffic down, plant trees (traffic calming measure) add safe biking and walking routes as well as more public transportation.
- 8. Slow traffic down! Milton is a cut through traffic town cars need to slow down!!! Add trees etc and safe off the road walking and biking options.

Survey #288 (3/23/2018)

8. There are no side walks between bradlee and robbins. Traffic especially from curry college to blue hill terrace often greatly exceeds speed limits. The congestion is so bad between 3pm and 6pm from curry college to royal st that the road is virtually unusable. Traffic coming from brush hill road onto rt 138 take right turns on red into oncoming traffic and without coming to a stop.

Survey #292 (3/23/2018)

7. Need left hand turn signal at brush hill and blue hill

Survey #293 (3/23/2018)

7. Addition of sidewalks beyond robbins st (going towards canton)

Survey #294 (3/23/2018)

- 3. Big trucks parked at intersection totally block vision 100% when trying to get onto rt 138 from side streets; cars go too fast as well on rt 138.
- 4. N/a, i do not walk or bike on this crazy street, rt 138
- 7. Do not add left-turn lanes...it is the side streets that need to figure out access to rt 138!

Survey #295 (3/23/2018)

6. Tucker school has kids who walk and bike to school bus 138.

Survey #296 (3/23/2018)

8. The idea that milton was ever voted as second best town to live in in america a few times was laughable. I feel trapped living in milton next to route 138 because it has no sidewalks. I would never feel comfortable walking a dog around milton street, canton street, route 138...just pick a street.

Survey #299 (3/23/2018)

8. Lighting, bike lanes, slowing the speed of traffic.

Survey #300 (3/23/2018)

- 3. Every morning i hesitate before venturing onto blue hill ave when the light changes to green due to aggressive drivers who run red lights. They may not value the lives they are endangering by doing so. However, i want to venture out safely without fear of being hit.
- 8. I feel i have addressed all concerns i can think of at the moment.

Survey #302 (3/23/2018)

- 4. I live at the corner of cheever st. And 138, and often cross there with my children. I find the crosswalk light responsive, however the stop line for vehicles is not clearly marked, and cars often will stop well beyond the stop line. I have also seen people run this light many times, particularly when heading southbound, coming up over the hill. I have almost been hit while carrying/strolling my children multiple times, and once returned from bringing my daughter to pre-school to find an accident scene in the crosswalk i had been in just moments before. Residents often don't shovel or rake access to ramps and crosswalk lights.
- 7. Better speed control (people drive like they're still on the highway) and more obviously visible crosswalks!
- 8. Per above, the cheever/blue hills terrace/138 intersection is especially treacherous for crossing as a pedestrian particularly because it's an unusually shaped intersection and cars often don't stop at the proper stop line or simply run through the light. At a minimum, please re-paint the stop line in the southbound lane to make the proper stop area more obvious. Lack of sidewalks after amor road prevent me from walking along 138 without having to cross to the other side.

Survey #304 (3/23/2018)

7. Bring the boston subway closer to the center of milton.

Survey #310 (3/23/2018)

8. Left turn lane at neponset valley parkway when proceeding toward milton/mattapan and better sidewalks and crossings near the blue hills reservation and blue hills trail museum. Speed limits more clearly marked throughout.

Survey #311 (3/23/2018)

8. Southbound traffic in the evening can be very bad. Most of this traffic is headed for the i93 junction in canton.

Survey #312 (3/23/2018)

- 6. From mattapan to the blue hills in canton is 4 miles that i would like to access safely for walking and biking.
- The number of semi trucks that use this route into boston should be considered when developing the area.

Survey #315 (3/23/2018)

- Not really any destination close-by, so rarely walk. Generally used as a route to hwy. So not walking that far. Have walked to curry, about 3.5 miles rt, and some areas without sidewalks were dangerous.
- Adding more destinations close enough to walk to would be the only reason/improvement that would get me to walk on that road.

Survey #317 (3/23/2018)

- 5. It's a highway no bicycles should be alowed
- 8. Traffic is horendious headed south from 3 pm until 7 pm. Perhaps a traffic study is necessary

Survey #319 (3/23/2018)

- 3. Drivers routinely go through the red light at the corner of 138 and cheever street/blue hill terrace. It is very dangerous!
- 4. The light at 138 and blue hill terrace seems to not be noticed by many drivers which creates a dangerous situation

Survey #320 (3/23/2018)

2. I live on milton st., directly off of 138. I usually take canton ave., but have to cross over 138, on dollar lane that turns into milton st. Very often drivers block the intersection so that i can't cross when my light is green. It's enormously frustrating.

Survey #321 (3/23/2018)

7. Add second lane south between royal and mueseum. Would decrease congestion.

Survey #322 (3/23/2018)

8. Aggressive drivers in speeding vehicles is a bad combination. People need to slow down, perhaps more traffice lights, reduce maximum speed and visible police presence.

Survey #322 (Survey #323)

- 1. Blue hill pkwy to canton ave. To route 138 or blue hill ave to unquity road to route 138"
- Blue hill recreation for hiking or sking traffic adds to the traffic congestion. Families trying find parking or waiting to get into the parking lots.
- 3. Bicycling or walking is very dangerous on either sides of the road especially trying to cross route 138

Survey #326 (3/23/2018)

- 1. I would be a bicyclist if i felt safe on blue hill ave, but i don't.
- 3. Inadequate bike lanes and sidewalks, so safety concerns as a driver when a runner is in the road.

Survey #333 (3/23/2018)

- 6. It's a drag strip....cars excelerate from one end to the other
- 8. I have heard that there is a concern for students from tucker school who cross 138. I want it to be safe for them. I want to afford them the same safety measures that we put in place for students at the other elementary schools in milton...

Survey #334 (3/23/2018)

4. Aggressive drivers and "poor bike manners" should not be combined categories. They are separate issues of separate importance with separate solutions.

Survey #343 (3/24/2018)

- I have been using 138 by bicycle for 50 years. As a young child, it was then safe to ride from my home in stoughton to my cousin's house in hyde park.
- Going north feels reasonably safe once you get past the ski area, due to the generous width of the shoulder. Traveling south is perilous due to the entry of vehicles and the overdevelopment on the canton stretch at royall street.
- 4. If sidewalks are widened, the resultant narrowing of the shoulders and high granite curbs can make things worse for cyclists. For example, the recent sidewalk improvements on canton ave. Needs to be balanced. Curbs need to be softer so as not to cause a bicycle to crash when you get squeezed into them by a passing vehicle.
- High volume of trucks. Cars tend to tailgate trucks, blocking their view of the shoulder. Cars passing without sufficient space, at high speeds create potentially deadly scenarios.
- 6. The canton stretch is deadly going south. The stretch from 128 to royall street needs to be divided. Going north, bicycles turning right from blue hill river road have a perilous stretch to the milton line. A protected bike lane from blue hill river road to canton ave would be a huge improvement.
- 7. This stretch of 138 is obviously an important artery. Aggressive traffic enforcement could provide an immediate benefit. The goal should be a smooth flow of vehicles at a safe and calm pace. Too many drivers use milton and the dcr roads as a commuting "short cut" trying to save time by avoiding the highway. This has destroyed the recreational utility of the dcr roads during commuter hours.

 Aggressive enforcement would deter some of this and get the vehicles back on the newly improved highway where they belong. Milton also needs to talk some sense into canton, who have let development at royall street/blue hill river road create a perilous situation.

 Survey #344 (3/24/2018)
- 4. Crossing at oak for tucker school-very little respect by vehicles for crosswalk!
- 7. Light/better marked crossing at oak for tucker school students.
- 8. Sidewalks widened/improved; more monitoring of speeding and aggressive driving; improve crossing at oak st.

Survey #345 (3/24/2018)

8. Traffic moves very fast with high volume and no dedicated bike lane only shoulder. A rumble strip between drivers lane and well marked bike lane would be awesome. Narrow the driver lane should help slow speed down.

Survey #346 (3/24/2018)

- 3. Not crashes, people driving too fast, trying to inappropriately pass, being on their phones.
- 4. "138 connects the city to blue hills, the region's most important "wild space" for outdoor recreation and the roadway there is virtually no way to access it by walking, running or cycling unless you want to risk your life with the insane way people drive through there. Which i have done. There needs to be a dedicated, physically protected way to move non vehicular traffic. And traffic calming "€ people are driving far too fast. More public transit would also be welcome. Turning very road in mass into i-95 is shortsighted.

Survey #348 (3/24/2018)

3. High volume is only encountered at commuter rush hour

Survey #349 (3/24/2018)

- Intersetions of neponset valley parkway and brush hill road turning into south bound lane at these intersections slowing the traffic back on 138.
- 6. See above about neponset valley road and brush hill road right hand turns onto south bound 138.
- 7. Light on neponset valley and 138 road with no right on red signs, put up no right on red at brush hill road and 138
- Light on neponset valley and 138 road with no right on red signs, put up no right on red at brush hill road and 138

Survey #352 (3/24/2018)

- 3. The closer you get to 95 intersecetion the worse traffic gets, but all lights are long
- 7. Ban trucks coming off of 95- dangerous and cause congestion

Survey #354 (3/24/2018)

- 2. Mostly from neponset valley parkway, into stoughton
- 3. During "rush hour" times it can take about an hour to get from neposet valley parkway/rt 138, through 2 traffic lights to the highway exit
- 8. The traffic congestion is getting out of control. It shouldn't take 45 min to an hour to drive a mile & a half. It's super frustrating for drivers, and i'm guessing this is leading to fender benders because of driver frustration. Also, coming back from stoughton toward hyde park, just before blue hills, where 2 lakes merge into 1 lane. This is a problem area. Nobody wants to merge and drivers get very aggressive.

Survey #356 (3/24/2018)

8. Way too much congestion!...the waits along rte. 138 during late afternoon and drive home hours are horrific.

Survey #357 (3/24/2018)

- 4. What percent of travelers on 138 are bicyclists? Less tha 1%
- 8. Looks like this study is gearing up for an outcome pushing for bike lanes and sidewalks. A good and noble idea, but a small percentage of users. I suggest focusing on better lighting, signage, traffic safety enforcement

Survey #360 (3/24/2018)

8. Cars on 138 very often run red lights so cross traffic lights seem very quick as cars have to hesitate before crossing 138 despite having a green light

Survey #362 (3/24/2018)

- 3. People speed, follow too closely. They treat it as a high speed highway.
- 4. Snow not cleared off sidewalks, so have to walk in road with extremely fast vehicles. Cars don't stop when i cross at a crosswalk. I think they don't think of it as a road that pedestrians use and seem surprised to see a pedestrian trying to cross.
- 7. Slow down vehicle traffic and make pedestrian crossings safer.
- 8. The crossing by tucker school is the one that i use the most. I see children crossing often at this and nearby pedestrian crossings in the direction away from mattapan, and cars don't seem to stop. Often 4 or 5 cars will pass me before one will stop. If there were some traffic slowing devices or enforcement of speeding laws and stopping for pedestrians or something along those lines, i think that would help. If there was some way to signal to drivers that this road is not a high speed highway that would help as well.

Survey #369 (3/24/2018)

- 7. Lower speed limit to 40mph and to 30 mph at intersections plus strict enforcement by police including general high police visibility.
- 8. State should hold contractors to a higher quality of repair when they open the road.

Survey #370 (3/24/2018)

7. My daughter had a bad crash recently by someone pulling out of a gas station, crossing the double line to go toward boston. She was traveling south on 138. We have seen several of the same accidents in that area. They need a way to cross 138 from those stations safely without hitting incoming traffic.

Survey #371 (3/24/2018)

8. Although there are some bike lanes, traffic on 138 goes way too fast for me to feel comfortable biking. I bike to work at umass boston, but i'm afraid of the bike commute between my house near pierce and curry college.

Survey #373 (3/24/2018)

2. From milton st dedham to houghtons pond. Also driving rte 138 to other towns.

Survey #377 (3/24/2018)

8. "signal at dollar lane..aggressive drivers running yellow and red lights..

Survey #379 (3/24/2018)

8. Robbins street. Drivers too fast and don't respect the signals

Survey #384 (3/24/2018)

3. People pulling around cars to pass when you stop to let a car pull out

Survey #386 (3/24/2018)

- 3. I live 3 houses in from rte 138. During morning and afternoon rush i try to avoid using it, that is, if i can. Since it is party of "my neighborhood," i'll often need 45+ minutes for a 15-20 minute trip.
- 4. I'll no longer walk nor bicycle on rte 138 even though i am a bicyclist and a walker and live 3 houses from rte 138. I am am afraid of injury, accidents, or worse.
- 5. At 66 years old, i'll no longer walk nor bicycle on rte 138 even though i am a bicyclist and a walker and live 3 houses from rte 138. I am am afraid of injury, accidents, or worse.

Survey #387 (3/24/2018)

8. This road has become too congested and people drive far too fast and are very reckless. It's a huge problem at rush hour and the traffic volume seems to only be increasing

Survey #388 (3/24/2018)

8. Road should be widened to two lanes approaching central st. Have had many drivers tailgating as i slow to make the turn.

Survey #389 (3/24/2018)

- 7. Shuttle service to mattapan station would be awesome.
- 8. Intersection with canton. It's such a bottleneck. Please improve?

Survey #393 (3/24/2018)

8. Atherton st. At the firehouse has frequent crashes. Firehouse down to the ski slopes gets all backed up to a standstill practically everyday at rush hours or if there is an accident. People use this road to cut through to the highway. Number of loud trucks is out of control especially in the wee hours of the night. I grew up right on rt 138 and parent still live there. They can't even get out of their driveway now much of the time and the dirt from the traffic and noise from trucks and motorcycles at night and early am is awful and keeps them from sleeping.

Survey #396 (3/24/2018)

8. Blinded when entering rt. 138 for cars jumping the lights

Survey #398 (3/24/2018)

8. I think there needs to be some sort of pedestrian crossing light or flashing light at oak st., where the kids cross for school. I've seen cars try to pass on the right when the crossing guard has traffic stopped, and almost hit a kid stepping off the sidewalk!

Survey #399 (3/24/2018)

- 7. Balance royal st. Traffic lights so that royal st wait (backup) is same as the rte. 138 vehicles
- 8. Traffic lights must favor rte.138 (the arterial) flow, even if side streets get backed up. It is wrong to phase the traffic lights to exit five or so cars from royal st. While backing 138 southbound volume to neponset valley parkway..

Survey #403 (3/24/2018)

8. 1) change speed limit. Why is is 45 mph after robbins, heading toward mattapan? That is a very populated area with a lot of side streets. When pulling out of side streets the line of sight is reduced due to people parking on blue hill. 2) there should a nb turning lane into curry college. 3) there should be a nb turning lane into neoponset pkwy. 4) when there is standstill traffic (primarily sb in evening) drivers block the side roads and traffic light intersections. 5) it would be nice to have defined bus stops, where cars can't park so the bus can pull aside.

Survey #404 (3/24/2018)

8. Streetlight bulb replacement in a timely fashion. As a middle-and woman who commutes via the trolley, the walk home is treacherous in the dark of winter. Also please replace the hydrant that is in the middle of 5he very narrow sidewalk and forces pedestrians to choose between the street and the untrimmed hedges.

Survey #417 (3/24/2018)

8. Southbound approaching rte128/93 the traffic is real bad. It needs to be moved much better. The wait in both directions is bad but southbound is rediculous. Please do something for a better, safer, easier and faster flow.

Survey #420 (3/25/2018)

3. Speed of vehicles. Also, turning in and out of neponsett pkwy

Survey #421 (3/25/2018)

- 1. I would like to be a bicyclist but feel the road is too unsafe for biking
- 8. There are many problems along 138 for drivers, pedestrians, and bicyclists, and i hope solutions are identified that can make the experience safer for everyone. However, i also don't want 138 to become more of a highway than it is already. Sections of 138 run beside and/or through the blue hills reservation and it is important to maintain the beauty of the area. For instance, no trees should be removed just to make more room for traffic. In fact, more trees should be planted as they are proven to calm traffic. Additionally, any work done should not lead to more animal deaths on the roads. Countless deer, raccoons, possums, and squirrels die along these sections of route 138. In other words, solutions that take into account the unique natural area as well as human safety should be sought.

Survey #426 (3/25/2018)

- Signage or flashing crossing light for parent/children crossing to get to tucker school.
- 8. It is very unsafe to cross at oak street to get to school. Someone is going to get hurt unless this crossing area gets a flashing light or something to warn drivers that there are people crossing.

Survey #427 (3/25/2018)

7. Flyovers and on/off ramps for local traffic. There should be parts that are limited access from side streets so that traffic can quickly move north/south, separately from local/pedestrians. Like parts of route 9 in newton

Survey #428 (3/25/2018)

8. The turn at neponset valley parkway, exits of blue hills parking lots

Survey #430 (3/25/2018)

- 2. Milton st./dollar lane where there are many accidents.
- Especially milton st./dollar lane area. From curry college to the blue hills needs to be redone.
- 8. Milton st./dollar lane continual crashes, and the massive line of traffic that forms heading to 95 from 2pm-7pm.

Survey #431 (3/25/2018)

- 6. Especially milton st./dollar lane curry on past the blue hills.
- 8. Milton st./dollar lane from 2pm-7pm headed toward 95 is ridiculous.

Survey #434 (3/25/2018)

- 6. Route 138 is a congested and poorly engineered state highway. Adding accommodations for pedestrians and bicyclists is not a safe or smart idea. Traffic backs up daily for miles, beginning at the poorly timed signals at the canton gas stations.
- 7. Since the traffic signals were installed at the canton gas stations a few years ago, backups have increased by several miles in both directions. The deadly "climbing lane" heading north just after the gas stations was a poor decision i travel this road at least once daily, and cannot believe the lack of merging signs, painted roadway lines, etc. That leads to the game of "chicken" with merging lanes. Whatever the reason for this lane drop, by allowing a maximum of 5 extra cars to exit the intersection and climb into this lane is offset by the road rage and inconsiderate drivers that are experienced here.
- 8. Having the long red light for north/south traffic at royall st intersection is infuriating for drivers stacked back for miles, while allowing multiple right/left/straight arrows allow minimum volume of traffic sometimes one single car to halt the majority of traffic volume through this intersection. The traffic backups for the accommodation of a few cars makes absolutely no sense here. Royall should have another exit or entrance elsewhere on 138 not necessarily in an intersection with multiple retail and gas stations congesting this area. Has anyone ever measured the miles that 138 backs up on any given weekday for several hours each afternoon?

Survey #435 (3/25/2018)

8. The lights at 138 & royall st. Need to be adjusted to allow 138 south traffic to move more freely, easing backups along 138 although making royall st. Vehicles wait longer.

Survey #436 (3/25/2018)

- 3. Being passed on the left by tractor trailers, speeding, drunk drivers
- 7. Local law enforcement actually pulling people over
- 8. Between brook rd and 28 (mattapan square) is the worst part.

 Trucks have driven into homes, cars into fences. You can not park on
 the side of the road in that area unless you want a new car.

Survey #437 (3/25/2018)

- 7. Reduce side street congestion on robbins street. It is a high speed cut through.
- 8. Lower speeds and reduce congestion approaching the blue hills area.

Survey #443 (3/25/2018)

- Sidewalks are not available on both sides of the parkway. Poor lighting makes it dangerous to cross. High speed limits in residential areas
- 8. Better lighting. Reduce speed limit from 45 mph in residential areas, especially between royall and blue hills parkway. Sidewalks on both

Survey #445 (3/25/2018)

- 1. It backs up all the way from route 128 almost to atherton for most of the afternoon."
- 8. Street signs over road when approaching major intersections with the name of the cross street.

Survey #447 (3/25/2018)

- 7. Add a "no jake brake" zone for big trucks to keep road noise at a minimum.
- Noise and congestion, commuters using the entire corridor as a "cut thru" to get to and from boston

Survey #449 (3/25/2018)

8. My highest concern for blue hills ave is that my children and i are not able to safely cross to the tucker school. Oak street is closed at school pick up and even with a crossing guard there are aggressive drivers that place all the children and adults in danger.

Survey #451 (3/25/2018)

- I am a recreational walker and occasionally have to cross rte. 138.
 Very dangerous with volume and velocity of traffic, depending on time of day.
- 3. I obey speed limits in the area. Aggressive drivers have passed me on rte. 138 (and canton avenue!) Several times. Curry college entrance is a problem -- turning traffic -- and excessive speeding/aggression of its commuters
- 4. Generally, i avoid walking routes that cross 138 bc of these issues -sadly, accessing dcr sidewalks on neponset valley parkway is extremely hazardous!
- I am not a cyclist but might be tempted if safe provisions were made for it.
- 6. Problem areas for all accessibility: thacher montessori, blue hills reservation trails (royall street to canton ave.); fuller village; curry college; atherton fire station, delphi academy, concord baptist church, neponset parkway intersection
- 7. I am not a traffic engineer but please consider additional traffic lights, flashing speed-limit signs, and pedestrian-activated / flashing crossing signs
- 8. Problem areas for all accessibility: thacher montessori school, blue hills reservation trails (royall street to canton ave.); fuller village; curry college; atherton fire station, delphi academy, concord baptist church, neponset parkway/green street/brush hill road intersections. Why is traffic congestion so toxic at p.m. Rush hour south-bound? Ned corcoran (local attorney) claims it is because traffic lights are not properly synchronized --- really?!

Survey #453 (3/25/2018)

8. Heading west on 138 toward canton is always extremely slow particularly through lights at brish hill road and lights at royall street. It's awful at afternoon rush hour.

Survey #457 (3/25/2018)

3. I actually feel like the section between royal st. And canton ave./brush hill rd. Is not bad at all.

Survey #459 (3/25/2018)

- 6. None. Route 138 is too narrow to accommodate a bike and pedestrian lane. Doing so could limit the ability to bypass turning vehicles and exacerbate the traffic issue.
- Smarter traffic lights would be great to allow more traffic to pass. It may also prevent the aggressiveness of drivers trying to turn onto 138. Also, citations to drivers who block intersections would be helpful.

Survey #462 (3/26/2018)

4. I don't believe it is safe to walk or bicycle on route 138

Survey #463 (3/26/2018)

- 2. To the blue hills or to canton or to get on 128 to westwood
- 8. Cars fly and tailgate, dangerous at times...those on bikes need their own space- not safe

Survey #466 (3/26/2018)

- 4. Along with vehicle drivers there are also cyclists who aggravate the issues with their own inconsiderate actions.
- 5. Cycling less on it each year because of speed of cars but especially large trucks.

Survey #470 (3/26/2018)

- 4. When coming off the neponset trail, how are you supposed to go south on 138/ blue hills parkway? It's not very clear or easy transission.
- 5. I tend to drive more than ride a bike on 138 save for the early connector from mattapan to blue hills parkway.

Survey #472 (3/26/2018)

- 3. Numerous potholes and construction at inconvenient point on the road
- 8. Improve the general congestion that is found in blue hill road from 4-7pm, add left turn lanes, and accommodate space for bikers and pedestrians by creating better sidewalks and shoulders.

Survey #479 (3/26/2018)

4. Bicycles in the way of traffic, bad behavior of bicycle riders, probably due to insufficient space for bicycle traffic. Separate off road path should be created away from vehicle traffic.

Survey #480 (3/27/2018)

3. Bicyclists are not vehicles nor pedestrians. They need to obey the rules of the road not when it's just convenient for them

Survey #481 (3/27/2018)

8. Mail a flyer out about bicyclists obey the rules of the road. The led lights might last longer or save the town money but they are poor lighting

Survey #482 (3/27/2018)

- 3. In my car taking my kids to blue hills ski area getting from canton ave. On to 138 at brush hill rd crossing. Great to have an arrow. Great if 138 cars did not go in to the intersection when teh light is red!. On my bicycle, great to have bike lanes painted on the shoulder and have the shoulder swept to keep broken glass from giving my a flat tire.
- 4. During traffic times i cannot make a left turn from green st north bound. Probably no solution.
- 8. Since cars leaving blue hills ski area are supposed to go right, anyone who wants to go south on 138 needs to go to the brush hill rd. Intersection via canton ave. So, make sure that intersection works for those cars. Now they compete to turn with cars comign the other way on brush hill rd and cars on 138 block the intersection during heavy traffic red lights. It's very stressful.

Survey #484 (3/27/2018)

4. Crossing 138 from green street to go north, there should be a pedestrian crossing light.

Survey #486 (3/27/2018)

- 1. I have 3 children under the age of 10 who walk to school on 138
- The sections don't all make sense to me. Our family uses the section between cheever/blue hill terrace and the intersection with blue hills parkway at the mattapan/milton line
- 3. My 10 year old sometimes walks to and from school by herself. She has to cross rte 138 at oak street. There are no signs warning drivers that a school is there and even when the crossing guard is present, cars don't always stop. We have been in the crosswalk and had cars pass the stopped vehicules at high speed. It's only a matter of time before a child gets hit.
- 4. The sidwalk on the east side of 138 is narrow and bumpy with tree roots. Some residents have overgrown landscape trees that obstruct the sidewalk. It's impossible in some places for one adult and one child to walk side by side (near churchill street)
- 5. There really should be a light at oak street and signaling that there is an elementary school around the corner. The other concern is that on the east side on craig street, there is a speed limit sign that says 45. So drivers speed up and have trouble slowing down when they get to the school. How is the increased speed limit ok in the middle of a town? Also, i can't recall a good sidewalk near curry college and atherton. I live close enough i could bike to the blue hills reservation but it really isn't possible safely right now.
- 8. I do not feel that it is safe at this time for my children to walk along rte 138 to school. Cars are accelerating as they get closer- not slowing down, there is no signaling and no crosswalk by the school. Further south, there is no sidewalk or bike lane- making access to the blue hills reservation by foot or bike impossible.

Survey #487 (3/27/2018)

- 4. Terribly unsafe attempting to cross 138 from green st to lot across the street. Which i do several times a week to run in the blue hills. I reside on green st.
- 8. Please add safe crosswalks between parking areas across 138 to access blue hill and little blue hiking areas.

Survey #488 (3/27/2018)

1. Many crashes in our yard- total one of our vehicles

Survey #491 (3/28/2018)

- 5. No one ploughs the snow from the sidewalks further up. Like between amor and robbins.
- 8. Why does 138 not have a legitimate mbta bus? There is the subsidiary bus but its not easily recognisable, not trackable on the mbta apps, only runs once an hour, there are no bus stops. With the coming blue hill ave commuter rail train it would be nice to get to mattapan on a regular, easily predictable, frequent bus.

Survey #493 (3/28/2018)

8. Would like a more frequent and legit bus that runs more than once an hour and can be tracked on apps. Would be nice to see sidewalk clearing of snow and over grown foliage.

Survey #495 (3/28/2018)

8. There really needs to be a way to cross 138 near the trailside museum. Particularly at green street. The area is supposed to be for, and promote, hiking and outdoor activities and yet doesn't allow for that in a safe way. Please add a crosswalk and slow traffic by green street!!

Survey #497 (3/28/2018)

- 4. Cars speeding. Cars coming into shoulder due to being on cell phone.
- 6. 138 and neponset valley is a particularly dangerous interesection when riding on a bike. Either direction.

Survey #500 (3/29/2018)

- Traveling southbound in the early morning, cars routinely pass me in no-passing zones. Sometimes cars pass me on the right, endangering cyclists and pedestrians.
- 7. Coming from their new warehouse in hyde park, trucks from katsiroubas produce routinely park along the side of the road southbound, early mornings, with drivers picking up coffee at dunkin donuts. They are blocking the right travel lane daily.
- 8. Mass dot's planned intersection improvement at neponset valley parkway would impair northbound cyclists with a very narrow lane and impatient drivers swerving around left-turning traffic. Mass dot restriped the lanes here a few years ago and safety was greatly impaired when southbound travel lanes did not align with northbound lanes. My concerns were addressed to milton police who had mass dot change the lane striping.

Survey #502 (3/30/2018)

1. If there were a way to prevent drivers from cutting to the right to pass on 138, that would be great."

Survey #504 (3/30/2018)

3. People do not care for cyclists and do not give enough room on the road

Survey #506 (3/30/2018)

3. Just last week an angry driver was tailgating us on 138 and passed in front

Survey #509 (3/30/2018)

- 1. Would love to see this become more pedestrian and bike friendly and use some traffic calming measures.
- 5. Unpleasant to walk. There are no street trees. It feels very highway like, not residential. The roadway is too wide. It can be narrowed quite a bit and still get the same volume of traffic through. Need traffic calming measures.
- 7. We would like traffic cameras on the lights that give tickets to cars for speeding.
- 8. "there is a large need for a complete streets implementation from robbins street towards the blue hills parkway in mattapan, that would connect to the dcr bikeway as well as pleasant walking experiences down here as well.

Survey #512 (3/31/2018)

- 2. I have lived on a side street that directly connects to route 138 for 27 years and use this corridor many times per day.
- 3. Cars racing through red lights at crosswalk in front of the concord baptist while pedestrians are attempting to cross this main street. I was also rear-ended while stopped to turn onto my side street and nearly killed by a car going 55mph claiming he did not see me during daylight hours with no adverse weather conditions. My car was pushed across the opposite lane into oncoming traffic.
- 4. We were nearly hit walking on the sidewalk by a speeding car that lost control and came up over the curve. We have had multiple close calls using the crosswalk lights when cars don't stop.
- 7. Restrict commercial development on this already overburdened main road and the impact of more lights and heavy traffic turning on/off this busy roadway. More bus service and shuttles connecting to mattapan, hyde park and e milton enabling local residence and college students other options besides driving.
- 8. Absolutely a signal cross walk at the entrance to the side street for tucker school!!! Electronic speed monitoring devices that alert drivers to the speed limit and their traveling speed. Repair sidewalks. Move the signal light box higher on the pole at the crosswalk at concord baptist. The current height is a hazard for children on bicycles especially without helmets.

Survey #513 (3/31/2018)

- 2. I have lived on a side street off route 138 for almost 30 years and travel this corridor many times per day.
- 3. Cars passing moving traffic on the right especially at rush hour. Log backups preventing us from exiting our side street. I was injured when a car going 55mph rear ended me and pushed me into oncoming traffic as i waited on route 138 to turn onto my street.
- 4. Road grit and trash including glass that is builds up on sidewalks and to the curb.
- 5. We were nearly hit walking on the sidewalk by a speeding car that lost control and came up over the curve. We have had to run to avoid cars racing through the red light at the signalized cross walk.
- 7. Absolutely a signalized cross walk at tucker school!!! Bus & shuttle service to mattapan, hyde park and east milton accessible to local residents and college students in an effort to decrease vehicle congestion.
- 8. Absolutely a signalized crossing at tucker school and improved signal at the concord baptist church. Restricting commercial development on this already danderous and congested roadway.

Survey #518 (4/1/2018)

- 6. Intersection of blue hill ave and oak st to provide safe access to the school for crossing and turning vehicles.
- 8. Safer access to tucker school at the oak street entrance to route 138

Survey #521 (4/1/2018)

Motorists near tucker elementary school frequently drive too fast, and fail to yield to pedestrians at crosswalks. This is an especially big problem during hours when crossing guards are not present.

Survey #522 (4/1/2018)

- 3. Making a left turn from brush hill rd (nw) onto brush hill ave (ssw) at certain times of day makes for 10 minute wait times at the light.
- Clearly marking the southbound lanes overhead better as you are heading into them might reduce a ton of last-minute traffic sorting that happens there.

Survey #524 (4/1/2018)

8. The traffic congestion on route 138 has increased dramatically in the last year. In addition drivers speed way too much on this very residential road, which makes biking and walking along it highly dangerous. Any help from this study would be great!

Survey #527 (4/1/2018)

- 4. I would avoid riding a bicycle on rt 138, because the road is too narrow and cars drive too fast.
- 8. In general, cars drive too fast on milton roads. I would like to see traffic calming measures on most roads, especially blue hills parkway.

Survey #531 (4/1/2018)

2. My children attend tucker school and we have friends at various locations on and off of route 138

Survey #535 (4/1/2018)

- 5. Drivers not stopping for pedestrians in cross walk
- 8. Stop signs at the intersection of oak and blue hill avenue

Survey #535 (Survey #536)

- Vehicle accidents from brook rd stop light to blue hill ave cars are going too fast
- 7. Bettter lighting. And the cars go to fast!! More state police or milton police would help fast drivers slow down! Ticket them!

Survey #551 (4/1/2018)

- 3. See people littering out their car windows please add signs reminding of littering fines
- 4. Terrible to cross with children on foot or on bikes, rude, fast drivers
- 6. **138 & brook rd, ****138 & oak street/tucker elementary school entry from 138
- 7. Traffic light with pedestrian crossing at 138 & oak st/tucker elementary school signs regarding excessive littering
- 8. Problems safely crossing the street with children on foot or on bikes.

Survey #553 (4/1/2018)

- 2. Unsafe crossing over to the tucker school when crossing guards are not present. Walked from my home to open house one evening and literally almost was hit by a car while i was in the crosswalk. Car in right lane stopped for me to cross, car on left sped through!! Such a scary moment!! I do not think it's an isolated incident either.
- Add crossing signals or a light where the tucker is located. So dangerous for our children to cross when the crossing guards are not present.

Survey #556 (4/1/2018)

- 3. Morning traffic, aggressive drivers are a huge safety concerns for the children
- 7. Add a "school zone flash" with reduced speed. People speed to much during arrival and release time. It is not sare for the kids
- 8. I think there should be a school zone on 138 and a turning line added to turn and drop off students so people on 138 can pass (with reduced speed). Or maybe add a traffic light

Survey #559 (4/2/2018)

4. "thank you for this opportunity to contribute to these efforts.

Survey #563 (4/2/2018)

4. Blue hill ave near blue hill parkway, very narrow sidewalk due to overgrown bushes of last house on street with hydrant. This is just where blue hill ave goes to 2 lanes. It is dangerous when there are snow banks.

Survey #569 (4/2/2018)

8. I think the stretch between bradlee rd and blue hills pkw could use more clear lane markings for driving, parking and bike lanes, as well as lowering of the speed limit. This is a very residential area and there are many kids walking/crossing roads to reach milton high or tucker elementary.

Survey #576 (4/2/2018)

7. Improve access to tucker school from rt 138: the cars queuing to turn right from rt 138 onto oak st at drop-off and pick-up both blocks other traffic and makes turning left on rt 138 onto oak, union, and churchill at those times of day almost impossible.

Survey #580 (4/2/2018)

- 3. Picking up my daughter after school when there are no crossing guards is extremely challenging. Drivers just don't stop.
- 8. The sidewalks during the winter were very poorly shoveled and it was difficult to park and walk my child into school. Crossing the street is challenging as there are no cross walks, except at tucker school, and without the crossing guards, you take your life in your hands.

Survey #584 (4/2/2018)

- 1. We either drive or walk to drop-off our child at tucker elementary.
- 3. High volume during tucker elementary drop-off. Traffic backs up trying to turn onto oak street heading toward blue hills parkway.
- 8. My biggest concern for pedestrian and driver safety is in the area of tucker elementary (primarily at oak street), as well as the congestion in that area at school drop-off and pick-up.

Survey #591 (4/2/2018)

4. I would never bike or walk along this route. Too dangerous.

Survey #592 (4/2/2018)

8. Interested in seeing improvements for the children traveling to and from school. (i.e. More responsive crosswalks for pedestrians indicating they want to cross the street)

Survey #594 (4/2/2018)

- Aggressive drivers do not stop even when we're in the crosswalk. I have difficulty crossing as an adult so imagine a child crossing to get to school.
- 5. Again, drivers refuse to stop for pedestrians in the crosswalk. Crossing 138 can be very intimidating.
- 8. The intersection of blue hill ave and oak st is very difficult to cross for both adults and children and since it is a school zone it would greatly benefit from some increased safety measures (flashing lights or a stop sign)!

Survey #595 (4/2/2018)

- Route 138 is not biker/walker friendly. Too many aggressive drivers that speed down 138.
- 4. It is almost impossible to cross 138. Drivers just do not stop for people even though you're in a crosswalk. As an adult, i find myself having to step out into traffic before a driver decides to stop. Completely infuriating!
- The feeling is that 138 is for drivers only with no regard for bicyclists or walkers.
- 8. The intersection of blue hill ave and oak st. This is a crosswalk that is used by many people most of whom are children. It is an intersection that is used by many school children for tucker elementary school. Once the crossing guard is gone it is nearly impossible to get across (using the crosswalk!) Because the drivers just do not stop. This intersection would benefit from some more safety measures like walk signals or a stop sign.

Survey #603 (4/3/2018)

3. Issues at delphi academy entrance during drop-off and pick-up times for kids.

Survey #605 (4/4/2018)

- 3. Inconsistent speed limits that vary back & forth too frequently. Traffic has increased exponentially now that gps has redirected drivers away from congested se expressway, i-93/i-95. 138 is now a cut thru from boston to the south, south west as drivers have realized it cuts through from expressway to rt 128 (whatever interstate it is now called) traffic regularly backs up from royall st to milton & atherton st. This is increased by stopped traffic trying to turn left at intersections, into curry, into blue hills ski area/trailside museum.
- 4. Don't need bike lanes where road almost needs to be a highway. Bike lanes are ridiculous to expect 5-10mph vehicles to interact with 30 45 mph vehicles.
- 5. Don't need bike lanes where road almost needs to be a highway. Bike lanes are ridiculous to expect 5-10mph vehicles to interact with 30 - 45 mph vehicles. Narrowing vehicle space for a couple dozen bikers who might use a road in the course of a year is simply a waste of time & money and unsafe for all involved.
- 6. Don't need bike lanes where road almost needs to be a highway. Bike lanes are ridiculous to expect 5-10mph vehicles to interact with 30 - 45 mph vehicles. Narrowing vehicle space for a couple dozen bikers who might use a road in the course of a year is simply a waste of time & money and unsafe for all involved. Bike lanes on blue hills parkway were stupid, on rt 138 would be insane.
- 7. Rt. 138 must be improved for vehicle traffic & adding accomodations for a handful of pedestrians & bicyclists that are infrequently in the area by altering lanes where vehicles are all the time is equivalent to narrowing sidewalks to one lane in order to widen traffic lanes.
- 8. Streets that intesect 138 from robbins st to parkway are often at an angle easing left turn entrance to the side street if coming out of mattapan, but also making right turns onto side streets more difficult, especially when the corners are not plowed. Theses angled streets are Survey #607 (4/4/2018)
- 3. For cycling lots of debris on the road shoulders

Survey #609 (4/5/2018)

8. Afternoon traffic delay making turn from canton ave. Onto 138 west heading for 128"€20 minutes = too long. No immediate solution comes to mind, but it's often a looooong crawl.

Survey #614 (4/5/2018)

3. For the short distance i drive, i rarely encounter problems.

Survey #617 (4/5/2018)

8. Speed limit and lights at 4-way intersections with a left turn signal

Survey #624 (4/5/2018)

8. The corner of blue hill parkway and blue hill ave is very problematic for pedestrians. There is a right lane for the cars so you can't walk in the road. The sidewalk is very narrow and the bushes in the area are overgrown. If it snows, pedestrians are really in danger.

Survey #625 (4/5/2018)

7. Focus on drop off at tucker school: both for parents/students drop off and others that are trying to go past oak but get stuck in the school traffic.

Survey #626 (4/5/2018)

8. Turning at intersection of 138 and brush hill road. Need dedicated left from canton ave and no turn on red coming from brush hill road.

Survey #627 (4/5/2018)

8. I don't think that route 138 need big improvements

Survey #629 (4/5/2018)

The turn from canton ave to brush hill rd there is so much congestion and turning left onto 138 is an issue. Would be nice to have a shielded arrow to make the turn

Survey #632 (4/5/2018)

- 2. From unquity road to tucker elementary and down to the next traffic light to return to unquity rd. At the end of the day.
- 3. Safety concerns regarding children and families crossing to tucker elem schoo

Survey #633 (4/6/2018)

- 6. At tucker elementary they don't even have school lights
- 8. There needs to be more safety for the children who go to school there cause it's extremely dangerous and i fear for my safety when i am dropping off and picking up

Survey #638 (4/6/2018)

5. I don't live near here so i am only driving on 138.

Survey #643 (4/6/2018)

8. "need better traffic for oak street were children cross to go to school.

Survey #645 (4/6/2018)

- 3. The outbound traffic (going towards canton) can be horrific depending on the day and time of day. I avoid it at all costs using this route for inbound travel (going towards mattapan sq).
- 4. Route 138 is a major route of traffic. Anyone who goes for a "bike ride" or a "stroll" is out of their mind!
- 5. I would never use this route for pleasure purposes. There is too much pollution for the constant automobile traffic. Anyone who goes for a "bike ride" or a "stroll" is out of their mind!
- Only in neighborhoods that have houses facing the street and have existing sidewalks.

Survey #646 (4/6/2018)

- 4. Drivers speed all the time. No crosswalks. Aggressive drivers.
- 8. Add pedestrian crosswalks and bike path. Difficult crossing 138 if i am walking or biking with my kids. Sidewalks are only on one side of 138 from austin str to robbins. No sidewalks on the opposite side of the street. Only 4 to 5 pedestrian crosswalks from austin to robbins. More pedestrian crosswalks speed limit signs needed.

Survey #649 (4/7/2018)

8. It's driven on as if it were a highway. The crossing by 128 is particularly scary on a bike.

Survey #652 (4/7/2018)

8. Signal at canton ave intersection with blue hills ave

Survey #655 (4/7/2018)

- 1. My sons and their friends are snowboarders and they have actually snowboarded home along rt 138 and down green st/hemenway drive. Also, i have seen many people with ski gear walking along 138 on west side attempting to cross to eat side where blue hill ski area located.
- 2. I live one street over from 138 opposite where the ski area is, so use this section of 138 multiple times each day.
- 8. There remain today road pedestrian crossing signs to warn drivers as they approached crossing in front of blue hill ski area. Prior to repaving of 138 several years ago covered the painted white street crossing lines. I called the mass. State highways dept and they said they would repaint the crossing lines. They never did. Nick vinke 617-407-4514

Survey #657 (4/7/2018)

- 7. Left turn lane at nep valley pkwy would be a good start. A left turn into the north trailside parking lot, from 138 south, can be next to impossible even on sunday afternoon. The sight line, for traffic coming over the hill towards the parking lot is very poor.
- 8. The area north of blue hill river rd where 138 goes from 2 lanes to 1 lane does not provide for safe merging. Cars turning left into the park&ride lot a little further north create another dangerous situation for traffic traveling north. A left turn lane at the main curry college entrance would also increase safety for northbound traffic.

Survey #658 (4/7/2018)

8. "rte 138 and brush hill road.. Northbound motorists ignoring the no left turn. Eastbound motorists on brush hill turning onto 138 southbound ignoring the traffic light (this also applies to westbound motrists turning onto 138 southbound)

Survey #662 (4/8/2018)

8. From 4-6:30 pm, it may take 30-45 minutes to travel the rte 138 corridor.

Survey #666 (4/9/2018)

- 1. I use it daily to drop off kids at tucker school. Also to access route
- 4. Pedestrian / student safety need to be enhanced at intersection of oak st., for tucker school. An extra lane, perhaps, for parents queuing up for morning drop off?
- 6. I don't know all the streets you list in the options. But i would request oak st. Intersection for tucker school.
- 7. Once again, student safety and parent drop-off lane for tucker school, at oak st. Local bus service to mattapan would be nice, too.

Survey #669 (4/10/2018)

8. Rte. 138 is unbearable due to congested traffic during particular hours - between usually 4-6pm. It can take 1/2 hour to drive distance usually taking 5-10 minutes. Stay clear during that time period.

Survey #673 (4/10/2018)

8. Turning off rt 138 to brush hill road is a big problem because of terrible road conditions (pot holes) on brush hill rd. Calls to dpw are not helping!!

Survey #678 (4/10/2018)

8. I am not a driver. I do often use the mattapan/cobb's corner van. I catch this van directly across 138 from the entrance to fuller blue hill campus. This is the best location and much safer and better than the intersection at light at brush hill. This has been confirmed by the van drivers. Because of my poor vision, i have to use extreme caution when crossing. A pedestrian crosswalk would really help.

Survey #682 (4/11/2018)

- 7. Better timing of lights to reduce congestion in the evening hours heading south towards the highway. Coming off canton ave/dollar lane to get on 138 is a nightmare in the evenings.
- 8. This isn't specifically a 138 but the major intersection where blue hill ave splits to brush hill, 138 and route 28 is a cluster. The new lights and the timing has created backupson river street coming from lower mills, mattapan square is even more gridlocked and cummins highway is starting to get the same gridlock as more and more people are using that. I know it's a mass dot, dcr and city of boston problem but if you could get together and review that intersection that would be amazing and appreciated.

Survey #687 (4/11/2018)

8. Traveling along 138 at commuter hours there is lots of traffic and it is very very slow-only one lane light at corner of brush hill and 138. It only allows 3-4 cars when it turns green.

Survey #688 (4/11/2018)

7. Mark road lines in the center and sides. Use same paint material they did at beginning of crossover to fuller village and brush hill ave, from canton ave.

Survey #694 (4/11/2018)

8. Widen the road into a 4 lane highway-2 on each lane; also a service lane when it's possible. Get into the 21st century!

Survey #697 (4/11/2018)

- 1. Very heavy traffic from 2-5pm-long waits on m-128 to get to fuller village.
- 2. 138 to caroline area to parking at foster bldg at blue hill

Survey #699 (4/11/2018)

8. Too much traffic for only 2 lanes; need to slow traffic down, lower speed limits after leaving fuller village taking right; this intersection is very bad; no lights; only a stop sign.

Survey #702 (4/12/2018)

8. Canton avenue is a narrow country road winding with curves creating several hidden now dangerous driveways. 30 mph signs are ignored. Implement physical traffic bumps or rumble strips and signs (warning hidden driveway) located between southern entrance and atherton street.

Survey #706 (4/13/2018)

4. Cars parked in bike lanes! Church parking along "no parking" areas/bikelane on weekends

Survey #708 (4/13/2018)

8. Canton ave.. Is a winding narrow country road now being used as a speedway, alternative to 138, 30 mph signs are ignored. Driveways obscured by curves are now dangerous. Implement physical traffic calming rumble strips and "caution hidden driveway" signs between southern entrance from 138 and atherton street. Intersection of brush hill rd and neponset valley prkway is also dangerous because of curves and speeding traffic. Implement a traffic light with timing adjusted to traffic flow.

Survey #709 (4/13/2018)

- 1. I would definitely be a pedestrian and possibly bicyclist too but it's far too dangerous where i live near the intersection with brush hill. I even drive to go across the street to the blue hills reservation. There aren't even crosswalks or sidewalks.
- 4. The situation is so bad that i am essentially prevented from walking and biking.
- 8. Make it possible to walk between fuller village and the blue hills reservation. Also to cross from fuller village to shuttle going towards mattapan. Reduce traffic and/or improve flow on 138 between dollar lane and 95, also on brush hill rd and canton ave coming into 138. Create noise barriers from surrey lane to brush hill. Fix brush hill rd. (it's in a state of serious deterioration).

Survey #711 (4/13/2018)

I see way too many aggressive drivers. I have seen on many occasions cars passing other cars on the shoulder or across the double lines.

Survey #714 (4/15/2018)

8. Any improvements should include enforcement of traffic rules for both motorists and bicyclist. Currently, bicyclists often travel between traffic lanes and ignore stop signs.

Survey #715 (4/16/2018)

4. Bike lane/shoulder often have lots of debris (rocks, branches, trash, etc). Also, storm drains, grates, and manhole covers are often located in the shoulder/bike lane.

Survey #725 (4/17/2018)

8. Extend 2 lane road going north beyond base of blue hill traffic lights to canton ave exit

Survey #728 (4/17/2018)

3. High traffic from brush hill road to 138 between 3:30 pm and 6:30 pm

Survey #730 (4/17/2018)

3. Traffic from canton ave to brush hill road and then taking left onto 138. Help

Survey #733 (4/18/2018)

- 3. Today i witnessed a cyclist get hit by a car (prius) that was turning into the park & ride east of dunkin donuts, 4/13/18 around 6pm.
- 8. Make it safe for cyclists or prohibit cyclists if it can't be made safe. Right now it's is too dangerous!

Survey #734 (4/20/2018)

- 3. Difficulty safely crossing 138 on horseback. Vehicle operators do not comply with safety laws around equines.
- 5. Poor connectivity between blue hills hiking area and paths to royall street equestrian workout ring.
- 7. Improve safety for pedestrians and horseback riding accommodations.

Survey #735 (4/18/2018)

3. Hard to cross when going from the park and ride parking lot to the hiking trails in the blue hills.

Survey #737 (4/23/2018)

- 1. Would like to be a pedestrian from brush hill road to the blue hills reservation but there is nowhere to safely cross 138 or walk.
- 3. No place to walk as pedestrian from brush hill to blue hills musuem/other hiking areas.
- 8. As a pedestrian, there is no way to safely cross 138 to get to blue hills museum and hiking trails, nor is there any sidewalk to walk on. Also, there are no cross walks on brush hill/138 light (and no walking light either). We are only .4 miles from blue hills parking lot but cannot walk there!

Survey #738 (4/23/2018)

- 3. It feels dangerous driving on, especially at the signaled intersections but also just in general. Neponset valley intersection needs traffic signals and better turn lane to make traffic move more safely and efficiently.
- 8. Neponset valley prkwy intersection needs to be improved with signals and turn lanes. Visible cross walks/flashing lights near blue hills trailside museum and curry college. Speed restrictions through residential sections and a guardrail protected bike lane along more secluded portion.

Survey #740 (4/24/2018)

6. "students at milton high school, pierce middle school, and tucker school (elementary school) who live west of route 138 are at great

Survey #742 (4/26/2018)

- 2. Between dollar lane and upper canton avenue. My child attends thacher montessori school at 1425 blue hill ave.
- 7. Brush hill and blue hill ave intersection it is very dangerous trying to take a left on to blue hill ave. This intersection requires cars taking a left from brush hill onto blue hill ave going toward 128 and cars taking a right toward 128 to merge. If the cars taking a left yield, they will never get through the light. Having no left arrow requires drivers to be aggressive in order to get through the intersection. Also, taking a right out of thacher montessori school (where my child goes) after this intersection, is very dangerous. I then have to take an almost immediate right onto the canton ave. There should be a wider section of road there for cars behind people taking a right to safely go around so traffic isn't backed up and to prevent stopped cars waiting to turn right from being rear-ended. Also, there should be an opening in the double yellow line on blue hill ave. Where cars can turn right onto canton ave, so it's obvious that it's legal to make a right here. Finally, there should be a light at the termination of neoponsite valley parkway to assist cars turning on from blue hill ave and those turning right onto blue hill ave from the parkway. Cars turning on to blue hill ave never stop at the stop sign and i've had to slam on my breaks even going under the speed limit to avoid an accident here.

Survey #743 (4/27/2018)

8. Sidewalks along brush hill rd. Very heavy traffic volume especially near curry college and heading to truman parkway. Very dangerous to walk along!

Appendix H: MassDOT Highway Division Project Development Process

Overview of the Project Development Process

Transportation decision-making is complex and can be influenced by legislative mandates, environmental regulations, financial limitations, agency programmatic commitments, and partnering opportunities. Decision-makers and reviewing agencies, when consulted early and often throughout the project development process, can ensure that all participants understand the potential impact these factors can have on project implementation. Project development is the process that takes a transportation improvement from concept through construction.

The MassDOT Highway Division has developed a comprehensive project development process which is contained in Chapter 2 of the *MassDOT Highway Division's Project Development and Design Guide*. The eight-step process covers a range of activities extending from identification of a project need, through completion of a set of finished contract plans, to construction of the project. The sequence of decisions made through the project development process progressively narrows the project focus and, ultimately, leads to a project that addresses the identified needs. The descriptions provided below are focused on the process for a highway project, but the same basic process will need to be followed for non-highway projects as well.

1. Needs Identification

For each of the locations at which an improvement is to be implemented, MassDOT leads an effort to define the problem, establishes project goals and objectives, and defines the scope of the planning needed for implementation. To that end, it has to complete a Project Need Form (PNF), which states in general terms the deficiencies or needs related to the transportation facility or location. The PNF documents the problems and explains why corrective action is needed. For this study, the information defining the need for the project will be drawn primarily, perhaps exclusively, from the present report. Also, at this point in the process, MassDOT meets with potential participants, such as the Metropolitan Planning Organization (MPO) and community members, to allow for an informal review of the project.

The PNF is reviewed by the MassDOT Highway Division district office whose jurisdiction includes the location of the proposed project. MassDOT also sends the PNF to the MPO, for informational purposes. The outcome of this step determines whether the project requires further planning, whether it is already well supported by prior planning studies, and, therefore, whether it is ready to move forward into the design phase, or whether it should be dismissed from further consideration.

2. Planning

This phase will likely not be required for the implementation of the improvements proposed in this planning study, as this planning report should constitute the outcome of this step. However, in general, the purpose of this implementation step is for the project proponent to identify issues, impacts, and approvals that may need to be obtained, so that the subsequent design and permitting processes are understood.

The level of planning needed will vary widely, based on the complexity of the project. Typical tasks include: define the existing context, confirm project need, establish goals and objectives, initiate public outreach, define the project, collect data, develop and analyze alternatives, make recommendations, and provide documentation. Likely outcomes include consensus on the project definition to enable it to move forward into environmental documentation (if needed) and design, or a recommendation to delay the project or dismiss it from further consideration.

3. Project Initiation

At this point in the process, the proponent, MassDOT Highway Division, fills out a Project Initiation Form (PIF) for each improvement, which is reviewed by its Project Review Committee (PRC) and the MPO. The PRC is composed of the Chief Engineer, each District Highway Director, and representatives of the Project Management, Environmental, Planning, Right-of-Way, Traffic, and Bridge departments, and the MassDOT Federal Aid Program Office (FAPO). The PIF documents the project type and description, summarizes the project planning process, identifies likely funding and project management responsibility, and defines a plan for interagency and public participation. First the PRC reviews and evaluates the proposed project based on the MassDOT's statewide priorities and criteria. If the result is positive, MassDOT Highway Division moves the project forward to the design phase, and to programming review by the MPO. The PRC may provide a Project Management Plan to define roles and responsibilities for subsequent steps. The MPO review includes project evaluation based on the MPO's regional priorities and criteria. The MPO may assign project evaluation criteria score, a Transportation Improvement Program (TIP) year, a tentative project category, and a tentative funding category.

4. Environmental Permitting, Design, and Right-of-Way Process

This step has four distinct but closely integrated elements: public outreach, environmental documentation and permitting (if required), design, and right-of-way acquisition (if required). The outcome of this step is a fully designed and permitted project ready for construction. However, a project does not have to be fully designed in order for the MPO to program it in the TIP. The sections below provide more detailed information on the four elements of this step of the project development process.

Public Outreach

Continued public outreach in the design and environmental process is essential to maintain public support for the project and to seek meaningful input on the design elements. The public outreach is often in the form of required public hearings, but can also include less formal dialogues with those interested in and affected by a proposed project.

Environmental Documentation and Permitting

The project proponent, in coordination with the Environmental Services section of the MassDOT Highway Division, will be responsible for identifying and complying with all applicable federal, state, and local environmental laws and requirements. This includes determining the appropriate project category for both the Massachusetts Environmental Protection Act (MEPA) and the National Environmental Protection Act (NEPA). Environmental documentation and permitting is often completed in conjunction with the **Preliminary Design** phase described below.

Design

There are three major phases of design. The first is **Preliminary Design**, which is also referred to as the 25-percent submission. The major components of this phase include full survey of the project area, preparation of base plans, development of basic geometric layout, development of preliminary cost estimates, and submission of a functional design report. Preliminary Design, although not required to, is often completed in conjunction with the Environmental Documentation and Permitting. The next phase is **Final Design**, which is also referred to as the 75-percent and 100-percent submission. The major components of this phase include preparation of a subsurface exploratory plan (if required), coordination of utility relocations, development of traffic management plans through construction zones, development of final cost estimates, and refinement and finalization of the construction plans. Once Final Design is complete, a full set of **Plans, Specifications, and Estimates (PS&E)** is developed for the project.

Right-of-Way Acquisition

A separate set of Right-of-Way plans are required for any project that requires land acquisition or easements. The plans must identify the existing and proposed layout lines, easements, property lines, names of property owners, and the dimensions and areas of estimated takings and easements.

5. Programming (Identification of Funding)

Programming, which typically begins during the design phase, can actually occur at any time during the process, from planning to design. In this step, which is distinct from project initiation, the proponent requests that the MPO place the project in the region's Transportation Improvement Program (TIP). The proponent requesting the project's listing on the TIP can be the community or it can be one of the MPO member agencies (the Regional Planning Agency, MassDOT, and the Regional Transit Authority). The MPO then considers the project in terms of state and regional needs, evaluation criteria, and compliance with the regional Transportation Plan and decides whether to place it in the draft TIP for public review and then in the final TIP.

6. Procurement

Following project design and programming of a highway project, the MassDOT Highway Division publishes a request for proposals. It then reviews the bids and awards the contract to the qualified bidder with the lowest bid.

7. Construction

After a construction contract is awarded, MassDOT Highway Division and the contractor develop a public participation plan and a management plan for the construction process.

8. Project Assessment

The purpose of this step is to receive constituents' comments on the project development process and the project's design elements. MassDOT Highway Division can apply what is learned in this process to future projects.

Project Development Schematic Timetable

Description	Schedule Influence	Typical Duration
Step I: Problem/Need/Opportunity Identification The proponent completes a Project Need Form (PNF). This form is then reviewed by the MassDOT District office which provides guidance to the proponent on the subsequent steps	The Project Need Form has been developed so that it can be prepared quickly by the proponent, including any supporting data that is readily available. The District office shall return comments	1 to 3 months
Step II: Planning Project planning can range from agreement that the problem should be addressed through a clear solution to a detailed analysis of alternatives and their impacts.	to the proponent within one month of PNF submission. For some projects, no planning beyond preparation of the Project Need Form is required. Some projects require a planning study centered on specific project issues associated with the	Project Planning Report: 3 to 24+ months
	proposed solution or a narrow family of alternatives. More complex projects will likely require a detailed alternatives analysis.	
Step III: Project Initiation The proponent prepares and submits a Project Initiation Form (PIF) and a Transportation Evaluation Criteria (TEC) form in this step. The PIF and TEC are informally reviewed by the Metropolitan Planning Organization (MPO) and MassDOT District office, and formally reviewed by the PRC.	The PIF includes refinement of the preliminary information contained in the PNF. Additional information summarizing the results of the planning process, such as the Project Planning Report, are included with the PIF and TEC. The schedule is determined by PRC staff review (dependent on project complexity) and meeting schedule.	1 to 4 months
Step IV: Design, Environmental, and Right of Way The proponent completes the project design. Concurrently, the proponent completes necessary environmental permitting analyses and files applications for permits. Any right of way needed for the project is identified and the acquisition process begins.	The schedule for this step is dependent upon the size of the project and the complexity of the design, permitting, and right-of-way issues. Design review by the MassDOT district and appropriate sections is completed in this step.	3 to 48+ months
Step V: Programming The MPO considers the project in terms of its regional priorities and determines whether or not to include the project in the draft Regional Transportation Improvement Program (TIP) which is then made available for public comment. The TIP includes a project description and funding source.	The schedule for this step is subject to each MPO's programming cycle and meeting schedule. It is also possible that the MPO will not include a project in its Draft TIP based on its review and approval procedures.	3 to 12+ months
Step VI: Procurement The project is advertised for construction and a contract awarded.	Administration of competing projects can influence the advertising schedule.	1 to 12 months
Step VII: Construction The construction process is initiated including public notification and any anticipated public involvement. Construction continues to project completion.	The duration for this step is entirely dependent upon project complexity and phasing.	3 to 60+ months
Step VIII: Project Assessment The construction period is complete and project elements and processes are evaluated on a voluntary basis. Source: Mass DOT Highway Division Project Days	The duration for this step is dependent upon the proponent's approach to this step and any follow-up required.	1 month

Source: MassDOT Highway Division Project Development and Design Guide